Planning and Zoning Board
Agenda
April 20, 2021

Chair James Sajczuk	**City Manager Adam Boukari**
Member Edward Potts	
Member Sandy Burgess	
Member Gary Thomas	
Member Virginia Johns	
School Board Member Tina Certain	

Planning and Zoning Board
At 6:00 PM

Meeting Date: April 20, 2021

Meeting Location: James A. Lewis Commission Chambers

Notice given pursuant to Section 286.0105, Florida Statutes. In order to appeal any decision made at this meeting, you will need a verbatim record of the proceedings. It will be your responsibility to ensure such a record is made.

PLANNING AND ZONING BOARD MEETING AGENDA

CALL TO ORDER

INVOCATION

PLEDGE TO THE FLAG

APPROVAL OF THE AGENDA

I. OLD BUSINESS

II. NEW BUSINESS

A. Approval of the Minutes of the March 9, 2021 PZB Minutes
B. Election of Vice Chair

C. Certificate of Appropriateness: A request by Raghu Consbruck and Govinda Cordua, applicants and property owners, for construction of a new single-family residence on a vacant parcel, located at the northeast corner of NW 146th Ave and NW 144th Street Tax Parcel No. 03679-001-000 (Quasi-Judicial Hearing)

D. Site Plan – KFC : A request by Daniel Young, P.E., of CHW, Inc., applicant and agent, for T&H Property Group, LLC, and Sarcav, LLC, property owners, for consideration of a Site Plan to construct a ±1,904 square foot quick service restaurant with associated parking, stormwater, landscaping, and site improvements on a ±1.06 acre subject property. Tax Parcel Number 03053-002-001 and a portion of 03053-002-000 (Quasi-Judicial Hearing).

E. Site Plan – Alchem Building 4: A request by Claudia Vega, P.E., of EDA Consultants, Inc., applicant and agent for Alchem Laboratories Corporation, property owner, for consideration of a Site Plan to construct a ±3,788 square foot building and associated site improvements on a ±0.76 acre property; Tax Parcel Numbers 03231-003-000 and 03231-004-001 (Quasi-Judicial Hearing).

F. Site-Specific Amendment to the Official Zoning Atlas: A request by Christopher Potts, P.E. of JBrown Professional Group, LLC, applicant and agent for Shakti Management, LLC, property owner, to amend the Official Zoning Atlas from Central Business District (CBD) to Commercial Intensive (CI) on a ±1.09 acre subject property. Tax Parcel Number 03869-004-000 (Quasi-Judicial Hearing).

G. Land Development Regulations Text Amendment : A request by Christopher Potts P.E.,of JBrown Professional Group, LLC to amend the City of Alachua Land Development Regulations (LDRs) as follows: amending Section 4.3.2 (I)(4), relating to the use-specific standards for minor utilities; amending Section 2.4.4 (D) (7), relating to standards for special exception permits (Legislative Hearing)

III. BOARD COMMENTS/DISCUSSION

IV. CITIZENS COMMENTS

ADJOURN

City of ALACHUA
THE GOOD LIFE COMMUNITY

Board/Committee Agenda Item

MEETING DATE: 4/20/2021
SUBJECT: Approval of the Minutes of the March 9, 2021 PZB Minutes

PREPARED BY: Kenyata Curtis, Planning Assistant

RECOMMENDED ACTION:

Approve the Minutes of the March 9, 2021 PZB Minutes.

Summary

Approval of the Minutes of the March 9, 2021 PZB Minutes.

ATTACHMENTS:

Description

- March 9, 2021 PZB Minutes

Planning and Zoning Board
Minutes
March 9, 2021

Chair James Sajczuk	**City Manager Adam Boukari**
Vice Chair Anthony Wright	
Member Sandy Burgess	
Member Gary Thomas	
Member Virginia Johns	
School Board Member Tina Certain	

Planning and Zoning Board
At 6:00 PM
to address the item(s) below.

Meeting Date: March 9, 2021

Meeting Location: James A. Lewis Commission Chambers

Notice given pursuant to Section 286.0105, Florida Statutes. In order to appeal any decision made at this meeting, you will need a verbatim record of the proceedings. It will be your responsibility to ensure such a record is made.

Planning and Zoning Board Meeting Minutes

CALL TO ORDER

Chair James Sajczuk called the meeting to order. School Board Member Tina Certain was absent.

INVOCATION

Member Gary Thomas led the invocation.

PLEDGE TO THE FLAG

Led by the Board.

APPROVAL OF THE AGENDA

Member Virginia Johns moved to approve the agenda; seconded by Member Sandy Burgess.

Passed by unanimous consent.

I. OLD BUSINESS

None.

II. NEW BUSINESS

A. Approval of the Minutes of the January 26, 2021 Special PZB Meeting

Member Johns moved to approve the minutes; seconded by Member Thomas.

Passed by unanimous consent.

B. Approval of the Minutes of the February 9, 2021 PZB Meeting

Member Johns moved to approved the minutes; seconded by Member Burgess.

Passed by unanimous consent.

C. Site Plan – Quick Serve Restaurant: A request by Daniel Young, P.E., of CHW, Inc., applicant and agent, for VRE Alachua, LLC, property owner, for consideration of a Site Plan to construct a ±1,930 square foot restaurant with drive-through service with associated site improvements on a ±1.15 acre property . Tax Parcel Number 03067-601-003 (Quasi-Judicial Hearing).

Planning Assistant Kenyata Curtis swore in all parties entering testimony.

Planner Adam Hall, AICP, presented the Staff Report.

Daniel Young, P.E., of CHW, Inc., accepted Staff's recommended conditions and availed himself for questions.

Member Johns inquired about the existing acceleration lane on US 441 being converted into a turning lane.

Mr. Young responded.

Member Johns moved that based upon the competent substantial evidence presented at this hearing, the presentation before this Board, and Staff's recommendation, this Board finds the application to be consistent with the City of Alachua Comprehensive Plan and in compliance with the Land Development Regulations and approves the Site Plan for a restaurant with drive-through service on Tax Parcel Number 03067-061-003, subject to the two (2) conditions provided in Exhibit "A" and located on page 17 of the March 9, 2021 Staff Report to the Planning & Zoning Board.

Motion Passed 4-0 in a roll call vote.

III. BOARD COMMENTS/DISCUSSION

Department Director Kathy Winburn stated there will be a meeting on April 20, 2021 at 6:00p.m.

IV. CITIZENS COMMENTS

None.

ADJOURN

Member Johns moved to adjourned; seconded by Member Burgess.

Motion Passed by unanimous consent.

ATTEST:

PLANNING AND ZONING BOARD OF THE CITY OF ALACHUA, FLORIDA

_____
Presiding Officer

_____
Staff Liaison

City of Alachua
THE GOOD LIFE COMMUNITY

Board/Committee Agenda Item

MEETING DATE: 4/20/2021
SUBJECT: Election of Vice Chair

PREPARED BY: Justin Tabor, AICP, Principal Planner

RECOMMENDED ACTION:

Elect a Vice Chair from among the PZB's membership.

Summary

Annually, the Planning & Zoning Board (PZB) elects from among its membership a Chair and Vice Chair to serve for a period of one (1) year, or until a successor is elected.

In August 2020, the PZB elected Member James Sajczuk to serve as Chair and Member Anthony Wright to serve as Vice Chair. On February 3, 2021, Vice Chair Wright tendered his resignation from the Board. As such, the PZB must elect a Vice Chair to serve the remainder of the annual term.

City of Alachua
THE GOOD LIFE COMMUNITY

Board/Committee Agenda Item

MEETING DATE: 4/20/2021
SUBJECT: Certificate of Appropriateness: A request by Raghu Consbruck and Govinda Cordua, applicants and property owners, for construction of a new single-family residence on a vacant parcel, located at the northeast corner of NW 146th Ave and NW 144th Street Tax Parcel No. 03679-001-000 (Quasi-Judicial Hearing)

PREPARED BY: Adam Hall, AICP, Planner

RECOMMENDED ACTION:
Staff recommends that the Planning & Zoning Board approve the Certificate of Appropriateness upon making the following motion:

Based upon the competent substantial evidence presented at this hearing, the presentation before this Board, and Staff's recommendation, this Board finds the application to be consistent with the City of Alachua Comprehensive Plan and in compliance with the Land Development Regulations and approves the application for a Certificate of Appropriateness.

Summary

This application is a request by Raghu Consbruck and Govinda Cordua, applicants and property owners, for the approval of a Certificate of Appropriateness for the following alterations to a property located within the Historic Overlay District: construction of a one story, single family dwelling unit.

The subject property is ±0.33 acres in area, and is located at the northeast corner of NW 146th Ave and NW 144th Street.

As part of the application package, the applicant has submitted pictures and specifications of the proposed structure, including building elevations and proposed possible colors of the new home. Pictures have also been submitted of the structures on nearby properties to demonstrate that the proposed building is consistent with the character of nearby structures.

Section 3.7.2(A) of the City's Land Development Regulations (LDRs) establishes the standards for the Historic Overlay District. The general purposes of the Historic Overlay District are as follows:

- to protect the beauty of the City and improve the quality of its environment through identification, recognition, conservation, maintenance, and enhancement of the City's historic and architectural resources;
- to protect designated land and buildings or structures having historic and architectural significance;
- to stabilize and improve property values;
- to foster civic pride; and,
- to encourage redevelopment in the City's downtown.

Section 3.7.2(A)(2)(a) of the LDRs establishes the City of Alachua Downtown Historic District, which includes all lots within an area bounded by NW 145th Avenue, NW 145th Terrace, NW 148th Place, and NW 138th Terrace.

ATTACHMENTS:

Description

- Staff Report
- Application and Supporting Materials
- Public Notice for 04/20/2021 Planning and Zoning Board

City of Alachua
Planning & Community Development Department
Staff Report

Planning & Zoning Board Hearing Date: **April 20, 2021**

Quasi-Judicial Hearing

SUBJECT:	A request for the approval of a Certificate of Appropriateness for the following alterations to a property located with the Historic Overlay District: construction of a new one story single family residence
APPLICANT/AGENT:	Raghu Consbruck and Govinda Cordua
PROPERTY OWNER:	Raghu Consbruck and Govinda Cordua
LOCATION:	Northeast corner of NW 146th Ave and NW 144th Street
PARCEL ID NUMBER:	03679-001-000
ACREAGE:	±0.33 acres
PROJECT PLANNER:	Adam Hall, AICP, Planner
RECOMMENDATION:	Staff recommends that the Planning & Zoning Board approve the Certificate of Appropriateness.
RECOMMENDED MOTION:	*Based upon the competent substantial evidence presented at this hearing, the presentation before this Board, and Staff's recommendation, this Board finds the application to be consistent with the City of Alachua Comprehensive Plan and in compliance with the Land Development Regulations and approves the application for a Certificate of Appropriateness.*

SUMMARY

This application is a request by Raghu Consbruck and Govinda Cordua, applicants and property owners, for the approval of a Certificate of Appropriateness for the following alterations to a property located within the Historic Overlay District: construction of a one story, single family dwelling unit.

The subject property is ±0.33 acres in area, and is located at the northeast corner of NW 146th Ave and NW 144th Street.

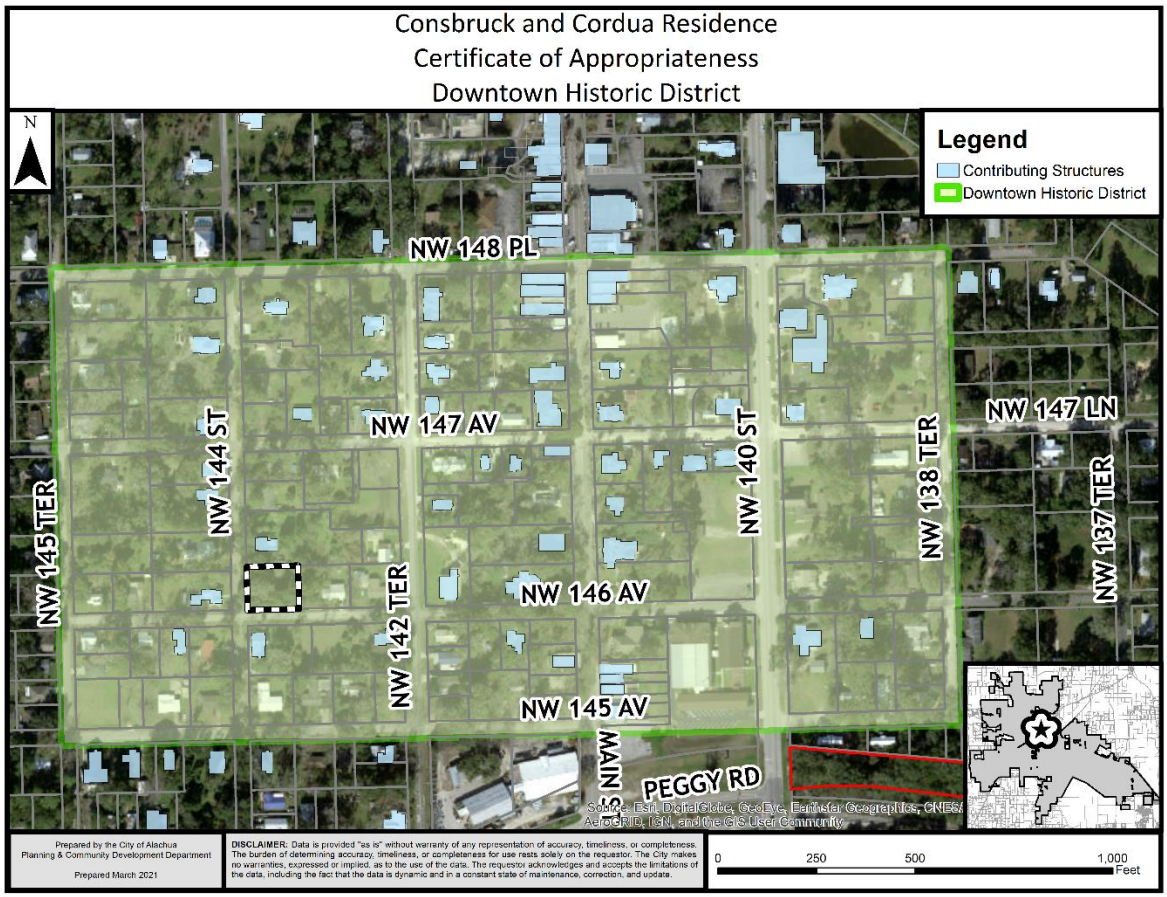
As part of the application package, the applicant has submitted pictures and specifications of the proposed structure, including building elevations and proposed possible colors of the new home. Pictures have also been submitted of the structures on nearby properties to demonstrate that the proposed building is consistent with the character of nearby structures.

Section 3.7.2(A) of the City's Land Development Regulations (LDRs) establishes the standards for the Historic Overlay District. The general purposes of the Historic Overlay District are as follows:

- to protect the beauty of the City and improve the quality of its environment through identification, recognition, conservation, maintenance, and enhancement of the City's historic and architectural resources;
- to protect designated land and buildings or structures having historic and architectural significance;
- to stabilize and improve property values;
- to foster civic pride; and,
- to encourage redevelopment in the City's downtown.

Section 3.7.2(A)(2)(a) of the LDRs establishes the City of Alachua Downtown Historic District, which includes all lots within an area bounded by NW 145th Avenue, NW 145th Terrace, NW 148th Place, and NW 138th Terrace. The area of the District, the subject property, and contributing historic structures are shown on Map 1.

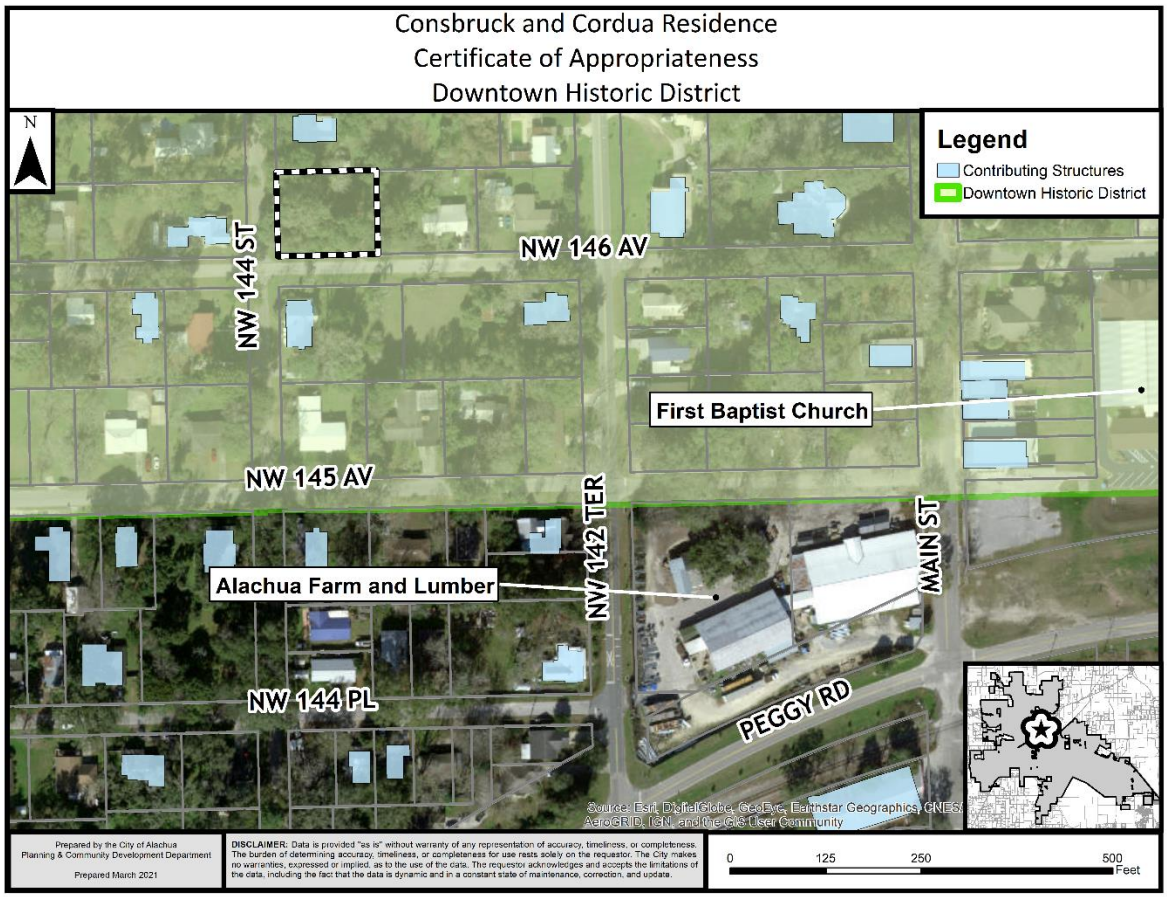
Map 1. City of Alachua Downtown Historic District & Location of Subject Property



Section 3.7.2(A)(5) of the LDRs states, "[o]nce land, buildings, or structures are classified within an Historic Overlay District, no demolition, new construction, addition, or alteration of exterior architectural features shall occur within the district without the issuance of a Certificate of Appropriateness in accordance with Section 2.4.6, *Certificate of Appropriateness*."

Map 2 depicts the vicinity surrounding the subject property. The subject property is located at the northeast intersection of NW 146th Ave and NW 144th Street, west of Main Street.

Map 2. Vicinity Map



CONSISTENCY WITH COMPREHENSIVE PLAN

The Goals, Objectives, and Policies (GOPs) identified below provide a basis to establish consistency with the Comprehensive Plan. There may be additional GOPs which the application is consistent with that are not listed below. An analysis of the application's consistency with the identified Goals, Objectives, and Policies below has been provided.

Future Land Use Element

GOAL 3: Historic, Cultural and Archeological Preservation: The City shall encourage the preservation of historic, cultural and archeological resources through the use of preservation districts, land acquisition, and partnerships with local, state and federal protection agencies.

Objective 3.1: Historic Preservation Districts:
The City of Alachua shall encourage property owners to maintain and improve buildings, grounds, streetscape and vistas and encourage settlement and revitalization of established neighborhoods.

Policy 3.5.a: The City shall ensure that future development within the district is sensitive to the historic character of any historic district.

Policy 3.9.a: An historic preservation overlay zoning classification shall be used to protect significant historic resources.

Policy 3.10.a: The City shall have Certificate of Appropriateness procedures for reviewing exterior changes to historic buildings, structures and sites in historic districts.

Analysis of Consistency with Goals, Objectives, & Policies: The applicant has submitted materials to support that the proposed structure will be consistent with and sensitive to the character of the historic district. The proposed façade material and design is similar in style to existing facades of contributing structures in the immediate area.

COMPLIANCE WITH LAND DEVELOPMENT REGULATIONS

Section 2.4.6(E) of the City's Land Development Regulations (LDRs) establishes the standards of review for an application for a Certificate of Appropriateness. Staff's evaluation of the application's compliance with the applicable standards of Section 2.4.6(E) is provided below.

(1) ***Affect Exterior Appearance***
Whether the proposed action will affect the exterior appearance of the subject land or structure or the appearance of any interior portion specifically identified in any relevant designation ordinance.

Evaluation: The applicant has submitted materials to support that the proposed structure will be consistent with and sensitive to the character of the historic district. The proposed façade material and design is similar in style to existing facades of contributing structures would be consistent with the appearance of structures identified as contributing structures within the Historic Overlay District.

(2) ***Affect Consistent with Historical, Architectural, or Other Relevant Qualities***
Whether any such affect is consistent with the historical, architectural, or other qualities which the relevant designation ordinance seeks to protect and enforce.

Evaluation: Section 3.7.2 of the LDRs establishes the provisions of the City's Downtown Historic District. The general purposes of the Historic Overlay District are: to protect the beauty of the City and improve the quality of its environment through identification, recognition, conservation, maintenance, and enhancement of the City's historic and architectural resources; to protect designated land and buildings or structures having historic and architectural significance; to stabilize and improve property values; to foster civic pride; and to encourage redevelopment in the City's downtown.

The proposed structure is consistent with the character of proximate structures (see exhibits and construction drawings submitted by the applicant). Staff finds the proposed structure would be consistent with the qualities of structures within the Historic Overlay District which the District seeks to protect. The proposed use of metal roof, siding and battens, and use of roof gables are all present on structures within the Historic Overlay District. The proposed paint colors are harmonious with existing paint colors of surrounding structures within the Historic Overlay District.

(3) ***Negative or Positive Impact on Neighboring Lands of Historical Significance***
Whether the proposed action will have a negative or positive impact on neighboring lands that have historic significance.

Evaluation: Based upon the preceding information, the proposed building will be consistent with the appearance of other proximate structures, including those identified as contributing structures in the Master Registry of the National Register of Historic Places. The proposed structure is anticipated to have a positive impact on neighboring lands.

(4) ***Effect of Denial***
Whether the denial of a Certificate of Appropriateness would deprive the owner of reasonable beneficial use of their land.

Evaluation: The approval of this application would permit the owners to construct a single family residence on a lot which permits single family dwelling units.

(5) ***Demolition***
In cases where a Certificate of Appropriateness is proposed for demolition of all or a portion of a Historic Property, the applicant shall demonstrate:

(a) That no other feasible alternative can be found;
(b) The extent to which the land may not be beneficially used without approval of the demolition;
(c) The ability to obtain a reasonable return from the existing structure(s); and
(d) Whether or not the building, structure, or land contributes to a Historic Overlay (HO) District, or no longer has historical significance.

Evaluation: No demolition has been proposed, therefore, this standard is not applicable.

City of ALACHUA

THE GOOD LIFE COMMUNITY

FOR PLANNING USE ONLY
Case #: _____
Application Fee: $_____
Filing Date: _____
Acceptance Date: _____
Review Type: P&Z

Certificate of Appropriateness Application

Reference City of Alachua Land Development Regulations 2.4.6

You must have all information for your application to be considered complete. Incomplete applications will not be accepted.

A. PROJECT TYPE

☐ Demolition ☐ Removal
☒ New Construction ☐ Addition
☐ Alteration of Exterior Features

B. APPLICANT

1. Applicant's Status ☒ Owner (title holder) ☐ Agent
2. Name of Applicant(s) or Contact Person(s): Raghu Consbruck and Title: Govinda Cordua
   Company (if applicable): _____
   Physical address: 14410 NW 146th Ave 352-284-9721
   Mailing Address: PO Box 42 cordua@gmail.com
   City: Alachua State: FL ZIP: 32615
3. If the applicant is agent for the property owner*:
   Name of Owner (title holder): _____
   Mailing Address: _____
   City: _____ State: _____ ZIP: _____
   Telephone: _____ FAX: _____ e-mail: _____

C. PROPERTY AFFECTED

1. Address of Subject Property: not assigned yet
2. Parcel ID Number(s): 03679-001-000
3. Existing Use of Property: Vacant
4. Future Land Use Map Designation: MED
5. Zoning Designation: RSF-3
6. Acreage: 0.33
7. Number of Existing Structures on the Property: 0

D. ATTACHMENTS

1. A detailed drawing which identifies the dimensions of the property, location of existing building(s) and those proposed (if applicable), distances of buildings from property lines, building setback lines (dimensioned), and names of adjacent streets. A current survey or site plan may be submitted to meet this requirement, if all requested information is provided.
2. Set of plans and specifications for the project. All drawings must be clear, concise, and drawn to scale. Plans must include and dimension all exterior elevations and provide a dimensioned floor plan illustrating all rooms within any proposed addition (if applicable).
3. Photographs of existing building(s) (all facades or elevations of building(s) and adjacent buildings). Photographs must clearly illustrate the appearance and conditions of existing building(s) affected by the project, close-up pictures of any specific elements under consideration by the project (for

example, windows or doors if proposed to be removed and/or replaced), as well as photographs depicting such elements and their relation with neighboring buildings.

4. If demolition is proposed, a report from a registered structural engineer stating the condition of the structure.
5. The applicant must provide a report which addresses the following:

   (1) ***Affect Exterior Appearance***

   Whether the proposed action will affect the exterior appearance of the subject land or structure or the appearance of any interior portion specifically identified in any relevant designation ordinance.

   (2) ***Affect Consistent with Historical, Architectural, or Other Relevant Qualities***

   Whether any such affect is consistent with the historical, architectural, or other qualities which the relevant designation ordinance seeks to protect and enforce.

   (3) ***Negative or Positive Impact on Neighboring Lands of Historical Significance***

   Whether the proposed action will have a negative or positive impact on neighboring lands that have historic significance.

   (4) ***Effect of Denial***

   Whether the denial of a Certificate of Appropriateness would deprive the owner of reasonable beneficial use of their land.

   (5) ***Demolition***

   In cases where a Certificate of Appropriateness is proposed for demolition of all or a portion of a Historic Property, the applicant shall demonstrate:

   (a) That no other feasible alternative can be found;

   (b) The extent to which the land may not be beneficially used without approval of the demolition;

   (c) The ability to obtain a reasonable return from the existing structure(s); and

   (d) Whether or not the building, structure, or land contributes to a Historic Overlay (HO) District, or no longer has historical significance.

6. Proof of ownership.
7. Proof of payment of taxes.
8. **Fee.** Please see fee schedule for fee determination. No application shall be accepted for processing until the required application fee is paid in full by the applicant. Any necessary technical review will be billed to the applicant at the rate of the reviewing entity. The invoice shall be paid in full prior to any legislative and/or quasi-judicial action of any kind on the petition, appeal, or development application.

All attachments are required for a complete application. **A completeness review of the application will be conducted within five (5) business days of receipt. If the application is determined to be incomplete, the application will be returned to the applicant.**

I/We certify and acknowledge that:

1. I /We must comply with the current Florida Building Code through the Building Department and obtain any necessary permits prior to receiving a final certificate of occupancy.
2. I/We must comply with the requirements of the Alachua County Fire Marshall.
3. I /We must obtain a Local Business Tax Receipt.
4. I/We must meet parking standards and any use specific standards for the zoning district
5. Falsifying information on this application may result in the Certificate of Appropriateness being revoked.

Signature of Applicant

Signature of Co-applicant

Raghu Consbruck

Typed or printed name and title of applicant

Govinda Cordua

Typed or printed name of co-applicant

March 9, 2021

Date

March 9, 2021

Date

State of Florida County of Alachua

The foregoing application is acknowledged before me this 9th day of MARCH, 2021 by GOVINDA CORDUA + RAGHU CONSBRUCK, who is/are personally known to me, or who has/have produced _____ as identification.

NOTARY SEAL

DAVID V. JAKUPKO
MY COMMISSION # GG 072467
EXPIRES: February 13, 2021
Bonded Thru Notary Public Underwriters

Signature of Notary Public, State of FLORIDA

5.1 This corner lot, currently unbuilt, will have a new single story home constructed for the owners who currently live less than a block from this site. The site is wooded with a diversity of trees and shrubs. The appearance of the site will change with the removal of certain trees as well as some undergrowth. Much of the vegetation bordering the two streets on the West and South sides of the lot will remain in order that the new dwelling will appear to be a cottage-like presence in the woods. Similarly, the main entrance on the South side will be significantly wooded with undergrowth neatly trimmed. The North side abuts the yard and home of the property's former owner. This house as well as the ones across the street to the West and the South are all on the historic register.

5.2 The new home will possess many characteristics of homes in the neighborhood. Gabled roofs, front porch, screened porch, board and batten façade with ends of gables finished with a smaller board and batten pattern referring to past traditions and styles. If affordable, the roof will be metal. If not, it will be architectural composite shingles. Both roof styles are present in the neighborhood.

5.3 This new home will be a very positive influence for the neighborhood. It will contain architectural features from cottage and bungalow forebearers.

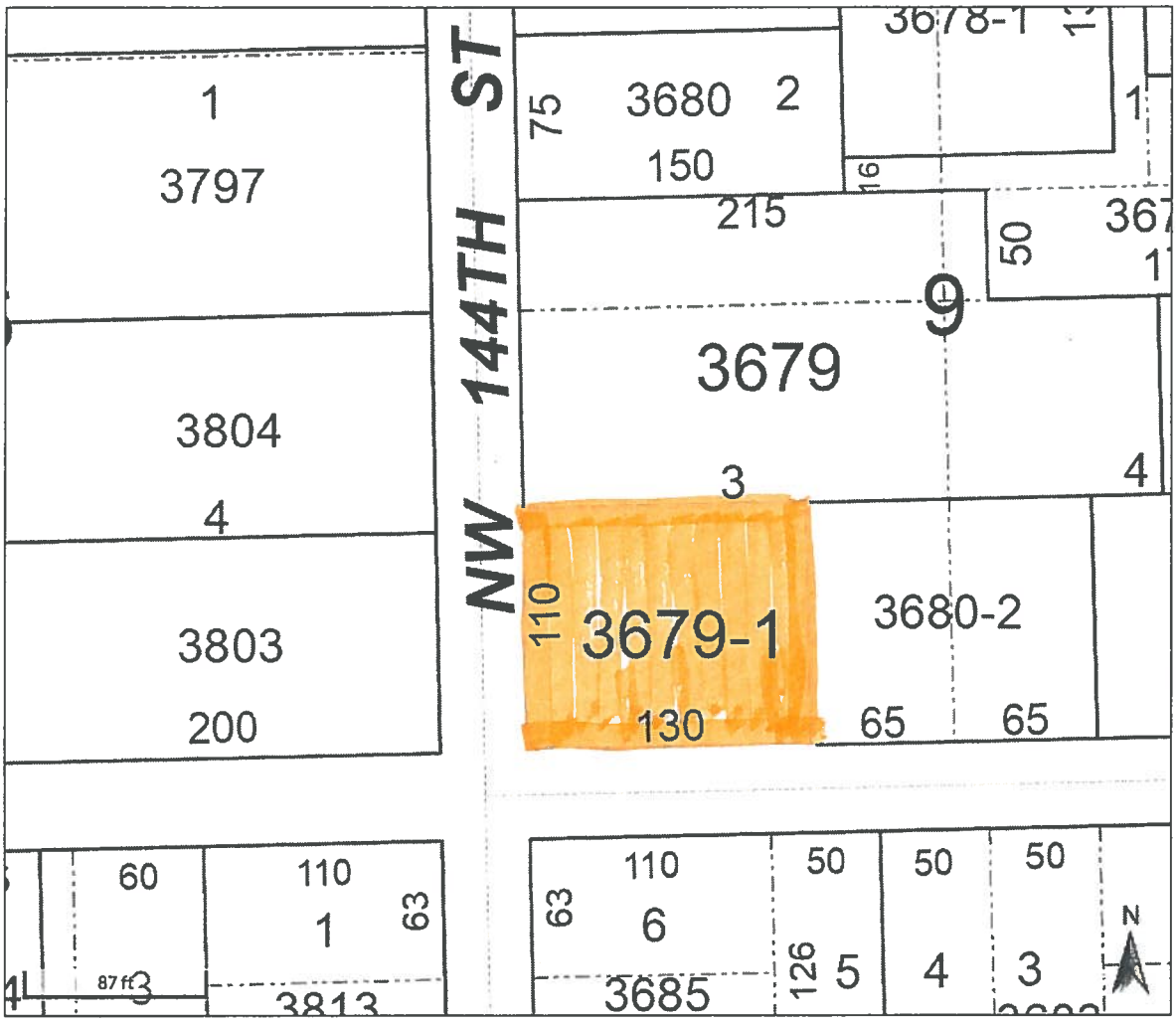
5.4 Denial of this request would devastate almost 4 years of living in neighborhood with the idea that someday we would be able to live in a larger cottage-style home. We have watched all the houses in the neighborhood that were "for sale" but found none that met all our criteria. Similarly, we have examined all the remaining lots in this historic area as they were put on the market. This one had the wooded nature we love and it would have its front door facing South as our current abode does. We feel part of this neighborhood and want to continue to be a positive addition to the people and places here.

5.5 This criterion does not relate to this application.

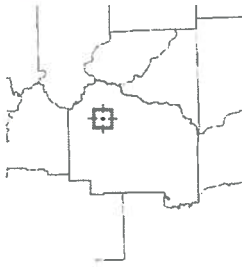
Consbruck-Cordua Residence
SPECIFICATIONS

1. Foundation: 2500 PSI reinforced concrete strip footings min. bearing 12" below grade with 8" CMU reinforced stemwall, compacted fill, treated for termites per code, with 4" thick 2500 PSI concrete slab over 12 mil Polyethylene vapor barrier, with 6 x6 # 10 WWF temperature steel.
2. Walls: 4-5/8" thick SIPs x 4' wide (Structural Insulated Panel) with 3.5" closed cell foam insulation, 5/16" Hardiboard panel surfaces on both sides, joints calked and covered with Hardiboard ¾" thick x 2.5" vertical battens, 1x4 and 1x6 trim at corners 1x8 or 1x10 trim at windows.
3. Roof: 8-1/4" thick SIPs by 4' wide (Structural Insulated Panel) with 7.25" closed cell foam insulation, 5/8" Zipboard plywood surfaces on both sides, joints taped and sealed.
4. Roofing: GAF self-adhered underlayment with 7mm 16" wide standing seam snap lock prefinished metal roofing.
5. Driveway: interlocking concrete pavers on crusher run compacted bedding
6. Deck: Pressure treated framing and a girders on cast in place 10" dia. Sonotube piers, 2500 PSI reinforced concrete pad footings, 6" nominal side composite decking.
7. Windows & Patio Doors: Prefinished insulated vinyl (white) casement and awning windows, sliding patio doors by Jeld-Wen or equal.
8. Skylights: Operable venting, with shade and electric remote control roof windows by VeLux, tube skylights by Velux or equal in bathrooms.
9. Entry Doors: Fiberglass pre-hung doors
10. Interior Doors: Flush smooth solid core wood doors.
11. Flooring: Prefinished Bamboo engineered wood floor over manufacturer's recommend vapor barrier, large format Ceramic Tile floors in bathrooms and laundry room.
12. Interior Walls and Ceilings: ½" painted drywall on 2x4 or 2x6 SPF # 2 or better wood studs, pressure treated floor plates, f1x2 or 2x2 furring as required on inside surface of exterior SIP walls.
13. HVAC: Central air high efficiency air conditioner and gas fired furnace ducted system, with outside air exchange and dehumidifier as designed by engineer.
14. Water Heater: Gas fired instantaneous water heater with recirculating capabilities.
15. Lighting: LED light fixtures, including recessed, surface, and pendant styles.
16. Electric Service: 200 amp panel with underground service entrance. Install conduits accessible to roof for future solar photovoltaic installation.
17. Site Landscaping: Natural mulch in hedge and "forest" areas. Lawn in backyard sunny space. Trim and shape existing perimeter hedge along both streets. Maintain healthy existing trees where desired. Gravel or compacted crushed limestone in extra parking space and (optional) existing driveway to neighbor. Grade site to drain as indicated on the Site Plan. Install drainbox, grate and plastic drain pipe as indicated for drainage at driveway area. Use caution to protect from construction traffic any trees (barricade where necessary) to be saved.

AYESHA SOLOMON
PROPERTY APPRAISER
KNOWLEDGEABLE - COMMUNITY FOCUSED - COMMITTED



Overview



Legend

Parcels

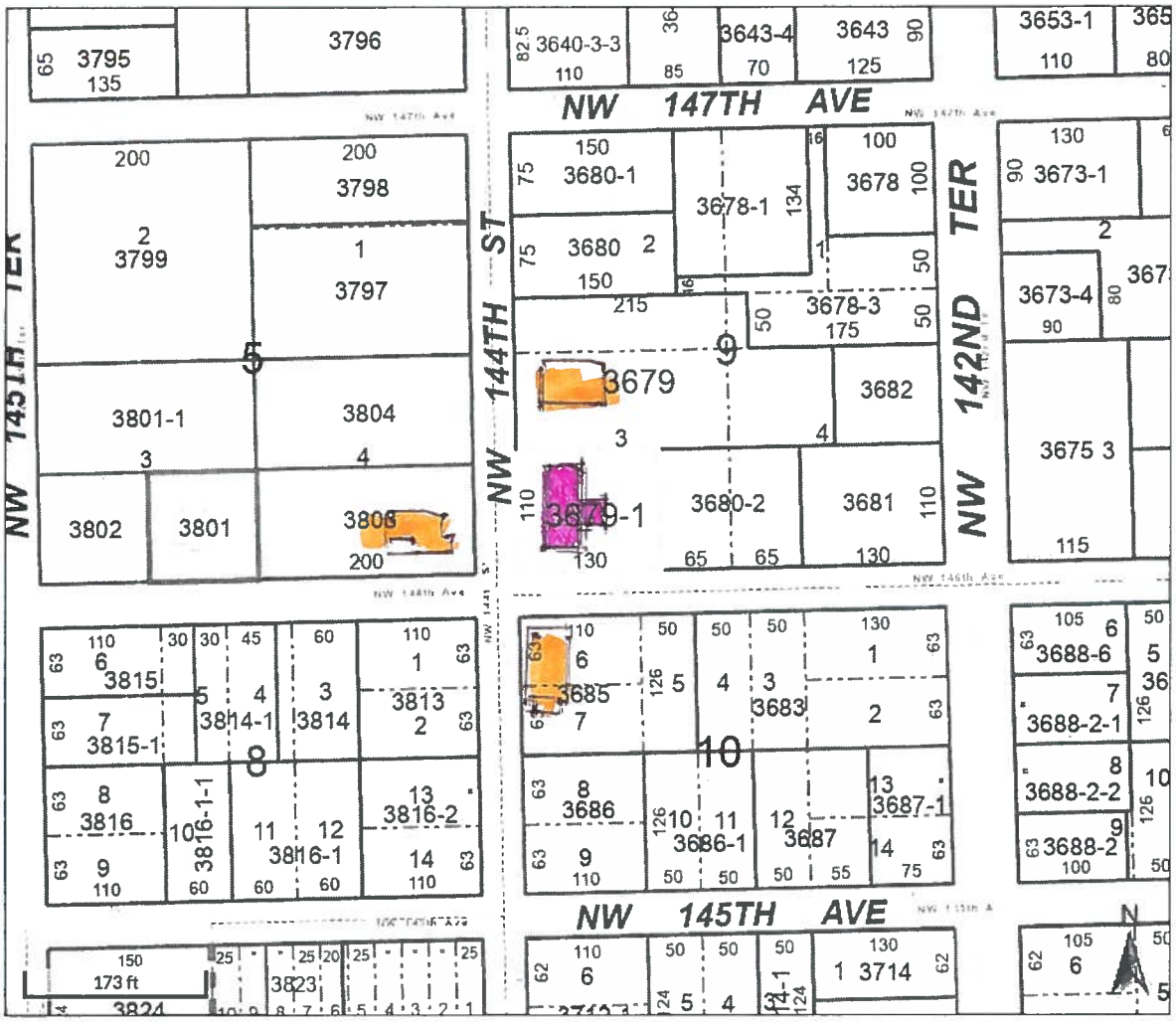
Lakes

Parcel ID	03679-001-000	**Prop ID**	501991	**Owner Address**	CONSBRUCK & CORDUA H/W
Sec/Twp/Rng	15-08-18	**Class**	SINGLE FAMILY		PO BOX 42
Property Address		**Acreage**	0.33		ALACHUA, FL 32616
District	1006 - ALACHUA				
Brief Tax Description	OLMSTEAD SURVEY PB C-79B COM SW COR OF LOT 3 POB N 110 FT E 130 FT S 110 FT W 130 FT POB OR 4795/55 (Note: Not to be used on legal documents)				

Date created: 3/2/2021
Last Data Uploaded: 3/2/2021 5:19:22 AM

Developed by Schneider GEOSPATIAL

AYESHA SOLOMON
PROPERTY APPRAISER
KNOWLEDGEABLE - COMMUNITY FOCUSED - COMMITTED



Parcel ID	03679-001-000	**Prop ID**	501991	**Owner Address**	CONSBRUCK & CORDUA H/W
Sec/Twp/Rng	15-08-18	**Class**	SINGLE FAMILY		PO BOX 42
Property Address		**Acreage**	0.33		ALACHUA, FL 32616
District	1006 - ALACHUA				
Brief Tax Description	OLMSTEAD SURVEY PB C-79B COM SW COR OF LOT 3 POB N 110 FT E 130 FT S 110 FT W 130 FT POB OR 4795/55				
	(Note: Not to be used on legal documents)				

Date created: 3/2/2021
Last Data Uploaded: 3/2/2021 5:19:22 AM

Developed by Schneider GEOSPATIAL

Sale Price: 18,000.-
Recording: 18.50
Doc. Stamp: 126.-

RECORDED IN OFFICIAL RECORDS
INSTRUMENT # 3271295 2 PG(S)
July 17, 2020 01:44:42 PM
Book 4795 Page 58
J K. JESS IRBY, ESQ Clerk Of Court
ALACHUA COUNTY Florida

Doc Stamp-Deed $126.00

Prepared by and return to:
Marvin W. Bingham, Jr.
Attorney at Law
Bingham & Mikolaitis, P. A.
P. O. Box 1930
Alachua, FL 32616-1930
386-462-5120
File #20-100

Tax Parcel #: 03679-000-000

[Space Above This Line For Recording Data]

Warranty Deed

This Warranty Deed made this 15th day of July, 2020 between **Gilman Sherwood Marshall, II, an unmarried man,** whose post office address is 14611 N. W. 144th Street, Alachua, FL 32615, grantor, and **Govinda Cordua and Raghu N. Consbruck, husband and wife,** whose post office address is P. O. Box 42, Alachua, FL 32616, grantees:

(Whenever used herein the terms "grantor" and "grantee" include all the parties to this instrument and the heirs, legal representatives, and assigns of individuals, and the successors and assigns of corporations, trusts and trustees)

Witnesseth, that said grantor, for and in consideration of the sum of TEN AND NO/100 DOLLARS ($10.00) and other good and valuable considerations to said grantor in hand paid by said grantees, the receipt whereof is hereby acknowledged, has granted, bargained, and sold to the said grantees, and grantees' heirs and assigns forever, the following described land, situate, lying and being in Alachua County, Florida to-wit:

Commence as a Point of Beginning at the Southwest corner of Lot Three (3), Block Nine (9) of OLMSTEAD'S SURVEY OF ALACHUA, Florida, as per plat thereof recorded in Plat Book C, Page 79B of the Public Records of Alachua County, Florida; thence run North along the East edge of Alachua Street in the town of Alachua, Florida, 110 feet; thence run East parallel with South Fourth Street 130 feet; thence run South parallel with said Alachua Street 110 feet to the North edge of South Fourth Street; thence run West along the North edge of South Fourth Street 130 feet to the Point of Beginning. The same being further described as the South 110 feet of West 130 feet of Lot Three (3) of Block Nine (9), Olmstead's Survey of Alachua.

Subject to and reserving unto grantor for the benefit of his adjoining lands and the future owners thereof, a 10' easement for ingress and egress being more particularly described as follows:

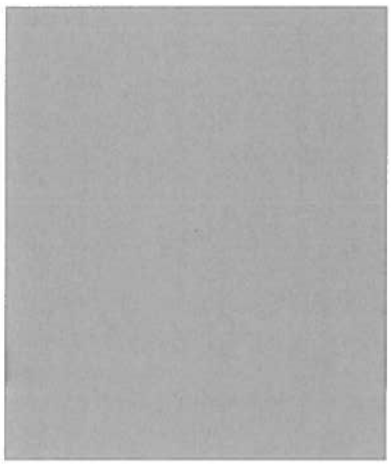
Commence at the Southwest corner of Lot 3, Block 9, of Olmstead's Survey of Alachua as in record Plat Book C, Page 79B of the Public Records of Alachua County, Florida; thence East, along the South line of said Block 9, a distance of 103.64 feet to the Point of Beginning; thence continue East a distance of 10.00 feet; thence, leaving said South line, North a distance of 110.02 feet; thence West a distance of 10.00 feet; thence South a distance of 110.02 feet to the Point of Beginning.

Subject to and together with covenants, easements, reservations and restrictions of record, and taxes for the year 2020 and all subsequent years.

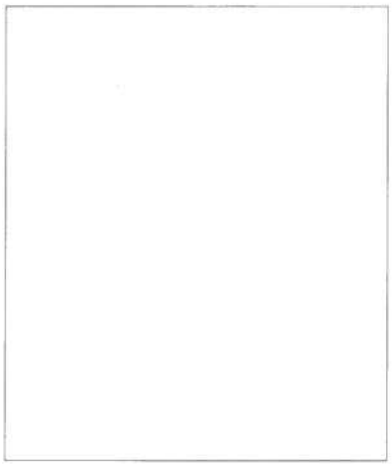
Together with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

To Have and to Hold, the same in fee simple forever.

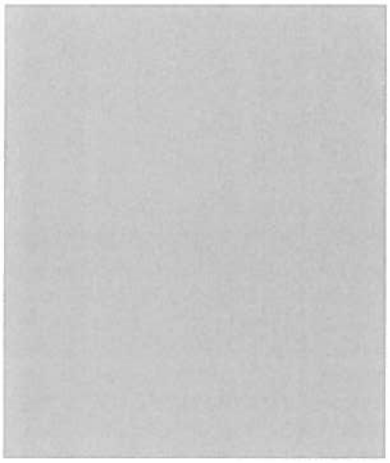
PAINT COLOR OPTIONS



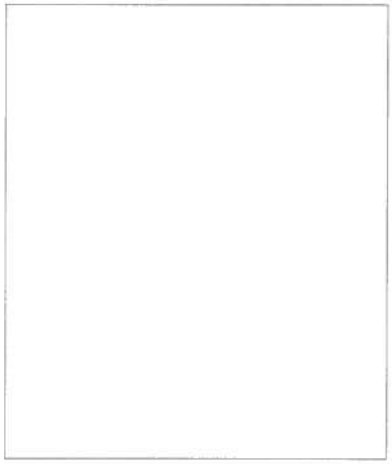
Exterior walls: Warm gray



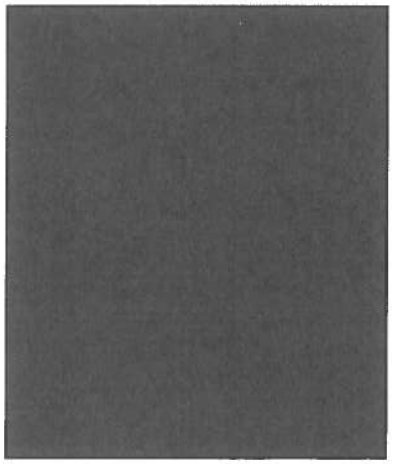
Trim: white



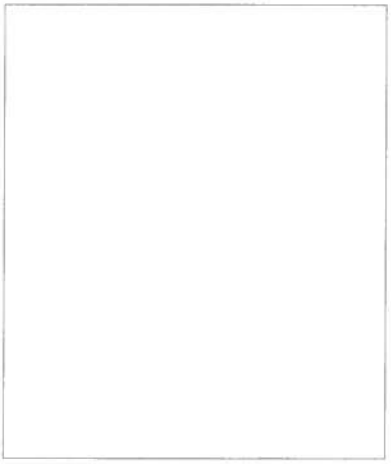
Exterior walls: Muted green



Trim: white



Exterior walls: Muted blue



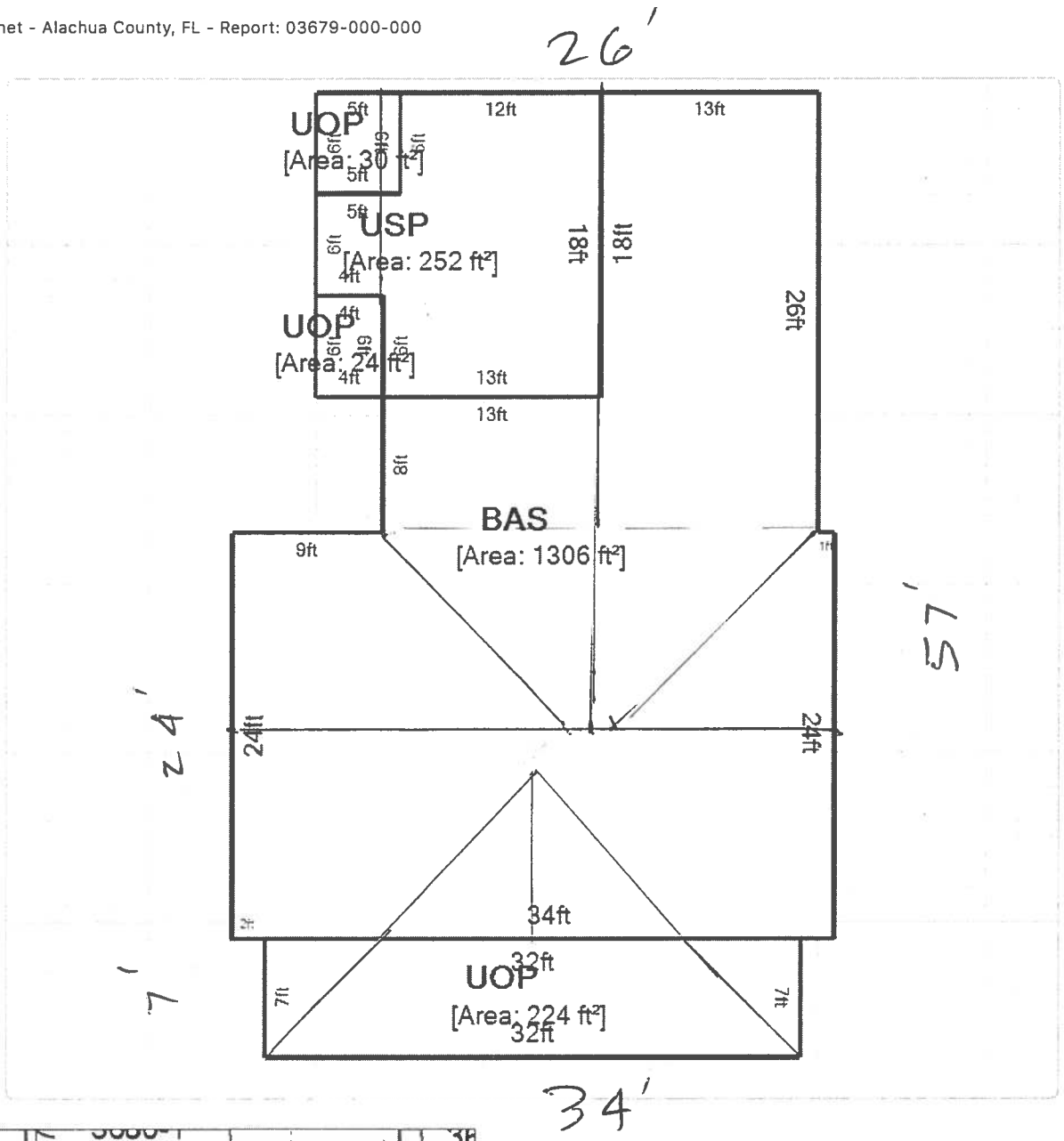
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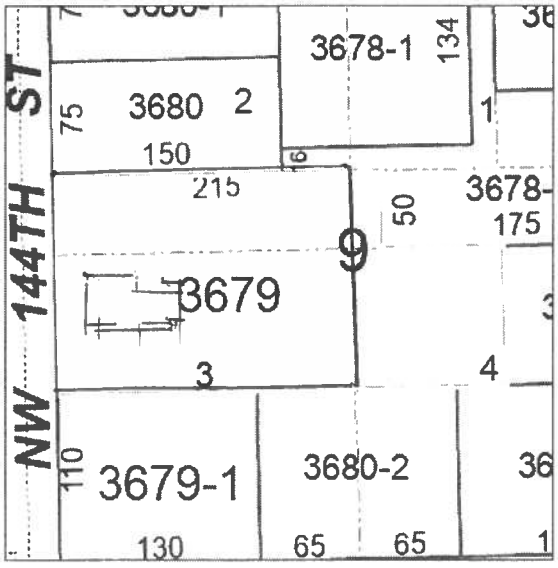


14611 NW 144th Street



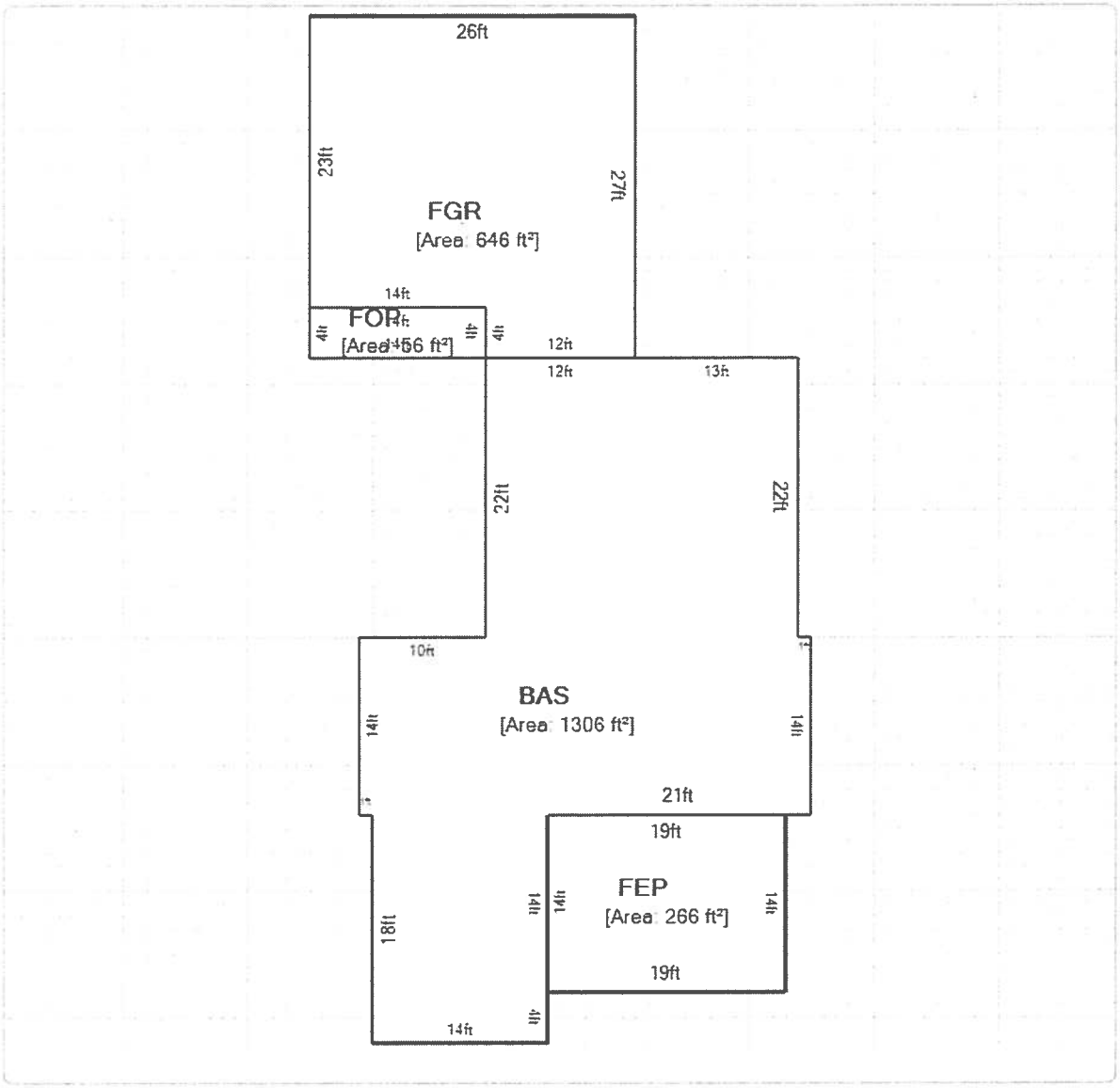
14602 NW 144th Street

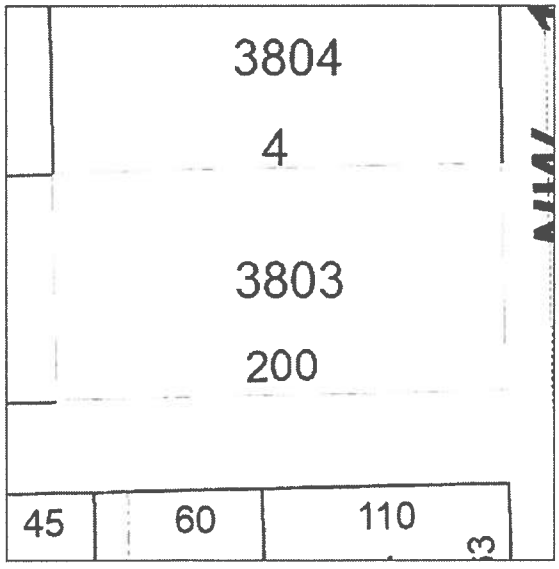




14611 NW 144th ST

No data available for the following modules: Extra Features, Photos.





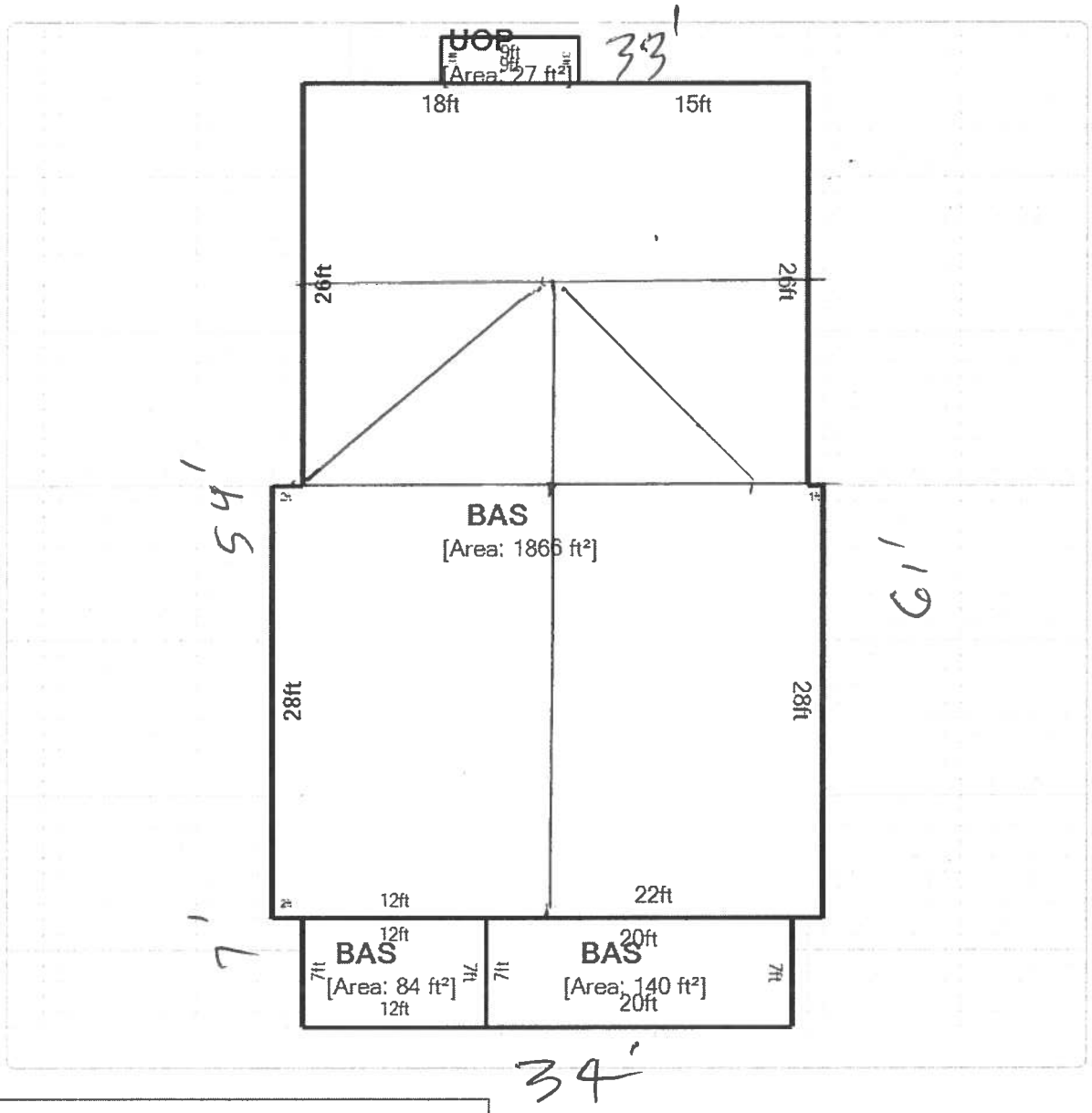
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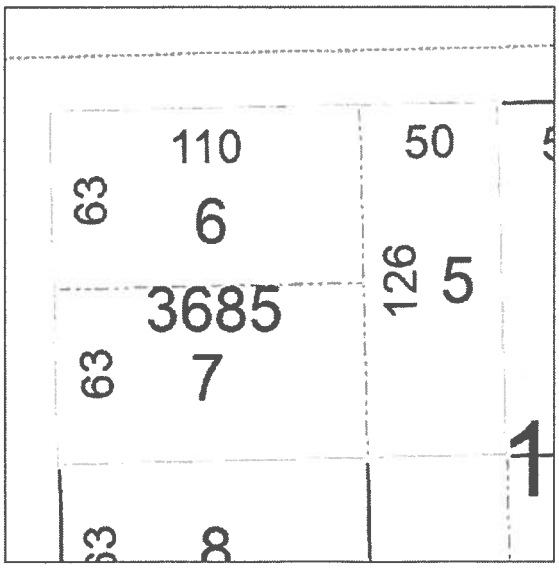


14319 NW 146th Avenue

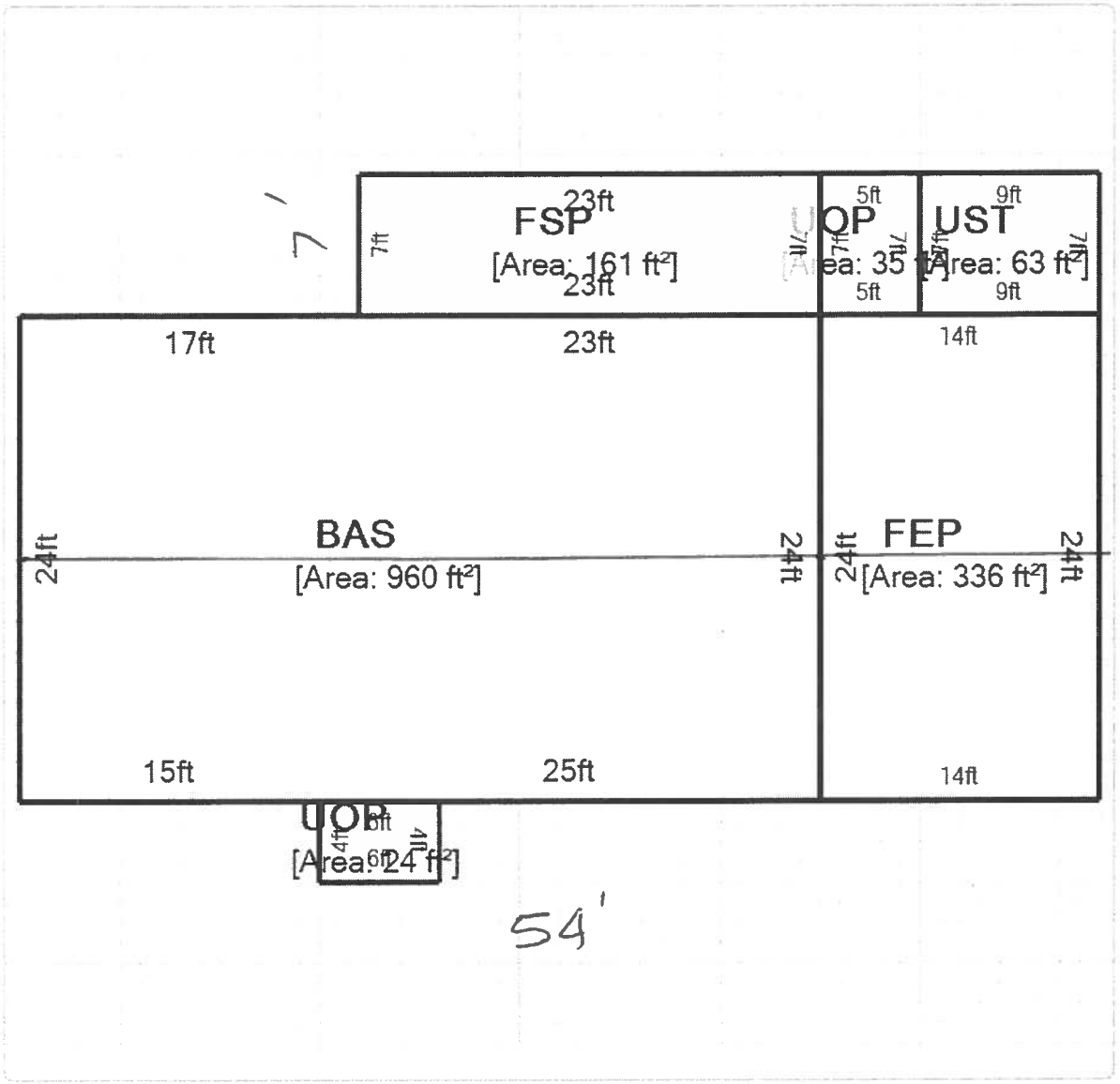


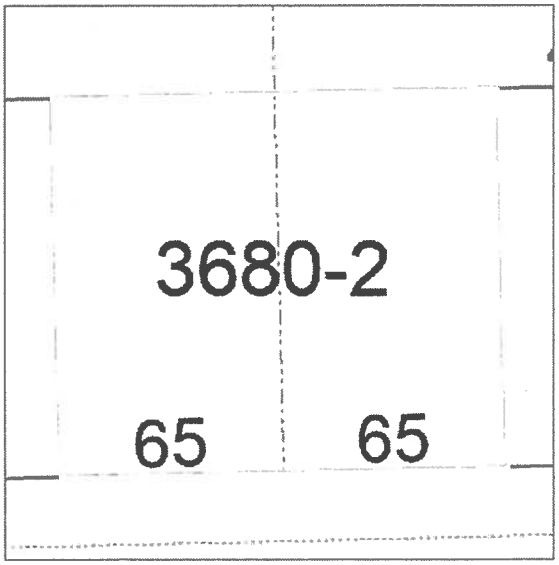
14304 NW 146th Avenue





No data available for the following modules: Extra Features, Permits, Photos.





No data available for the following modules: Extra Features, Photos.



14313 NW 148th Place - West Elevation



14313 NW 148th Place - North Elevation



14625 NW 144th Street



14285 NW 148th Place

CITY OF ALACHUA

P. O. BOX 9
ALACHUA, FL 32616-0009

(386) 418-6110
UTILITIES@CITYOFALACHUA.ORG
WWW.CITYOFALACHUA.COM

Receipt: 176803 03/10/2021 12:44 PM

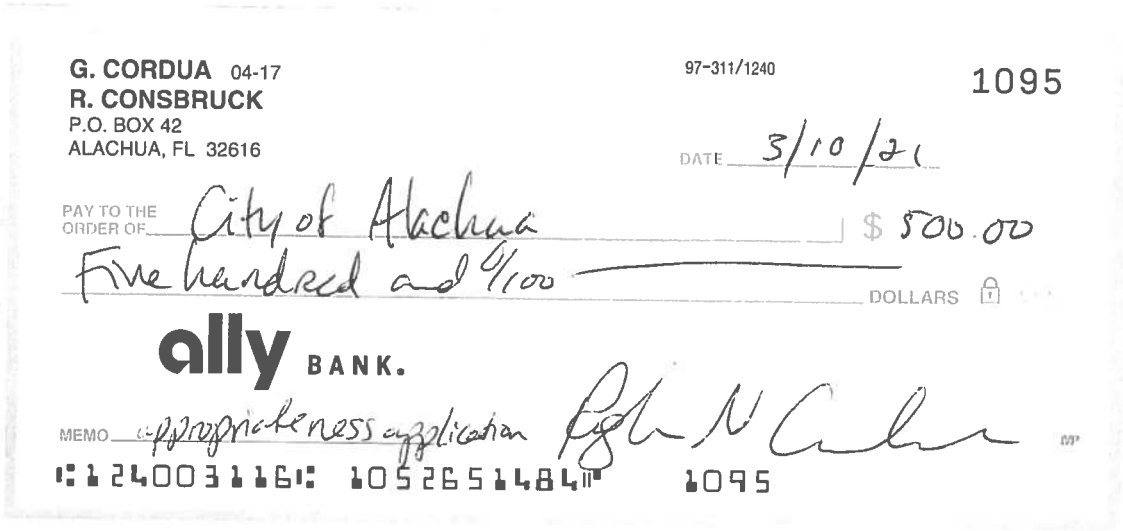
Cashier: mi_fields
Received Of: GOVINDA CORDUA AND RAGHU CONSBRU

PO BOX 42
ALACHUA FL 32615

The sum of: 500.00

BDINV	00003897		500.00
		Balance Due: 0.00	
		Total	500.00

TENDERED: CHECKS 1095 500.00



G. CORDUA 04-17
R. CONSBRUCK
P.O. BOX 42
ALACHUA, FL 32616

97-311/1240

1095

DATE 3/10/21

PAY TO THE ORDER OF City of Alachua $ 500.00

Five hundred and 0/100 DOLLARS

ally BANK.

MEMO appropriateness application

⑆124003116⑆ 105265148⑈ 1095

Text Label

14011 NW 144 TH ST.

80 78 76 74 72

460'-0" APPROX.

SHED

PROPERTY LINE 130.0'

6' HIGH WOOD FENCE

12.4'

GRAVEL PARKING

6" PVC DRAIN

4' GATE

34'

76.0
75.5

15'-6"

77.0 GRADE

15' SETBACK

OVERHEAD ELECTRIC TO NEIGHBOR

6'

77.4 PAVR

77.5

78.2

4'

77.4

16'

GARAGE
77.5

14'

22'

PAVERS

CONCRETE PAVER DRIVEWAY

77.0

76.5

18" GRATE

UTILITY POLE

12'

78.0

DECK

10'

WATER TAP LOCATION (PROPOSED)

WATER METER

W

SWALE

REMOVE TREE

74'

EXISTING DIRT DRIVEWAY 10' WIDE APPROX.

75.5 GRADE

16'

HOUSE
78.0

GENERAL DIRECTION OF SURFACE DRAINAGE

SCREENED PORCH

12'

7.5' SETBACK

110.0'

NW 144 TH ST.

EDGE OF PAVEMENT

110.0'

GRADE BREAK

2' ROOF OVERHANG

REMOVE TREE

20' SETBACK

PROPERTY LINE

SWALE

6'

14'

OPEN PORCH

76.5 GRADE

TYPICAL, TREE TO REMAIN.

72

14602 NW 144 ST.

60'-0" APPROX.

22'-0"

20' SETBACK

77.0

77.9

13.0'

DECK SIDE WALK

20'-6"

APPROX. SEWER TAP LOCATION.

PROPERTY LINE 130'

55'

EDGE OF PAVEMENT

6.7'

80 78 76 74

NW 146 TH AVE.

MANHOLE

BUILDING INFORMATION

HEATED	1972 SF
SCREENED PORCH	192 SF
OPEN PORCH	36 SF
GARAGE/CARPORT	476 SF
TOTAL	2676 SF

PROJECT INFORMATION:

PARCEL I.D. #	03679-001-000
LOT SIZE	110' x 130'
ZONING	RSF-3
FLOOD ZONE	X
LOT AREA	14,300 SF
	= .328 ACRES
BLDG. COVERAGE	2652 SF = 18.5%
DECKS	200 SF
PAVERS	132 SF
DRIVEWAY	352 SF

SITE PLAN

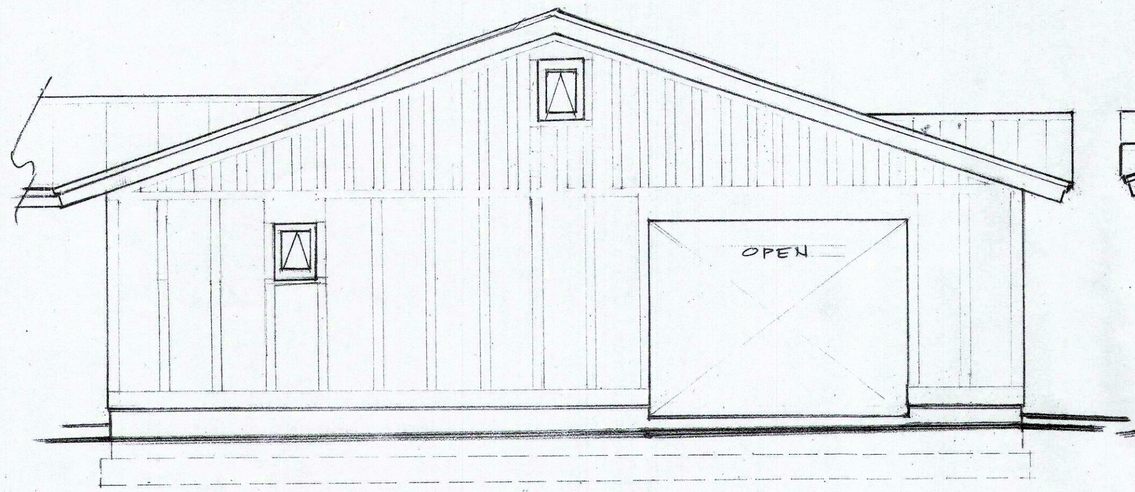
1"=10'

BASED ON SURVEY DATED 6-16-2020
BY DEREN LAND SURVEYING

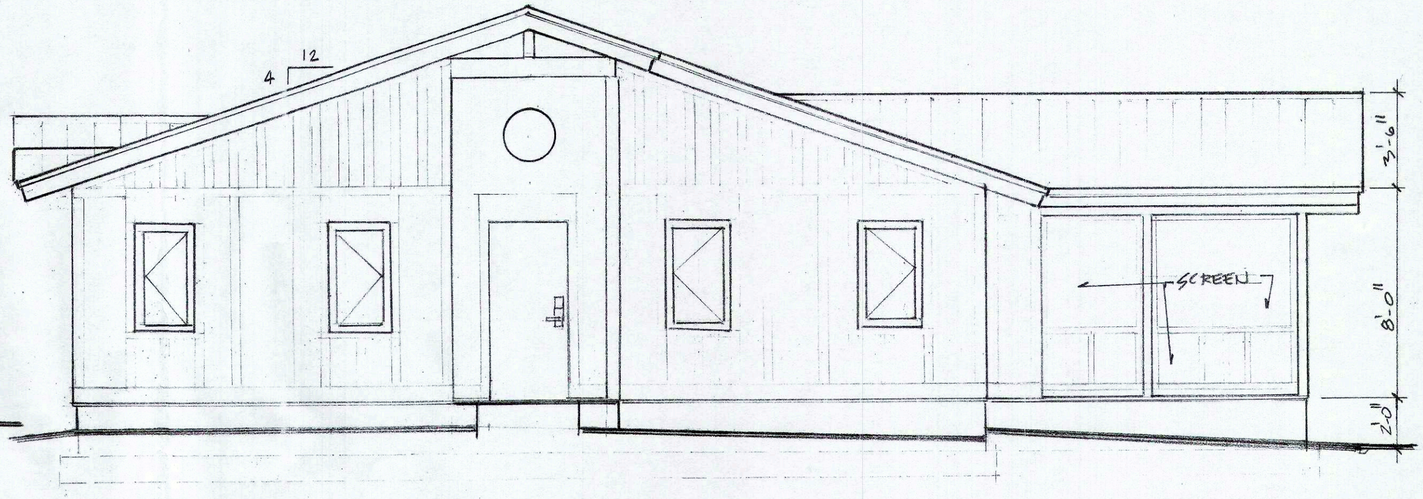
CONSBRUCK-CORDUA
NEW RESIDENCE
14444 NW 146 th AVE, ALACHUA, FL
DMC ARCHITECT 941-400-4027

SITE PLAN
1"=10'
3-8-21

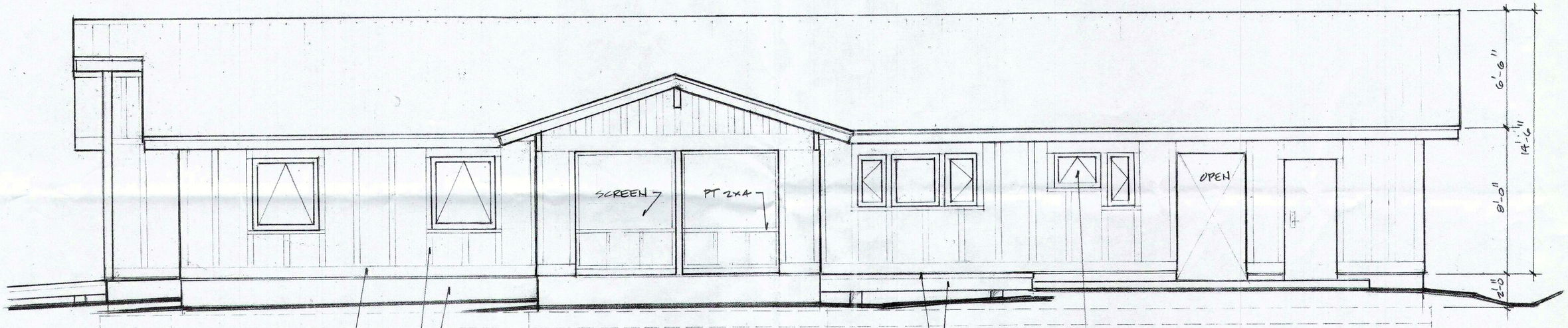
1 OF



NORTH ELEVATION



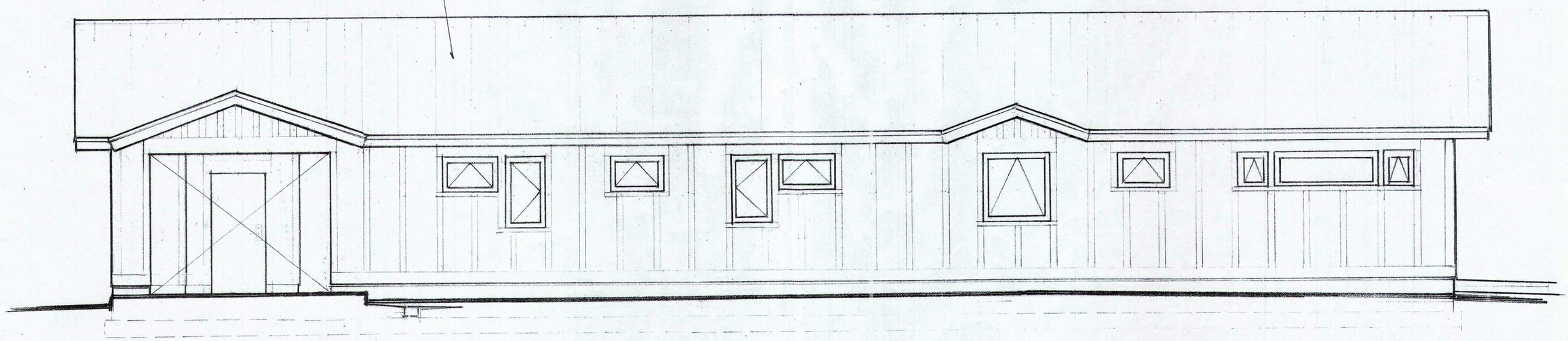
SOUTH ELEVATION



HARDIBOARD TRIM
HARDIBOARD SIDING & BATTENS
CMU STEMWALL
STANDING SEAM METAL ROOF

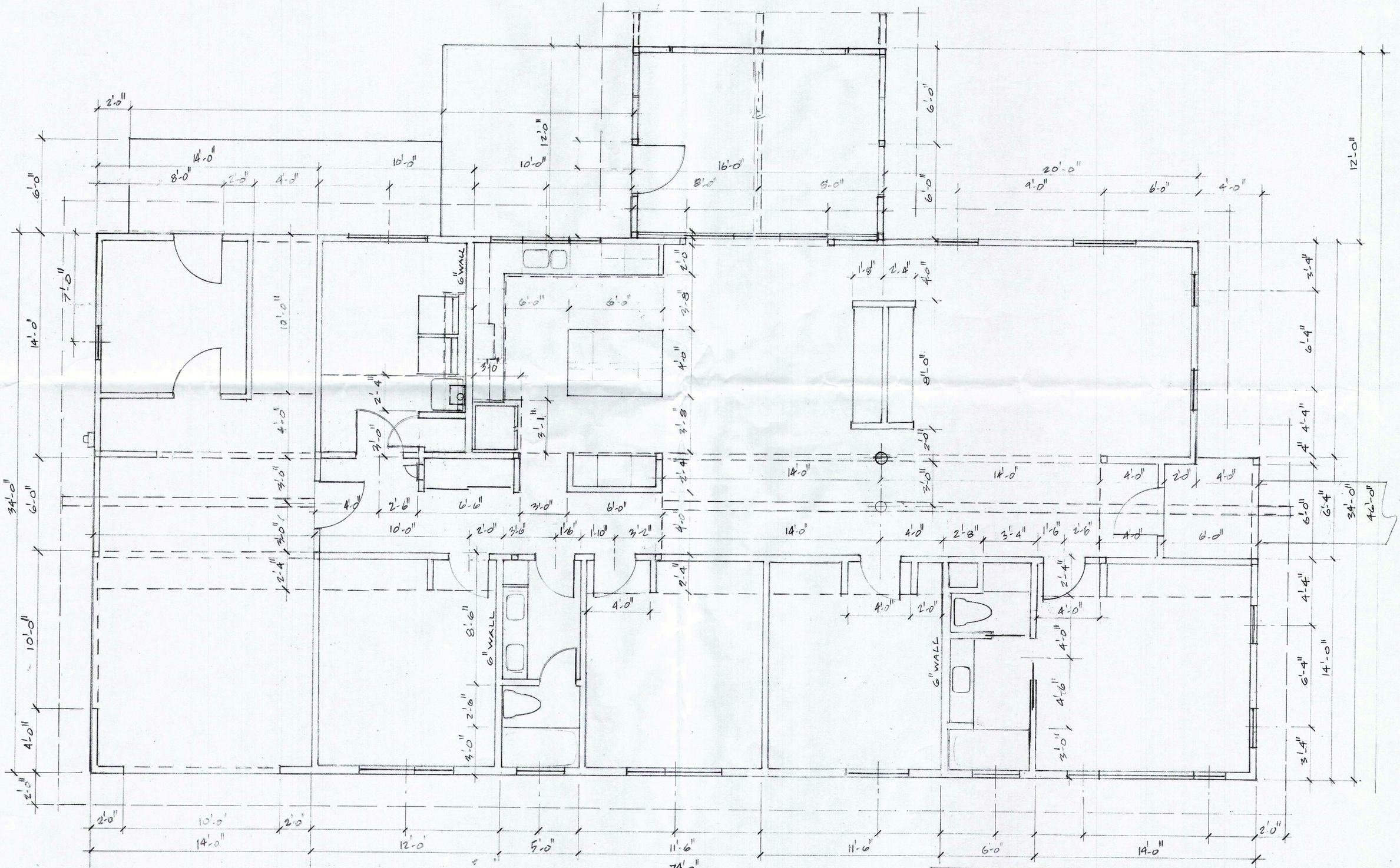
EAST ELEVATION

P.T. FRAME
COMPOSITE DECK
VINYL WINDOWS



WEST ELEVATION

CONSBRUCK - CORDUA NEW RESIDENCE ~~14444~~ NW 146 TH AVE, ALACHUA, FL DMC ARCHITECT 941-400-4027	ELEVATIONS 1/4" = 12" 3-8-21	2 OF



CONSBRUCK-CORDUA NEW RESIDENCE 14444 NW 146 th AVE, ALACHUA, FL DMC ARCHITECT 941-400-4027	FLOOR PLAN 1/4" = 12" 3-8-21	3

l u-turn caused a crash

u-turn, pulled into W. U.S. Highway 27 heading eastbound into oncoming raffic and collided with a 016 Chevrolet Silverado. The Silverado driver, 9-year-old Alice Knight, lso of Fort White, had riginally been behind the fitsubishi.

The two drivers and firage passenger 19-year-ld Jacob Petrovich of Fort /hite were transported to ea hospitals with neck, oulder and possible back juries.

Both vehicles were operable and were towed om the scene.

According to the High Springs Police Department, alcohol was not suspected to be related to the crash.

# # #

Email cwalker@alachuatoday.com

y of
ACHUA

BOARD
CY

accepting applications
wing Board:
gency (CRA) Advisory
s of the CRAAB is to
CRA Board regarding
t Area, including any
dance with the CRA
ership qualifications

other municipal office;
within the Community
ap of the Community
the CRA's webpage at

serve for a three-year

nd monthly meetings
third Monday of every

ll be available on the
.com, and at City Hall
a submission deadline
planned that the CRA
oint an individual for
10, 2021, at 5:00 p.m.
d June 12, 2024.

cation process may be
Coordinator, at

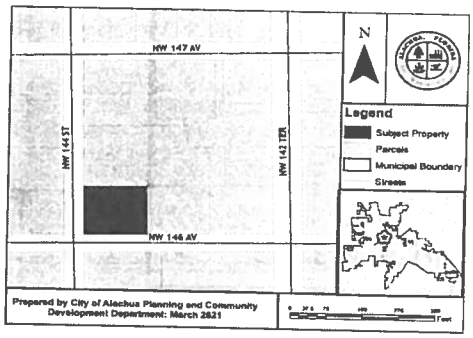
April 08 & 15, 2021)

y Today
ews - 386.462.3355

City of ALACHUA

NOTICE OF PUBLIC HEARING BEFORE THE PLANNING AND ZONING BOARD OF THE CITY OF ALACHUA, FLORIDA

Notice is hereby given that the Planning and Zoning Board of the City of Alachua will hold a public hearing on April 20, 2021, at 6:00 p.m. The hearing will be held in the James A. Lewis Commission Chambers in City Hall, located at 15100 NW 142nd Terrace, Alachua, Florida, to consider the following: a request by Raghu Consbruck and Govinda Cordua, property owners, for the approval of a Certificate of Appropriateness for the following alterations to a property located within the Historic Overlay District: construction of a new one story single family residence. Tax Parcel No. 03679-001-000; FLUM: Medium Density Residential; Zoning: Residential Single Family – 3 (RSF-3).



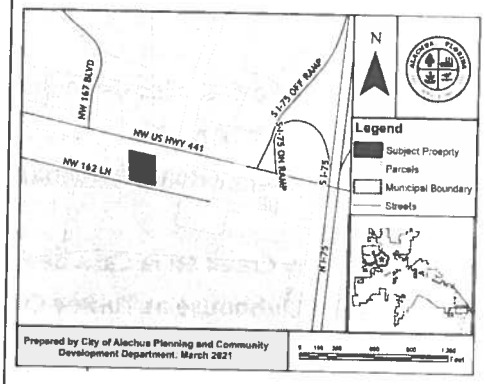
At the public hearing, all interested parties may appear and be heard with respect to the application. Copies of the application are available for public inspection at the Planning and Community Development Department, 15100 NW 142nd Terrace, Alachua, Florida, on any regular business day between the hours of 7:30 a.m. to 6:00 p.m. Written comments on the application may be sent to the following address: City of Alachua, Planning and Community Development, P.O. Box 9, Alachua, FL 32616. Notice is given pursuant to Section 286.0105, Florida Statutes, that, in order to appeal any decision made at the public hearing, you will need a record of the proceedings, and that, for such purpose, you may need to ensure that a verbatim record of the proceedings is made, which includes the testimony and evidence upon which the appeal is to be based. In accordance with the Americans with Disabilities Act, any persons with a disability requiring reasonable accommodation in order to participate in this meeting should call the City Clerk at (386) 418-6100 x 1501 at least 48 hours prior to the public hearing

(Published: Alachua County Today - April 08, 2021)

City of ALACHUA

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Notice is hereby given that the Planning and Zoning Board of the City of Alachua will hold a public hearing on April 20, 2021, at 6:00 p.m. The hearing will be held in the James A. Lewis Commission Chambers in City Hall, located at 15100 NW 142nd Terrace, Alachua, Florida, to consider the following: A request by Daniel Young, P.E, of CHW, Inc., applicant and agent, for T&H Property Group, LLC and Sarcav, LLC, property owners, for consideration of a Site Plan to construct a ±1,904 square foot quick service restaurant with associated parking, stormwater, landscaping, and site improvements on a ±1.06 acre subject property. Consisting of Tax Parcel Number 03053-002-001 and a portion of 03053-002-000; FLUM: Commercial; Zoning: Commercial Intensive (CI); Overlay District: Gateway Overlay District.



At the public hearing, all interested parties may appear and be heard with respect to the application. Copies of the application are available for public inspection at the Planning and Community Development Department, 15100 NW 142nd Terrace, Alachua, Florida, on any regular business day between the hours of 7:30 a.m. to 6:00 p.m. Written comments on the application may be sent to the following address: City of Alachua, Planning and Community Development, P.O. Box 9, Alachua, FL 32616. Notice is given pursuant to Section 286.0105, Florida Statutes, that, in order to appeal any decision made at the public hearing, you will need a record of the proceedings, and that, for such purpose, you may need to ensure that a verbatim record of the proceedings is made, which includes the testimony and evidence upon which the appeal is to be based. In accordance with the Americans with Disabilities Act, any persons with a disability requiring reasonable accommodation in order to participate in this meeting should call the City Clerk at (386) 418-6100 x 1501 at least 48 hours prior to the public hearing.

(Published: Alachua County Today - April 08, 2021)

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is your best source for local news - 386.462.3355

City of Alachua
THE GOOD LIFE COMMUNITY

Board/Committee Agenda Item

MEETING DATE: 4/20/2021
SUBJECT: Site Plan – KFC : A request by Daniel Young, P.E., of CHW, Inc., applicant and agent, for T&H Property Group, LLC, and Sarcav, LLC, property owners, for consideration of a Site Plan to construct a ±1,904 square foot quick service restaurant with associated parking, stormwater, landscaping, and site improvements on a ±1.06 acre subject property. Tax Parcel Number 03053-002-001 and a portion of 03053-002-000 (Quasi-Judicial Hearing).

PREPARED BY: Adam Hall, AICP, Planner

RECOMMENDED ACTION:
Staff recommends that the Planning & Zoning Board approve the Site Plan for a restaurant with drive-through service on Tax Parcel Number 03053-002-001 and a portion of 03053-002-000, subject to the four (4) conditions provided in Exhibit "A" and located on page 20 of the April 20, 2021 Staff Report to the Planning & Zoning Board, upon making the following motion:

Based upon the competent substantial evidence presented at this hearing, the presentation before this Board, and Staff's recommendation, this Board finds the application to be consistent with the City of Alachua Comprehensive Plan and in compliance with the Land Development Regulations and approves the Site Plan for a restaurant with drive-through service on Tax Parcel Number 03053-002-001 and a portion of 03053-002-000, subject to the four (4) conditions provided in Exhibit "A" and located on page 20 of the April 20, 2021 Staff Report to the Planning & Zoning Board.

Summary

This application is a request by Daniel Young, P.E., of CHW, Inc., applicant and agent, for the T&H Property Group, LLC and Sarcav, LLC., property owners, for consideration of a Site Plan to construct a ±1,904 square foot restaurant with drive-through service.

The subject property is ±1.06 acres in size and is located to the south of US HWY 441, to the west of I-75, and to the north of 162nd Lane. The property is adjacent to the recently approved Burger King development. It will be served by two means of ingress/egress. One will be a new shared access connection to US 441. This connection will be shared with the parcel to the west. There is no development proposal for this property to the west at this time. The second point of ingress/egress will be a shared access with the Burger King restaurant to the east. Stormwater treatment will be provided on site.

ATTACHMENTS:

Description

- Staff Report and Staff Supporting Materials
- KFC Site Plan
- Application and Supporting Materials
- Public Notice Materials for the 04/20/21 Planning and Zoning Board Hearing

City of Alachua
Planning & Community Development Department
Staff Report

Planning & Zoning Board Hearing Date: April 20, 2021
Quasi-Judicial Hearing

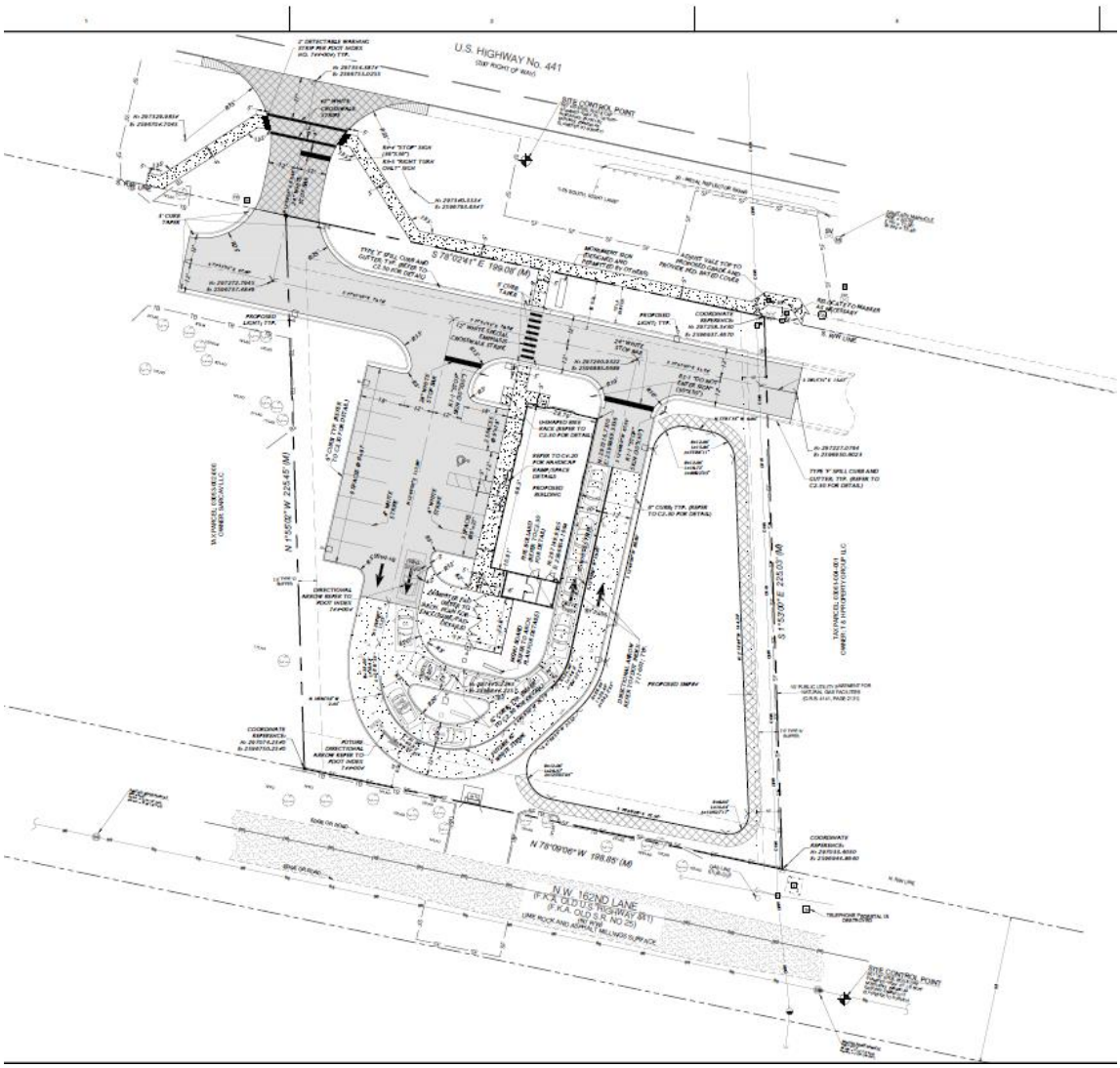
SUBJECT:	A request for a Site Plan to construct a ±1,904 square foot restaurant with drive-through service and associated site improvements
APPLICANT/AGENT:	Daniel Young, P.E., of CHW, Inc.
PROPERTY OWNER:	T&H Property Group, LLC and Sarcav, LLC.
LOCATION:	South of US Highway 441, west of I-75, north of NW 162nd Lane
PARCEL ID NUMBER:	03053-002-001 and a portion of 03053-002-000
FLUM DESIGNATION:	Commercial
ZONING:	Commercial Intensive (CI)
OVERLAY:	Gateway Overlay District
ACREAGE:	±1.06 acres
PROJECT PLANNER:	Adam Hall, AICP
RECOMMENDATION:	Staff recommends that the Planning & Zoning Board approve the Site Plan for a restaurant with drive-through service on Tax Parcel Number 03053-002-001 and a portion of 03053-002-000, subject to the four (4) conditions provided in Exhibit "A" and located on page 20 of the April 20, 2021 Staff Report to the Planning & Zoning Board.
RECOMMENDED MOTION:	*Based upon the competent substantial evidence presented at this hearing, the presentation before this Board, and Staff's recommendation, this Board finds the application to be consistent with the City of Alachua Comprehensive Plan and in compliance with the Land Development Regulations and approves the Site Plan for a restaurant with drive-through service on Tax Parcel Number 03053-002-001 and a portion of 03053-002-000, subject to the four (4) conditions provided in Exhibit "A" and located on page 20 of the April 20, 2021 Staff Report to the Planning & Zoning Board.*

SUMMARY

This application is a request by Daniel Young, P.E., of CHW, Inc., applicant and agent, for the T&H Property Group, LLC and Sarcav, LLC., property owners, for consideration of a Site Plan to construct a ±1,904 square foot restaurant with drive-through service.

The subject property is ±1.06 acres in size and is located to the south of US HWY 441, to the west of I-75, and to the north of 162nd Lane. The property is adjacent to the recently approved Burger King development. It will be served by two means of ingress/egress. One will be a new shared access connection to US 441. This connection will be shared with the parcel to the west. There is no development proposal for this property. The second point of ingress/egress will be a shared access with the Burger King restaurant to the east. Stormwater treatment will be provided on site.

Illustration 1. KFC Site Plan – Site & Dimension Plan



Map 1. FLUM



Map 2. Zoning Map



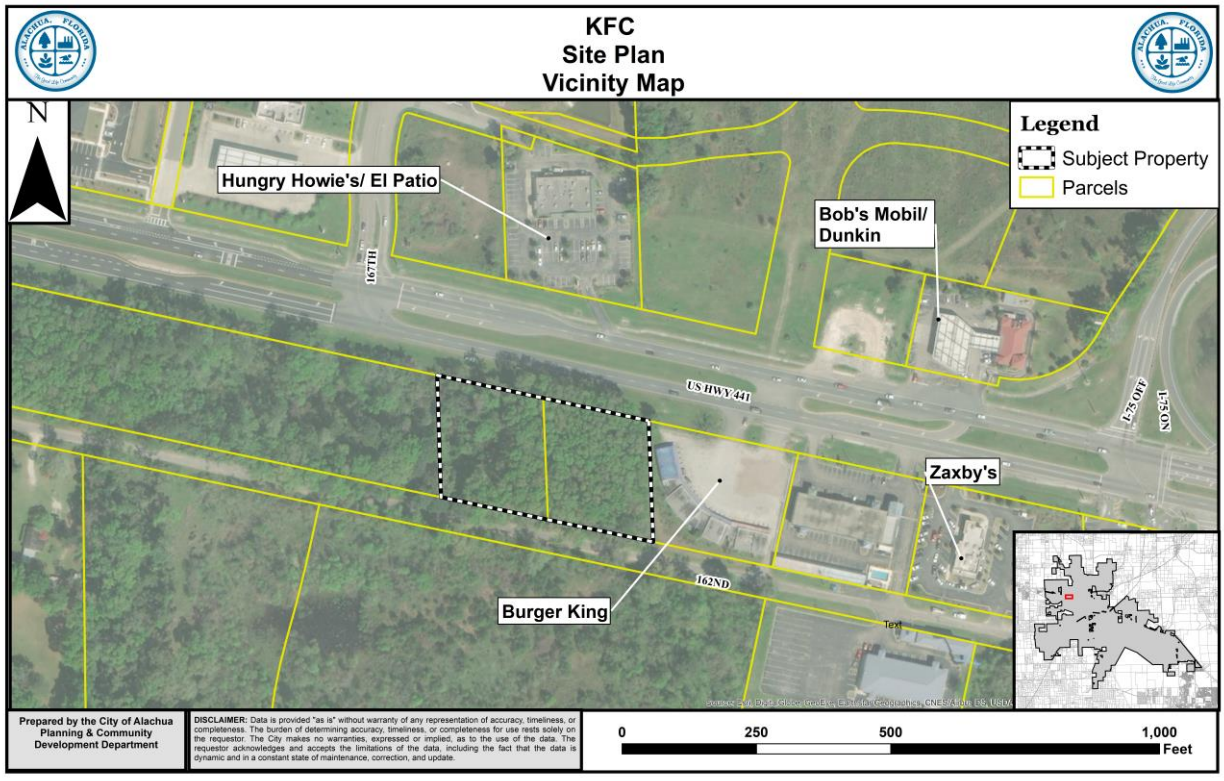
SURROUNDING USES

The existing uses, Future Land Use Map (FLUM) Designations, and zoning districts of the surrounding area are identified in Table 1. Map 1 provides an overview of the vicinity of the subject property. (NOTE: The information below is intended to provide a general overview of the area surrounding the subject property and to generally orient the reader. It is not intended to be all-inclusive, and may not identify all existing uses, FLUM Designations, and/or zoning districts surrounding the subject property.

Table 1. Surrounding Land Uses

Direction	Existing Use(s)	FLUM Designation(s)	Zoning District(s)
North	US 441/ Hungry Howie's/ HighPoint Crossing	Commercial	CI (Commercial Intensive)
South	Vacant Commercial	Commercial	CI (Commercial Intensive)
West	Vacant Commercial	Commercial	CI (Commercial Intensive)
East	Burger King	Commercial	CI (Commercial Intensive)

Map 3. Vicinity Map



NEIGHBORHOOD MEETING

The purpose of a Neighborhood Meeting is to educate the owners of nearby land and any other interested members of the public about the project and to receive comments regarding the project. As required by Section 2.2.4 of the LDRs, all property owners within 400 feet of the subject property were notified of the meeting and notice of the meeting was published in a newspaper of general circulation.

A virtual Neighborhood Meeting was held virtually on November 19, 2020 to educate the owners of nearby land and any other interested members of the public about the application. As required by Section 2.2.4 of the LDRs, all property owners within 400 feet of the subject property and any organizations or persons who have registered to receive notification of applications for development were notified of the meeting. In addition, a notice of the meeting was published in a newspaper of general circulation. The applicant's agent was present and available to answer questions. As evidenced by materials submitted by the applicant, the meeting was not attended by any members of the general public.

CONSISTENCY WITH THE COMPREHENSIVE PLAN

The Goals, Objectives, and Policies (GOPs) identified below are provided to establish a basis of the application's consistency with the Comprehensive Plan. There may be additional GOPs which the application is consistent with that are not identified within this report. An evaluation and findings of consistency with the identified GOPs is also provided below.

Future Land Use Element

Goal 1: *Future Land Use Map 2035*

The City shall maintain a Future Land Use Map in order to effectively guide development in a sustainable manner and to ensure economic prosperity and stability while maintaining a high quality of life for all of its present and future citizens, businesses, and visitors.

Objective 1.3: *Commercial*

The City shall establish three commercial land use categories: Community Commercial, Commercial, and Central Business District. These land use categories shall provide a broad range of commercial uses, including retail sales and services, personal services, offices, and tourist-oriented uses, in order to provide for the availability of goods and services, both to the citizens and visitors of Alachua and to the citizens of the North Central Florida region.

Policy 1.3.b: *Commercial*: The Commercial land use category is established to provide for general commercial uses, as well as more intense commercial and highway commercial uses. This is the land use category in which large-scale, regional commercial uses may locate. The following uses are allowed within the Commercial land use category:

1. Retail sales and services;
2. Personal services;
3. Financial institutions;
4. Outdoor recreation and entertainment;
5. Tourist-related uses;
6. Hotels and motels;
7. Commercial shopping centers;
8. Auto-oriented uses;
9. Traditional Neighborhood Planned Developments;
10. Employment Center Planned Developments;
11. Commercial recreation centers;
12. Office/business parks;
13. Limited industrial services; and,
14. **Eating establishments.**

Analysis of Consistency with Goal 1, Objective 1.2, and Policy 1.2.a: The proposed site plan would be consistent with permitting an eating establishment. There is no proposed change of future land use designation or zoning designation.

Objective 5.1: Natural features: The City shall coordinate Future Land Use designations with appropriate topography, soils, areas of seasonal flooding, wetlands and habitat during review of proposed amendments to the Future Land Use Map and the development review process. Natural features may be included as amenities within a development project.

Policy 5.1.a: Topography: The City shall protect the natural topography of the City, including steep and seepage slopes, by requiring new development to include techniques to minimize negative impacts on the natural terrain. An emphasis will be placed on retaining the natural function of seepage slopes during development. Additionally, retention of existing native vegetation will be encouraged as one method of protecting slopes.

Policy 5.1.b: Soils: The City shall ensure soil protection and intervention measures are included in the development review process.

Policy 5.1.c: Flood prone areas: The City shall require as part of the development review process the identification of FEMA flood zone areas. Where necessary, base flood elevations and minimum finished floor elevations shall be established. The City shall also require finished floor elevations on subdivision plats, site plans and building permit plans when necessary to determine compliance with flood prone area regulations. The City shall establish standards for a limitation on filling in flood prone areas.

Policy 5.1.d: Wetlands: The City shall utilize statewide wetland delineation methodology in accordance with Florida Administrative Code (FAC)

and regulations adopted by the FDEP and the Suwannee River Water Management District.

Policy 5.1.e: Habitat: The City shall require as part of the development review process, an inventory of listed species for all new developments in areas identified as known habitat for listed species if listed species are known to exist in close proximity to the development. The survey shall include detailed information regarding type, quantity, location, and habitat requirements for any listed species identified. A de minimus threshold for properties required to complete the inventory shall be established in the City's Land Development Regulations.

Analysis of Consistency with Objective 5.1 and Policies 5.1.a – e: Based on the best available data, there are no significant environmental resources that would limit or impact development.

Objective 5.2: Availability of facilities and services: All new development shall be planned and constructed concurrently with the availability of facilities and services necessary for the development.

Analysis of Consistency with Objective 5.2: The subject property is located within the City's potable water and wastewater service areas, as defined in Policies 1.2.a and 4.2.a of the Community Facilities and Natural Groundwater Aquifer Recharge Element of the Comprehensive Plan, and will be required to connect to the City of Alachua's potable water and wastewater system.

Objective 9.1: *Connections to Water and Wastewater Systems*

The City will require new development and significant redevelopment projects to connect to the City's potable water and wastewater systems when such connections can be made to such systems.

Policy 9.1: Any new development or redevelopment within a Commercial or Industrial land use category within the corporate limits, where potable water and wastewater service are available, as defined in Policy 1.2.a and Policy 4.2.b of the Community Facilities Element of the City's Comprehensive Plan, shall connect to the City's potable water and wastewater system.

Analysis of Consistency with Goal 9 and Policy 9.1: The subject property is within the potable water and wastewater service area. Any development of the subject property will be required to connect to the potable water and wastewater systems at the time of development.

Transportation Element

Objective 1.1: Level of Service: The City shall establish a safe, convenient and efficient level of service standard for all motorized and non-motorized transportation systems.

Analysis of Consistency with Objective 1.1: An analysis of the impacts to transportation facilities has been provided within this report. The proposed development would not result in a decrease in the level of service for transportation facilities monitored for concurrency.

Community Facilities & Natural Groundwater Aquifer Recharge Element

Policy 1.2.a: The City shall establish a Community Wastewater Service Area, which includes all areas where wastewater service is available. Wastewater service shall be deemed available if:

1. A gravity wastewater system, wastewater pumping station, or force main exists within ¼ mile of the property line of any residential subdivision with more than 5 units, or any multi-family residential development, or any commercial development, or any industrial development and the gravity wastewater system, wastewater pumping station, or force main can be accessed through public utility easements or right of ways. The distance shall be measured as required for construction of the infrastructure along public utility easements and right of ways.

Analysis of Consistency with Policy 1.2.a: The subject property is located within the wastewater service area, and any future development on the subject property will be required to connect to the wastewater system.

Policy 2.1.a: The City hereby establishes the following level of service standards for solid waste disposal facilities:

FACILITY TYPE	LEVEL OF SERVICE STANDARD
Solid Waste Landfill	.73 tons per capita per year

Analysis of Consistency with Objective 2.1.a: An analysis of the impacts to solid waste facilities has been provided within this report. The proposed amendment would not result in a decrease in the level of service for solid waste disposal facilities.

Policy 4.1.b: The City shall establish a Community Potable Water Service Area, which includes all areas where potable water service is available. Water service shall be deemed available if:

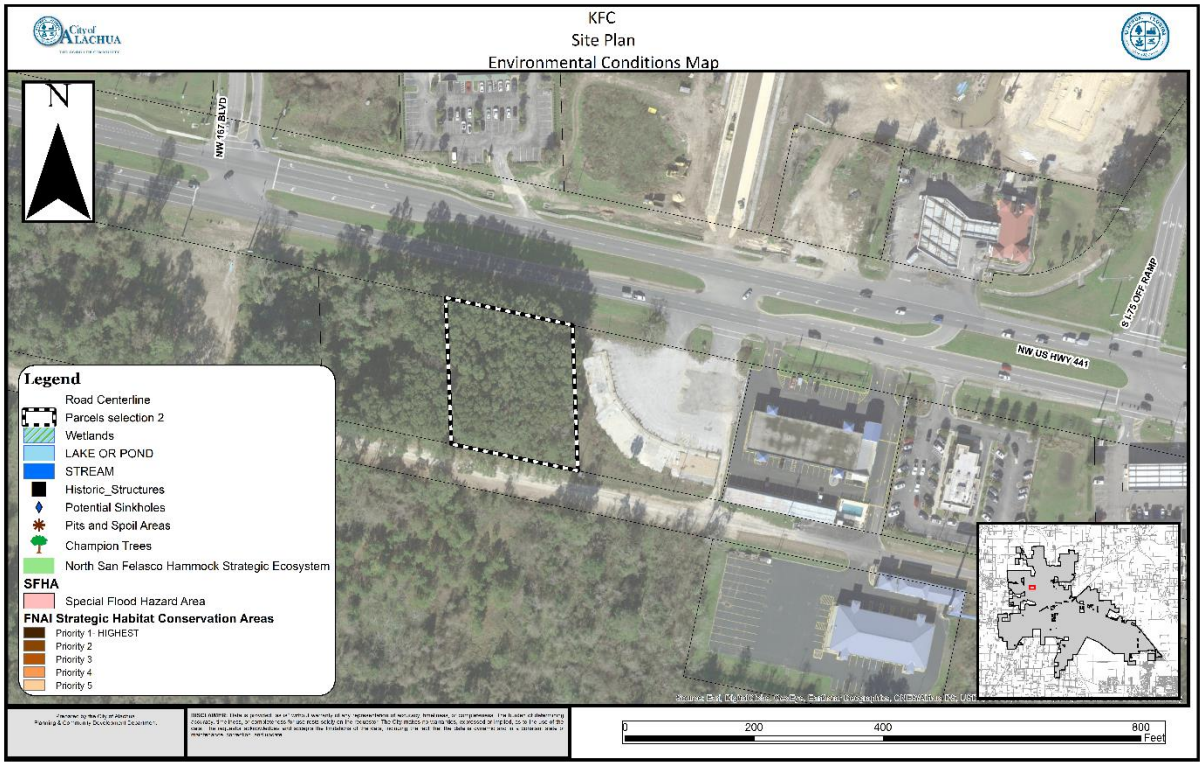
1. A water main exists within ¼ mile of any residential subdivision with more than 5 units, or any multi-family residential development, or any commercial development, or any industrial development and water service can be accessed through public utility easements or right of ways. The distance shall be measured

as required for construction of the infrastructure along public utility easements and right of ways.

Analysis of Consistency with Policy 4.1.b: The subject property is located within the potable water service area, and any future development on the subject property will be required to connect to the potable water system.

ENVIRONMENTAL CONDITIONS ANALYSIS

Map 4. Environmental Features



Wetlands

According to National Wetlands Inventory, no potential wetlands are located on the subject property. Any wetlands identified must be delineated and protected in accordance with the applicable protection standards.

Evaluation: No wetlands have been identified on subject property. If wetlands are identified on subject property at a later time, the applicable standards in the City's Comprehensive Plan, Land Development Regulations, and Suwannee River Water Management District (SRWMD) regulations would apply to those areas identified as wetlands; therefore, there are no issues related to wetland protection.

Strategic Ecosystems

Strategic Ecosystems were identified by an ecological inventory project in a report prepared for Alachua County Department of Growth Management in 1996. The purpose of the inventory was to identify, inventory, map, describe, and evaluate the most significant natural biological communities in private ownership in Alachua County.

Evaluation: The subject property is not located within or adjacent to a Strategic Ecosystem, therefore, the development will have no impact upon any Strategic Ecosystem(s) identified within the ecological inventory report.

Regulated Plant & Animal Species

Based on the best available data, there are no regulated plant or animal species that would limit or impact development.

Evaluation: Based on the best available data, there are no regulated plant or animal species that would limit or impact development. If, however, a regulated plant or animal species is encountered during development, the applicant would be responsible for meeting all applicable state, federal, and local laws regarding the regulated species.

Soil Survey

Each soil type found on the subject property is identified below. The hydrologic soil group is an indicator of potential soil limitations. The hydrologic soil group, as defined for each specific soil, refers to a group of soils which have been categorized according to their runoff-producing characteristics. These hydrologic groups are defined by the Soil Survey of Alachua County, Florida, dated August 1985. The chief consideration with respect to runoff potential is the capacity of each soil to permit infiltration (the slope and kind of plant cover are not considered, but are separate factors in predicting runoff). There are four hydrologic groups: A, B, C, and D. “Group A” soils have a higher infiltration rate when thoroughly wet and therefore have a lower runoff potential. “Group D” soils have very lower infiltration rates and therefore a higher runoff potential.

Table 2. Soil Analysis

Soil Type	Hydrologic Group	Drainage Class	Small Commercial Buildings	% of Subject Property (may not total to 100% due to rounding)
Arredondo-Urban land complex, 0 to 5 percent slopes	A	Well drained	Not limited	68.1%
Kendrick sand, 2 to 5 percent slopes	B	Well drained	Somewhat limited	31.9%

Evaluation: The soil type located within the subject property do not pose any significant limitations for small commercial buildings. Therefore, there are no issues related to soil suitability. Any future development would require that any soil limitations be addressed at the time of development.

Flood Potential

Panel 120D of the Federal Emergency Management Agency (FEMA) Flood Insurance Rate Map (FIRM) Series, dated June 16, 2006, indicates that the subject property contains areas with Flood Zone "X" designations (areas determined to be outside of the 500-year floodplain).

Evaluation: The subject property is located in Flood Zone “X” (areas determined to be outside of the 500-year floodplain. Therefore, there are no issues related to flood potential.

Karst-Sensitive Features

Karst sensitive areas include geologic features, such as fissures, sinkholes, underground streams, and caverns, and are generally the result of irregular limestone formations. The subject property is located within an area where sinkholes may potentially allow hydrologic access to the Floridan Aquifer System, however, best available data indicates that no sinkholes or known indicators of sinkhole activity are located on the subject property.

Evaluation: There are no geologic features located on the subject property which indicate an increased potential for karst sensitivity.

Wellfield Protection Zone

Policy 7.2.1 of the Future Land Use Element of the Comprehensive Plan establishes a 500 foot radial buffer around city-owned potable water well.

Evaluation: The subject property is not located within a City of Alachua wellhead protection zone as identified on the City of Alachua Wellfield Primary Protection Zones Map of the Comprehensive Plan, therefore, there are no issues related to wellfield protection.

Historic Structures and Markers

The subject property does not contain any historic structures or markers as determined by the State of Florida and the Alachua County Historic Resources Inventory.

Evaluation: There are no issues related to historic markers or structures.

FINDINGS OF FACT: COMPLIANCE WITH LAND DEVELOPMENT REGULATIONS

SITE PLAN STANDARDS

Section 2.4.9(E) of the City's Land Development Regulations (LDRs) establishes the standards with which all site plans must be found to be compliant. The application has been reviewed for compliance with the standards of Section 2.4.9(E.) An evaluation and findings of the application's compliance with the standards of Section 2.4.9(E) is provided below.

(E) Site Plan Standards

A Site Plan shall be approved only upon a finding the applicant demonstrates all of the following standards are met:

(1) ***Consistency with Comprehensive Plan***
The development and uses in the Site Plan comply with the Goals, Objectives and Policies of the Comprehensive Plan.

Evaluation & Findings: An analysis of the application's consistency with the Comprehensive Plan has been provided in this report.

(2) ***Use Allowed in Zone District***
The use is allowed in the zone district in accordance with Article 4: *Use Regulations*.

Evaluation & Findings: The subject property is zoned Commercial Intensive (CI). The site plan proposes a new quick service restaurant, which is classified as a "restaurant with drive-through or drive-in service". This use is defined as follows in Article 10 of the LDRs: "*an establishment where provision is made on the premises for the selling, dispensing, or serving of food, refreshments, or beverages to persons in automobiles and/or in other than a completely enclosed*

building on the premises, including those establishments where customers may serve themselves and may eat or drink the food, refreshments or beverages in automobiles on the premises. A restaurant which provides drive-in facilities of any kind shall be deemed a drive-in restaurant. Such an establishment by design encourages or permits customers to obtain goods while remaining in their motor vehicles." Table 4.1-1 of the LDRs sets forth the uses allowed within each zoning district. A restaurant with drive-through or drive-in service is permitted in the CI zoning district.

(3) ***Zone District Use-Specific Standards***

The development and uses in the Site Plan comply with Section 4.3, *Use-Specific Standards*.

Evaluation & Findings: Section 4.4.4 (A) (2) provides use-specific standards for restaurants with drive-through service. The proposed site plan complies with these use-specific standards. These standards are below:

(a)Off-street parking and loading. All principal uses with drive-through or drive-in services shall comply with all applicable standards governing vehicle-stacking areas and drive-in facilities set forth in Section 6.1, Off-street parking and loading standards.

(b)Transitional use area requirement. When such use is located within a CI, ILW or IG district which adjoins a residential district, and is located upon the same frontage as the residential district without an intervening street, drive-throughs or drive-ins may not be closer than 100 feet from the residential district boundary line.

(4) ***Development and Design Standards***

The development proposed in the Site Plan and its general layout and design comply with all appropriate standards in Article 6: *Development Standards*.

Evaluation & Findings: The application has been reviewed for and is found to be in compliance with all relevant provisions of Article 6, *Development Standards*, including but not limited to *Section 6.1*, Off Street Parking & Loading Standards, *Section 6.2*, Tree Protection/Landscape/Xeriscape Standards, *Section 6.3*, Fencing Standards, *Section 6.7*, Open Space Standards, Section 6.8, *Design Standards for Business Uses*, and *Section 6.9*, Environmental Protection Standards.

Additionally, the subject property is located within the US 441/I-75 Gateway Overlay District. Please reference the section of this report entitled "Findings of Fact: Compliance with Land Development Regulations, US 441/I-75 Gateway Overlay District" for further information regarding compliance with the applicable standards for development within the US 441/I-75 Gateway Overlay District.

(5) ***Subdivision Standards***
In cases where a subdivision has been approved or is pending, the development proposed in the Site Plan and its general layout and design comply with all appropriate standards in Article 7: *Subdivision Standards*.

Evaluation & Findings: There is no subdivision of the property proposed, therefore this standard is not applicable.

(6) ***Complies with All Other Relevant Laws and Ordinances***
The proposed site plan development and use complies with all other relevant City laws and ordinances, state and federal laws, and regulations.

Evaluation & Findings: The application is consistent with all other relevant City ordinances and regulations.

COMPLIANCE WITH SECTION 6.6.3, RESIDENTIAL PROTECTION STANDARDS

Section 6.6.3 of the City's Land Development Regulations (LDRs) establishes additional conditions that may be imposed for development located within 500 feet of any residential district or adjacent to any existing single-family or two-family development. There are no residential zoned lands within 500' of the subject property and the property is not adjacent to existing single-family or two-family development.

SECTION 6.8, DESIGN STANDARDS FOR BUSINESS USES

Section 6.8 of the City's Land Development Regulations (LDRs) establishes design standards for business uses. The standards established within Section 6.8.2 apply to business use types (unless otherwise exempt), except for single tenant retail sales and services uses greater than or equal to 20,000 square feet or unless otherwise exempted within Subsection 6.8.1(B.). The standards established within Section 6.8.3 apply to single tenant retail sales and services uses greater than 20,000 square feet.

The proposed development is classified as a restaurant with drive-through or drive-in service, which is a business use type and therefore subject to compliance with the requirements of Section 6.8.2. The standards of Section 6.8.2 establish requirements for front façades; façades facing streets or lands zoned for or used for residential uses; screening of mechanical equipment; and connections to existing sidewalk systems. Façade requirements primarily relate to glazing (e.g., windows/glass); façade massing; materials used for side and rear façades; and a prohibition on the use of certain building materials. The application has been reviewed for and is found to be in compliance with the standards of Section 6.8.2.

US 441/I-75 GATEWAY OVERLAY DISTRICT

Section 3.7.2(C) of the LDRs establishes the US 441/I-75 Gateway Overlay District. These standards are intended to recognize of the importance of promoting the City as an attractive, vibrant, and economically prosperous community. The purpose of the US 441/I-75 Gateway Overlay District include welcoming existing and future residents and visitors to the City; establishing consistent and harmonious design standards in order to unify the visual quality

of the Gateway Overlay District; creating an enhanced visual gateway to an area that serves as a main entrance to the City; improving the sense of place and community; and maintaining and enhancing property values.

The standards of Section 3.7.2(C) apply to all lands located within 2,000 feet of the U.S. Highway 441 / Interstate 75 interchange. The subject property is located within the US 441/I-75 Gateway Overlay District and is therefore subject to compliance with the requirements of Section 3.7.2(C).

Section 3.7.2(C)(5) establishes development standards for development within the US 441/I-75 Gateway Overlay District. These standards establish requirements for building design and orientation; fencing; outside storage areas; street buffers; the location of parking and loading areas; and signage.

The application has been reviewed for and is found to be in compliance with the applicable standards of Section 3.7.2(C).

PUBLIC FACILITIES IMPACT

The analysis of each public facility provided below represents an analysis of the impacts which would be generated by the proposed development. A related application is under review, which indicates that the proposed use would be a 1,904 square foot restaurant with drive-through service (ITE Code 934).

At present, the impacts which would be generated by the proposed development are acceptable and are not anticipated to degrade the Level of Service (LOS) of any public facility.

As the proposed use is non-residential, an analysis of the recreational and education level of services impact was not completed for this application.

Table 3. Affected Comprehensive Plan Roadway Segments[1]

Segment Number[2, 3]	Segment Description	Lanes	Functional Classification	Area Type	LOS
1 (32)	I-75 (from NCL to US 441)	6D	Freeway	COMM	C
2 (31)	I-75 (from US 441 to SCL)	6D	Freeway	COMM	C
5 (107)	US Hwy 441 (from SR235 to I-75)	4D	Principle Arterial	COMM	D
6 (4107)	US 441 (from I-75 to CR235A)	4D	Principle Arterial	Urban Trans	D
7 (14)	US 441 (from CR235A to NW 188th Street)	4D	Principle Arterial	Urban Trans	D
16	CR235A (from US 441 to NCL)	2U	Major Collector	COMM	D
17	CR235A (from US441 to CR 235)	2U	Major Collector	COMM	D

1 Source: City of Alachua Comprehensive Plan, Traffic Circulation Element.
2 For developments generating 1,000 trips or fewer, affected roadway segments are identified as all those wholly or partially located within ½ mile of the development's ingress/egress, or to the nearest intersecting major street, whichever is greater.
3 FDOT roadway segment number shown in parenthesis. For the purposes of concurrency management, COA Comprehensive Plan segments that make up a portion of a larger FDOT roadway segment will be evaluated together when determining post development roadway capacity.

Table 4. Potential Trip Generation[1]

	Land Use	AADT (Enter/Exit)	AM Peak Hour (Enter/Exit)	PM Peak Hour (Enter/Exit)
Restaurant with Drive-Through Service (1,904 square feet)	Fast Food Restaurant with Drive-Through Window [2] (ITE Code 934)	897 (449/449)	77 (39/38)	62 (32/30)

1 Source: ITE Trip Generation, 10th Edition.
2 Formulas: AADT – 470.94 trips per 1ksf x 1.904 sf (50% entering/50% exiting); AM Peak Hour – 40.19 trips per 1 ksf x 1.904 sf (51% entering/49% exiting); PM Peak Hour – 32.67 trips per 1 ksf x 1.904 sf (52% entering/48% exiting). Approximately 50% of trips calculated as pass by.

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Table 5. Potential Impact on Affected Comprehensive Plan Roadway Segments

Traffic System Category	1 I-75 (from NCL to US 441)(32)[1]	2 I-75 (from US 441 to SCL) (31)[1]	5 US Hwy 441 (from SR235 to I-75)(107) [1]	6 US 441 (from I-75 to CR235A) (4107) [1]	7 US 441 (from CR235A to NW 188th Street)(14) [1]	16 CR235A (from US 441 to NCL) [1]	17 CR235A (from US441 to CR 235) [1]
Maximum Service Volume[2]	91,600	91,600	45,700	39,000	43,000	15,120	15,120
Existing Traffic[3]	45,638	69,956	24,199	28,490	22,482	1,501	5,379
Reserved Trips[4]	508	508	3,745	3,286	3,286	809	191
Available Capacity[4]	45,454	21,136	11,056	7,224	17,232	12,810	9,550
Potential Impact Generated by Proposed Site Plan[5]	113	113	227	453	91	23	23
Residual Capacity after Proposed Site Plan[6]	**45,341**	**21,023**	**10,829**	**6,771**	**17,241**	**12,787**	**9,527**
PM Peak Hour Traffic Analysis	1 I-75 (from NCL to US 441)(32)[1]	2 I-75 (from US 441 to SCL) (31)[1]	5 US Hwy 441 (from SR235 to I-75)(107) [1]	6 US 441 (from I-75 to CR235A) (4107) [1]	7 US 441 (from CR235A to NW 188th Street)(14) [1]	16 CR235A (from US 441 to NCL) [1]	17 CR235A (from US441 to CR 235) [1]
Maximum Service Volume[2]	8,250	8,250	3,510	3,510	3,870	1,359	1,359
Existing Traffic[3]	4,792	7,345	2,299	2,707	2,136	166	486
Reserved Trips[4]	38	38	321	290	290	84	20
Available Capacity[4]	3,420	867	890	513	1,444	1,109	853
Potential Impact Generated by Proposed Site Plan [5]	8	8	16	31	6	2	2
Residual Capacity after Proposed Site Plan[6]	**3,412**	**859**	**874**	**482**	**1,438**	**1,107**	**851**

1 FDOT roadway segment number shown in parenthesis. For the purposes of concurrency management, COA Comprehensive Plan segments that make up a portion of a larger FDOT roadway segment will be evaluated together when determining post development roadway capacity.
2 Source: FDOT 2018 Quality/Level of Service Handbook, Generalized Annual Average Daily Volumes and Generalized Peak Hour Two-Way Volumes for Areas Transitioning to Urbanized Areas or Areas of 5,000 Not in Urbanized Areas.
3 Florida State Highway System Level of Service Report 2016, Florida Department of Transportation, District Two (published August 2017).
4 Source: City of Alachua March 2021 Development Monitoring Report.
5 Pass by rates for AM and PM Peak hours provided by ITE Trip Generation Manual 10th Edition. Daily pass by rates imputed from AM and PM peak rates.
6 The application is for a Final Development Order. Facility capacity and concurrency will be reserved.

Evaluation: It is anticipated that the proposed site plan will not adversely affect the Level of Service ("LOS") for the affected roadway segments, and the potential impact of a 1,904 square foot restaurant with drive-through service is therefore acceptable.

Potable Water Impacts

Table 6. Potable Water Impacts

System Category	Gallons Per Day
Current Permitted Capacity*	2,300,000
Less Actual Potable Water Flows*	1,295,000
Reserved Capacity*	152,524
Potential Potable Water Demand from Proposed Application **	1,523
Residual Capacity	**850,953**
Percentage of Permitted Design Capacity Utilized	**63.00%**

Sources:
** City of Alachua December 2020 Development Monitoring Report*
***Chapter 64-E, FAC, 15 square feet per seat x 20 gpd*

Evaluation: The proposed site plan would increase the maximum potential demand from the development of the subject property to 1,523 gallons per day. This analysis is based on the maximum development potential of a 1,904square foot restaurant with drive-through service. It is anticipated that the proposed site plan will not adversely affect the Level of Service ("LOS") for potable water facilities, and the impacts are therefore acceptable.

Sanitary Sewer Impacts

Table 7. Sanitary Sewer Impacts

System Category	Gallons Per Day
Treatment Plant Current Permitted Capacity*	1,500,000
Less Actual Treatment Plant Flows*	675,000
Reserved Capacity*	145,298
Projected Potential Wastewater Demand from Proposed Application **	1,523
Residual Capacity	**678,179**
Percentage of Permitted Design Capacity Utilized	**54.79%**

Sources:
** City of Alachua December 2020 Development Monitoring Report*
***Chapter 64-E, FAC, 15 square feet per seat x 20 gpd*

Evaluation: The proposed site plan would increase the maximum potential demand from the development of the subject property to 1,523 gallons per day. This analysis is based on the maximum development potential of 1,904 square foot restaurant with drive-through service. It is anticipated that the proposed site plan will not adversely affect the Level of Service ("LOS") for sanitary sewer facilities, and the impacts are therefore acceptable.

Solid Waste Impacts

Table 9. Solid Waste Impacts

System Category	Lbs Per Day	Tons Per Year
Existing Demand[1]	41,192.00	7,517.54
Reserved Capacity[2]	9,953.23	1,816.46
Projected Solid Waste Demand from Application[3]	22.9	4.17
New River Solid Waste Facility Capacity[4]	**50 years**	

Sources:
1 University of Florida, Bureau of Economic & Business Research, Estimates of Population by County and City in Florida, April 1, 2016; Policy 2.1.a, CFNGAR Element (Formula: 9,936 persons x 0.73 tons per year)
2 City of Alachua December 2020 Development Monitoring Report
3 Source: Sincero and Sincero; Environmental Engineering: A Design Approach. Prentice Hall, New Jersey, 1996. Formula used: (((12 lbs. / 1,000 ft2/day x 1,930 sq. ft.) x 365)/2,000)
4 New River Solid Waste Facility, April 2019

Evaluation: The proposed site plan would increase the maximum potential demand from the development of the subject property by approximately 4.17 tons per year. It is anticipated that the proposed site plan will not adversely affect the Level of Service ("LOS") of solid waste facilities; therefore, the impacts are acceptable.

EXHIBIT "A"
TO
KFC
SITE PLAN
STAFF REPORT

CONDITIONS:

1. The applicant agrees it shall provide the Cross Access Easements as depicted on the Site Plan. All Cross Access Easements shall be approved by the City and recorded in the Public Records of Alachua County prior to applying for a building permit. The applicant shall incur all costs associated with preparing and recording such Easements.
2. The applicant agrees it shall obtain all other applicable local, state, and federal permits before the commencement of the development.
3. The applicant agrees to provide off-site tree mitigation in accordance with Resolution 21-14 of the City of Alachua, and in accordance with the calculation of the fee for off-site mitigation as provided within a memorandum from Caeli Tolar, PLA, of Causseaux, Hewett, & Walpole, Inc., dated March 25, 2021. The fee for off-site mitigation shall be paid to the City prior to the public hearing associated with this Site Plan and final development order.

4. The applicant agrees that Conditions 1 – 3 as stated above does not inordinately burden the land and shall be binding upon the property owner, including any subsequent property owners, successors, or assigns, and that the development shall comply with Conditions 1 – 3 as stated herein.

EXHIBIT "B"

TO

KFC

SITE PLAN
STAFF REPORT

SUPPORTING APPLICATION MATERIALS SUBMITTED BY CITY STAFF TO THE PLANNING AND ZONING BOARD

City of Alachua

ADAM BOUKARI
CITY MANAGER

PLANNING & COMMUNITY DEVELOPMENT
DIRECTOR KATHY WINBURN, AICP

March 31, 2021

Daniel Young, P.E.
CHW, Inc.
11801 Research Drive
Alachua, FL 32615

Also submitted electronically to daniely@chw-inc.com

RE: Notice of Hearing to be Scheduled for KFC Site Plan

Dear Mr Young:

On March 25, 2021 the City of Alachua received your updated application for the above referenced project.

Based on review of the materials submitted, the City has determined that the application can now be scheduled for a hearing before the Planning and Zoning Board.

You must provide three (3) *double-sided, three-hole punched, color sets* of the complete application package, seven (7) sets of plans, and a digital copy of all materials (including plans) in PDF format on a CD *no later than 10 business days prior to the PZB Meeting at which your application is scheduled to be heard.* The application has been scheduled for the April 20, 2021 Planning and Zoning Board meeting; therefore, the above referenced materials must be submitted to the City no later than 12 PM on *Tuesday, April 6, 2021.* Materials may be submitted earlier than this date.

In addition, Section 2.2.9(D) of the Land Development Regulations requires the applicant to place posted notice signs on the subject property at least 14 days prior to the public hearing. Therefore, posted notice signs must be placed on the property no later than *Monday, April 5, 2021*. These signs will be available at City Hall after 12 PM on Thursday, April 1, 2021.

If you plan to utilize a PowerPoint presentation or would like other materials to be available for reference during the public hearing, please submit the presentation or materials no later than 12:00 PM on the last business day prior the PZB meeting (no later than *Monday, April 19, 2021*). Any presentation or materials may be submitted by emailing them to planning@cityofalachua.com.

Should you have any questions, please feel free to contact me at (386) 418-6100, ext. 1603 or via email at ahall@cityofalachua.com.

Sincerely,

Adam Hall, AICP
Planner
c: Kathy Winburn, AICP, Planning Director
File

City of Alachua

ADAM BOUKARI
CITY MANAGER

PLANNING & COMMUNITY DEVELOPMENT
DIRECTOR KATHY WINBURN, AICP

March 18, 2021

Also sent electronically to daniely@chw-inc.com

Daniel Young, P.E.
Causseaux, Hewett, & Walpole, Inc.
11801 Research Drive
Alachua, FL 32615

RE: DRT Comments for KFC Alachua Site Plan
Letter No. 2

Dear Mr Young:

On March 9, 2021, the City received your revised application for the KFC Alachua Site Plan, which proposes the construction of a ±1,904 square foot restaurant with drive through on a ±1.00 acre subject property (Parcel No. 03053-002-001 and a portion of 03053-002-000).

The application has been reviewed for compliance with the applicable review standards, including the City's Comprehensive Plan and Land Development Regulations (LDRs). Based upon Staff's review, revisions must be made to the application.

Please address all insufficiencies outlined below in writing and provide an indication as to how they have been addressed by 10:00 AM on **Thursday, March 25, 2021**. A total of four (4) copies of the application package, plans, and a digital copy of all materials in PDF format on a CD or sent by emailing a Cloud / FTP link must be provided by this date. Upon receipt of your revised application, Staff will notify you of any remaining insufficiencies which must be resolved, if any, before the item may be forwarded to the Planning and Zoning Board for their consideration. If all comments are addressed by the resubmission date above, the application may be scheduled for the April 20, 2021 PZB Meeting. If all comments are addressed after 10:00 AM, March 25, 2021, but before 5 PM on April 6, 2021, the application may be scheduled for the May 11, 2021 Planning and Zoning Board.

Please address the following insufficiencies (begins on next page):

Deficiencies to be Addressed

Unless otherwise noted, references to code Sections refer to City of Alachua Land Development Regulations. Where applicable, new comments are noted.

2. *Section 3.7.2(C)(5), Gateway Overlay District Standards*
   b. Section 3.7.2(C)(5)(a)(v) requires glazing to constitute a minimum of 35% of the ground floor area when a building faces and is substantially visible from US Highway 441. For the proposed development, this standard is applicable to the north, east, and west elevations. These elevations do not appear to meet the 35% glazing requirement. Please provide glazing calculations for all elevations.

NEW COMMENT: Provided calculation appears to use the base floor area of the building. However, the requirement is intended for the ground floor façade area to be glazed a minimum of 35%. This is calculated by measuring the applicable building wall between the finished frade and the underside of the roof of the façade. When a building has a parapet, the ground floor façade area shall be calculated by measuring to the top of the parapet. This calculation should be done separately for each façade visible from US 441 and shown on architectural sheets. Staff has estimated that the façade area for the east and west elevations is 1,284 square feet, which means that the minimum glazing for these elevations is 449 square feet. The east elevation requires 167 more square feet of glazing. The west elevation requires 92 square feet of glazing. The front elevation façade area is estimated to be 620 square feet, which means that the minimum glazing for this elevation is 217 square feet. The front elevation requires 37 square feet in additional glazing. Also, identify which windows, if any, are spandrel glass.

   d. Please identify the three architectural elements being utilized on the front façade in accordance with Section 3.7.2 (C)(5)(a)(vi).

NEW COMMENT: Please identify which architectural element standard is addressed with each item listed in response. Please note that these elements are for front facades; the walk in cooler cladding would not apply to the required to elements.

4. *Section 6.2, Tree Protection / Landscape / Xeriscape Standards*
   c. There appear to be 2 (two) 31" live oaks being proposed to be removed per demolition plan, however, the list of heritage trees being removed on Sheet LS-1 only lists 1(one) 31" tree being removed. Please revise mitigation calculations.

   NEW COMMENT: Please revise mitigation memo. The City Commission will be required to accept the payment by resolution prior to any action by the Planning and Zoning Board.

   f. Verify calculations of interior parking lot landscaping requirements as set forth in Section 6.2.2(D)(2). Trees are required at the minimum rate of one canopy or ornamental/understory tree for every 1,800 square feet, or portion thereof, of the total parking lot area, with a minimum of 10 shrubs per landscaped island. Parking lot area is calculated as the paved or unpaved area dedicated to the short- or long-term parking or unloading of vehicles, including associated drive-aisles adjacent to parking bays or loading areas. Note that, per Section 6.2.2(D) (2) (a) (iii) f. this requirement is reduced by 50% when all parking bays are to the side or rear of the property. Provided calculation is 4,360 square feet, but staff estimates this area to be approximately 12,400 square feet. 4 (four) additional canopy trees would be needed. These trees appear to already be provided with the current submission. Please revise calculation in Table located on Sheet LS-1.

NEW COMMENT: The hatched area does not capture entire parking lot area. The drive-through lanes and by pass lane are integral elements of the parking lot area and must be included in the calculation of area. It appears that the number of trees required are present so no additional trees are needed to meet the requirement of this section. However, an additional 40 shrubs are needed. A potential location would be to the east of the by pass lane, adjacent to proposed bald cypress.

j. Section 6.2.3 (E) requires 4 (four) canopy trees for every 100 lineal feet of arterial frontage placed approximately every 25' and 3 (three) understory trees for every 100 lineal feet of arterial frontage. 4 (four) additional canopy trees and 5 (five) understory trees required along US Highway 441 frontage.

NEW COMMENT: Three additional canopy trees needed within 25' of front property line to meet this requirement. Trees placed in basin cannot be counted towards arterial frontage screening requirements. The landscape plan sheet for the adjacent property to the east has been included with this letter.

9. *Public Services / Fire Rescue / Engineering Review Comments*

b. The applicant must address the comments provided by A.J. "Jay" Brown, P.E., of JBrown Professional Group, Inc., as provided in a letter dated February 10, 2021.

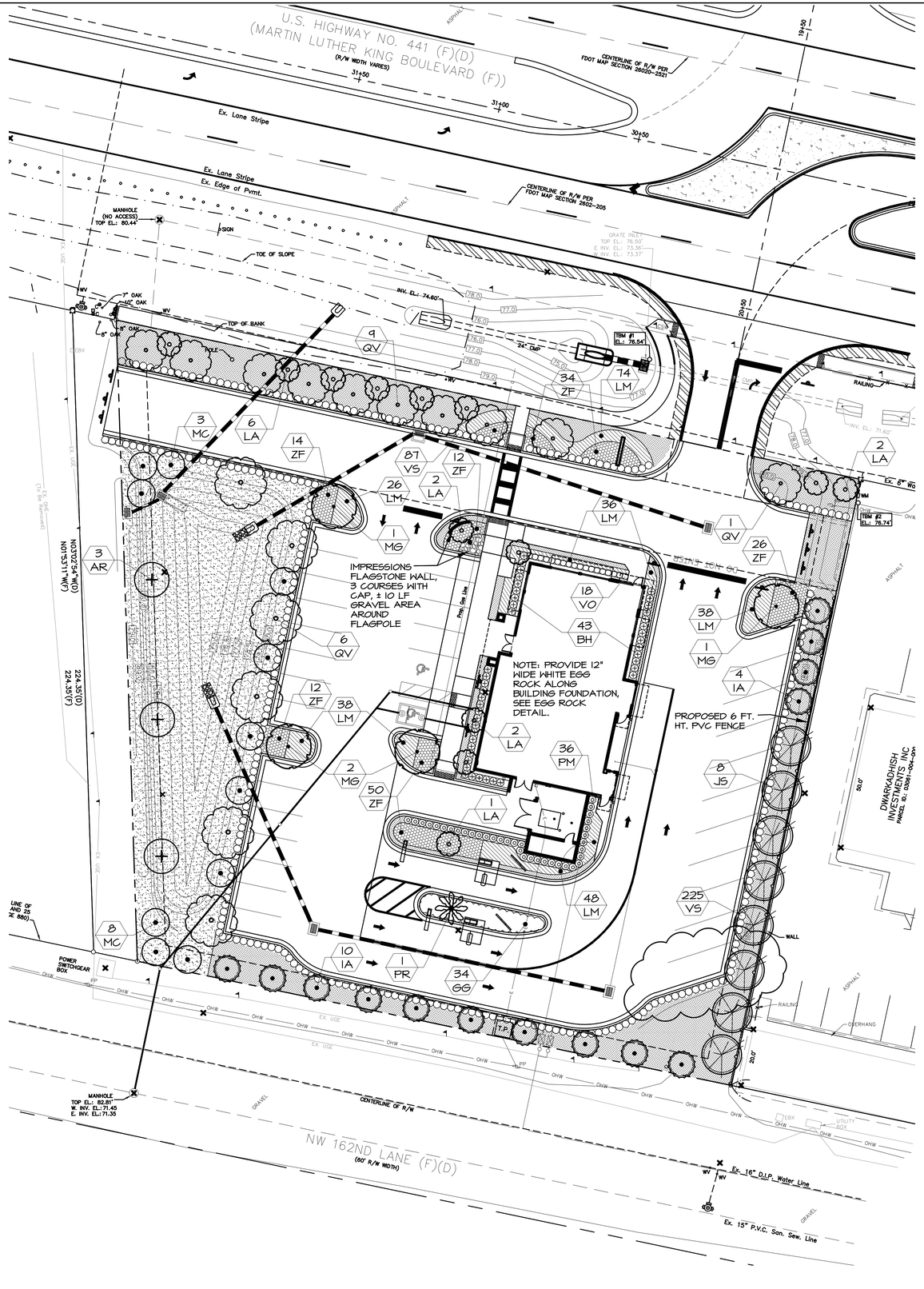
NEW COMMENT: Please see minor comments in attached email. Please revise.

If you have any questions regarding the information above, please contact me at 386-418-6100 x 1603 or via e-mail at ahall@cityofalachua.com. We look forward to receiving your revised application.

Sincerely,

Adam Hall, AICP
Planner

cc: Kathy Winburn, AICP, Planning & Community Development Director
Justin Tabor, AICP, Principal Planner
Project File



PLANT MATERIAL LIST

SYMBOL	QUANTITY	BOTANICAL NAME	COMMON NAME	SPECIFICATIONS	NATIVE	SPACING
PR	1	PHOENIX ROEBELLENII	*PYGMY DATE PALM*	5' OVERALL HEIGHT, TRIPLE STEM	NO	AS SHOWN
MG	4	MAGNOLIA "D D BLANCHARD"	*D D BLANCHARD MAGNOLIA*	10' HT X 5' SPD, 2" CAL, 30 GALLON	YES	AS SHOWN
QV	16	QUERCUS VIRGINANA	*LIVE OAK*	10' HT X 5' SPD, 2" CAL, 30 GALLON	YES	AS SHOWN
AR	3	ACER RUBRUM	*RED MAPLE*	10' HT X 5' SPD, 2" CAL, 30 GALLON	YES	AS SHOWN
JS	8	JUNIPERUS SILICICOLA	*RED CEDAR*	10' HT X 4' SPD, 2" CAL, 30 GALLON	YES	AS SHOWN
IA	14	ILEX ATTENUATA "EAGLESTON"	*EAGLESTON HOLLY*	8' HT X 4' SPD, 2" CAL, 30 GALLON	YES	AS SHOWN
LA	13	LAGERSTROEMIA INDICA 'MUSKOGEE'	*LAVENDER CRAPE MYRTLE*	8' HT X 4' SPD, MULT-STEM, 30 GALLON	NO	AS SHOWN
MC	11	MYRICA CERIFERA	*WAX MYRTLE*	8' HT X 4' SPD, MULT-STEM, 30 GALLON	YES	AS SHOWN
PM	36	PODOCARPUS MACROPHYLLUS	*PODOCARPUS*	48" HT X 30" SPD, 15 GALLON	NO	30" OC
VS	307	VIBURNUM SUSPENSUM	*SANDANKWA VIBURNUM*	24" HT X 24" SPD, 7 GALLON	NO	30" OC
VO	18	VIBURNUM SUSPENSUM	*SANDANKWA VIBURNUM*	24" HT X 24" SPD, 7 GALLON	NO	24" OC
GG	34	GALPHIMA GRACILIS	*YELLOW THRYALLIS*	24" HT X 24" SPD, 3 GALLON	NO	30" OC
ZF	148	ZAMIA FLORIDANA	*COONTIE*	15" HT X 15" SPD, 3 GALLON	NO	30" OC
BH	43	ILEX CORNUTA 'BURFORDII COMPACTA'	*DWARF BURFORD HOLLY*	18" HT X 18" SPD, 3 GALLON	NO	30" OC
LM	260	LIRIOPE MUSCARI 'SUPER BLUE'	*SUPER BLUE LIRIOPE*	10" HT X 10" SPD, 1 GALLON	NO	24" OC

CODE REQUIREMENTS

INTERIOR REQUIREMENTS

ONE TREE / 1,800 SF
28,060 VUA / 1,800 SF = 16 TREES X 0.5 = 8 TREES
10 SHRUBS PER LANDSCAPE ISLAND
7 ISLANDS X 10 = 70 SHRUBS
184 SHRUBS PROVIDED

PERIMETER / BUFFER REQUIREMENTS

NORTH - 242 LF / 100 = 2.42
2.42 X 4 = 10 CANOPY TREES
2.42 X 3 = 8 UNDERSTORY TREES

EAST - PARKING LOT BUFFER 190 LF / 100 = 1.90
1.90 X 4 = 8 CANOPY TREES
1.90 X 2 = 4 UNDERSTORY TREES
TYPE A BUFFER, OPTION 3
6 FT. HT. PVC FENCE PROPOSED

SOUTH - 190 LF / 100 = 1.90 (OVERHEAD POWER LINES)
PARKING LOT BUFFER 1.90 X 4 = 8 CANOPY TREES
1.90 X 2 = 4 UNDERSTORY TREES

WEST - PARKING LOT BUFFER 160 LF / 100 = 1.6
1.6 X 4 = 6 CANOPY TREES
1.6 X 2 = 3 UNDERSTORY TREES
TYPE A BUFFER, OPTION 1
200 / 60 = 3 CANOPY TREES
200 / 60 = 3 UNDERSTORY TREES

57 TREES TOTAL PERIMETER

FOUNDATION
90 LF / 100 = 0.9
0.9 X 4 = 4 TREES

TOTAL TREES REQUIRED
8 + 57 + 4 = 69 TREES
69 TREES PROVIDED

LANDSCAPE INSTALLATION NOTES

1) ALL PLANT MATERIALS SHALL BE FLORIDA #1 OR BETTER AS GIVEN IN, GRADES AND STANDARDS FOR NURSERY PLANTS, LATEST EDITION, PUBLISHED BY THE FLORIDA DEPARTMENT OF AGRICULTURE AND CONSUMER SERVICES.

2) LOCATION OF PLANTS ON THE PLAN ARE DIAGRAMMATIC- SEE THE LANDSCAPE ARCHITECT FOR QUESTIONS ON EXACT LOCATIONS. THE PLANT MATERIALS LIST IS PROVIDED FOR THE CONVENIENCE OF THE LANDSCAPE CONTRACTOR. SHOULD THERE BE ANY DISCREPANCY BETWEEN THE PLANT LIST AND THE PLAN, THE PLAN SHALL PREVAIL.

3) IT IS THE LANDSCAPE CONTRACTORS RESPONSIBILITY TO INSURE THAT ALL PLANT BED AREAS HAVE PROPER DRAINAGE FOR OPTIMUM GROWTH OF LANDSCAPE MATERIAL BEFORE INSTALLATION BEGINS.

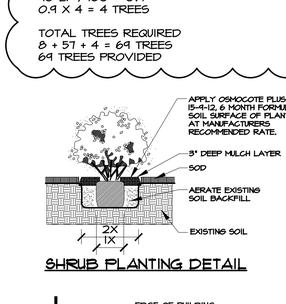
4) THE CONTRACTOR SHALL INSURE THAT ALL PLANTING ISLANDS AND OTHER AREAS SHALL BE CLEAN OF TRASH, CONSTRUCTION DEBRIS, OR OTHER WASTE MATERIALS TO A DEPTH OF 24" PRIOR TO LANDSCAPE INSTALLATION.

5) ALL PLANT BEDS AND TREE RINGS SHALL BE TREATED WITH A PRE-EMERGENT HERBICIDE AND THEN TOP-DRESSED WITH 3" DEEP TRIPLE SHREDDED BLACK MULCH. ALL NEW TREES SHALL BE STAKED.

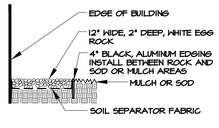
6) NEW SOD SHALL BE EMPIRE ZOYSIA SOD ± 6,200 S.F., AND ARGENTINE BAHIA ± 8,150 S.F., IN THE LOCATIONS SHOWN ON THE PLAN. CONTRACTOR SHALL DETERMINE EXACT QUANTITIES IN THE FIELD. SOD AREAS SHALL BE MAINTAINED BY THE CONTRACTOR FROM THE TIME OF INSTALLATION TO THE TIME OF FINAL ACCEPTANCE.

7) TREES SHRUBS, AND GROUND COVER SHALL BE INSTALLED USING THE FOLLOWING PROCEDURE: PLANT PITS SHALL BE EXCAVATED TO TWICE THE DIAMETER OF THE PLANT ROOT BALL. AERATE EXISTING SOIL BEFORE BACKFILLING PIT.

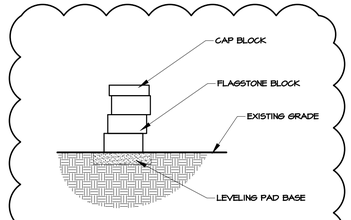
8) TREE INSTALLATION: ALL REQUIRED TREES SHALL BE INSTALLED 1" - 2" ABOVE FINISH GRADE. TREES INSTALLED OR BURIED TOO DEEP SHALL BE RESET TO THIS STANDARD. REMOVE THE TOP 1/3 OF THE WIRE BASKETS ON ALL B & B STOCK.



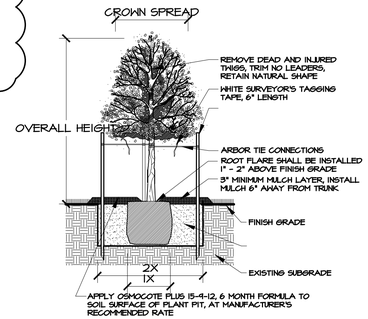
SHRUB PLANTING DETAIL



EGG ROCK INSTALLATION DETAIL

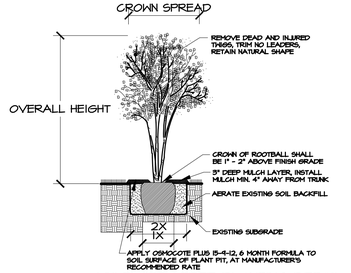


FLAGSTONE WALL DETAIL



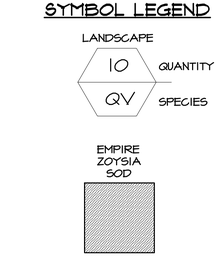
SHADE TREE STAKING AND PLANTING DETAIL

CONTRACTOR SHALL REMOVE STAKING AND GUYING MATERIALS AFTER ONE GROWING SEASON.



MULTI-STEM TREE PLANTING DETAIL

SYMBOL LEGEND



L. ALYSON UTTER
FLORIDA LANDSCAPE ARCHITECT No. LA0001163

Consultant:
ANDERSON LESNIAK LIMITED, INC.
landscape architects land planners
4921 S. WESTSHORE BOULEVARD
TAMPA, FLORIDA 33611
(813) 831-9595 FAX 831-5485
anderson.lesniak@tampabay.rr.com
www.andersonlesniak.net

Developer:
BRAVOFLORIDA, LLC.
3018 U.S. HIGHWAY 301 N.
SUITE NO. 100
TAMPA, FL. 33619
(813) 374.8848

NO.	COMMENTS	DATE
1	PER REVIEW COMMENTS	6/23/20

LANDSCAPE PLAN

BURGER KING
16130 NW U.S. HIGHWAY 441
CITY OF ALACHUA, FLORIDA

SCALE: 1"=20'
DATE: 01.21.20
CAD FILE: L-22
JOB NUMBER: ALACHUA COUNTY
SHEET L-1

S:\Projects 2020\20102 Burger King @ US 441 Alachua\2 CIVIL\L-22.dwg - Jun 23, 2020 @ 10:32am

RE: Request for Review Site Plan for KFC Alachua

From : Tim Boehlein, EI <tim.boehlein@jbprogroup.com>
Wed, Mar 17, 2021 10:37 AM
2 attachments

Subject : RE: Request for Review Site Plan for KFC Alachua

To : Adam Hall <ad_hall@cityofalachua.org>

Cc : A. J. 'Jay' Brown, Jr., PE <jay.brown@jbprogroup.com>

Good morning Adam,

Please find a ached our le er of approval for the KFC Alachua site plan. We find this project approvable from an engineering standpoint.

The only concern we have is that we would s. ll encourage them to provide energy dissipation at S-11 at the outfall of the FDOT right of way to prevent erosion, however this is ultimately up to the FDOT. One other very minor thing we wanted to point out was that they have one mislabeled contour at the basin bo□om on Sheet C2.10. This should be an 80 rather than an 81.

Please let us know if you have any additional questions or concerns,

Thank you

Tim Boehlein, E.I., LEED Green Associate
Staff Engineer

JBrown PROFESSIONAL GROUP
Civil Engineering • Land Surveying • Planning
www.JBProGroup.com
3530 NW 43rd Street • Gainesville, FL 32606
Office: (352) 375-8999 x 104 • Fax: (352) 375-0833 • Cell: (352) 575-5455

RE: Independent Review of the QSR (KFC)

From : Kanely, Brian <brian.kanely@volkert.com> Wed, Mar 10, 2021 10:59 AM
Subject : RE: Independent Review of the QSR (KFC)
To : Adam Hall <ad_hall@cityofalachua.org>

Adam:
The revised Traffic Impact Analysis addresses all my concerns listed in my Independent Review dated February 17, 2021. Please note the following in the revised TIA:

1. The size (square footage) of the QSR decreased slightly. This resulted in slightly lower numbers for the trip generation.
2. The traffic data was updated, this also resulted in slightly different numbers from the original TIA.
3. The slight change in the trip generation and traffic data did not result in any significant changes is the results and conclusions of the TIA.
4. I concur with the TIA's conclusions and recommendations.
5. The new figures 1 and 7 are major additions to the TIA. They clearly show the existing conditions, proposed developments, intent of the future frontage road and future extension of NW 167th Blvd and the access management issues.

Based on the changes made in the revised TIA, I recommend its approval.

Please contact me if you have any questions.

Brian

Brian Kanely, P.E.
Sr. Traffic Engineer
Volkert, Inc.
3501 S Main Street, Suite 2
Gainesville, FL 32601
352-240-7459 - Direct Line
352-372-9594 x3006
Cell: 352-262-3580
brian.kanely@volkert.com
The information contained in this e-mail, including any accompanying documents or attachments, is from Volkert, is intended only for the use of the individual or entity named above, and is privileged and confidential. If you are not the intended recipient, be aware that any disclosure, dissemination, distribution, copying or use of the contents of this message is strictly prohibited. If you have received this message in error, please notify Volkert immediately at our corporate office (251) 342-1070. Thank you for your cooperation

City of Alachua

ADAM BOUKARI
CITY MANAGER

PLANNING & COMMUNITY DEVELOPMENT
DIRECTOR KATHY WINBURN, AICP

February 18, 2021

Also sent electronically to daniely@chw-inc.com

Daniel Young, P.E.
Causseaux, Hewett, & Walpole, Inc.
11801 Research Drive
Alachua, FL 32615

RE: DRT Comments for KFC Alachua Site Plan

Dear Mr Young:

On January 28, 2021, the City received your initial application for the KFC Alachua Site Plan, which proposes the construction of a ±1,904 square foot restaurant with drive through on a ±1.00 acre subject property (Parcel No. 03053-002-001 and a portion of 03053-002-000).

The application has been reviewed for compliance with the applicable review standards, including the City's Comprehensive Plan and Land Development Regulations (LDRs). Based upon Staff's review, revisions must be made to the application. A meeting is scheduled for 2:00 PM on Monday, February 22, 2021 to review these comments.

Please address all insufficiencies outlined below in writing and provide an indication as to how they have been addressed by 5:00 PM on Tuesday, March 9, 2021. A total of four (4) copies of the application package, plans, and a digital copy of all materials in PDF format on a CD or sent by emailing a Cloud / FTP link must be provided by this date. Upon receipt of your revised application, Staff will notify you of any remaining insufficiencies which must be resolved, if any, before the item may be forwarded to the Planning and Zoning Board for their consideration. If all comments are addressed by the resubmission date above, the application may be scheduled for the April 20, 2021 PZB Meeting.

Please address the following insufficiencies (begins on next page):

Deficiencies to be Addressed

Unless otherwise noted, references to code Sections refer to City of Alachua Land Development Regulations. Where applicable, new comments are noted.

1. *Land Rights*
   a. Proposed development appears to also occur on parcel no. 03061-004-001 (Burger King site). Please confirm that property is under current ownership as the subject parcel (03053-002-001).

2. *Section 3.7.2(C)(5), Gateway Overlay District Standards*
   a. Section 3.7.2(C)(5)(a)(v) requires glazing to constitute a minimum of 35% of the ground floor area when a building faces and is substantially visible from US Highway 441. For the proposed development, this standard is applicable to the north, east, and west elevations. These elevations do not appear to meet the 35% glazing requirement. Please provide glazing calculations for all elevations.
   b. Per Section 3.7.2(C)(5)(d), live oak shall be used as the required canopy tree. Revise the landscape plan to incorporate live oak as the primary canopy tree utilized for the plan.
   c. Section 3.7.2(C)(5)(g) requires cross access between the subject property and adjacent parcels. A cross access easement shall be required. Coordinate preparation of the easement with Compliance & Risk Management (386-418-6118).
   d. Please identify the three architectural elements being utilized on the front façade in accordance with Section 3.7.2 (C)(5)(a)(vi).
   e. Please provide color elevations.

3. *Section 6.1, Off-Street Parking & Loading Standards*
   a. Depict and dimension the minimum required drive-through stacking spaces. Per Section 6.1.8, a minimum 4 stacking spaces are required from the entrance of the drive through to the order boxes and from the order boxes to the pick-up window. The minimum dimension of each stacking space is 9 feet by 20 feet, and shall not impede other on-site traffic movements or movements into or out of parking spaces.

4. *Section 6.2, Tree Protection / Landscape / Xeriscape Standards*
   a. Per Section 6.2.1 (A), please verify that it is not possible to retain 37" live oak located near proposed ingress/egress. Are there proposed utilities that could reasonably be shifted to accommodate retention? Preservation would result in 12 tree credits.
   b. There appear to be 2 (two) 31" live oaks being proposed to be removed per demolition plan, however, the list of heritage trees being removed on Sheet LS-1 only lists 1(one) 31" tree being removed. Please revise mitigation calculations.
   c. Per Section 6.2.2 (D)(1)(c)(i), 3 (three) canopy trees per acre are required, not 1 (one) as displayed in Table on Sheet LS-1. Please revise.
   d. 4 (four) additional canopy trees required (1 (one) additional for overall site landscaping requirement and 3 (three) for building east façade) on east side of building to meet landscaping requirements of Section 6.2.2 (D)(1)(c).
   e. Parking lot buffer requirements are not met for parking area located west of the building. Per Section 6.2.2(D)(2)(b), parking lot perimeter buffers shall be located immediately adjacent to the curb or pavement and shall have a minimum of 4 canopy trees and 2 understory trees for every 100 lineal feet. 3 (three) additional canopy trees and 1 (one) additional understory tree required for parking lot buffer.
   f. Verify calculations of interior parking lot landscaping requirements as set forth in Section 6.2.2(D)(2). Trees are required at the minimum rate of one canopy or ornamental/understory tree for every 1,800 square feet, or portion thereof, of the total

parking lot area, with a minimum of 10 shrubs per landscaped island. Parking lot area is calculated as the paved or unpaved area dedicated to the short- or long-term parking or unloading of vehicles, including associated drive-aisles adjacent to parking bays or loading areas. Note that, per Section 6.2.2(D) (2) (a) (iii) f. this requirement is reduced by 50% when all parking bays are to the side or rear of the property. Provided calculation is 4,360 square feet, but staff estimates this area to be approximately 12,400 square feet. 4 (four) additional canopy trees would be needed. These trees appear to already be provided with the current submission. Please revise calculation in Table located on Sheet LS-1.

g. To meet perimeter buffering requirements of Section 6.2.2 (D) (3), 3 (three) additional canopy trees required along eastern perimeter are needed, and 1 (one) additional canopy tree required along southern perimeter.
h. Per Section 6.2.2(C) (8), include a narrative explaining how Florida Friendly Landscaping practices have been incorporated into landscape plan.
i. Podocarpus appears to be functioning more as a hedge as presented in the proposed landscape plan. Spacing and location under other trees would seem to prevent a single planting from becoming a tree as commonly defined.
j. Section 6.2.3 (E) requires 4 (four) canopy trees for every 100 lineal feet of arterial frontage placed approximately every 25' and 3 (three) understory trees for every 100 lineal feet of arterial frontage. 4 (four) additional canopy trees and 5 (five) understory trees required along US Highway 441 frontage.

5. *Section 6.4, Exterior Lighting Standards*
   a. Photometric plan does not provide data proximate to wall mounted fixtures. Confirm the plan accounts for wall mounted fixtures and that data points are provided proximate to wall mounted fixtures.
   b. Provide specifications or details for wall mounted lighting. Sheet A4.0 references details shown on Sheet E4.0, but this sheet was not submitted with plans.

6. *Section 6.5, Signage*
   a. Please note that signage is not reviewed for approval or permitting at time of site plan, but as it has been provided on plans, Staff offers the comments below for information purposes. Please revise site plan to note monument signage and wall signage are not permitted through the site plan process.
   b. East and west side signage not permitted as there are no street frontages facing those facades.
   c. Free standing sign must be in the "monument" style. That is, a sign with continuous support from the bottom of the sign to the ground. In addition, as the sign is located within the Gateway Overlay District, there must be 3' landscape area surrounding the sign.
   d. Front wall sign appears to exceed 10% of façade wall area.

7. *Section 6.8, Design Standards for Business Uses*
   a. Per Section 6.8.2 (A)(2)(b), facades must incorporate wall offsets at least two feet in depth a minimum of every 30'. The façade area with "World Famous Chicken" copy on west elevation is 31' wide without offset.
   b. Per Section 6.8.2 (A)(2)(b), facades must incorporate wall offsets at least two feet in depth a minimum of every 30'. The façade area with striped painting on east elevation is 38' wide without offset.
   c. Per Section 6.8.2(A)(3), mechanical equipment shall be fully concealed from visibility from a street. A/C units are visible above the top of parapet walls of the east and west elevations, and will be visible from US 441. Please address.
   d. Please indicate façade materials in order to demonstrate compliance with Section 3.7.2 (C)(5)(c)(ii).

e. Metal appears to be used as finish material above windows. Per Section 3.7.2 (C)(5)(a)(ii), metal shall not be used a finish building material.

8. *Concurrency Impact Analysis*
   a. Concurrency Impact Analysis narrative cites October 2020 Development Monitoring Report as data used in analysis. Please revise using latest report available at time of application – December 2020.
   b. Traffic Impact Analysis and Concurrency Analysis appear to use two separate calculations for trips generated by proposed development. Please confirm square feet proposed.

9. *Public Services / Fire Rescue / Engineering Review Comments*
   a. The applicant must address the comments provided by Tom Ridgik, P.E., Engineering Supervisor, in a memorandum dated February 15, 2021.
   b. The applicant must address the comments provided by A.J. "Jay" Brown, P.E., of JBrown Professional Group, Inc., as provided in a letter dated February 10, 2021.
   c. The applicant must address the comments provided by Brian Kanely of Volkert Inc. as provided in a letter dated February 17, 2021 (forthcoming).

10. *General Information*
   a. Alachua County Environmental Protection Department (EPD) requires the review and approval of all new commercial automatic irrigation systems. It is recommended that the applicant contact Alachua County EPD to coordinate this review and approval. Additional information is available at: https://alachuacounty.us/Depts/epd/WaterResources/WaterConservation/Pages/Irrigation-Efficiency-Design-and-Maintenance-Code.aspx
   b. Alachua County Environmental Protection Department (EPD) requires the review and approval of stormwater management systems, unless otherwise exempt. It is recommended that the applicant contact Alachua County EPD to coordinate this review and approval. Additional information is available at: https://library.municode.com/fl/alachua_county/codes/code_of_ordinances?nodeId=PTIIADCO_TIT7HESA_CH77WAQUSTMAPR_ARTIIISTTRCO

If you have any questions regarding the information above, please contact me at 386-418-6100 x 1603 or via e-mail at ahall@cityofalachua.com. We look forward to receiving your revised application.

Sincerely,

Adam Hall, AICP
Planner

cc: Kathy Winburn, AICP, Planning & Community Development Director
Justin Tabor, AICP, Principal Planner
Project File

City of Alachua

ADAM BOUKARI
CITY MANAGER

RODOLFO VALLADARES, P.E.
PUBLIC SERVICES DIRECTOR

INTER-OFFICE COMMUNICATION

DATE: **Feb 15, 2021**

TO: **Kathy Winburn, AICP**
Planning & Community Development Director

FROM: **Rodolfo Valladares, P.E.**
Public Services Director
Tom Ridgik, P.E.
Engineering Supervisor TR 2/15/2021

RE: **KFC Site Plan**

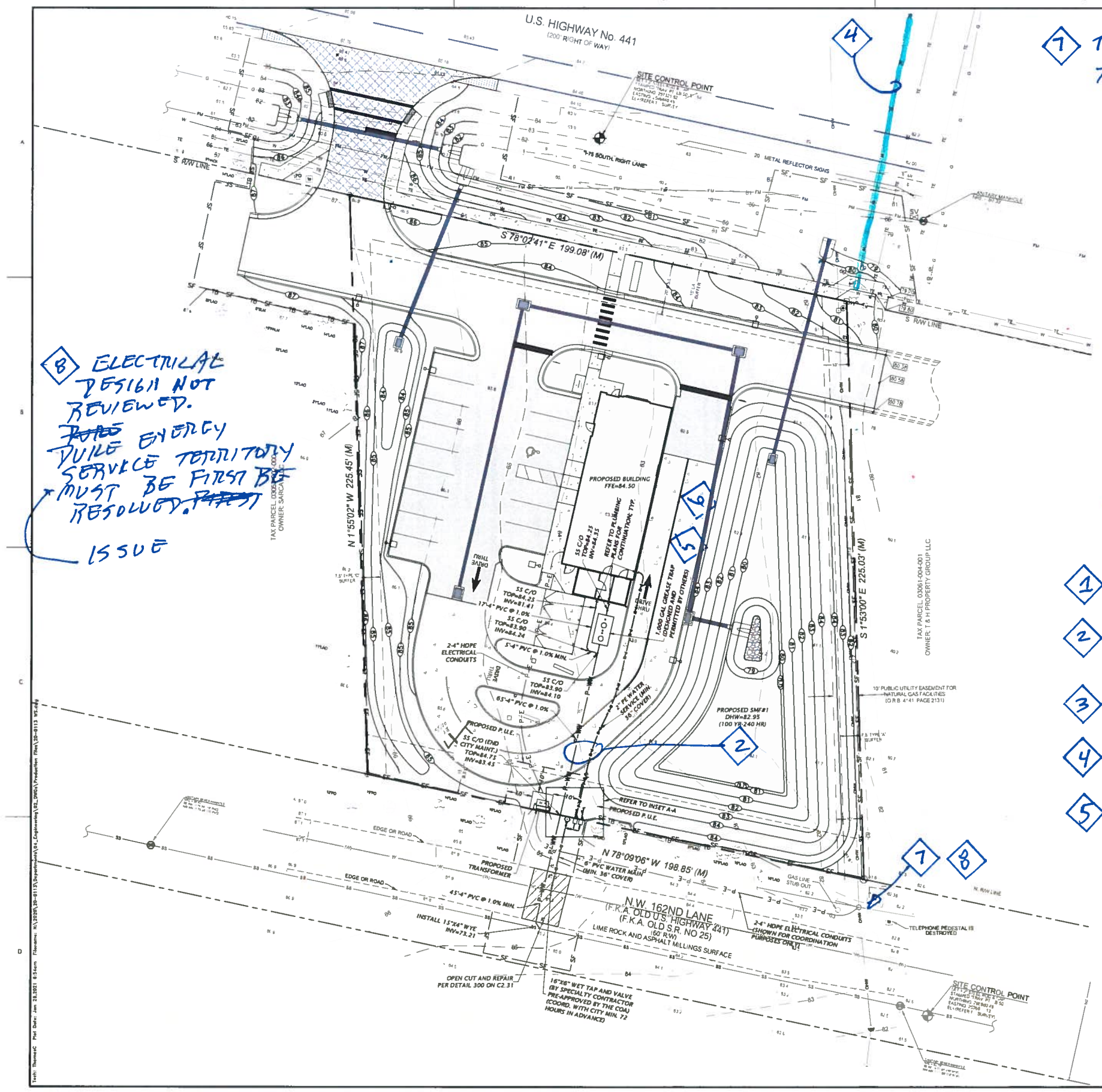
Public Services has reviewed the subject project (Jan 28 Submittal) and offer the following comments. Review was specific to the Public Services Utilities.

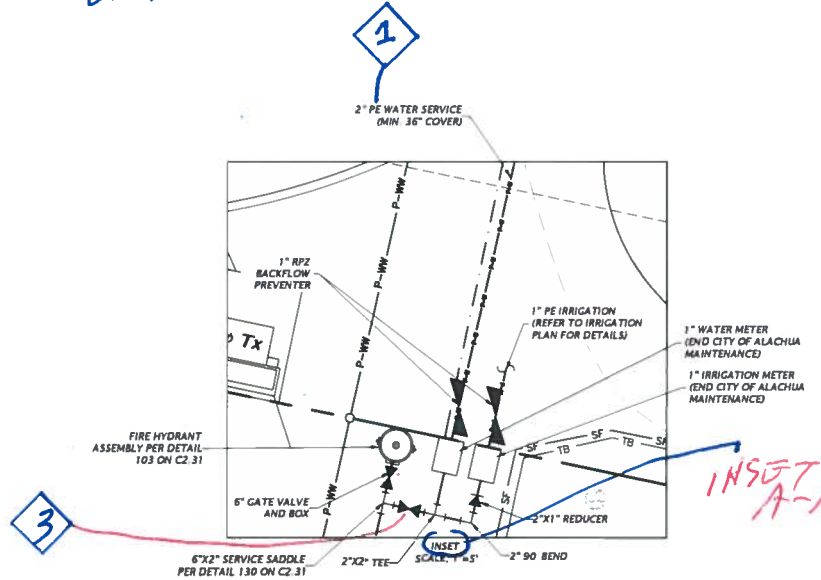
NO.	COMMENTS
1.	**Comment on Jan 28 Submittal** **Sheet C0.00** Minor Comment: General Note 6: Suggest deleting "SEWER" so text now reads "AN ON-SITE STORMWATER COLLECTION SYSTEM".
2.	**Comment on Jan 28 Submittal** **Sheet C0.00** General Note 5: This note says that CoA will provide the electric service. As the documents now show, the proposed transformer connects to a CoA power line. But please note that the KFC site is in the Duke Energy service territory, not the CoA service territory. Duke Energy would need to grant permission for CoA to be the electric service provider for this site. This reviewer is of the understanding that the normal procedure is for the owner/designer to initiate contact with Duke Energy.

NO.	COMMENTS
3	**Comment on Jan 28 Submittal** **Sheet C0.10** **Water & Wastewater General Note 12:** Please add the following sentence to this note: "ALL PVC SERVICE LINES SHALL BE SUITABLE FOR POTABLE WATER SERVICE AND RATED NSF 61." **General Notes 25 & 26:** These two notes are very similar, but not identical. Suggest deleting General Note 25.
4	**Comment on Jan 28 Submittal** **Sheet C1.10** The two detectable warning strips are within the right of way. Thus, these strips shall conform to CoA standard detail 403. Please call this out, and include standard detail 403 in the site plans.
5	**Comment on Jan 28 Submittal** **Sheet C3.10** Minor: Call out the inset as "INSET A-A".
6	**Comment on Jan 28 Submittal** **Sheet C3.10** "PE" is called out on this sheet for polyethylene water piping. Please add "PE" to the Legend Sheet.
7	**Comment on Jan 28 Submittal** **Sheet C3.10** Please call out a minimum distance of 3' 0" between the P-W & P-WW.
8	**Comment on Jan 28 Submittal** **Sheet C3.10** Inset A-A: For the gate valve leading to the potable water and irrigation meters, call out "2-INCH GATE VALVE & BOX".
9	**Comment on Jan 28 Submittal** **Sheet C3.10** Attached markup of C3.10 shows a shaded blue water line. Please call this line out as "ABANDONED".

NO.	COMMENTS
10	**Comment on Jan 28 Submittal** **Sheet C3.10** The grease trap is called out as 1000 gallons. Final sizing of the grease trap requires that the owner submit grease trap sizing calcs as per CoA Design & Construction Manual Supplement and then approved by CoA. Thus suggest adding a grease trap note, similar to: "BEFORE INSTALLING 1000 GAL GREASE TRAP, SUBMIT SIZING CALCULATIONS, WHICH MUST BE APPROVED BY COA"
11	**Comment on Jan 28 Submittal** **Sheet C3.10** FYI For the recent Burger King project, which was designed by CHW, it eventually became known that a vent line from the grease trap to the building is required. Suggest that the need for such a vent line be discussed with owner.
12	**Comment on Jan 28 Submittal** **Sheet C3.10** As previously stated, the site is in the Duke Energy service territory. Duke must agree to CoA providing the power. Until this issue is resolved, no technical comments related to the electric design will be made.
13	**General Comment** Please update the following details – see Bryan. • 127 • 130
14	**General Comment** Please add the following details – see Bryan. • 121 - Backflow Preventer and Enclosure • 145 - Gate Valve – Water Mains – 4-inch & larger. • 227 - Grease Interceptor • 230 -Ring and Cover. • 403 - ADA Detectable Warning
15	
END OF COMMENTS	

Please advise if you have any questions or require additional information.
cc: Justin Tabor – AICP Principal Planner
Adam Hall – AICP Planner
Harry Dillard – Lead Engineering Technician





7 THIS SHOWS TRANSFORMER CONDUIT CONNECTED TO COA POWER LINE. THIS SITE IS WITHIN DUKE ~~ENERGY~~ ENERGY ~~SERVICE~~ SERVICE TERRITORY. COA CAN ONLY SUPPLY APPROVAL IF DUKE ENERGY GRANTS APPROVAL.

8 ELECTRICAL DESIGN NOT REVIEWED. ~~DUKE~~ DUKE ENERGY SERVICE TERRITORY ~~MUST BE FIRST BE~~ MUST BE RESOLVED. ~~PERMIT~~

ISSUE

1 DEFINE "PE" ON LEGEND SHEET.

2 CALL OUT ~~3'~~ 3'0" MIN DISTANCE BETWEEN P-W & P-WW.

3 CALL OUT "2" GATE VALVE AND BOX"

4 WATER MAIN SHADED IN BLUE IS ABANDONED. EITHER CALL OUT AS SUCH, OR DELETE.

5 ADD NOTE SIMILAR TO: "DO NOT INSTALL 1000 GAL GREASE TRAP UNTIL DILLY AFTER COA APPROVES SUBMITTED SIZING CALCULATIONS."

6 ~~SPECIFIC~~ FOR RECENT BURGER KING JOB, VENT LINE WAS REQUIRED BETWEEN GREASE ~~AND~~ AND BLDG. SUGGEST THAT NEED OF VENT LINE BE DISCUSSED WITH ~~PLUM~~ KFC RESTAURANT DESIGNER.

11801 Research Drive
Alachua, FL 32615
(352) 331-1976
www.chw-inc.com
est. 1988 FLORIDA CA-5075
CHW Professional Consultants

SCALE: 1"=20'
VERIFY SCALE BAR IS TRUE FOR ORIGINAL DRAWING.
IF NOT ONE INCH ON THIS SHEET, ADJUST SCALES ACCORDINGLY

SUBMITTAL
1/28/21 COA, SRWMD, AND FDOT

CLIENT: DE FOODS, LLC.
PROJECT: KFC ALACHUA, FL
SHEET TITLE: DETAILED UTILITY PLAN

DESIGNED: T.E. CONWAY
DETAILED: C. MENEHENNETT, E.I.
QUALITY CONTROL: D.H. YOUNG, P.E.
PROJECT MANAGER: DANIEL H. YOUNG
PROJECT NO: 20-0113

Daniel H. Young, P.E.
State of Florida, Professional Engineer, License No. 70780
This item has been digitally signed and sealed by Daniel H. Young, P.E. on the date indicated here: 01/28/2021
Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

FL PE No. 70780

SHEET NO: C3.10

JBrown Professional Group
CIVIL ENGINEERING • LAND SURVEYING • PLANNING
3530 NW 43rd Street • Gainesville, FL 32606 • 352.375.8999 • JBProGroup.com

February 10, 2021

Mr. Adam Hall, AICP
Planner
City of Alachua
Office of Planning & Community Development
P.O. Box 9
Alachua, FL 32616-0009

Re: KFC – Alachua

Dear Mr. Hall:

As you requested, we have reviewed the KFC - Alachua submittal drawings and other materials provided to us for the above referenced project. The drawings reviewed were created by CHW and dated 01/28/2021. Our review generated a few minor comments and recommendations that are outlined below.

C0.21 - SWPPP

1. Please provide a silt fence and tree protection detail.
2. Silt fence is not necessary along the west side of the project due to the existing topography.
3. Suggest tree barricade along the south boundary west of the proposed sanitary connection.

C0.30 – Demolition Plan

1. Please freeze the proposed transformer on this sheet.

C200 – Dimension Plan

1. Suggest placing a 6” flush header curb at the future stub on the west side of the site to protect the asphalt.
2. Consider placing an additional direction area for the by-pass lane adjacent to the arrow for the drive thru at the edge of the asphalt pavement.

C2.10 – Detailed Grading & Drainage Plan

1. Please callout locations where curb will be spill out vs. standard type F.
2. There are 2 callouts within the building showing a storm C/O with traffic rated cover. These are not pointing to anything. Please show how storm cleanouts will tie into storm system.
3. Please label the centerline of the swale along the west side of the property and provide a typical section.
4. Please label the rip-rap located at the S-5 outfall.
5. Please label the 78 contour at the bottom of the basin.

6. Suggest shifting S-10 slightly to the north. Currently, the 18" pipe will be exposed at the back of the structure.
7. Please clarify whether the pipe from S-10 to S-11 outfall structure will be a 15" or 18" RCP pipe. The detail shows an 18" pipe, the label says 15".
8. Please show a detail of S-3 outfall structure.
9. Please add a storm sewer profile from S-1 to S-2.
10. Please provide concrete energy dissipation pad at S-2, S-4, and S-11 to minimize erosion in right of way.

C2.30 – Construction Details

1. Please verify that 2" of asphalt within the FDOT right of way will be acceptable with the FDOT.
2. Please add a detail of Type F Spillout curb if this is to be used on the site.

C3.10 – Utility Plan

1. Per the FDEP, there is a 10' preferred and 6' minimum separation requirement for wastewater and water lines. Please verify that this separation will be met.
2. Please verify the material of the wastewater sewer that will be tied into. The upstream manhole shows PVC, the downstream manhole shows RCP.
3. Please callout a 15"x4" PVC wye and bend w/ 2 couplings at the connection of the proposed wastewater to the existing wastewater sewer.
4. Please revise cleanout elevations on wastewater line to a minimum of 3' depth. One invert is listed above the top of the structure, others are very shallow.
5. Please callout inverts on the grease trap.
6. Please callout gate valve on water fitting inset.

Landscape / Photometric Plans

1. Please revise location of light pole in median on south side of parking lot. This light pole if located right behind a proposed tree.

Sincerely,

A. J. "Jay" Brown, Jr., PE
President, JBrown Professional Group Inc.

Cc: Daniel Young

VOLKERT

Volkert, Inc.
3501 South Main Street
Suite 2
Gainesville, FL 32601

Office 352.372.9594
Fax 352.371.3988
gainesville@volkert.com

www.volkert.com

February 17, 2021

Mr. Adam Hall, AICP
Planner, Office of Planning and Community Development
City of Alachua
15100 NW 142nd Terrace
P.O. Box 6
Alachua, FL 32616

Subject: Independent Review of a Quick Service Restaurant on US 441 in the City of Alachua.

Dear Mr. Hall:

Attached is the completed independent review of the Traffic Impact Analysis for the Quick Service Restaurant (KFC) on US 441 in the City of Alachua.

Thank you for the opportunity to provide transportation engineering services for the City of Alachua, FL. If you have any questions or comments or require additional information, please contact me at 352-240-7459 (direct line) or 352-262-3580 (cell phone).

Sincerely,

Volkert, Inc.

Brian Kanely

Brian Kanely, P.E.
Senior Traffic Engineer

CC: Bo Sanchez, P.E.

Attachment: Independent Review

Independent Review of the Traffic Impact Analysis for a
Quick Service Restaurant (KFC)
Located in the City of Alachua, Florida
Prepared by Volkert, Inc., February, 2021

Introduction

At the request of the City of Alachua, FL, Planning and Community Development Department, Volkert, Inc., conducted an independent review of the Traffic Impact Analysis for a Quick Service Restaurant in the City of Alachua. The Traffic Impact Analysis was prepared by CHW in December, 2020.

The Quick Service Restaurant (QSR) is located on the south side of US 441 approximately 830 feet west of the I-75 southbound ramp. The site is immediately west of the new Burger King restaurant. The site will have access to US 441 via a connection to a frontage road that is located immediately south of US 441. The Project is expected to generate 505 net new daily trips.

The independent review of the Traffic Impact Analysis (TIA) will include, but is not limited to, the following items:

- Review of the study area.
- Review of the trip generation data.
- Review of data collection (existing and future traffic volumes) and reserved trips.
- Review of project trip distribution.
- Review of intersection analysis (stop and traffic signal controlled).
- Review of access management.
- Review of the TIA conclusions and recommendations.
- Review of any additional items.

Review of the Traffic Impact Analysis

Review of the Study Area

Figure 1 shows the project Study Area and does contain the major features along US 441 in a very simplified manner. The section of US 441 between I-75 and NW 167th Blvd (access to the Publix shopping area) has experienced rapid development in the last several years. Google Earth does not have an updated representation of this area. To assist with the review and understanding of the TIA, a detailed drawing/aerial of the Study Area is recommended. This drawing/aerial would show all the recent development, development under construction (like the Hampton Inn), the current traffic lane/full median openings/directional median opening configurations on US 441, the new road on the north side of US 441 that accesses the Hampton Inn, the frontage road in front of the Burger King, etc. Such a drawing/aerial would significant help in understanding the TIA and the issues with trip distribution and access management.

Volkert Recommendations:

1. A drawing/aerial should be provided that shows the current and future proposed developments along US 441 and roadway lane configurations/median openings, etc. along US 441 in the Study Area.

Review of Trip Generation Data

This section of the TIA describes and documents the trip generation for the Project. The total daily trips (1000 trips) and total PM peak hour trips (69) are correctly calculated per the ITE *Trip Generation Manual, 10th Edition.* When pass-by traffic is accounted for there are 505 net new daily trips and 34 net new PM peak hour trips. Quick service restaurants have a high percentage of pass-by traffic; traffic that is already on the roadway and the motorist decides to stop at the land use while 'passing-by'. The percentage of pass-by traffic that was accounted for is consistent with ITE data. The trip generation data was correctly calculated.

Volkert Recommendations:

1. The ITE trip generation data sheets should be included in the appendix of the TIA.

Review of Data Collection and Reserved Trips

This section of the TIA describes the collection of traffic data and application of reserved trips. The traffic data collection and application of a historical growth rate along US 441 based on FDOT traffic data was correctly calculated. Use of a minimum traffic growth rate of 1% is standard practice in traffic studies and is consistent with other projects in the City of Alachua.

Reserved trips are trips associated with developments that are currently under construction and/or permitted by the City for construction. The City maintains a list of reserved trips for the various roadway segments on the major roads in the City. It is routine practice to include the reserved trips in the traffic data calculation for developing future traffic projections. The reserved trips in the TIA were correctly calculated and applied to the future traffic projections.

Volkert Recommendations:

1. None

Review of Project Trip Distribution

This section of the TIA assigns the project trips to the roadway network. The key feature in this section is to determine the directional distribution of the trips onto the roadway network based on the time of day (percentage of trips headed south to I-75/Gainesville and to the north to High Springs). For small projects like the QSR, standard practice is to mimic the existing directional distribution that is observed with similar projects in the immediate area. For this QSR, the directional distribution for existing traffic was used to determine the project trip directional distributions. The results obtained are reasonable and consistent with previous traffic studies along this this section of US 441. Volkert concurs with the project trip distribution in the TIA.

Volkert Recommendations:

1. None.

Review of Intersection Analysis

This section of the TIA analyses the Study Area intersections for deficiencies that include any traffic movements operating with a LOS of F, volume/capacity (v/c) ratios that exceed 1.0 and 95th percentile queue lengths exceeding the available storage (are the traffic storage lanes long enough to accommodate the traffic and not overflow into the adjacent travel lane). The appropriate traffic engineering software was used to analyze the intersections and Volkert concurs with the TIA findings.

The intersection analysis found two deficiencies as follows:

1. The first deficiency is the northbound movement at the US 441/Burger King/Hampton Inn connection. This deficiency has been corrected with the full median opening being converted to a directional median opening. The new directional median opening is now in operation.
2. The second deficiency is the southbound movement on NW 167th Blvd at the traffic signal at US 441. The two southbound lanes on NW 167th Blvd have a storage length of 150 feet. Any southbound traffic that backs up past 150 feet blocks the driveway into the Raceway Gas Station. The analysis showed that a storage length of 175 feet was required. There is actually additional southbound storage on NW 167th Blvd north of the Raceway Driveway, but 150 feet was used in the intersection analysis. This is not a significant issue and any modifications to NW167th Blvd are not the responsibility of the QSR.

The QSR is a small project with a low number of peak hour trips. This small number of trips would not be expected to significantly impact the adjacent roadway conditions with the reserved trips added.

Volkert Recommendations:

1. None

Review of Access Management

This section of the TIA discusses how traffic will access the QSR site. The access to the QSR site is from a frontage road along the south side of US 441. Access to the frontage road is from a proposed right in/right out driveway located along the southbound section of US 441 midway between the traffic signal at NW 167th Blvd and the right in/right out driveway on the frontage road that provides access to the Burger King site. As pointed out in the TIA, traffic that is exiting the QSR and headed west on US 441, can exit the proposed new right in/right out driveway on US 441, drive into the left turn lane at the new directional median opening and make a U-turn and proceed west on US 441. This is the preferred exiting for the QSR site rather than travel east on the frontage road, exit the right in/right out driveway by the Burger King and then make a U-turn at the directional median opening that provides access to the Dunkin Donuts site on the north side of US 441. As discussed in the TIA, the proposed right in/right out driveway on US 441 that provides access to the frontage road falls slightly short of meeting FDOT Access Management

Spacing Standards. However, the benefits of locating the new right in/right out driveway midway between the NW 167th Blvd traffic signal and the right in/right out driveway for the Burger King more than offset just falling short of the FDOT Spacing Standards. Volkert concurs with the recommendation in the TIA for locating the new right in/right out driveway for the QSR midway between the traffic signal at NW 167th Blvd and the right in/right out driveway that provides access to the Burger King site.
It should be noted that the frontage road will provide access to future development along the south side of US 441 between the QSR and the NW167th Blvd traffic signal. The frontage road will provide for safer traffic operations along this section of US 441 and is a design feature seen at many locations.

It would be informative if the TIA included a brief discussion and a drawing of the frontage road along the south side of US 441. This would include the ultimate start/stop point of the frontage road, any future access points along the frontage road and US 441, etc. At the end of this review is a copy of the drawing in the TIA Appendix B. This type of drawing and detail on one of more 8.5 x 11 sheets of paper would adequately show the access management features discussed in the TIA.

Volkert Recommendations:

1. A scale drawing should be provided that shows in detail the access management features for the project. The drawing should include;
    a. The section of US 441 between the US 441 & I-75 SB ramp traffic signal and the traffic signal at US 441 and NW 167th Blvd.
    b. The median openings and pavement markings along US 441.
    c. The details of the frontage road on the south side of US 441 including the access for the Burger King and QSR.
2. The TIA should include a brief discussion of the frontage road along the south side of US 441. This would include:
    a. The ultimate start/stop points of the frontage road.
    b. Any future access point along the frontage road.
    c. Any other details pertain to the frontage road.

Review of the TIA Conclusions and Recommendations

This section of the TIA states the report's conclusions and recommendations. These are:

1. The traffic associated with the project creates no deficiencies at the study area intersections; therefore no transportation improvements/modifications are required.
2. The spacing standards for the proposed right in/right out access from US 441 to the frontage road in front of the QSR fall slightly short of the FDOT spacing standards. However, the new right in/right out access allows QSR traffic to access the new directional median opening on US 441 to make a U-turn and proceed west. Being able to access this new directional median opening is preferable over using the median opening at the Dunkin Donuts because it provides more storage length and also better distributes the traffic along the frontage road.

Additional Volkert Comments

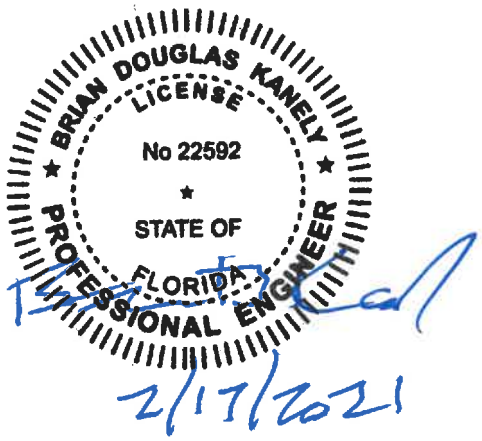
1. The TIA is well prepared and adequately address the traffic issues associated with the proposed QSR.
2. As discussed in the review, providing additional drawings/aerials that document the existing conditions in the study area and the proposed details of the frontage road and access management for the project will be extremely helpful. These drawings should be on 8.5 x 11 paper for easy printing. This additional information would help individuals not familiar with the study area understand the traffic information and circulation discussed in the TIA.

Volkert, Inc. appreciates the opportunity to assist the City of Alachua with this independent review of the TIA for the QSR. Please contact me if you have any additional questions or need additional information.

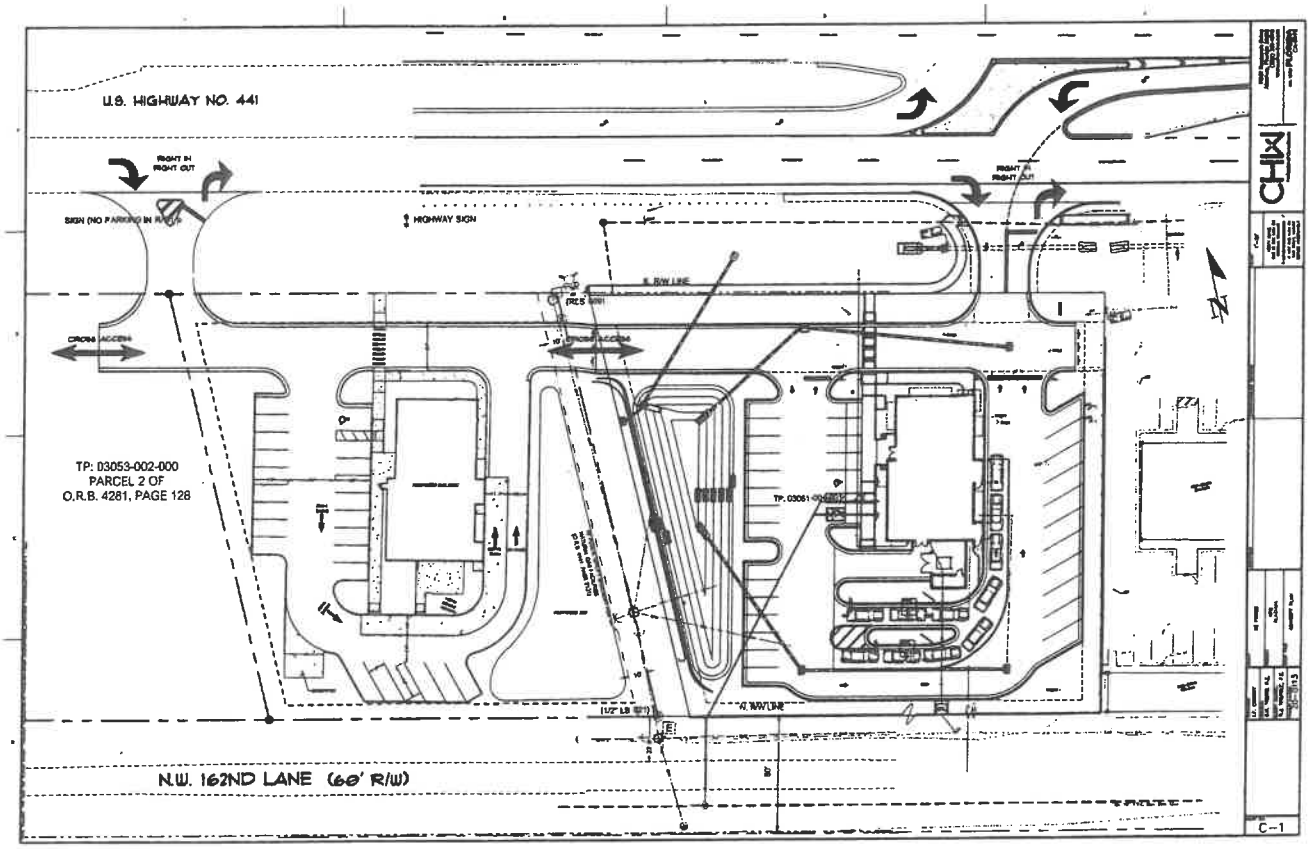
Prepared by:

Brian D. Kanely, P.E.
Senior Traffic Engineer
Volkert, Inc.
February 17, 2021

Attachment: Drawing from TIA, Appendix B



Drawing From TIA, Appendix B



City of Alachua

ADAM BOUKARI
CITY MANAGER

PLANNING & COMMUNITY DEVELOPMENT
DIRECTOR KATHY WINBURN, AICP

February 2, 2021

Daniel Young, P.E.
11801 Research Drive
Alachua, FL 32615

Also submitted electronically to daniely@chw-inc.com

RE: Application Acceptance for KFC Alachua Site Plan

Dear Mr. Young:

On January 28, 2021 the City of Alachua received your site plan application for the KFC Alachua project, which proposes the construction of a ±1,904 square foot restaurant with drive through on a ±1.00 acre subject property (Parcel No. 03053-002-001 and a portion of 03053-002-000).

According to Section 2.2.6 of the Land Development Regulations (LDRs), upon receipt of an application, a completeness review shall be conducted to determine that the application contains all the necessary information and materials, is in proper form and of sufficient detail, and is accompanied by the appropriate fee. The Planning Department has reviewed the aforementioned application for completeness and finds that the application is complete.

The contents of the applications have not been thoroughly reviewed. An in-depth review of the content of the application will be performed subsequently and any further issues with content will be provided to you at the Development Review Team meeting, which will be scheduled separately from this letter. Should you have any questions, please feel free to contact me at (386) 418-6100, ext. 108 or via email at ahall@cityofalachua.com.

Sincerely,

Adam Hall, AICP
Planner

c: Kathy Winburn, AICP, Planning Director
Justin Tabor, AICP, Principal Planner
File

SITE PLANS
FOR:

KFC
ALACHUA

ALACHUA COUNTY, FLORIDA

SECTION 9, TOWNSHIP 8 SOUTH, RANGE 18 EAST

SUBMITTED TO:
SUWANNEE RIVER WATER MANAGEMENT DISTRICT
FLORIDA DEPARTMENT OF ENVIRONMENTAL PROTECTION
FLORIDA DEPARTMENT OF TRANSPORTATION
CITY OF ALACHUA

GENERAL NOTES

1. DEVELOPMENT DATA:

	PROPOSED DEVELOPMENT		
OVERALL SITE AREA=	43,520 S.F.	100.0%	1.00 ACRES
BUILDING FOOTPRINT AREA=	1,904 S.F.	4.4%	
CONC./PAVE AREA=	19,727 S.F.	45.3%	
TOTAL IMPERVIOUS AREA=	21,631 S.F.	49.7%	
OPEN AREA=	21,889 S.F.	50.3%	

DESCRIPTION: THIS PROJECT CONSISTS OF THE CONSTRUCTION OF A ±1,904 SF QUICK SERVE RESTAURANT, WITH DOUBLE DRIVE THROUGH PARKING AREA, STORMWATER MANAGEMENT FACILITY, AND ASSOCIATED UTILITY INFRASTRUCTURE.

2. MINIMUM BUILDING / YARD SETBACKS PER COMMERCIAL INTENSIVE ZONING ARE AS FOLLOWS:
PRIMARY BUILDING (CI)
FRONT: 20'
SIDE: N/A
REAR: 15'

MINIMUM LOT AREA	NONE
MINIMUM LOT WIDTH	NONE
MAX BUILDING HEIGHT	65'
MAX LOT COVERAGE	NONE
PROPOSED FAR	0.04

3. PARKING CALCULATIONS:
CAR: 1 SPACE PER 150 SQ. FT. FLOOR AREA
1,904 SQ. FT. / 150 SQ. FT. = MINIMUM 13 SPACES
13 SPACES X 125% = MAXIMUM 16 SPACES
PROVIDED: 15 SPACES TOTAL

ACCESSIBLE SPACES:
REQUIRED = 1
PROVIDED = 1

BICYCLE: 1 SPACE/10 CAR SPACES = 15 / 10 = 1.5 SPACES REQUIRED;
2 SPACES PROVIDED

4. DATE OF BOUNDARY SURVEY:
A BOUNDARY SURVEY WAS COMPLETED BY CHW AND DATED OCTOBER 18, 2020.

5. UTILITIES:
-POTABLE WATER AND FIRE SERVICE WILL BE PROVIDED BY CITY OF ALACHUA.

-SANITARY SEWER SERVICE WILL BE PROVIDED BY CITY OF ALACHUA.

-ELECTRIC SERVICE WILL BE PROVIDED BY DUKE ENERGY.

-GAS SERVICE WILL BE PROVIDED BY GRU.

6. DRAINAGE:
AN ON-SITE STORMWATER COLLECTION SYSTEM WILL BE CONSTRUCTED AND CONNECT TO A PROPOSED STORMWATER MANAGEMENT FACILITY. THE FACILITY MEETS CITY OF ALACHUA AND SRWMD DESIGN CRITERIA.

7. NATURAL FEATURES:
TOPOGRAPHY WILL SLOPE FROM WEST TO EAST WITH MODERATE SLOPES. THERE ARE NO OTHER NATURAL SITE FEATURES.

8. ZONING/LAND USE INFORMATION AND COMPLIANCE:
COMMERCIAL INTENSIVE (CI) ZONING DISTRICT DESIGNATION.

THE PROPOSED FACILITIES ARE CONSISTENT WITH THE COMMERCIAL INTENSIVE DEFINITION PROVIDED IN ARTICLE 4 OF THE LDRS. PER ARTICLE 4 OF THE LDRS, RESEARCH AND DEVELOPMENT IS A PERMITTED USE WITHIN THE COMMERCIAL INTENSIVE ZONING DISTRICT.

PER ARTICLE 6 OF THE LDRS, THE PROPOSED SITE IS CONSISTENT WITH ALL APPLICABLE DEVELOPMENT STANDARDS. COMPLIANCE WITH REQUIRED PARKING CAN BE FOUND IN NOTE 3 ON THIS SHEET. THERE ARE EXISTING TREES ON SITE AND THE PROPOSED LANDSCAPING FULFILLS GENERAL SITE, MITIGATION, AND BUFFER REQUIREMENTS. THE PROPOSED STORMWATER CONVEYANCE SYSTEM AND EROSION PROTECTION PROPOSED FULFILL ENVIRONMENTAL PROTECTION STANDARDS THROUGH THE USE OF BMPS.

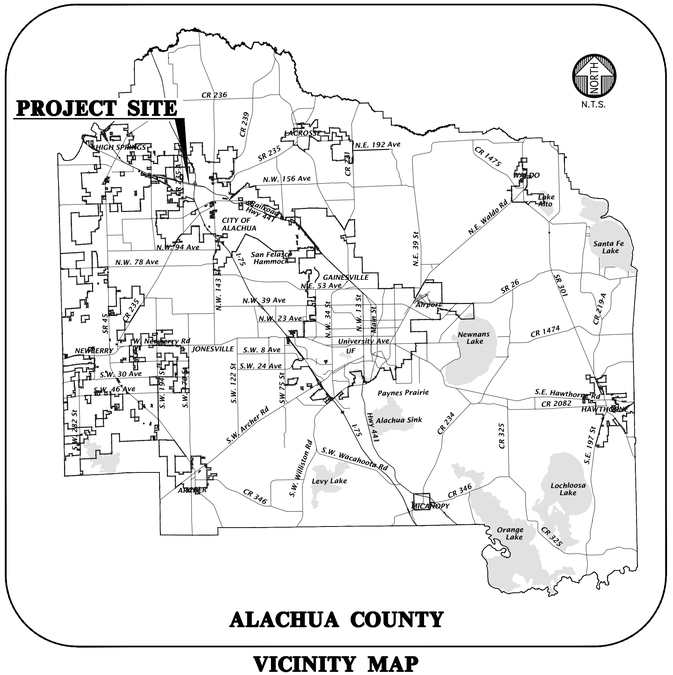
9. CONCURRENCY IMPACT ANALYSIS:
SUBMITTED UNDER SEPARATE COVER

10. LEGAL DESCRIPTION:

(PER O.R.B. 4645, PAGE 1470 & 1471)
A PARCEL OF LAND SITUATED IN SECTION 9, TOWNSHIP 8 SOUTH, RANGE 18 EAST, ALACHUA COUNTY, FLORIDA, BEING MORE PARTICULARLY DESCRIBED AS FOLLOWS:

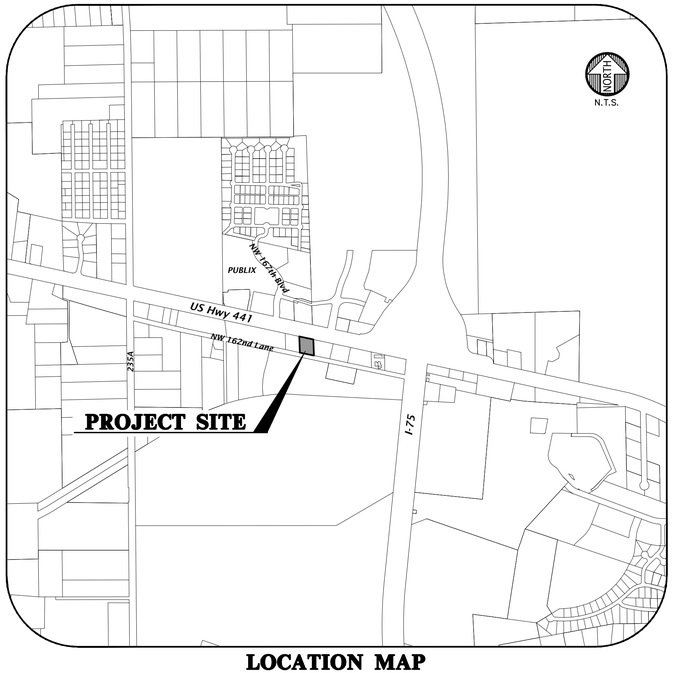
FOR A POINT OF REFERENCE COMMENCE AT THE SOUTHEAST CORNER OF SAID SECTION 9; THENCE NORTH 03°37'28" WEST, A DISTANCE OF 286.74 FEET TO THE NORTH RIGHT OF WAY LINE OF OLD S.R. NO. 25 (OLD US HIGHWAY 441); THENCE NORTH 79°23'05" WEST, ALONG SAID NORTH RIGHT OF WAY LINE, A DISTANCE OF 2353.88 FEET TO THE SOUTHEAST CORNER OF PARCEL 2 OF THE LANDS DESCRIBED IN OFFICIAL RECORDS BOOK 4233, PAGE 1337 OF THE PUBLIC RECORDS OF ALACHUA COUNTY, FLORIDA AND THE POINT OF BEGINNING; THENCE CONTINUE NORTH 79°23'05" WEST, ALONG SAID NORTH RIGHT OF WAY LINE, A DISTANCE OF 198.88 FEET; THENCE NORTH 03°02'56" WEST, A DISTANCE OF 225.41 FEET TO THE SOUTHERLY RIGHT OF WAY LINE OF NEW HIGHWAY NO. 441 (200' WIDE RIGHT OF WAY); THENCE SOUTH 79°05'00" EAST, ALONG SAID SOUTHERLY RIGHT OF WAY LINE, A DISTANCE OF 199.13 FEET TO THE NORTHEAST CORNER OF SAID PARCEL 2; THENCE SOUTH 03°02'54" EAST, ALONG THE EAST LINE OF SAID PARCEL 2, A DISTANCE OF 224.33 FEET TO THE POINT OF BEGINNING.

CONTAINING 1.00 ACRES, MORE OR LESS.



ALACHUA COUNTY

VICINITY MAP



LOCATION MAP

BEFORE YOU DIG !
CALL SUNSHINE STATE ONE CALL OF FLORIDA
AT LEAST TWO FULL BUSINESS DAYS BEFORE DIGGING OR DISTURBING EARTH
1-800-432-4770
Know what's **below.** **Call** before you dig.

SHEET INDEX

SHEET NUMBER	DESCRIPTION
C0.00	COVER SHEET AND INDEX
1 OF 1	ALTA/NSPS LAND TITLE SURVEY
C0.10	GENERAL NOTES
C0.11	LEGEND
C0.20	STORMWATER POLLUTION PREVENTION NOTES
C0.21	STORMWATER POLLUTION PREVENTION PLAN AND DETAILS
C0.30	DEMOLITION SITE PLAN
C1.10	DETAILED HORIZONTAL CONTROL AND SITE PLAN
C1.20	ACCESSIBILITY SITE PLAN AND DETAILS
C2.10	DETAILED GRADING AND DRAINAGE PLAN
C2.30 - C2.32	CONSTRUCTION DETAILS
C3.10	DETAILED UTILITY PLAN
LS-1	LANDSCAPE NOTES, DETAILS, & SCHEDULE
LS-2	LANDSCAPE PLAN
IR-1	IRRIGATION PLAN, DETAILS, & SCHEDULE
P1 - P2	PHOTOMETRICS
A1.0	ARCHITECTURAL FLOOR PLAN
A4.0-A4.1	ARCHITECTURAL EXTERIOR ELEVATIONS

SURVEYOR OF RECORD
MARTIN G. DARDIS, P.L.S.
CHW
11801 RESEARCH DRIVE
ALACHUA, FL 32615
(352) 331-1976

LANDSCAPE ARCHITECT
CAELI M. TOLAR, L.A.
CHW
11801 RESEARCH DRIVE
ALACHUA, FL 32615
(352) 331-1976

DEVELOPER
DAN WHITNEY
DE FOODS, LLC.
101 ACCORD PARK DRIVE
NORWELL, MA 02061

PROPERTY OWNER
T & H PROPERTY GROUP, LLC.
3580 NORTH MAIN STREET
GAINESVILLE, FL 32609

PHOTOMETRIC
BILL STORMANT, PE
5304 NW 173 ST.
ALACHUA, FL. 32615
stormantwt@cox.net

ENGINEER OF RECORD
DANIEL H. YOUNG, P.E.
CHW
11801 RESEARCH DRIVE
ALACHUA, FL 32615
(352) 331-1976

11801 Research Drive
Alachua, Florida 32615
(352) 331-1976
www.chw-inc.com
est. 1988 FLORIDA CA-9075
CHW Professional Consultants

SCALE: N/A
VERIFY SCALE
BAR IS ONE INCH ON ORIGINAL DRAWING
IF NOT ONE INCH ON THIS SHEET ADJUST SCALES ACCORDINGLY
0 1"

CONSTRUCTION/BID REVISIONS:

SUBMITTALS:
1/28/21 COA, SRWMD, AND FDOT
2/19/21 COA, SRWMD
3/18/21 FDOT
3/25/21 COA, SRWMD

CLIENT: DE FOODS, LLC.
PROJECT: KFC ALACHUA, FL
SHEET TITLE: COVER SHEET AND INDEX

TECHNICIAN: T.F. COWART
DESIGNER: C. MEHENNETT, E.I.
PROJECT ENGINEER: D.H. YOUNG, P.E.
PROJECT NUMBER: 20-0113

DANIEL H. YOUNG
FL PE No. 70780

SHEET NO.: C0.00

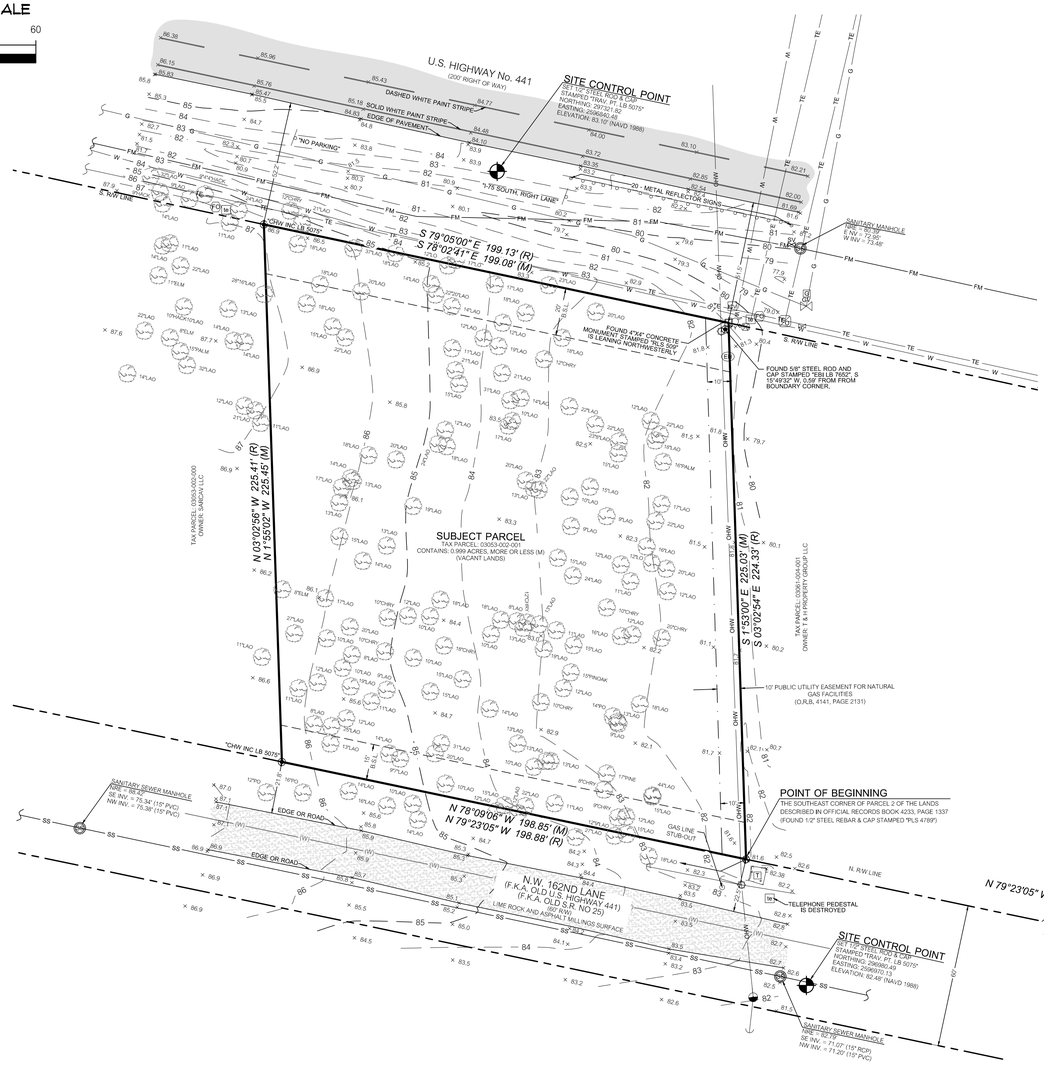
Tech: colem Plot Date: Mar 24,2021 5:34pm Filename: H:\2020\20-0113\Department\04_Engineering\02_DWGs\Production Files\20-0113 CV.dwg

ALTA / NSPS LAND TITLE SURVEY

LOCATED IN SECTION 9, TOWNSHIP 8 SOUTH, RANGE 18 EAST, CITY OF ALACHUA, ALACHUA COUNTY, FLORIDA

GRAPHIC SCALE

0 15 30 60



LEGAL DESCRIPTION:

(PER O.R.B. 4645, PAGE 1470 & 1471)
A PARCEL OF LAND SITUATED IN SECTION 9, TOWNSHIP 8 SOUTH, RANGE 18 EAST, ALACHUA COUNTY, FLORIDA, BEING MORE PARTICULARLY DESCRIBED AS FOLLOWS:

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CONTAINING 1.00 ACRES, MORE OR LESS.

SURVEYOR'S NOTES:

1. BEARINGS SHOWN HEREON ARE BASED ON STATE PLANE FLORIDA NORTH ZONE 83/86 DATUM PROJECTED FROM ALACHUA COUNTY DENSIFICATION SURVEY 1988 - RECOVERED TRAVERSE CONTROL POINTS #4602 & 4603.
2. ELEVATIONS SHOWN HEREON ARE NORTH AMERICAN DATUM OF 1988 (NAD88) PROJECTED FROM RECOVERED BENCHMARK C 51- 6"X6" CONCRETE MONUMENT AND BRASS CAP STAMPED C 51 1934 HAVING A PUBLISHED ELEVATION OF 75.14 FEET.
3. THE SURVEYOR HAS NO KNOWLEDGE OF UNDERGROUND FOUNDATIONS WHICH MAY ENCROACH.
4. INSTRUMENTS OF RECORD REFLECTING EASEMENTS, RIGHTS-OF-WAY, AND OR OWNERSHIP WERE FURNISHED TO THE SURVEYOR VIA FIDELITY NATIONAL TITLE INSURANCE COMPANY, POLICY NUMBER 126272-1-CT-18-1023-2018.2736609-21358O291, DATED MARCH 12, 2018 AN AMENDED OCTOBER 24, 2019. A SEARCH OF THE PUBLIC RECORDS HAS NOT BEEN DONE BY THIS SURVEYOR. NO INSTRUMENTS OF RECORD REFLECTING EASEMENTS, RIGHTS-OF-WAY, AND OR OWNERSHIP WERE FURNISHED TO THE SURVEYOR FOR PARCEL 2.
5. INFORMATION FROM FEDERAL EMERGENCY MANAGEMENT AGENCY, (F.E.M.A.) FLOOD INSURANCE RATE MAP(S), SHOWN ON THIS MAP WAS CURRENT AS OF THE REFERENCED DATE. MAP REVISIONS AND AMENDMENTS ARE PERIODICALLY MADE BY LETTER AND MAY NOT BE REFLECTED ON THE MOST CURRENT MAP.
6. FENCES, SYMBOLS, MONUMENTATION AND UTILITIES SHOWN HEREON MAY BE EXAGGERATED FOR PICTORIAL PURPOSES ONLY AND MAY NOT BE SHOWN TO SCALE.
7. IN THE OPINION OF THIS SURVEYOR, THE PERIMETER LINES AS SHOWN HEREON REPRESENT THE LOCATION OF THE BOUNDARY LINES OF THE SUBJECT PARCEL IN RELATION TO THE DESCRIPTION OF RECORD AND THOSE EXISTING LAND CORNERS FOUND TO BE ACCEPTABLE BY THIS SURVEYOR.
8. ADDITIONAL POINTS MAY BE FOUND BY TURNING ON THE SV-NODE* LAYERS IN THE SUPPLIED DIGITAL FILE.
9. TOPOGRAPHIC INFORMATION SHOWN HEREON BASED ON GROUND SURVEY. CONTOURS SHOWN HEREON REFLECT 1-FOOT INTERVALS.
10. THERE ARE NO STRIPED PARING SPACES ON THE SUBJECT PARCEL..
11. THERE WAS NO EVIDENCE OF RECENT EARTH MOVING WORK, BUILDING CONSTRUCTION, OR BUILDING ADDITIONS OBSERVED IN THE PROCESS OF CONDUCTING THE FIELDWORK.
12. THERE ARE NO KNOWN PROPOSED CHANGES IN STREET RIGHT OF WAY LINES, IF SUCH INFORMATION IS MADE AVAILABLE TO THE SURVEYOR BY THE CONTROLLING JURISDICTION. THERE WAS NO EVIDENCE OF RECENT STREET OR SIDEWALK CONSTRUCTION OR REPAIRS OBSERVED IN THE PROCESS OF CONDUCTING THE FIELDWORK.

LEGEND:

- = BENCHMARK
- = FOUND 4"X4" CONCRETE MONUMENT (MARKED AS NOTED)
- = FOUND 5/8" STEEL REBAR & CAP (MARKED AS NOTED)
- = FOUND 1/2" STEEL REBAR & CAP MARKED PLS 4789
- = CONCRETE POWER POLE
- = GAS VALVE
- = GUY ANCHOR
- = TELEPHONE PEDESTAL
- = GUY POLE
- = WOODEN POWER POLE
- = FIRE HYDRANT
- = SIGN
- = TREE (SIZE AND TYPE AS NOTED)
- = SANITARY SEWER MANHOLE
- = FIBER OPTIC BOX
- = FIBER OPTIC LINE MARKER
- = WATER VALVE
- = ELECTRIC HANDHOLE
- = CONTOUR LINE
- = SANITARY SEWER LINE
- = UNDERGROUND WATER LINE
- = WATER LINE (PER MAPS)
- = GAS LINE (PER MAPS)
- = OVERHEAD WIRE
- = END OF FEATURE NOT DETERMINED

(M) = DATA BASED ON FIELD MEASUREMENTS
(R) = DATA BASED ON DEED OF RECORD
P.U.E. = PUBLIC UTILITIES EASEMENT
P.O.B. = POINT OF BEGINNING
O.R.B. = OFFICIAL RECORDS BOOK
R/W = RIGHT OF WAY
TP = TAX PARCEL
NRE = NORTH RIM ELEVATION
FEMA = FEDERAL EMERGENCY MANAGEMENT AGENCY
NAVD = NORTH AMERICAN VERTICAL DATUM
X 83.5 = SPOT ELEVATION (PERVIOUS SURFACE)
X 83.75 = SPOT ELEVATION (IMPERVIOUS SURFACE)
LO = LIVE OAK
LAO = LAUREL OAK
CHRY = CHERRY
PO = PIN OAK
INV = INVERT
PVC = POLYVINYL CHLORIDE

- = ASPHALT SURFACE
- = LIME ROCK & ASPHALT MILLINGS SURFACE
- = CONCRETE SURFACE

FLOOD ZONE:

THIS PROPERTY IS LOCATED IN FEDERAL FLOOD ZONE "X", AREAS DETERMINED TO BE OUTSIDE THE 0.2% ANNUAL CHANCE FLOODPLAIN, AS INTERPOLATED FROM F.I.R.M. PANEL NO. 12001C0120DD, EFFECTIVE DATE: JUNE 16, 2006.

ZONING INFORMATION:

(PER MUNICODE.COM)

ZONED:	COMMERCIAL, INTENSIVE (CI)
BUILDING SETBACKS:	FRONT - 20 FEET
	SIDE - 0 FEET
	REAR - 15 FEET
BUILDING HEIGHT:	65 FEET MAXIMUM

SURVEYOR'S CERTIFICATION:

TO: T & H PROPERTY GROUP, LLC; FIDELITY NATIONAL TITLE INSURANCE COMPANY & COMMUNITY TITLE, LLC :

THIS IS TO CERTIFY THAT THIS MAP OR PLAT AND THE SURVEY ON WHICH IT IS BASED WERE MADE IN ACCORDANCE WITH THE 2016 MINIMUM STANDARD DETAIL REQUIREMENTS FOR ALTA/NSPS LAND TITLE SURVEYS, JOINTLY ESTABLISHED AND ADOPTED BY ALTA AND NSPS, AND INCLUDES ITEMS 1, 2, 3, 4, 5, 6(b), 8, 9, 11, 13, 14, 16, 17 AND 19, OF TABLE A THEREOF. THE FIELD WORK WAS COMPLETED ON OCTOBER 18, 2020.

MARTIN G. DARDIS, P.S.M.
FLORIDA SURVEYOR & MAPPER CERTIFICATE NUMBER 5097
MARTIND@CHW-INC.COM

DATE OF PLAT OR MAP:

11801 Research Drive
Alachua, FL 32615
(352) 331-1976
www.chw-inc.com
est. 1988 FLORIDA CA-5075

CHW
Professional Consultants

SCALE: 1" = 30'

VERIFY SCALE
BAR IS ONE INCH ON ORIGINAL DRAWING
IF NOT ONE INCH ON THIS SHEET ADJUST SCALES ACCORDINGLY.
0 1"

SEE SURVEYOR'S CERTIFICATE

CERTIFIED TO:

SURVEY DATE: 10/18/2020
REVISION DATE:
PROJECT NUMBER: 20-0113

TECHNICIAN: KWM
CREW CHIEF: NG
DRAWN BY: MGD
FIELD BOOK & PAGE: 599 / 76 & 80

This map prepared by:
Certificate of Authorization No. L.B. 5075
NOT VALID WITHOUT THE ORIGINAL SIGNATURE AND SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER

MARTIN G. DARDIS
SEE SURVEYOR'S CERTIFICATE
Professional Surveyor & Mapper Fla. License No. 5097

SHEET NO.: 1 OF 1

Tech: kwm Plot Date: Mar 01 2021 12:20pm Filename: \\civil\Users\section\kwm\Desktop\20-0113\20-0113 Topo.dwg

GENERAL NOTES

1. THE TOPOGRAPHIC AND EXISTING INFORMATION SHOWN HEREON WAS TAKEN FROM AN ALTA/NSPS LAND TITLE SURVEY AND COMPLETED BY THIS COMPANY, CHW, INC., ON OCTOBER 18, 2020.

2. THE LOCATION OF ALL EXISTING UTILITIES SHOWN ON THE PLANS HAS BEEN DETERMINED FROM THE BEST INFORMATION AVAILABLE. THE ENGINEER ASSUMES NO RESPONSIBILITY FOR ACCURACY. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO NOTIFY THE VARIOUS UTILITIES AND TO MAKE NECESSARY ARRANGEMENTS FOR ANY RELOCATION OF THESE UTILITIES WITH THE OWNER OF THE UTILITY. THE CONTRACTOR SHALL EXERCISE CAUTION WHEN CROSSING ANY UNDERGROUND UTILITY, WHETHER SHOWN ON THE PLANS OR LOCATED BY THE UTILITY COMPANY. THE RESPECTIVE UTILITY COMPANIES SHALL RELOCATE ALL UTILITIES THAT INTERFERE WITH THE PROPOSED CONSTRUCTION. THE CONTRACTOR SHALL COOPERATE WITH THE UTILITY COMPANIES DURING THE RELOCATION OPERATIONS. ANY DELAY OR INCONVENIENCE CAUSED TO THE CONTRACTOR BY THE VARIOUS UTILITIES SHALL BE INCIDENTAL TO THE CONTRACT AND NO EXTRA COMPENSATION WILL BE ALLOWED.

3. THE CONTRACTOR SHALL EXERCISE EXTREME CAUTION IN AREAS OF BURIED UTILITIES AND SHALL PROVIDE AT LEAST 48 HOURS NOTICE TO THE APPROPRIATE UTILITY COMPANIES IN ORDER TO ALLOW MARKING OF THE LOCATIONS OF EXISTING UNDERGROUND FACILITIES IN ADVANCE OF CONSTRUCTION BY CALLING THE FLORIDA SUNSHINE STATE ONE-CALL CENTER, INC. AT 1-800-432-4770 OR 811. IT IS THE CONTRACTOR'S RESPONSIBILITY TO NOTIFY "SUNSHINE" 48 HOURS PRIOR TO ANY CLEARING OF CONSTRUCTION TO IDENTIFY ALL UTILITY LOCATIONS. NO CONSTRUCTION ACTIVITY MAY OCCUR UNTIL THE UTILITIES HAVE BEEN PROPERLY MARKED.

4. THE CONTRACTOR SHALL FIELD VERIFY THE HORIZONTAL LOCATION AND VERTICAL LOCATION OF ALL EXISTING UTILITIES WITHIN THE LIMITS OF THE PROJECT ENVELOPE SHOWN PRIOR TO COMMENCING WORK. THE CONTRACTOR SHALL CALL ALL UTILITY COMPANIES TO HAVE THE LOCATIONS OF ALL UTILITIES FIELD MARKED PRIOR TO COMMENCEMENT OF CONSTRUCTION. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER PRIOR TO CONTINUING CONSTRUCTION.

5. THE CONTRACTOR IS RESPONSIBLE FOR REPAIRING ANY DAMAGE TO EXISTING FACILITIES, ABOVE OR BELOW GROUND THAT MAY OCCUR AS A RESULT OF THE WORK PERFORMED BY THE CONTRACTOR.

6. ALL PRIVATE AND PUBLIC PROPERTY AFFECTED BY THIS WORK SHALL BE RESTORED TO A CONDITION EQUAL TO OR BETTER THAN EXISTING CONDITIONS BEFORE COMMENCING CONSTRUCTION WORK, UNLESS SPECIFICALLY EXEMPTED BY THE PLANS. ADDITIONAL COSTS ARE INCIDENTAL TO OTHER CONSTRUCTION AND NO EXTRA COMPENSATION WILL BE ALLOWED.

7. ALL WORK PERFORMED SHALL COMPLY WITH THE REGULATIONS AND ORDINANCES OF THE VARIOUS GOVERNMENTAL AGENCIES HAVING JURISDICTION OVER THE WORK INCLUDING LANDSCAPING.

8. IT IS THE CONTRACTOR'S RESPONSIBILITY TO BECOME FAMILIAR WITH THE PERMIT AND INSPECTION REQUIREMENTS OF THE VARIOUS GOVERNMENTAL AGENCIES. THE CONTRACTOR SHALL OBTAIN ALL NECESSARY PERMITS PRIOR TO CONSTRUCTION AND SCHEDULE INSPECTIONS ACCORDING TO AGENCY AND/OR MUNICIPALITY INSTRUCTIONS.

9. IT SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR TO COMPLY WITH AND ENFORCE ALL APPLICABLE SAFETY REGULATIONS.

10. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROTECTING EXCAVATIONS AGAINST COLLAPSE AND SHALL PROVIDE BRACING, SHEETING OR SHORING AS NECESSARY. TRENCHES SHALL BE KEPT DRY WHILE PIPES ARE BEING PLACED. DEWATERING SHALL BE USED AS REQUIRED, AND PERMITTED THROUGH LOCAL GOVERNMENTAL AGENCIES AND WATER MANAGEMENT DISTRICT PER CURRENT REGULATIONS AT THE SOLE COST OF THE CONTRACTOR.

11. CONTRACTOR TO REVIEW GEOTECHNICAL REPORT AND BORINGS PRIOR TO BIDDING THE PROJECT AND FOLLOW OUTLINED CONSTRUCTION TECHNIQUES.

12. THE CONTRACTOR IS RESPONSIBLE FOR COORDINATING APPLICABLE TESTING WITH THE SERVICES OF AN APPROVED TESTING LABORATORY AND/OR SOILS ENGINEER, APPLICABLE REGULATORY AGENCIES, AND AS MAY BE FOUND IN THE ENGINEERING CONSTRUCTION DRAWINGS OR SPECIFICATIONS. CONTRACTOR TO VERIFY ALL TESTING WITH THE OWNER PRIOR TO COMMENCING CONSTRUCTION. UPON COMPLETION OF THE WORK, THE TESTING LABORATORY AND/OR SOILS ENGINEER MUST SUBMIT TO THE OWNER'S ENGINEER CERTIFICATIONS STATING THAT ALL REQUIREMENTS HAVE BEEN MET.

13. INSTALL SILT FENCE PRIOR TO SITE DEMOLITION OR NEW SITE CONSTRUCTION. INSTALL SILT FENCE PER FLORIDA STORMWATER EROSION AND SEDIMENTATION CONTROL INSPECTOR'S MANUAL AND PROVIDE TOE-IN. THE CONTRACTOR SHALL MAINTAIN THE SILT FENCE IN WORKING ORDER THROUGHOUT THE CONSTRUCTION PHASE. THE PROJECT SILT FENCE SHALL BE INSPECTED DAILY AND ANY CORRECTIVE MEASURES SHALL BE COMPLETED WITHIN 24 HOURS.

14. ALL TREE BARRICADES AND SILT FENCING SHALL BE INSTALLED AND INSPECTED BY THE CITY OF ALACHUA PRIOR TO COMMENCEMENT OF ANY DEMOLITION OR CONSTRUCTION ACTIVITIES.

15. THE CONTRACTOR IS TO PREPARE THE SITE PRIOR TO BEGINNING ACTUAL CONSTRUCTION IN ACCORDANCE WITH THE GEOTECHNICAL REPORT.

16. ALL DELETERIOUS MATERIAL (I.E. MUCK, PEAT, BURIED DEBRIS) IS TO BE EXCAVATED IN ACCORDANCE WITH THESE PLANS OR AS DIRECTED BY THE OWNER'S ENGINEER OR OWNER'S SOIL TESTING COMPANY. DELETERIOUS MATERIAL IS TO BE STOCKPILED AND REMOVED FROM THE SITE. EXCAVATED AREAS ARE TO BE BACKFILLED WITH APPROVED MATERIALS AND COMPACTED AS SHOWN ON THESE AREAS.

17. CONTRACTOR SHALL CLEAR AND GRUB ONLY THOSE PORTIONS OF THE SITE NECESSARY FOR CONSTRUCTION. DISTURBED AREAS SHALL BE SODDED, SEEDED, MULCHED, OR PLANTED WITH OTHER APPROVED LANDSCAPE MATERIAL, AS DIRECTED BY THESE PLANS, IMMEDIATELY FOLLOWING CONSTRUCTION PER LOCAL INSPECTOR.

18. WORK BEING PERFORMED UNDER THIS CONTRACT SHALL INTERFACE SMOOTHLY WITH OTHER WORK BEING PERFORMED ON THE SITE BY OTHER CONTRACTORS AND/OR UTILITY COMPANIES. IT WILL BE NECESSARY FOR THE CONTRACTOR TO COORDINATE AND SCHEDULE HIS ACTIVITIES, WHERE NECESSARY, WITH OTHER CONTRACTORS AND UTILITY COMPANIES.

19. ALL PAVEMENT DIMENSIONS SHOWN ARE TO EDGE OF PAVEMENT UNLESS OTHERWISE NOTED.

20. THE GOVERNING STANDARDS AND SPECIFICATIONS, UNLESS STATED OTHERWISE SHALL BE PER FLORIDA DEPARTMENT OF TRANSPORTATION DESIGN STANDARDS DATED FY 2020-2021, AND STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION DATED JANUARY 2021, AS AMENDED BY CONTRACT DOCUMENTS. ALL MATERIALS AND METHODS SHALL MEET FDOT SPECIFICATIONS AND SHALL BE PRODUCED OR OBTAINED FROM AN FDOT APPROVED SOURCE.

21. ALL NEW TRAFFIC CONTROL DEVICES (SIGNS AND PAVEMENT MARKINGS) SHALL CONFORM TO THE MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES (MUTCD) AND FDOT STANDARDS.

22. ALL STRIPING WITHIN THE FDOT RIGHT OF WAY SHALL BE PLACED FIRST AS TEMPORARY STRIPING FOLLOWED BY APPLICATION OF THERMOPLASTIC STRIPING 30 DAYS LATER.

23. CONTRACTOR IS RESPONSIBLE FOR MAINTAINING PROPER BENCHMARKS ON-SITE. EXISTING BENCH MARKS SCHEDULED FOR REMOVAL SHALL BE RELOCATED AT CONTRACTORS EXPENSE AND RE-ESTABLISHED BY A LICENSED SURVEYOR.

24. ALL HANDICAP RAMPS SHALL COMPLY WITH THE FLORIDA ACCESSIBILITY CODE AND AMERICANS WITH DISABILITIES ACT.

25. A PRE-CONSTRUCTION CONFERENCE SHALL BE REQUIRED. THE CONTRACTOR, ENGINEER OF RECORD, AND THE OWNER SHALL MEET WITH THE CITY OF ALACHUA PRIOR TO INITIATION OF THE SITE CONSTRUCTION.

26. ANY CHANGE ORDER REQUESTS, SITE REVISIONS, AND PAY REQUESTS MUST BE SUBMITTED TO AND APPROVED BY THE ENGINEER OF RECORD.

27. CONTRACTOR IS RESPONSIBLE FOR ALL DEWATERING AS NEEDED THROUGHOUT ALL CONSTRUCTION ACTIVITIES COVERED BY THESE PLANS. DEWATERING SHALL BE DONE IN ACCORDANCE WITH FDOT STANDARD SPECIFICATIONS, JANUARY 2021 EDITION, SECTION 455-28.

28. THE CONTRACTOR IS RESPONSIBLE FOR THE PERFORMANCE AND COST OF ALL CLEARING AND GRUBBING AND ALL WORK OF REMOVAL, DISPOSAL, AND REPAIR OR REPLACEMENT OF EXISTING IMPROVEMENTS WHERE SHOWN IN THE PLANS, OR ORDERED BY THE ENGINEER TO BE REMOVED, OR WHERE REQUIRED BECAUSE OF THE CONSTRUCTION OPERATIONS, IN ORDER TO CONSTRUCT THE PROPOSED IMPROVEMENTS (THIS INCLUDES BUT IS NOT LIMITED TO PROPOSED PIPING, STRUCTURES, UTILITIES, PAVING, CURBING, ETC.).

29. AN AS-BUILT SURVEY MAY BE REQUIRED BY REGULATORY AGENCIES. CONTRACTOR TO COORDINATE WITH PROJECT OWNER FOR COMPLETION OF AS-BUILT SURVEYS PRIOR TO PROJECT / PERMIT CLOSE-OUT.

MAINTENANCE OF TRAFFIC (MOT) NOTES

1. *THE CONTRACTOR IS RESPONSIBLE FOR CREATING A MAINTENANCE OF TRAFFIC (MOT) PLAN FOR CONSTRUCTION ACTIVITY THAT OCCURS WITHIN THE PUBLIC RIGHT-OF-WAY, INCLUDING BUT NOT LIMITED TO SIDEWALK WORK AND ACTIVITIES THAT REQUIRE A LANE (OR ROAD) CLOSURE, SUCH AS CONNECTION TO SEWER MANHOLES AND WATER MAINS. THE MOT PLAN MUST BE CREATED BY A REGISTERED PROFESSIONAL ENGINEER WHO IS CERTIFIED TO DO SO BY THE FDOT MOT CERTIFICATION TRAINING. THE MOT PLAN MUST ALSO BE IN ACCORDANCE WITH FDOT STANDARDS PLANS AND FDOT STANDARD SPECIFICATIONS REQUIREMENTS AND MUST BE REVIEWED AND APPROVED BY THE FDOT.*
2. *THE CONTRACTOR SHALL SUBMIT THE MOT TO THE APPROPRIATE REGULATORY AUTHORITY PRIOR TO WORK REQUIRING THE MOT FOR APPROVAL. NO WORK IN THE ROW SHALL OCCUR UNTIL THE MOT IS APPROVED.*

DEMOLITION GENERAL NOTES

1. THE CONTRACTOR SHALL BE RESPONSIBLE TO DISPOSE OF ALL DEMOLITION MATERIALS IN A SAFE AND LAWFUL MANNER. THE CONTRACTOR SHALL SALVAGE TO THE OWNER ANY ITEM AS DETERMINED BY THE OWNER. ONCE DEMOLISHED, MATERIAL SHALL BE DISPOSED OF PROPERLY AND IMMEDIATELY.

2. REMOVE ALL IMPROVEMENTS DEFINED ON THE DEMOLITION PLAN. SALVAGE ITEMS TO OWNER AS DEFINED BY THE OWNER'S REPRESENTATIVE AND CONSTRUCTION DOCUMENT SPECIFICATIONS.

3. EXISTING PAVEMENT AND SIDEWALK EDGES THAT BORDER NEW CONSTRUCTION OR DEMOLITION ARE TO BE SAW-CUT TO PROVIDE A SMOOTH TRANSITION.

4. ALL EXISTING TREES ARE TO REMAIN UNLESS OTHERWISE NOTED.

5. ROOTS LARGER THAN 1 INCH IN DIAMETER ON TREES TO BE PRESERVED THAT ARE ENCOUNTERED DURING CONSTRUCTION MUST BE CUT CLEANLY AND COVERED OVER WITH SOIL BY THE END OF THE WORKING DAY.

6. ALL ASPHALT AND LIMEROCK WILL BE COMPLETELY REMOVED FROM AREAS THAT WILL BE LANDSCAPED. IN PARTICULAR, AREAS WHERE ASPHALT WILL BE REMOVED MUST HAVE THE TOP HARD SURFACE, LIMEROCK, AND COMPACTED SOIL REMOVED. REPLACEMENT SOIL SHALL BE CLEAN DEEP FILL OF PH 5.5 - 6.5. THE DEPTH OF UNCOMPACTED SOIL PRIOR TO PLANTING MUST BE AT LEAST 3 FEET TO ACCOMMODATE FUTURE TREE ROOT GROWTH. NO LIMEROCK, LARGE STONES, OR OTHER CONSTRUCTION DEBRIS CAN REMAIN IN AREAS TO BE LANDSCAPED.

PAVING, GRADING, AND DRAINAGE GENERAL NOTES

1. THE CONTRACTOR IS RESPONSIBLE FOR EROSION/SEDIMENTATION CONTROL PRACTICES DURING CONSTRUCTION TO MINIMIZE ON-SITE EROSION/SEDIMENTATION AND TO PROTECT AGAINST DAMAGE TO OFF SITE PROPERTY. THE FOLLOWING PRACTICES SHALL BE EMPLOYED:

A. EROSION AND SEDIMENTATION CONTROL SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. AREAS OF OFF-SITE DISCHARGE DURING CONSTRUCTION SHALL BE PROTECTED WITH A SEDIMENT BARRIER PER FLORIDA STORMWATER EROSION AND SEDIMENTATION CONTROL INSPECTOR'S MANUAL TO PREVENT OFF-SITE DISCHARGE OF SEDIMENTS. A SILT BARRIER SHALL SPECIFICALLY BE REQUIRED, CONSTRUCTED, AND MAINTAINED AS INDICATED ON THIS SHEET. TEMPORARY SEED AND MULCH SHOULD BE USED TO CONTROL ON-SITE EROSION WHEN IT IS NOT PRACTICAL TO ESTABLISH PERMANENT VEGETATION. SOD SHALL BE PLACED AS EARLY AS POSSIBLE ON ALL SLOPES STEEPER THAN 5 (FT) HORIZONTAL TO 1 (FT) VERTICAL. SOD SHALL BE PINNED AS REQUIRED. ALL EROSION AND SEDIMENTATION CONTROL MEASURES SHALL BE MAINTAINED IN WORKING ORDER THROUGHOUT THE CONSTRUCTION PHASE. THE CONTRACTOR SHALL INSPECT AND REPAIR AS NECESSARY THE EROSION/SEDIMENTATION PROTECTION AT THE END OF EACH WORKING DAY.

NOTE: EROSION/SEDIMENTATION CONTROL SHALL BE PLACED PRIOR TO SITE EXCAVATION AND SHALL REMAIN IN PLACE UNTIL SITE VEGETATION AND LANDSCAPING IS COMPLETE.

B. ALL INLET STRUCTURES AND PIPE SHALL BE PROTECTED FROM SILTATION BY CONSTRUCTING INLET PROTECTION AS DEFINED BY THESE PLANS OR IN THE FDOT STANDARDS. IF SILTATION OCCURS, THE CONTRACTOR IS RESPONSIBLE TO REMOVE SILTATION AS PART OF THE BASE CONTRACT AT NO ADDITIONAL COST TO THE OWNER.

C. EXCAVATED STORMWATER FACILITIES SHALL BE CONSTRUCTED AS PART OF THE INITIAL CONSTRUCTION. THE FACILITIES SHALL BE ROUGH GRADED TO THE DESIGN ELEVATIONS. AFTER THE CONTRIBUTING DRAINAGE AREA IS STABILIZED. THE FACILITIES BOTTOM SHALL BE OVER-EXCAVATED BY SIX INCHES, SCARIFIED, BACKFILLED WITH ARCHER FILL (HAVING NO MORE THAN 5% PASSING NO. 200 SIEVE), AND GRADED TO FINAL DESIGN GRADES. EXCESS AND UNSUITABLE SOILS SHALL BE REMOVED FROM THE BASIN (REMOVE ALL ACCUMULATED SILTS, CLAYS, ORGANIC, AND DEBRIS). FINALLY, SCARIFY AND RAKE BOTTOM AND VEGETATE.

D. PERMANENT VEGETATIVE STABILIZATION SHALL BE APPLIED ON FINE GRADED SITES AS SOON AS PRACTICAL. TEMPORARY SEEDING SHOULD BE EMPLOYED TO PREVENT EXPOSURE OF BARREN SOILS UNTIL PERMANENT VEGETATION CAN BE APPLIED.

E. ALL SLOPES 1:3 OR STEEPER REQUIRE LAPPED OR PEGGED SOD.

F. EROSION, SEDIMENT AND TURBIDITY CONTROL ARE THE RESPONSIBILITY OF THE CONTRACTOR. THESE DELINEATED MEASURES ARE THE MINIMUM REQUIRED, WITH ADDITIONAL CONTROLS TO BE UTILIZED AS NEEDED, DEPENDENT UPON ACTUAL SITE CONDITIONS AND CONSTRUCTION OPERATION.

G. ALL SYNTHETIC BALES, SILT FENCE, AND OTHER EROSION CONTROL MEASURES SHALL BE REMOVED AT THE COMPLETION OF THE PROJECT.

2. THE CONTRACTOR SHALL MAINTAIN IN HIS POSSESSION A COPY OF THE WATER MANAGEMENT DISTRICT CONSTRUCTION PERMIT. HE SHALL BE RESPONSIBLE FOR ADHERENCE TO ALL CONDITIONS CONTAINED IN THE PERMIT.

3. PROPOSED SPOT ELEVATIONS REPRESENT FINISHED PAVEMENT OR GROUND SURFACE GRADE UNLESS OTHERWISE NOTED ON DRAWINGS.

4. CONTRACTOR SHALL SUBMIT FOR REVIEW TO THE OWNER AND OWNER'S ENGINEER SHOP DRAWINGS ON ALL PRECAST AND MANUFACTURED ITEMS TO BE USED ON THIS SITE. FAILURE TO OBTAIN APPROVAL BEFORE INSTALLATION MAY RESULT IN REMOVAL AND REPLACEMENT AT CONTRACTOR'S EXPENSE. ENGINEER'S APPROVAL OF A SHOP DRAWING DOES NOT RELIEVE THE CONTRACTOR'S RESPONSIBILITY FOR THE PERFORMANCE OF THE ITEM.

5. THE COST OF ALL TESTING OF COMPACTION AND OTHER REQUIRED TESTS SHALL BE PAID BY THE CONTRACTOR AND MADE AVAILABLE TO THE ENGINEER OF RECORD DURING SITE INSPECTIONS.

6. GENERAL CONTRACTOR TO CONTACT ENGINEER OF RECORD AND THE OWNER REPRESENTATIVE 48 HOURS IN ADVANCE PRIOR TO BACKFILLING TRENCHES FOR FIELD INSPECTION AND PRIOR TO LAYING ASPHALT FOR FIELD INSPECTION.

7. CONTRACTOR IS TO SUBMIT FDOT APPROVED ASPHALT DESIGN MIXES TO THE OWNER'S REPRESENTATIVE AND ENGINEER OF RECORD BEFORE ANY WORK IS TO COMMENCE ON PROJECT. THE MIXTURE AT THE PLANT OR ON THE ROAD SHALL NOT EXCEED 335 DEGREES. THE CONTRACTOR SHALL NOTIFY THE OWNER'S REPRESENTATIVE AND PROVIDE TEMPERATURE READINGS PRIOR TO LAYING ASPHALT.

8. AS DETERMINED NECESSARY AND DIRECTED BY ALACHUA COUNTY PUBLIC WORKS DEPARTMENT OR ENGINEER OF RECORD, THE CONTRACTOR SHALL UNDERCUT ALL UNSUITABLE MATERIAL 24 INCHES BELOW THE BOTTOM OF ANY PROPOSED LIMEROCK BASE, AND SHALL BACKFILL WITH FILL MATERIAL MEETING FDOT STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION. SEE FDOT INDEX 120-001 AND 120-002.

9. PROVIDE LEVEL PLATFORM IN FRONT OF ALL EGRESS DOORS. THE FLOOR SURFACE ON BOTH SIDES OF A DOOR SHALL BE AT THE SAME ELEVATION. THE FLOOR SURFACE OR LANDING ON EACH SIDE OF THE DOOR SHALL EXTEND FROM THE DOOR IN THE CLOSED POSITION A DISTANCE EQUAL TO THE DOOR WIDTH AND SHALL COMPLY WITH SECTION 4.13.6 MANEUVERING CLEARANCES AT DOORS OF THE FLORIDA ACCESSIBILITY CODE FOR BUILDING CONSTRUCTION.

10. RAMPS SHALL HAVE LEVEL LANDINGS AT THE BOTTOM AND TOP OF EACH RAMP RUN. CURB RAMPS ARE NOT REQUIRED TO HAVE LANDINGS. LANDINGS SHALL HAVE THE FOLLOWING FEATURES:

A. THE LANDING SHALL BE AT LEAST AS WIDE AS THE RAMP RUN LEADING TO IT.

B. ALL LANDINGS ON RAMPS SHALL BE NOT LESS THAN 60" CLEAR, AND THE BOTTOM OF EACH RAMP SHALL HAVE NOT LESS THAN 72" OF STRAIGHT AND LEVEL CLEARANCE.

C. IF RAMPS CHANGE DIRECTION AT LANDINGS, THE MINIMUM LANDING SIZE SHALL BE 60"X60". IF A RAMP RUN HAS A RISE GREATER THAN 6" OR A HORIZONTAL PROJECTION GREATER THAN 72" THEN IT SHALL HAVE HANDRAILS ON BOTH SIDES. HANDRAILS ARE NOT REQUIRED ON CURB RAMPS. HANDRAILS SHALL BE SHOWN ON THE SITE PLAN.

11. THE CONTRACTOR SHALL STOCKPILE TOPSOIL AND CONSTRUCTION MATERIALS IN AREAS DESIGNATED BY THE OWNER.

12. THE CONTRACTOR IS RESPONSIBLE FOR PROVIDING RECORD DRAWINGS AS NOTED IN NOTE #29 UNDER SITE GENERAL NOTES.

13. ALL CONCRETE USED SHALL BE 2,500 PSI MINIMUM.

14. ALL WELLS, CLEANOUTS, MANHOLE TOPS, PULL BOX COVERS AND OTHER UTILITY APPURTENANCES IN THE AREA OF REDEVELOPMENT SHALL BE PROTECTED AND TOPS ADJUSTED TO MATCH PROPOSED GRADES.

15. CONTRACTOR SHALL SAW CUT, TACK, AND MATCH EXISTING PAVEMENT AT LOCATIONS WHERE NEW PAVEMENT MEETS ANY EXISTING PAVEMENT.

16. SOD SHALL BE PLACED AROUND ALL STRUCTURES AS DIRECTED BY THE FDOT INDEX 524-001 AND FDOT INDEX 425- AND 430- SERIES AS APPROPRIATE. ALL OTHER DISTURBED AREAS SHALL BE SEEDED AND MULCHED.

17. ALL STORM SEWER CURB AND DITCH BOTTOM INLETS SHALL CONFORM TO THE APPLICABLE FDOT INDEX. ALL DRAINAGE STRUCTURES WITH GRATES THAT ARE LOCATED IN GRASSED AREAS SHALL HAVE THE GRATE CHAINED TO THE STRUCTURE USING AN EYE BOLT AND CHAIN.

18. ALL CONCRETE STRUCTURES SHALL HAVE ALL EXPOSED EDGES CHAMFERED 3/4" AND CLASS I SURFACE FINISH.

19. ALL HDPE FITTINGS AND CONNECTORS SHALL BE WATER TIGHT. SEE SPECIFICATIONS FOR MORE INFORMATION.

20. COMPACTION OF ALL MATERIALS SHALL BE LIMITED TO STATIC MODE ONLY, UNLESS DIRECTED OTHERWISE BY THE ENGINEER OF RECORD.

21. ALL RCP PIPE JOINTS SHALL BE WRAPPED WITH FILTER FABRIC IN ACCORDANCE WITH FDOT STANDARD SPECIFICATION SECTION 430.

WATER AND WASTEWATER GENERAL NOTES

1. MATERIALS AND CONSTRUCTION METHODS FOR WATER AND WASTEWATER SYSTEMS SHALL BE IN ACCORDANCE WITH THE LOCAL REGULATORY AGENCY CODES, PLANS, AND SPECIFICATIONS FOR CONSTRUCTION, LATEST REVISION THEREOF AND SUPPLEMENTAL SPECIFICATIONS THERETO. APPROVAL AND CONSTRUCTION OF ALL UTILITY EXTENSIONS AND CONNECTIONS MUST BE COORDINATED THROUGH THE REGULATORY AGENCY DEPARTMENT FOR PUBLIC UTILITIES. MATERIALS AND CONSTRUCTION METHODS SHALL COMPLY WITH THE CITY OF ALACHUA DEPARTMENT OF PUBLIC SERVICE DESIGN AND CONSTRUCTION MANUAL, LATEST EDITION.

2. IT IS THE CONTRACTOR'S RESPONSIBILITY TO NOTIFY ALL UTILITY COMPANIES TO DISCONNECT OR REMOVE THEIR FACILITIES PRIOR TO REMOVING OR DEMOLISHING ANY EXISTING STRUCTURES FROM THE SITE.

3. THE CONTRACTOR IS RESPONSIBLE FOR ANY NECESSARY UTILITY FIELD LOCATION AND RELOCATION AS REQUIRED.

4. THE COST OF ALL TESTING OF COMPACTION AND OTHER REQUIRED TESTS SHALL BE PAID BY THE CONTRACTOR AND MADE AVAILABLE TO THE ENGINEER OF RECORD DURING SITE INSPECTIONS.

5. THE CONTRACTOR SHALL PERFORM AN INFILTRATION/EXFILTRATION TEST ON GRAVITY SEWERS IN ACCORDANCE WITH THE REGULATORY JURISDICTION. SAID TESTS ARE TO BE CERTIFIED BY THE ENGINEER AND SUBMITTED TO THE REGULATORY AGENCY FOR APPROVAL. COORDINATION AND NOTIFICATION OF PARTIES US THE CONTRACTOR'S RESPONSIBILITY.

6. ALL FORCE MAINS SHALL BE SUBJECT TO A HYDROSTATIC PRESSURE TEST IN ACCORDANCE WITH THE REGULATORY AGENCY HAVING JURISDICTION. SAID TESTS ARE TO BE CERTIFIED BY THE ENGINEER AND SUBMITTED TO THE REGULATORY AGENCY FOR APPROVAL. COORDINATION AND NOTIFICATION OF PARTIES IS THE CONTRACTOR'S RESPONSIBILITY.

7. CONTRACTOR SHALL SUBMIT FOR REVIEW TO THE OWNER AND OWNER'S ENGINEER SHOP DRAWINGS ON ALL PRECAST AND MANUFACTURED ITEMS TO BE USED ON THIS SITE. FAILURE TO OBTAIN APPROVAL BEFORE INSTALLATION MAY RESULT IN REMOVAL AND REPLACEMENT AT CONTRACTOR'S EXPENSE. ENGINEER'S APPROVAL OF A SHOP DRAWING DOES NOT RELIEVE THE CONTRACTOR'S RESPONSIBILITY FOR THE PERFORMANCE OF THE ITEM.

8. A HORIZONTAL SEPARATION OF TEN FEET PREFERRED, BUT NO LESS THAN SIX FEET, SHALL BE MAINTAINED BETWEEN POTABLE WATER MAINS AND GRAVITY OR PRESSURE WASTEWATER MAINS. WASTEWATER FORCE MAINS, AND RECLAIMED WATER MAINS NOT REGULATED UNDER PART III OF CHAPTER 62-610, F.A.C. A HORIZONTAL SEPARATION OF TEN FEET PREFERRED, BUT NO LESS THAN THREE FEET, SHALL BE MAINTAINED BETWEEN POTABLE WATER MAINS AND VACUUM WASTEWATER MAINS. A HORIZONTAL SEPARATION OF THREE FEET SHALL BE MAINTAINED BETWEEN POTABLE WATER MAINS AND STORM SEWERS, STORMWATER FORCE MAINS, AND RECLAIMED WATER MAINS REGULATED UNDER PART III OF CHAPTER 62-610, F.A.C.

9. WHEN POTABLE WATER MAINS CROSS OTHER PIPES, THE TWO PIPES SHALL HAVE JOINTS A MINIMUM OF SIX FEET FROM THE CROSSING. WHEN POTABLE WATER MAINS CROSS UNDERNEATH OTHER PIPES, THE MINIMUM VERTICAL SEPARATION IS TWELVE INCHES. WHEN POTABLE WATER MAINS CROSS ABOVE PRESSURE WASTEWATER MAINS, WASTEWATER FORCE MAINS, AND RECLAIMED WATER MAINS, THE MINIMUM VERTICAL SEPARATION IS TWELVE INCHES. WHEN POTABLE WATER MAINS CROSS ABOVE GRAVITY AND VACUUM WASTEWATER MAINS, STORM SEWERS, AND STORMWATER FORCE MAINS, THE PREFERRED VERTICAL SEPARATION IS TWELVE INCHES AND THE THE MINIMUM VERTICAL SEPARATION IS SIX INCHES.

10. ALL WATER MAINS SHALL HAVE A MINIMUM OF 36 INCHES OF COVER.

11. RESTRAINED JOINTS SHALL BE PROVIDED AT ALL FITTINGS AND HYDRANTS IN ACCORDANCE WITH AWWA STANDARDS.

12. ALL PVC SERVICE LINES SHALL BE SUITABLE FOR POTABLE WATER SERVICE AND RATED NSF 61.

13. THE SITE WORK CONTRACTOR SHALL ENGAGE THE SERVICES OF A LICENSED UNDERGROUND UTILITY AND EXCAVATION CONSTRACTOR TO INSTALL THE NEW WATER SERVICE LINE.

14. ALL SANITARY SEWER SERVICE LATERALS SHALL BE 4" PVC SDR 35 OR 6" PVC SDR 35 WITH A CLEAN-OUT LOCATED PER THE PLANS. MINIMUM SLOPE FOR 4" LATERALS SHALL BE 1.0% AND A MINIMUM CLEANOUT SPACING OF 75 FEET ON-CENTER AND MINIMUM SLOPE FOR 6" LATERALS SHALL BE 0.6% AND A MINIMUM CLEANOUT SPACING OF 100 FEET ON-CENTER.

15. PUBLIC UTILITY EASEMENTS WILL BE PROVIDED AS REQUIRED FOR ALL UTILITIES SHOWN HEREON BY METES AND BOUND DESCRIPTION AND IN ACCORDANCE WITH THE REGULATORY AGENCY DEPARTMENT FOR PUBLIC UTILITIES.

16. ALL UNDERGROUND FIRE LINE PIPING SHALL BE PVC (C900), AND ALL ABOVE GRADE FIRE LINE PIPING SHALL BE DIP.

17. AS A MINIMUM, PROVIDE TRACER WIRE FROM THE WET TAP TO THE PADS FOR THE FIRE LINE AND POTABLE WATER METER.

FDOT GENERAL NOTES

1. ALL WORK PERFORMED WITHIN THE FLORIDA DEPARTMENT OF TRANSPORTATION RIGHT-OF-WAY SHALL CONFORM TO THE FOLLOWING:

- *A. STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION (JULY 2020).*
- *B. FDOT STANDARD PLANS (FY 2020-21 ROAD CONSTRUCTION)*
- *C. FDOT DESIGN MANUAL (2020)*
- *D. FDOT FLEXIBLE PAVEMENT DESIGN MANUAL FOR NEW CONSTRUCTION AND PAVEMENT REHABILITATION*

SHOULD A CONFLICT ARISE BETWEEN THE DETAILS SHOWN IN THE PLANS AND THE DEPARTMENT OF TRANSPORTATION STANDARDS THE ENGINEER/ PERMITTEE SHALL IMMEDIATELY CONFER WITH THE DEPARTMENT'S ENGINEER IN ORDER TO RESOLVE THE DISCREPANCY. IN NO CASE WILL ANYTHING LESS THAT THE DEPARTMENT'S MINIMUM STANDARD BE ALLOWED.

2. ALL TRAFFIC STRIPING AND MARKINGS ARE TO BE LEAD-FREE, NON-SOLVENT BASED THERMOPLASTIC.

3. REMOVAL OF EXISTING STRIPING SHALL BE ACCOMPLISHED USING THE HYDRO-BLAST METHOD.

4. ALL CURB AND GUTTER AND SIDEWALK WILL BE REMOVED AND REPLACED JOINT TO JOINT.

5. ALL DISTURBED AREA WITH THE DEPARTMENT OF TRANSPORTATION RIGHT OF WAY WILL RESTORED TO ORIGINAL OR BETTER CONDITION BY GRADING AND SODDING THE AREA DISTURBED (BERMUDA IN RURAL, CENTIPEDE IN UTILITY STRIPS).

11801 Research Drive
Alachua, Florida 32615
(386) 418-1976
www.chw-inc.com
est. 1988 FLORIDA CA-9075

CHW Professional Consultants

SCALE: N/A

VERIFY SCALE
BAR IS ONE INCH ON
ORIGINAL DRAWING.
IF NOT ONE INCH ON
THIS SHEET ADJUST
SCALES ACCORDINGLY.
0 1"

CONSTRUCTION/BID REVISIONS:

SUBMITTALS:
1/28/21 COA, SRWMD, AND FDOT
3/8/21 COA, SRWMD
3/18/21 FDOT
3/25/21 COA, SRWMD

CLIENT: DE FOODS, LLC.
PROJECT: KFC ALACHUA, FL
SHEET TITLE: GENERAL NOTES

TECHNICIAN: T.F. COWART
DESIGNER: C. MCKENNETT, E.I.
QUALITY CONTROL: D.H. YOUNG, P.E.
PROJECT NUMBER: 20-0113

DANIEL H. YOUNG
FL PE No. 70780

SHEET NO: C0.10

Tech: cowart Plot Date: Mar 24,2021 8:24pm Filename: N:\2020\20-0113\Department\04_Engineering\02_PWD\Production Files\20-0113 GT.dwg

ABBREVIATIONS

	SYMBOLS
'	*FEET (WHEN USED WITH LENGTHS)*
°	*DEGREES*
'	*MINUTES (WHEN USED WITH ANGLES)*
"	*SECONDS*
%	*PERCENT*
@	*AT*
	A
AASHTO	*ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS*
AC	*ACRES*
ADA	*AMERICAN WITH DISABILITIES ACT*
ANSI	*AMERICAN NATIONAL STANDARDS INSTITUTE*
ARCH	*ARCHITECT*
ARV	*AIR RELEASE VALVE*
ASTM	*AMERICAN SOCIETY FOR TESTING AND MATERIALS*
AWWA	*AMERICAN WATER WORKS ASSOCIATION*
	B
BC	*BACK OF CURB*
BFP	*BACKFLOW PREVENTER*
BLDG	*BUILDING*
BM	*BENCHMARK*
BMP	*BEST MANAGEMENT PRACTICE*
BOC	*BACK OF CURB*
BVCS	*BEGIN VERTICAL CURVE STATION*
BVCE	*BEGIN VERTICAL CURVE ELEVATION*
BW	*BOTTOM OF WALL*
BSL	*BUILDING SETBACK LINE*
	C
CATV	*CABLE TELEVISION*
CI	*CURB INLET*
CIP	*CAST IRON PIPE*
CMP	*CORRUGATED METAL PIPE*
CO	*CLEANOUT*
CONC	*CONCRETE*
COORD	*COORDINATE*
CR	*COUNTY ROAD*
C/O	*CLEANOUT*
	D
DBH	*DIAMETER AT BREAST HEIGHT*
DDC	*DOUBLE DETECTOR CHECK*
DE	*DRAINAGE EASEMENT*
DEG	*DEGREE*
DIA	*DIAMETER*
DIP	*DUCTILE IRON PIPE*
DWG	*DRAWING*
	E
℄	*RATE OF ELEVATION*
E	*EAST*
EA	*EACH*
EL	*ELEVATION*
ELEV	*ELEVATION*
EOP	*EDGE OF PAVEMENT*
EOR	*ENGINEER OF RECORD*
ERCP	*ELLIPTICAL REINFORCED CONCRETE PIPE*
ESMT	*EASEMENT*
EVCS	*END VERTICAL CURVE STATION*
EVCE	*END VERTICAL CURVE ELEVATION*
EX	*EXISTING*
	F
FAC	*FLORIDA ADMINISTRATIVE CODE*
FBR	*FLORIDA BEARING RATIO*
FC	*FRICTION COURSE*
FDEP	*FLORIDA DEPARTMENT OF ENVIRONMENTAL PROTECTION*
FDOT	*FLORIDA DEPARTMENT OF TRANSPORTATION*
FFE	*FINISHED FLOOR ELEVATION*
FH	*FIRE HYDRANT*
FHWA	*FLORIDA HIGHWAY ADMINISTRATION*
FIG	*FIGURE*
FM	*FORCE MAIN*
FOC	*FACE OF CURB*
FS	*FLORIDA STATUTES*
FT	*FEET*
	G
GALV	*GALVANIZED*
GM	*GAS MAIN*
GV	*GATE VALVE*
	H
HDPE	*HIGH DENSITY POLYETHYLENE*
HP	*HIGH POINT*
	I
ID	*IDENTIFICATION*
INV	*INVERT*
INV EL	*INVERT ELEVATION*
IP	*IRON PIPE*
	K
K	*VERTICAL CURVE RATE OF CHANGE*
	L
L	*LENGTH*
LA	*LANDSCAPE ARCHITECT*
LBR	*LIMEROCK BEARING RATIO*
LDR	*LAND DEVELOPMENT REGULATION*
LF	*LINEAR FEET*
LP	*LOW POINT*
LT	*LEFT*
	M
MAX	*MAXIMUM*
ME	*MATCH EXISTING*
MH	*MANHOLE*
MIN	*MINIMUM*
MISC	*MISCELLANEOUS*
MUTCD	*MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES*
	N
N	*NORTH*
N-E	*NORTHING - EASTING*
N/A	*NOT APPLICABLE*
NAVD	*NORTH AMERICAN VERTICAL DATUM OF 1988*
NGVD	*NATIONAL GEODETIC VERTICAL DATUM OF 1929*
NO	*NUMBER*
NPDES	*NATIONAL POLLUTANT DISCHARGE ELIMINATION SYSTEM*
NTS	*NOT TO SCALE*
	O
OC	*ON CENTER*
OHW	*OVERHEAD WIRE*
ORB	*OFFICIAL RECORDS BOOK*
OSHA	*OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION*
	P
PAVT	*PAVEMENT*
PC	*POINT OF CURVATURE*
PCC	*POINT OF COMPOUND CURVE*
PE	*POLYETHYLENE*
PERF	*PERFORATED*
PROP	*PROPOSED*
PT	*POINT OF TANGENCY*
PVC	*POLYVINYL CHLORIDE*
PVI	*POINT OF VERTICAL INTERSECTION*
	R
R	*RADIUS*
RCP	*REINFORCED CONCRETE PIPE*
RPM	*RAISED REFLECTIVE PAVEMENT MARKER*
RPZ	*REDUCED PRESSURE ZONE*
RT	*RIGHT*
RWM	*RECLAIMED WATER MAIN*
R/W	*RIGHT-OF-WAY*
	S
S	*SOUTH*
SAN	*SANITARY*
SHWE	*SEASONAL HIGH WATER ELEVATION*
SL	*SLOPE*
SP	*SUPERPAVE*
SR	*STATE ROAD*
SS	*SANITARY SEWER*
ST	*STORM*
STA	*STATION*
STD	*STANDARD*
	T
TCE	*TEMPORARY CONSTRUCTION EASEMENT*
TEMP	*TEMPORARY*
TOB	*TOP OF BANK*
TV	*TELEVISION*
TW	*TOP OF WALL*
TYP	*TYPICAL*
	U
USF	*UNITED STATES FOUNDRY*
USGS	*UNITED STATES GEOLOGICAL SURVEY*
UTIL	*UTILITY*
	V
V	*VERTICAL*
VC	*VERTICAL CURVE*
VCP	*VITRIFIED CLAY PIPE*
	W
W	*WEST*
W	*WATER*
W/	*WITH*
WM	*WATER MAIN*
WW	*WASTEWATER*
WWF	*WELDED WIRE FABRIC*

SIGNAGE

SIGNS ARE PER FDOT SPECIFICATIONS OR PER MUTCD. SIGN POSTS AND INSTALLATION SHALL BE PER FDOT INDEX NO. 11860. SIGN PLACEMENT SHALL BE PER FDOT INDEX NO. 17302.

FTP-20-06 (12" X 18") PER FDOT INDEX NO. 17355

R1-1 "STOP" - SEE PLANS FOR SIZE

SITE INFORMATION

EX. PROPERTY LINE

LANDSCAPE BUFFER LINE

BUILDING SETBACK LINE

WETLAND LIMITS LINE

WETLAND SETBACK LINE

CENTER LINE

EASEMENT LINE

RIGHT-OF-WAY LINE

SILT FENCE LINE

TREE BARRICADE LINE

EX. STRUCTURE OR BUILDING

PROPOSED BUILDING

PROPOSED ASPHALTIC PAVEMENT

PROPOSED CONCRETE PAVEMENT

PROPOSED DETECTABLE WARNING SURFACE

DIRECTIONAL TRAFFIC ARROW PER FDOT INDEX NO. 17346

WATERSHED DIVIDE

EX. ELEVATION CONTOUR

PROPOSED CONTOUR

EX. SPOT ELEVATION

PROPOSED SPOT ELEVATION

DIRECTION OF SURFACE DRAINAGE FLOW

PROPOSED SWALE LINE

EX. FENCE

PROPOSED FENCE

EX. TREE (SIZE & TYPE)

EX. TREE (TREE ID)

EX. TREE TO BE REMOVED (SIZE & TYPE)

EX. TREE TO BE REMOVED (TREE ID)

PROJECT BENCHMARK

STORMWATER

THE PROPOSED STORMWATER STRUCTURES DEPICTED BELOW ARE DRAWN PER FDOT SPECIFICATIONS AND TO SCALE WHEN SHOWN ON THE PLAN SHEETS.

EX. GRAVITY STORMWATER MAIN

PROPOSED GRAVITY STORMWATER MAIN (PIPE LENGTHS ARE FROM N-E LOCATION OF A STRUCTURE TO N-E LOCATION OF A STRUCTURE)

EX. STORMWATER MANHOLE

PROPOSED 48" DIA. STORMWATER MANHOLE PER FDOT INDEX. NO. 425-001 AND 425-010

PROPOSED CIRCULAR AREA DRAIN

PROPOSED SQUARE AREA DRAIN

PROPOSED TYPE 1 CURB INLET TOP PER FDOT INDEX NO. 425-020 (SEE PLANS FOR BOTTOM SPECIFICATION)

PROPOSED TYPE 2 CURB INLET TOP PER FDOT INDEX NO. 425-020 (SEE PLANS FOR BOTTOM SPECIFICATION)

PROPOSED TYPE 3 CURB INLET TOP PER FDOT INDEX NO. 425-020 (SEE PLANS FOR BOTTOM SPECIFICATION)

PROPOSED TYPE 4 CURB INLET TOP PER FDOT INDEX NO. 425-020 (SEE PLANS FOR BOTTOM SPECIFICATION)

PROPOSED TYPE 5 CURB INLET TOP PER FDOT INDEX NO. 425-021 (SEE PLANS FOR BOTTOM SPECIFICATION)

PROPOSED TYPE 6 CURB INLET TOP PER FDOT INDEX NO. 425-021 (SEE PLANS FOR BOTTOM SPECIFICATION)

PROPOSED TYPE 9 CURB INLET TOP PER FDOT INDEX NO. 425-024(SEE PLANS FOR BOTTOM SPECIFICATION)

PROPOSED TYPE 'C' DITCH BOTTOM INLET TOP PER FDOT INDEX NO. 425-052 (SEE PLANS FOR GRATE MATERIAL AND BOTTOM SPECIFICATION)

PROPOSED TYPE 'D' DITCH BOTTOM INLET TOP PER FDOT INDEX NO. 425-052 (SEE PLANS FOR GRATE MATERIAL AND BOTTOM SPECIFICATION)

PROPOSED TYPE 'E' DITCH BOTTOM INLET TOP PER FDOT INDEX NO. 425-052 (SEE PLANS FOR GRATE MATERIAL AND BOTTOM SPECIFICATION)

PROPOSED TYPE 'F' DITCH BOTTOM INLET TOP WITH STEEL GRATE PER FDOT INDEX NO. 425-053 (SEE PLANS FOR BOTTOM SPECIFICATION)

PROPOSED TYPE 'G' DITCH BOTTOM INLET TOP WITH STEEL GRATE PER FDOT INDEX NO. 425-053 (SEE PLANS FOR BOTTOM SPECIFICATION)

PROPOSED TYPE 'H' DITCH BOTTOM INLET TOP PER FDOT INDEX NO. 425-052 (SEE PLANS FOR GRATE MATERIAL AND BOTTOM SPECIFICATION)

PROPOSED TYPE 'J' DITCH BOTTOM INLET TOP WITH STEEL GRATE PER FDOT INDEX NO. 425-054 (SEE PLANS FOR BOTTOM SPECIFICATION)

PROPOSED U-TYPE CONCRETE ENDWALLS WITH GRATES PER FDOT INDEX NO. 430-010 (SEE PLANS FOR SIZE)

PROPOSED FLARED END SECTION PER FDOT INDEX NO. 430-020 (SEE PLANS FOR SIZE)

PROPOSED CROSS DRAIN MITERED END SECTION PER FDOT INDEX NO. 430-021 (SEE PLANS FOR SIZE)

PROPOSED SIDE DRAIN MITERED END SECTION PER FDOT INDEX NO. 430-022 (SEE PLANS FOR SIZE)

PROPOSED STORMWATER STRUCTURE ID TAG

POTABLE AND RECLAIMED WATER

EX. POTABLE WATER MAIN

PROPOSED POTABLE WATER MAIN

EX. RECLAIMED WATER MAIN

PROPOSED RECLAIMED WATER MAIN

11.25° BEND W/ MECHANICALLY RESTRAINED JOINTS (POTABLE AND RCW)

22.5° BEND W/ MECHANICALLY RESTRAINED JOINTS (POTABLE AND RCW)

45° BEND W/ MECHANICALLY RESTRAINED JOINTS (POTABLE AND RCW)

90° BEND W/ MECHANICALLY RESTRAINED JOINTS (POTABLE AND RCW)

TEE (POTABLE AND RCW)

CROSS (POTABLE AND RCW)

BLOWOFF ASSEMBLY (POTABLE AND RCW)

REDUCER (POTABLE AND RCW)

EX. GATE VALVE AND BOX (POTABLE AND RCW)

PROPOSED GATE VALVE AND BOX (POTABLE AND RCW)

EX. AIR RELEASE VALVE (POTABLE AND RCW)

PROPOSED AIR RELEASE VALVE (POTABLE AND RCW)

EX. FIRE HYDRANT ASSEMBLY

PROPOSED FIRE HYDRANT ASSEMBLY

PROPOSED SAMPLE POINT

EX. WATER METER (POTABLE AND RCW)

PROPOSED POTABLE WATER METER

PROPOSED POTABLE WATER BACK FLOW PREVENTER

PROPOSED RECLAIMED WATER METER

EX. WATER WELL

EX. HOSE BIB (POTABLE AND RECLAIMED)

PROPOSED HOSE BIB (POTABLE AND RECLAIMED)

PROPOSED FITTING ID TAG (POTABLE AND RECLAIMED)

WASTEWATER

EX. GRAVITY WASTEWATER MAIN

PROPOSED GRAVITY WASTEWATER MAIN (PIPE LENGTHS ARE FROM N-E LOCATION OF A STRUCTURE TO N-E LOCATION OF A STRUCTURE)

EX. WASTEWATER FORCE MAIN

PROPOSED WASTEWATER FORCE MAIN

EX. WASTEWATER MANHOLE

PROPOSED WASTEWATER MANHOLE

EX. WASTEWATER CLEANOUT

PROPOSED WASTEWATER CLEANOUT

PROPOSED WASTEWATER GREASE TRAP

PROPOSED WASTEWATER MANHOLE ID

11.25° BEND W/ MECHANICALLY RESTRAINED JOINTS (WW FORCE MAIN)

22.5° BEND W/ MECHANICALLY RESTRAINED JOINTS (WW FORCE MAIN)

45° BEND W/ MECHANICALLY RESTRAINED JOINTS (WW FORCE MAIN)

90° BEND W/ MECHANICALLY RESTRAINED JOINTS (WW FORCE MAIN)

WYE W/ MECHANICALLY RESTRAINED JOINTS (WW FORCE MAIN)

EX. PLUG VALVE AND BOX (WW FORCE MAIN)

PROPOSED PLUG VALVE AND BOX (WW FORCE MAIN)

EX. AIR RELEASE VALVE (WW FORCE MAIN)

PROPOSED AIR RELEASE VALVE (WW FORCE MAIN)

MISCELLANEOUS UTILITIES

THE PROPOSED UTILITIES BELOW ARE DESIGN BY OTHERS AND ARE DEPICTED FOR COORDINATION PURPOSES ONLY. REFER TO PLANS BY OTHERS FOR EXACT LOCATIONS, DIMENSION, AND DETAILS.

PROPOSED AT&T LINE

EX. BURIED CABLE LINE

PROPOSED BURIED CABLE LINE

EX. BURIED TELEPHONE LINE

PROPOSED TELEPHONE LINE

EX. CABLE TELEVISION LINE

PROPOSED CABLE/TELEVISION LINE

EX. FIBER OPTIC LINE

EX. UNDERGROUND TELEPHONE LINE

EX. TELEPHONE PEDESTAL

EX. TELEVISION/CABLE PEDESTAL

EX. CHILLED WATER MAIN

PROPOSED CHILLED WATER MAIN

EX. FIRE MAIN

PROPOSED FIRE MAIN

EX. IRRIGATION LINE

PROPOSED IRRIGATION LINE

EX. STEAM LINE

PROPOSED STEAM LINE

PROPOSED CLAY ELECTRIC LINE

EX. ELECTRIC LINE

PROPOSED ELECTRIC LINE

EX. ENERGY LINE

PROPOSED PRIVATE LIGHTING LINE

EX. OVERHEAD WIRE LINE

EX. UNDERGROUND ELECTRIC LINE

EX. LIGHT

EX. UTILITY POLE

EX. UTILITY POLE

EX. WOOD POWER POLE

EX. GUY ANCHOR

PROPOSED TRANSFORMER

EX. GAS LINE

PROPOSED GAS LINE

EX. GAS MARKER

EX. GAS MARKER

NOTES:
1. THIS LEGEND IS ALL INCLUSIVE AND MAY INCLUDE ITEMS NOT A PART OF THIS PLAN SET.

2. SYMBOLS SHOWN ON THIS SHEET ARE FOR ILLUSTRATIVE PURPOSES ONLY. UNLESS NOTED OTHERWISE, SYMBOLS IN THESE PLANS MAY NOT BE REPRESENTATIVE OF SIZE.

11801 Research Drive
Alachua, Florida 32615
(386) 418-1976
www.chw-inc.com
est. 1988 FLORIDA CA-9075
CHW Professional Consultants

SCALE: N/A
VERIFY SCALE
BAR IS ONE INCH ON ORIGINAL DRAWING
IF NOT ONE INCH ON THIS SHEET ADJUST SCALES ACCORDINGLY
0 1"

CONSTRUCTION/BID REVISIONS:

SUBMITTALS:
1/28/21 COA, SRWMD, AND FDOT
3/10/21 COA, SRWMD
3/18/21 FDOT
3/25/21 COA, SRWMD

COUNT: DE FOODS, LLC.
PROJECT: KFC, ALACHUA, FL
SHEET TITLE: LEGEND

TECHNICIAN: T.F. COWART
DESIGNER: C. HENNENNETT, E.I.
QUALITY CONTROL: D.H. YOUNG, P.E.
PROJECT NUMBER: 20-0113

DANIEL H. YOUNG
FL PE No. 70780

SHEET NO: C0.11

Tech: colem Plot Date: Mar 24,2021 5:24pm Filename: W:\2020\20-0113\Departments\04_Engineering\02_DWGS\Production Files\20-0113 CV.dwg

I. INTRODUCTION

THIS DOCUMENT WAS PREPARED IN ORDER TO ILLUSTRATE COMPLIANCE WITH CHAPTER 62-621.300 (4) OF THE FLORIDA ADMINISTRATIVE CODE, WHICH PERTAINS TO THE GENERIC PERMIT FOR STORMWATER DISCHARGE FROM LARGE AND SMALL CONSTRUCTION ACTIVITIES. THE ADMINISTRATIVE CODE GRANTS THE FLORIDA DEPARTMENT OF ENVIRONMENTAL PROTECTION (FDEP) THE AUTHORITY TO REGULATE POINT SOURCE DISCHARGE OF STORMWATER FROM CONSTRUCTION SITES. THIS DOCUMENT ESTABLISHES A STORMWATER POLLUTION PREVENTION PLAN FOR THE SITE USING STANDARD PRACTICE AND BEST MANAGEMENT PRACTICES (BMPs) AND IS ORGANIZED TO CORRESPOND TO 62-621.300(4)(a) GENERIC PERMIT FOR STORMWATER DISCHARGE FROM LARGE AND SMALL CONSTRUCTION ACTIVITIES FDEP FORM 62-621.300(4)(b) IS TO BE SUBMITTED IN CONJUNCTION WITH THIS DOCUMENT. CONTRACTOR(S) MUST FILL IN THE ACTIVITIES SEQUENCE (SECTION II. B. BELOW) AND SIGN THIS SHEET (REFER TO SIGNATURE TABLE THIS SHEET) AND RETAIN THIS SHEET AND ALL FOLLOWING SWPPP SHEETS ON SITE. ALTERNATIVELY, CONTRACTOR(S) MAY PREPARE AND SIGN THEIR OWN SWPP PLAN MEETING THE REQUIREMENTS. IF THE SITE OR CONTRACTOR ACTIVITIES REQUIRE ANY BMPs THAT ARE NOT DESCRIBED ON THIS SHEET, THE CONTRACTOR MUST PREPARE AN ALTERNATIVE OR ADDITIONAL SWPPP DOCUMENT CONTAINING THE NECESSARY BMPs.

II. SITE DESCRIPTION

COUNTY:	*ALACHUA COUNTY, FLORIDA*
SECTION, TOWNSHIP, RANGE:	*SECTION 23, TOWNSHIP 8 SOUTH, RANGE 18 EAST*
COUNTY PARCEL NO.:	*03053-002-001 & PORTION OF 03053-002-000*
STREET ADDRESS:	*SOUTH OF US 441 AND EAST OF NW 167TH BLVD.*
PROJECT AREA:	*1.06 ACRES*
SITE LOCATION MAP:	*SEE COVER SHEET OF CONSTRUCTION DRAWINGS*

A. NATURE OF CONSTRUCTION ACTIVITY

THE PROPOSED DEVELOPMENT IS THE CONSTRUCTION OF A ± 1,904 S.F. QUICK SERVICE RESTAURANT AND ASSOCIATED PAVEMENT FOR DRIVEWAYS AND PARKING, SIDEWALKS, UTILITIES, AND STORMWATER MANAGEMENT FACILITY(S). THE PROJECT SITE IS LOCATED SOUTH OF US 441 AND EAST OF NW 167TH BLVD., ALACHUA FLORIDA. THE PROJECT SITE TOTAL AREA IS APPROXIMATELY 1.00 ACRES.

B. SEQUENCE OF MAJOR SOIL DISTURBING ACTIVITIES - CONTRACTOR MUST FILL IN DAYS

DAYS

FROM_____TO_____ PRIOR TO CONSTRUCTION, SILT FENCING AND TREE PROTECTION FENCING SHALL BE INSTALLED AND ALL EXISTING STORM DRAINAGE SWALE AND INLETS SHALL BE PROTECTED IN ACCORDANCE WITH THE STATE OF FLORIDA EROSION AND SEDIMENT CONTROL DESIGNER AND REVIEWER MANUAL, DATED JULY 2013, AND THESE PLANS.

FROM_____TO_____ THE CONSTRUCTION SERVICE ENTRANCE SHALL BE STABILIZED TO MINIMIZE THE CREATION OF DUST AND OFF-SITE TRACKING OF SEDIMENTS.

FROM_____TO_____ ONLY THE AREA COMPRISING THE PROPOSED STORMWATER MANAGEMENT FACILITY(S) SHALL BE CLEARED AND GRUBBED OF UNWANTED VEGETATION.

FROM_____TO_____ THE PROPOSED STORMWATER MANAGEMENT FACILITY(S) SHALL BE CONSTRUCTED.

FROM_____TO_____ IF SUITABLE, THE EXCAVATED SOIL FROM THE FACILITY(S) MAY BE USED AS FILL FOR ON-SITE GRADING THAT IS DEPICTED IN THESE CONSTRUCTION PLANS. THE CONTRACTOR SHALL DISPOSE OF ALL UNSUITABLE MATERIAL ON-SITE OR OFF-SITE TO A PERMITTED LOCATION.

FROM_____TO_____ THE REMAINING PORTION OF THE SITE THAT IS TREATED BY THE CONSTRUCTED STORMWATER MANAGEMENT FACILITY(S) SHALL BE CLEARED AND GRUBBED.

FROM_____TO_____ THE PERMANENT ROADWAYS/DRIVEWAYS SHALL BE ROUGHLY GRADED.

FROM_____TO_____ THE UNDERGROUND UTILITIES INFRASTRUCTURE AND STORMWATER PIPING SYSTEM SHALL BE INSTALLED. ANY DE-WATERING (PUMPED) SHALL BE DIVERTED TO THE ASSOCIATED STORMWATER MANAGEMENT FACILITY(S).

FROM_____TO_____ THE PERMANENT ROADWAY/DRIVEWAY SUBGRADE SHALL BE COMPACTED, A LIMEROCK BASE SHALL BE ESTABLISHED, AND THEN FOLLOWED BY AN OVERLAY OF ASPHALT.

FROM_____TO_____ UPON SIGNIFICANT COMPLETION OF CONSTRUCTION, THE STORMWATER PIPING SYSTEM SHALL BE FLUSHED OUT TO REMOVE ACCUMULATED DEBRIS AND SEDIMENT.

FROM_____TO_____ UPON COMPLETION OF THE DEBRIS AND SEDIMENT REMOVAL FROM THE STORMWATER PIPING SYSTEM, THE PROPOSED STORMWATER MANAGEMENT FACILITY(S) SHALL BE FINE GRADED AND SHALL BE EXCAVATED A MINIMUM OF SIX INCHES BELOW THE DESIGN BOTTOM ELEVATION AND REPLACED WITH FILL HAVING A MINIMUM PERMEABILITY RATE OF 20 FEET/DAY WITH A MAXIMUM OF 5% SOIL FINES PASSING THE NO. 200 SIEVE. THE BOTTOM SHALL BE SCARIFIED AND STABILIZED ACCORDING TO THESE PLANS. ONCE COMPLETED, NO HEAVY MACHINERY SHALL BE ALLOWED WITHIN THE STORMWATER MANAGEMENT FACILITY(S).

FROM_____TO_____ ALL REMAINING DISTURBED AREAS WITHIN THE CONSTRUCTION AREA SHALL BE COMPLETELY GRASSED AND/OR LANDSCAPED ACCORDING TO THESE PLANS. TURF ESTABLISHMENT SHALL BE PER FDOT STANDARD SPECIFICATIONS SECTION 570. EVIDENCE OF GROWTH MUST BE PRESENT PRIOR TO REMOVAL OF SILT FENCING AND OTHER EROSION CONTROL APPLICATIONS.

C. SITE DEVELOPMENT DATA:

TOTAL ON-SITE PROJECT AREA:	*1.00 ACRES*
TOTAL OFF-SITE PROJECT AREA:	*0.06 ACRES*
TOTAL SITE AREA TO BE DISTURBED:	*1.32 ACRES*
TOTAL IMPERVIOUS AREA (AS SHOWN IN CONSTRUCTION DRAWINGS):	*0.52 ACRES*
TOTAL DETENTION VOLUME:	*0.54 ACRE-FEET*
TOTAL OPEN AREA:	*0.54 ACRES*

D. SOIL CONDITIONS AND STORMWATER QUALITY

THE NRCS DATA FOR THE SITE REVEALS THAT THE SITE SOILS ARE COMPRISED OF KENDRICK SAND AND ARREDONDO-URBAN LAND COMPLEX. ECS FLORIDA, LLC. CONDUCTED A GEOTECHNICAL EXPLORATION OF THE PROPOSED STORMWATER MANAGEMENT FACILITY(S) LOCATION IN OCTOBER OF 2020. THE INVESTIGATION REVEALED THAT THE PROJECT SITE'S SURFACE SOILS HAVE THE FOLLOWING CHARACTERISTICS:

DEPTH TO EFFECTIVE OR MOBILIZED AQUIFER:	*5.0 FEET*
DEPTH OF SEASONAL HIGH WATER TABLE:	*5.0 FEET*
HORIZONTAL HYDRAULIC CONDUCTIVITY:	*18 FEET/DAY*
UNSATURATED VERTICAL INFILTRATION:	*12 FEET/DAY*

DESIGN PERCOLATION RATES FOR THE STORMWATER MANAGEMENT FACILITY(S) WERE DETERMINED BASED ON LABORATORY PERMEABILITY TEST RESULTS FROM BORING SAMPLES TAKEN WITHIN THE LIMITS OF THE STORMWATER MANAGEMENT FACILITY(S).

THE STORMWATER MANAGEMENT FACILITY(S) WAS DESIGNED TO PROVIDE RATE AND VOLUME CONTROL AND WATER QUALITY TREATMENT OF THE STORMWATER RUNOFF RESULTING FROM THE POST-DEVELOPMENT SITE UNDER 100-YEAR CRITICAL STORM EVENT RAINFALL CONDITIONS. THE STORMWATER POLLUTION PREVENTION PLAN (C0.21) DEPICTS THE POST-DEVELOPMENT WATERSHED(S) LIMITS AND THE TABLE BELOW SUMMARIZES EACH WATERSHED.

WATERSHED ID	*POST DEVELOPMENT AREA (ACRES)*	*POST DEVELOPMENT RUNOFF FACTOR (CN)*	*POST DEVELOPMENT IMPERVIOUS AREA (ACRES)*	*STORMWATER MANAGEMENT FACILITY TYPE*	*FACILITY DETENTION CAPACITY (ACRE-FEET)*	*100-YEAR FLOOD ELEVATION (FT)*
1	*0.78*	*0.92*	*0.52*	*DRY RETENTION*	*0.54*	*82.95*

E. SITE MAP

PLEASE SEE THE STORMWATER POLLUTION PREVENTION PLAN (C0.21) FOR DETAILS.

F. STORMWATER OUTFALL LOCATION AND RECEIVING WATER BODY

THE STORMWATER MANAGEMENT FACILITY 1 OUTFALL STRUCTURE IS LOCATED AT THE NORTH PORTION OF THE PROJECT (29°48'08.4" N, 82°31'07.3" W) AND THE ASSOCIATED RECEIVING WATER BODY IS MILL CREEK SINK.

III. CONTROLS TO REDUCE POLLUTION

AS OUTLINED IN THE SUWANNEE RIVER WATER MANAGEMENT DISTRICT (SRWMD) PERMIT, ALL CONSTRUCTION ACTIVITIES SHALL BE CONDUCTED IN A MANNER AS TO NOT VIOLATE STATE WATER QUALITY STANDARDS. PRIOR TO CONSTRUCTION, THE CONTRACTOR SHALL IMPLEMENT AND MAINTAIN ALL EROSION AND SEDIMENT CONTROL MEASURES REQUIRED TO RETAIN SEDIMENT ON-SITE. IF SITE CONDITIONS ARE SUCH THAT ADDITIONAL CONTROL MEASURES ARE REQUIRED OTHER THAN WHAT IS SPECIFIED IN THE EROSION AND SEDIMENTATION CONTROL PLAN, THEN THE CONTRACTOR SHALL IMPLEMENT ADDITIONAL BEST MANAGEMENT PRACTICES. THESE MEASURES MUST BE INSPECTED AND MAINTAINED THROUGHOUT THE CONSTRUCTION PHASE AND UNTIL AS DIRECTED BY THESE PLANS. THE STORMWATER POLLUTION PREVENTION PLAN (C0.21) AND SECTION IV BELOW PROVIDE DETAILS ON THE SPECIFIC CONTROL MEASURES TO REDUCE STORMWATER POLLUTION.

IV. EROSION AND SEDIMENT CONTROLS

A. STABILIZATION PRACTICES

EXISTING TREES AND NATURAL VEGETATION TO REMAIN ON-SITE SHALL BE PROTECTED BY TREE BARRICADE FENCING AS DEPICTED ON THE STORMWATER POLLUTION PREVENTION PLAN (C0.21). TYPE III SILT FENCING SHALL PROTECT ALL DRAINAGE STRUCTURES AND SHALL BUFFER AREAS WITH POTENTIAL TO CONTRIBUTE OFF-SITE RUNOFF AND AS SPECIFICALLY DEPICTED ON THE STORMWATER POLLUTION PREVENTION PLAN (C0.21). STABILIZATION MEASURES SHALL BE INITIATED FOR EROSION AND SEDIMENT CONTROL ON DISTURBED AREAS AS SOON AS PRACTICAL, BUT IN NO CASE MORE THAN 7 DAYS, IN PORTIONS OF THE SITE WHERE CONSTRUCTION HAS TEMPORARILY OR PERMANENTLY CEASED. AS SPECIFIED IN SECTION II.B. ABOVE, UPON COMPLETION OF CONSTRUCTION, ALL STORMWATER MANAGEMENT FACILITIES SHALL BE SCRAPED CLEAN OF ACCUMULATED SEDIMENT AFTER THE COMPLETION OF CONSTRUCTION. ALL TURF ESTABLISHMENT SHALL BE PERFORMED MEETING THE REQUIREMENTS OF SECTION 570 OF THE STANDARD SPECIFICATIONS. EVIDENCE OF GROWTH MUST BE PRESENT PRIOR TO FINAL RELEASE.

B. STRUCTURE PRACTICES

AS DEPICTED IN THE STORMWATER POLLUTION PREVENTION PLAN (C0.21), A STORMWATER MANAGEMENT SYSTEM WILL BE CONSTRUCTED AND WILL BE COMPRISED OF A DRY RETENTION FACILITY AND A STORM PIPE CONVEYANCE SYSTEM. TO PREVENT EROSION DURING CONSTRUCTION, TYPE III SILT FENCING WILL BE INSTALLED IN THE LOCATIONS SHOWN ON THE PLANS. ALL EXISTING AND PROPOSED STORM DRAINS AND DRAINAGE SWALES SHALL BE PROTECTED ACCORDING TO THE STATE OF FLORIDA EROSION AND SEDIMENT CONTROL DESIGNER AND REVIEWER MANUAL, DATED JULY 2013 OR PER DETAILS PROVIDED ON SHEET C0.21 UNTIL CONSTRUCTION IS COMPLETE. THE STORM PIPE CONVEYANCE SYSTEM SHALL BE FLUSHED OUT TO REMOVE ALL ACCUMULATED DEBRIS AND SEDIMENT UPON COMPLETION OF CONSTRUCTION.

C. DRAINAGE LOCATIONS THAT SERVE AREAS WITH LESS THAN 10 DISTURBED ACRES

AS SPECIFIED IN THE "SEQUENCE OF MAJOR SOIL DISTURBING ACTIVITIES," THE SMF(S) WILL BE CONSTRUCTED PRIOR TO CLEARING AND GRUBBING OUTSIDE OF THE SMF(S) AREAS AND CONSTRUCTION OF THE PERMANENT PAVED AREAS. THE TOTAL CONTRIBUTING DRAINAGE AREA TO THE STORMWATER MANAGEMENT SYSTEM IS APPROXIMATELY 0.21 ACRES AND WILL CONSIST OF APPROXIMATELY 0.78 ACRES OF DISTURBED CONSTRUCTION AREA. THEREFORE, NO ADDITIONAL SEDIMENT TRAP BASINS ARE NECESSARY TO PROVIDE SEDIMENT STORAGE ON-SITE DURING CONSTRUCTION. AS SHOWN ON THE STORMWATER POLLUTION PREVENTION PLAN (C0.21), THE PROPOSED STORMWATER MANAGEMENT SYSTEM WILL PREVENT OFF-SITE EROSION DURING CONSTRUCTION. SILT FENCES OR EQUIVALENT SEDIMENT CONTROLS SHALL BE INSTALLED AT SIDE SLOPE AND DOWN SLOPE BOUNDARIES, INLET LOCATION, OUTLET LOCATIONS, AND OTHER LOCATIONS AS SHOWN ON THE STORMWATER POLLUTION PREVENTION PLAN, AS REQUIRED. BY COMPLETION OF CONSTRUCTION, THE SIDE SLOPES, SWALES, AND ALL DISTURBED AREAS SHALL BE STABILIZED WITH GRASS AND LANDSCAPING AS SPECIFIED ON THE CONSTRUCTION DRAWINGS.

D. DRAINAGE LOCATIONS THAT SERVE AREAS WITH MORE THAN 10 DISTURBED ACRES

NOT APPLICABLE, SEE SECTION C, ABOVE.

V. STORMWATER MANAGEMENT

A. BEST MANAGEMENT PRACTICES

AFTER CONSTRUCTION, THE STORMWATER MANAGEMENT SYSTEM SHALL BE MAINTAINED IN ACCORDANCE WITH THE SPECIFIED STORMWATER MAINTENANCE NOTES IN THE INCLUDED CONSTRUCTION DRAWINGS AND/OR RESPECTIVE MAINTENANCE REPORTS. SPECIFICALLY, THE PROPOSED SMF(S) SHALL BE MOWED REGULARLY IN THE SPECIFIED AREAS, STORM PIPES AND STRUCTURES WILL BE INSPECTED SEMI-ANNUALLY AND CLEANED ANNUALLY, SMF(S) SIDE SLOPES SHALL BE MAINTAINED TO PREVENT EROSION, AND LANDSCAPING AND GRASS THAT PREVENTS EROSION SHALL BE MAINTAINED. ADDITIONALLY, REMEDIAL ACTIONS SHALL BE TAKEN SHOULD THE SMF(S) NOT PERFORM AS DESIGNED.

B. VEGETATED SWALES

WHEN VEGETATED SWALES ARE UTILIZED, SILT FENCING OR EQUIVALENT SEDIMENT CONTROLS SHALL BE INSTALLED AT ADEQUATE INTERVALS TO COLLECT SEDIMENT ALONG THE SWALE. THE SEDIMENT SHALL BE REMOVED WHEN SEDIMENT REACHES ONE-THIRD OF THE HEIGHT OF THE SILT FENCING. SEE THE STORMWATER POLLUTION PREVENTION PLAN (C0.21) FOR DETAILS AND LOCATIONS, AS REQUIRED.

C. VELOCITY DISSIPATION DEVICES AT DISCHARGE POINTS

WHEN DISCHARGE POINTS ARE NOT LOCATED UNDER WATER, RIP RAP PADS HAVE BEEN PROVIDED AT LOCATIONS WHERE NECESSARY DUE TO ANTICIPATED DISCHARGE VELOCITIES. PLEASE SEE THE CONSTRUCTION PLANS FOR DETAILS AND LOCATIONS, AS NEEDED.

VI. CONTROLS FOR OTHER POTENTIAL POLLUTANTS

A. WASTE DISPOSAL

THE CONTRACTOR SHALL PROVIDE LITTER COLLECTION CONTAINERS WITHIN THE PROJECT BOUNDARIES DURING CONSTRUCTION. CONTRACTOR SHALL DISPOSE OF ALL UNSUITABLE MATERIALS AND CONSTRUCTION DEBRIS IN ACCORDANCE WITH ALL APPLICABLE LOCAL, STATE, AND FEDERAL REQUIREMENTS.

B. DUST CONTROL

TO PREVENT OFF-SITE VEHICULAR TRACKING OF SEDIMENTS AND DUST GENERATION, A STABILIZED CONSTRUCTION ENTRANCE SHALL BE ESTABLISHED BY THE SITE CONTRACTOR. PLEASE SEE THE STORMWATER POLLUTION PREVENTION PLAN (C0.21) FOR DETAILS AND LOCATION(S).

C. EXISTING VERSUS PROPOSED POTABLE AND SANITARY SEWER SYSTEMS

IF TEMPORARY SANITARY SYSTEMS ARE UTILIZED DURING CONSTRUCTION, THE CONTRACTOR SHALL PROPERLY CONTROL AND DISCHARGE ANY SANITARY WASTE IN ACCORDANCE WITH ALL APPLICABLE LOCAL, STATE, AND FEDERAL REGULATIONS.

D. FERTILIZER & PESTICIDES

THE USE OF FERTILIZERS, HERBICIDES, AND PESTICIDES ON THE PROJECT SITE, WILL BE DIRECTED BY THE LANDSCAPE PLAN AND THE FDOT STANDARD SPECIFICATIONS SECTION 570, TO SUPPORT THE GROWTH OF THE PROPOSED VEGETATION. ESTABLISHING THIS VEGETATION WILL AID IN THE STABILIZATION OF THE PROJECT SITE AND REDUCE EROSION. APPLICATION RATES FOR THE FERTILIZERS, HERBICIDES, AND PESTICIDES SHALL BE IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS TO GUARD AGAINST OVER-USE, WHICH CAN LEAD TO VIOLATIONS OF STATE WATER QUALITY STANDARDS.

E. TOXIC MATERIAL

THE CONSTRUCTION SITE WILL BE IN FULL COMPLIANCE WITH STATE AND FEDERAL REQUIREMENTS. A PLASTIC MAT, TAR PAPER, OR OTHER IMPERVIOUS MATERIAL SHALL BE PLACED UNDER AREAS WHERE TOXIC LIQUIDS ARE TO BE OPENED AND STORED.

F. HAZARDOUS MATERIALS

ALL HAZARDOUS MATERIALS SHALL BE STORED IN A SECURE LOCATION, UNDER COVER, AND IN APPROPRIATE TIGHTLY, SEALED CONTAINERS WHEN NOT IN USE. ALL PRODUCTS SHALL BE STORED IN AND USED FROM THE ORIGINAL CONTAINER WITH THE ORIGINAL PRODUCT LABEL. CONTAINERS MUST BE STORED IN A MANNER TO PROTECT THEM FROM THE ELEMENTS AND INCIDENTAL DAMAGE. THE MINIMUM PRACTICAL QUANTITY OF ALL SUCH MATERIALS SHALL BE KEPT ON THE JOB SITE AND SCHEDULED FOR DELIVERY AS CLOSE TO TIME OF USE AS PRACTICAL.

ALL PRODUCTS SHALL BE USED IN STRICT COMPLIANCE WITH THE INSTRUCTIONS ON THE PRODUCT LABEL.

SUFFICIENT EQUIPMENT AND/OR MATERIALS SHALL BE KEPT ONSITE TO CONTAIN AND CLEAN UP SPILLS OF HAZARDOUS MATERIALS IN THE AREAS WHERE THESE MATERIALS ARE STORED OR USED. SPILL CONTROL AND CONTAINMENT KIT SUPPLIES SHALL BE OF SUFFICIENT QUANTITIES AND APPROPRIATE CONTENT TO CONTAIN A SPILL FROM THE LARGEST ANTICIPATED PIECE OF EQUIPMENT AND FROM THE LARGEST ANTICIPATED QUANTITIES OF PRODUCTS STORED ON THE SITE AT ANY GIVEN TIME.

CONTRACTOR TO CONTAIN AND CLEAN UP ANY SPILLS IMMEDIATELY AFTER THEY OCCUR. ANY SPILLS OF PETROLEUM PRODUCTS OR HAZARDOUS MATERIALS IN EXCESS OF REPORTABLE QUANTITIES AS DEFINED BY EPA, STATE, OR LOCAL AGENCY REGULATIONS SHALL BE REPORTED TO THE APPROPRIATE AGENCIES IN THE REQUIRED TIME FRAMES. THE CONTRACTOR SHALL PROVIDE A WRITTEN NOTICE TO THE OWNER IMMEDIATELY UPON IDENTIFICATION OF ANY SPILL.

ALL EXCESS, USED, OR SPILLED PRODUCTS, INCLUDING CONTAMINATED SOIL, SHALL BE DISPOSED OF BY THE CONTRACTOR IN STRICT COMPLIANCE WITH INSTRUCTIONS ON THE PRODUCT LABEL AND ALL APPLICABLE REGULATIONS.

VII. APPROVED STATE AND LOCAL PLANS

THE CONSTRUCTION DRAWINGS FOR THE PROJECT WERE APPROVED AND PERMITTED BY THE FOLLOWING AGENCIES:

- *CITY OF ALACHUA*
- *FLORIDA DEPARTMENT OF TRANSPORTATION*
- *SUWANNEE RIVER WATER MANAGEMENT DISTRICT*
- *FLORIDA DEPARTMENT OF ENVIRONMENTAL PROTECTION*

VIII. CONSTRUCTION ACTIVITY DISCHARGES

IN ACCORDANCE WITH THIS PLAN, THERE ARE NO ANTICIPATED DISCHARGES OF STORMWATER ASSOCIATED WITH CONSTRUCTION ACTIVITIES.

IX. CHANGES TO THE POLLUTION PREVENTION PLAN

THIS STORMWATER POLLUTION PREVENTION PLAN SHALL BE AMENDED TO REFLECT ANY APPLICABLE CHANGE IN A STATE, REGIONAL, OR LOCAL PERMIT FOR WHICH THE PERMITTEE RECEIVES WRITTEN NOTICE. WHEN WRITTEN NOTICE IS RECEIVED, THE PERMITTEE SHALL PROVIDE A RE-CERTIFICATION OF THIS POLLUTION PREVENTION PLAN, WHICH HAS BEEN REVISED TO ADDRESS SUCH CHANGES. AMENDMENTS TO THE PLAN SHALL BE PREPARED, SIGNED, DATE, AND KEPT AS ATTACHMENTS TO THE ORIGINAL PLAN.

X. ALTERNATIVE PERMIT REQUIREMENTS

NO ALTERNATIVE PERMIT REQUIREMENTS ARE REQUESTED.

XI. MAINTENANCE

THE CONTRACTOR IS RESPONSIBLE FOR THE MAINTENANCE, INSPECTION SCHEDULE, AND REPAIRS OUTLINED IN THIS PLAN. MAINTENANCE SHALL CONTINUE THROUGHOUT THE PROJECT UNTIL WORK IS COMPLETE. THE CONTRACTOR IS RESPONSIBLE FOR REMOVING ALL TEMPORARY EROSION AND SEDIMENT CONTROL DEVICES AFTER CONSTRUCTION IS COMPLETE. IN ADDITION TO THE TIMES MENTIONED IN THE PREVIOUS SECTIONS, THE CONTRACTOR SHALL INITIATE ANY REPAIRS WITHIN 24 HOURS OF BEING REPORTED. IN THE EVENT THAT THE SMF(S) DO NOT PERFORM PROPERLY OR IF A SINKHOLE DEVELOPS, THE PROJECT ENGINEER SHALL BE NOTIFIED TO ASSIST IN COORDINATING REMEDIAL ACTION. ACCUMULATED SEDIMENT SHALL BE REMOVED FROM SILT FENCING WHEN IT HAS REACHED ONE-THIRD THE HEIGHT OF THE SILT FENCE. UPON FINAL COMPLETION OF CONSTRUCTION AND ACCEPTANCE BY BOTH THE CITY AND OWNER, THE OPERATION AND MAINTENANCE ENTITY WILL BE "DE FOODS, LLC.".

XII. INSPECTIONS

THE CONTRACTOR SHALL INSPECT ALL POINTS OF POTENTIAL DISCHARGE FROM THE PROJECT SITE FOR ALL DISTURBED AREAS ON THE CONSTRUCTION SITE AT LEAST ONCE EVERY SEVEN CALENDAR DAYS AND WITHIN 24 HOURS OF THE END OF A STORM THAT IS 0.50 INCHES OR GREATER. FOR POINTS OF DISCHARGE INTO SURFACE WATERS OF THE STATE OR AN MS4, A QUALIFIED INSPECTOR (PROVIDED BY THE OPERATOR) SHALL PERFORM THE REQUIRED INSPECTIONS. THE CONTRACTOR SHALL INSTALL A RAIN GAUGE AT THE SITE TO MONITOR AND DOCUMENT RAINFALL EVENTS 0.50 INCHES OR GREATER. LOCATIONS WHERE THE SITE IS COMPLETELY CONSTRUCTED AND STABILIZED, SUCH INSPECTIONS SHALL BE CONDUCTED AT LEAST ONCE A MONTH. ALL INSPECTIONS SHALL BE RECORDED ON THE CONSTRUCTION INSPECTION FORM. THE CONTRACTOR MAY USE THEIR OWN FORM (MEETING FDEP SWPPP REQUIREMENTS) OR A SAMPLE FORM FROM FDEP. A SAMPLE CONSTRUCTION FORM IS AVAILABLE AT: "HTTPS://FLORIDADEP.GOV/WATER/STORMWATER/DOCUMENTS/CONSTRUCTION-SWPPP". MORE SPECIFICALLY, THE INSPECTION SHALL ENSURE THE FOLLOWING CATEGORIES.

A. DISTURBED AREAS

ALL DISTURBED AREAS AND AREAS USED FOR MATERIAL STORAGE SHALL BE INSPECTED FOR POLLUTANTS ENTERING THE STORMWATER SYSTEM. THE STORMWATER MANAGEMENT SYSTEM AND EROSION AND SEDIMENT CONTROL MEASURES IDENTIFIED IN THE PLAN SHALL BE INSPECTED TO ENSURE THEY ARE OPERATING CORRECTLY. LOCATIONS WHERE VEHICLES ENTER AND EXIT THE SITE SHALL BE INSPECTED FOR EVIDENCE OF OFF-SITE SEDIMENT TRACKING.

B. MAINTENANCE PERFORMANCE

BASED ON THE RESULTS OF THE INSPECTION, ALL MAINTENANCE OPERATIONS NEEDED TO ASSURE PROPER COMPLIANCE WITH THIS PLAN SHALL BE DONE IN A TIMELY MANNER, BUT IN NO CASE LATER THAN 7 DAYS FOLLOWING THE INSPECTION.

C. REPORTING REQUIREMENTS

ALL INSPECTIONS SHALL BE RECORDED ON THE CONSTRUCTION INSPECTION FORM. THIS FORM IS CREATED TO SUMMARIZE THE SCOPE OF THE INSPECTION, THE NAME(S) AND QUALIFICATION OF THE INSPECTOR(S), THE DATE OF INSPECTION, RAINFALL DATA, OBSERVATIONS, THE ACTIONS TAKEN TO CORRECT INCIDENTS OF NON-COMPLIANCE WITH THE PROVISIONS OF THIS PLAN. IF NO INCIDENTS OF NON-COMPLIANTS ARE OBSERVED, THE REPORT SHALL CONTAIN A CERTIFICATION THAT THE FACILITY IS IN COMPLIANCE WITH THE STORMWATER POLLUTION PREVENTION PLAN AND THE ASSOCIATED PERMIT.

XIII. NON-STORMWATER DISCHARGES

IN ADDITION TO STORMWATER RUNOFF, THIS PLAN APPLIES TO RUNOFF FROM IRRIGATION OPERATIONS AND CONSTRUCTION PRACTICES. THIS PLAN DOES NOT PERTAIN TO DISCHARGES FROM FIRE FIGHTING ACTIVITIES.

XIV. CONTRACTORS CERTIFICATION

THE CONTRACTORS OR SUB-CONTRACTORS SHALL PHOTOCOPY AND COMPLETE THE FORM ON THIS PAGE. IT SHALL BE PROVIDED TO THE OWNER AND KEPT ON FILE PURSUANT TO SECTION XV REGARDING PROJECT RECORDS.

XV. RETENTION OF RECORDS

THE PERMITTEE SHALL RETAIN COPIES OF STORMWATER POLLUTION PREVENTION PLANS AND ALL REPORTS REQUIRED BY THIS PERMIT, AND RECORDS OF ALL DATA USED TO COMPLETE THE NOTICE OF INTENT TO BE COVERED BY THIS PERMIT, FOR A PERIOD OF AT LEAST THREE YEARS FROM THE DATE THAT THE SITE IS FINALLY STABILIZED.

THE PERMITTEE SHALL RETAIN A COPY OF THE STORMWATER POLLUTION PREVENTION PLAN AND ALL REPORTS, RECORDS, AND DOCUMENTATION REQUIRED BY THIS PERMIT AT THE CONSTRUCTION SITE, OR AN APPROPRIATE ALTERNATIVE LOCATION AS SPECIFIED IN THE NOTICE OF INTENT, FROM THE DATE OF PROJECT INITIATION TO THE DATE OF FINAL STABILIZATION.

XVI. NOTICE OF TERMINATION

NOTICE OF TERMINATION:

1. *WHERE A SITE HAS BEEN FINALLY STABILIZED AND ALL STORMWATER DISCHARGES AUTHORIZED BY THIS PERMIT ARE ELIMINATED, THE PERMITTEE SHALL SUBMIT A NOTICE OF TERMINATION (DEP FORM 62-621.300(6)), SIGNED IN ACCORDANCE WITH PART VII.C OF DEP DOCUMENT NO. 62-621.300(4)(a), WITHIN 14 DAYS OF FINAL STABILIZATION OF THE SITE TO TERMINATE COVERAGE UNDER THIS PERMIT.*
2. *ELIMINATION OF STORMWATER DISCHARGES ASSOCIATED WITH CONSTRUCTION ACTIVITY MEANS THAT ALL DISTURBED SOILS AT THE SITE HAVE BEEN FINALLY STABILIZED AND TEMPORARY EROSION AND SEDIMENT CONTROL MEASURES HAVE BEEN REMOVED OR WILL BE REMOVED AT AN APPROPRIATE TIME, OR THAT ALL STORMWATER DISCHARGES ASSOCIATED WITH CONSTRUCTION ACTIVITY FROM THE SITE THAT ARE AUTHORIZED BY THIS GENERIC PERMIT HAVE OTHERWISE BEEN ELIMINATED.*
3. *FOR CONSTRUCTION ACTIVITIES WHERE THE OPERATOR CHANGES, THE EXISTING OPERATOR SHALL FILE AN N.O.T. IN ACCORDANCE WITH THIS PART WITHIN 14 DAYS OF RELINQUISHING CONTROL OF THE PROJECT TO A NEW OPERATOR.*

THE PERMITTEE SHALL SUBMIT A NOTICE OF TERMINATION TO THE FOLLOWING ADDRESS:

NPDES STORMWATER NOTICES CENTER, MS# 2510
FLORIDA DEPARTMENT OF ENVIRONMENTAL PROTECTION
2600 BLAIR STONE ROAD
TALLAHASSEE, FLORIDA 32399-2400

PROJECTS THAT DISCHARGED STORMWATER ASSOCIATED WITH CONSTRUCTION ACTIVITY TO A MUNICIPAL SEPARATE STORM SEWER SYSTEM (MS4) SHALL SUBMIT A COPY OF THE N.O.T. TO THE OPERATOR OF THE MS4.

Contractor/Subcontractor Certification Statement
Stormwater Pollution Prevention Plan

Site Name: KFC Alachua
Site Location: South of US 441 and East of NW 167th Blvd.
Alachua County, Florida

THE CONTRACTOR(S) OR SUB-CONTRACTOR(S) RESPONSIBLE FOR COMPLYING WITH THIS STORMWATER POLLUTION PREVENTION PLAN SHALL SIGN THE CERTIFICATION STATEMENT BELOW. MULTIPLE COPIES OF THIS CERTIFICATION STATEMENT MAY BE NECESSARY DEPENDING ON THE NUMBER OF SUB-CONTRACTORS ASSOCIATED WITH THE PROJECT

I CERTIFY UNDER PENALTY OF LAW THAT I UNDERSTAND, AND SHALL COMPLY WITH, THE TERMS AND CONDITIONS OF THE STATE OF FLORIDA GENERIC PERMIT FOR STORMWATER DISCHARGE FROM LARGE AND SMALL CONSTRUCTION ACTIVITIES AND THIS STORMWATER POLLUTION PREVENTION PLAN PREPARED THEREUNDER.

DATE	RESPONSIBLE INDIVIDUAL'S NAME	RESPONSIBLE INDIVIDUAL'S SIGNATURE	TITLE	COMPANY NAME, ADDRESS, AND PHONE NUMBER

11801 Research Drive
Alachua, Florida 32615
(386) 418-1976
www.chw-inc.com
est. 1988 FLORIDA CA-9075
CHW Professional Consultants

SCALE: N/A

VERIFY SCALE
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THIS SHEET ADJUST
SCALES ACCORDINGLY
0 1"

CONSTRUCTION/BID REVISIONS:

SUBMITTALS:
1/28/21 COA, SRWMD, AND FDOT
3/8/21 COA, SRWMD
3/18/21 FDOT
3/25/21 COA, SRWMD

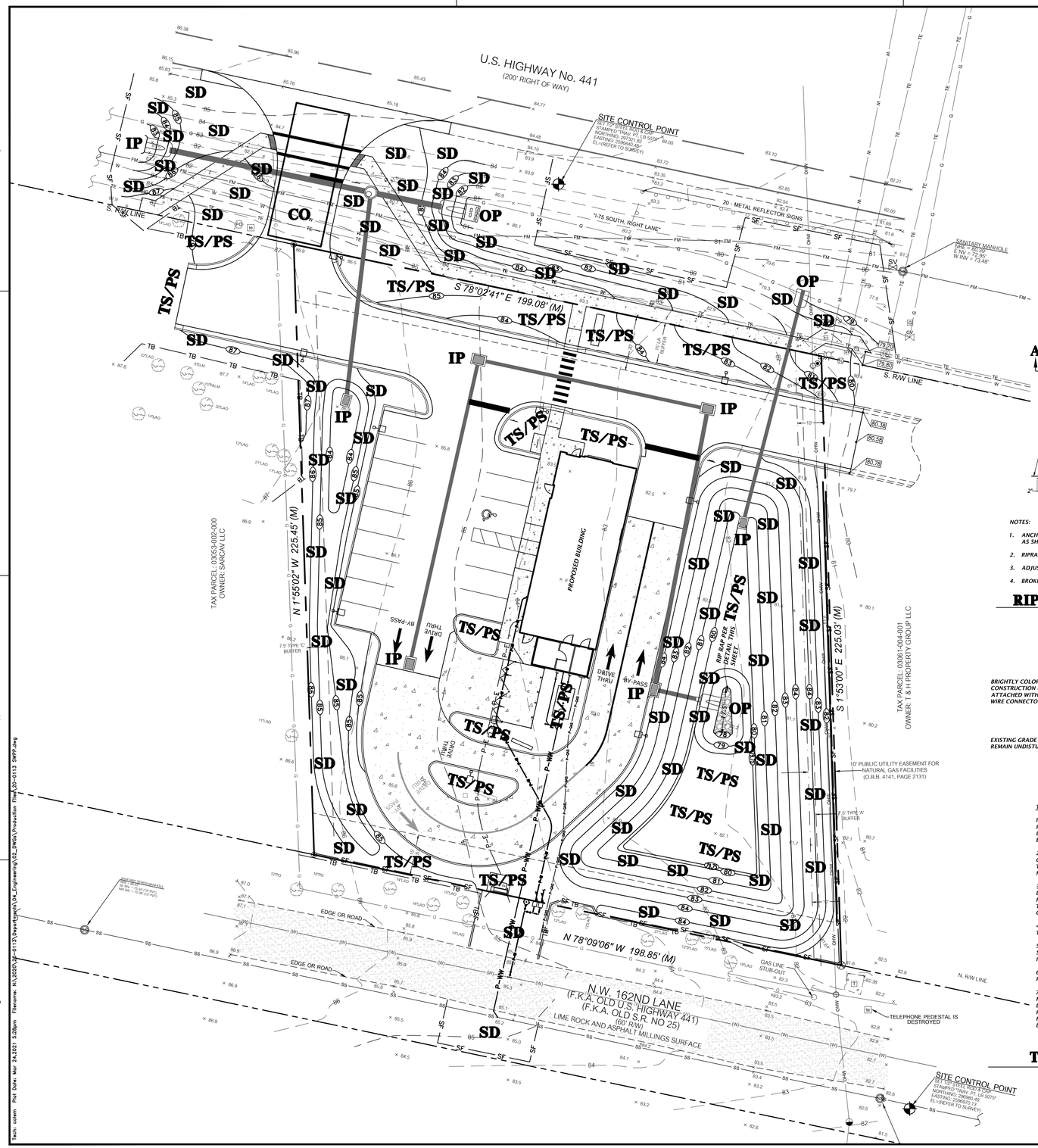
CLIENT: DE FOODS, LLC.
PROJECT: KFC ALACHUA, FL
SHEET TITLE: STORMWATER POLLUTION PREVENTION NOTES

TECHNICIAN: T.F. COWART
DESIGNER: C. MEDDERMENTT, E.I.
CONSTRUCTION OBSERVATION: D.H. YOUNG, P.E.
PROJECT NUMBER: 20-0113

DANIEL H. YOUNG
FL PE No. 70780

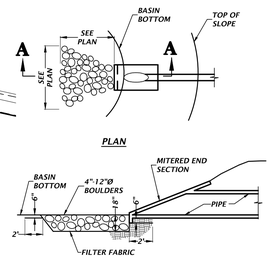
SHEET NO.: C0.20

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STORMWATER POLLUTION PREVENTION LEGEND

TS	= TEMPORARY SEEDING
PS	= PERMANENT SEEDING
ML	= MULCHING
SD	= SOD STABILIZATION
SF	= SILT BARRIER
TB	= TREE BARRIER
IP	= INLET PROTECTION
OP	= OUTLET PROTECTION
CO	= CONSTRUCTION ENTRANCE/EXIT

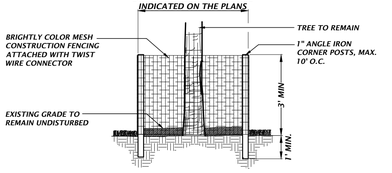


NOTES:

1. ANCHOR FILTER FABRIC BY EXTENDING BEYOND RIPRAP AND BACKFILLING AS SHOWN ON DETAIL.
2. RIPRAP SHALL NOT EXTEND ONTO SIDE SLOPES.
3. ADJUST SHAPE OF APRON WHERE REQUIRED TO MATCH TOE OF SLOPE.
4. BROKEN SLOPE IS NOT ACCEPTABLE.

RIP-RAP EROSION PROTECTION

NTS



TREE PROTECTION NOTES

1) PROTECTIVE BARRIERS SHALL BE CONSTRUCTED, AS NECESSARY, TO PREVENT THE DESTRUCTION OR DAMAGING OF REGULATED TREES THAT ARE LOCATED WITHIN 50 FEET OF ANY CONSTRUCTION ACTIVITY OR STORAGE OF EQUIPMENT AND MATERIALS.

2) PROTECTIVE BARRIERS SHALL BE PLAINLY VISIBLE AND SHALL CREATE A CONTINUOUS BOUNDARY AROUND TREES OR VEGETATION CLUSTERS IN ORDER TO PREVENT ENCROACHMENT BY MACHINERY, VEHICLES OR STORED MATERIALS.

3) NO TRENCHING ALLOWED WITHIN THE PROTECTIVE BARRIER ZONE. HAND DIG TO INSTALL UTILITY IF APPROVED BY CITY MANAGER OR DESIGNEE. WHERE ROOTS GREATER THAN ONE INCH IN DIAMETER ARE DAMAGED OR EXPOSED, THEY SHALL BE CUT CLEANLY AND RE-COVERED WITH SOIL WITHIN ONE HOUR OF DAMAGE OR EXPOSURE.

4) PROTECTIVE BARRIERS SHALL REMAIN IN PLACE AND INTACT UNTIL SUCH TIME AS LANDSCAPE OPERATIONS BEGIN.

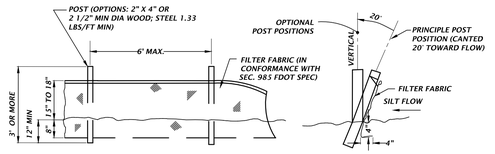
5) LANDSCAPE PREPARATION IN THE PROTECTED AREA SHALL BE LIMITED TO SHALLOW DISCING OF THE AREA. DISCING SHALL BE LIMITED TO A DEPTH OF 4 INCHES.

6) NO BUILDING MATERIALS, MACHINERY OR HARMFUL CHEMICALS SHALL BE PLACED WITHIN PROTECTIVE BARRIERS.

7) THE AMERICAN NATIONAL STANDARDS INSTITUTE A-300 PART V: MANAGEMENT OF TREES AND SHRUBS DURING SITE PLANNING, SITE DEVELOPMENT, AND SITE CONSTRUCTION OR OTHER NATIONALLY RECOGNIZED ARBORICULTURAL STANDARDS SHALL BE USED AS GUIDELINES FOR TREE PROTECTION, PLANTING, PRUNING AND CARE DURING DEVELOPMENT AND CONSTRUCTION.

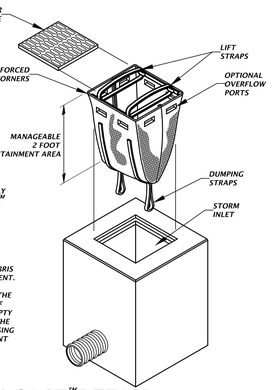
TREE PROTECTION DETAIL

NTS



TYPE III SILT FENCE DETAIL

NTS



INSTALLATION:

REMOVE THE GRATE FROM INLET. IF USING OPTIONAL OIL ABSORBENTS; PLACE ABSORBENT PILLOW IN UNIT. STAND THE GRATE ON END. MOVE THE TOP LIFTING STRAPS OUT OF THE WAY AND PLACE THE GRATE INTO THE DANDY SACK™ SO THAT THE GRATE IS BELOW THE TOP STRAPS AND ABOVE THE LOWER STRAPS. HOLDING THE LIFTING DEVICES, INSERT THE GRATE INTO THE INLET.

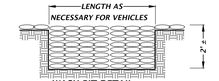
MAINTENANCE:

REMOVE ALL ACCUMULATED SEDIMENT AND DEBRIS FROM VICINITY OF UNIT AFTER EACH STORM EVENT. AFTER EACH STORM EVENT AND AT REGULAR INTERVALS, LOOK INTO THE DANDY SACK™ IF THE CONTAINMENT AREA IS MORE THAN 1/3 FULL OF SEDIMENT, THE UNIT MUST BE EMPTIED. TO EMPTY UNIT, LIFT THE UNIT OUT OF THE INLET USING THE LIFTING STRAPS AND REMOVE THE GRATE. IF USING OPTIONAL OIL ABSORBENTS; REPLACE ABSORBENT WHEN NEAR SATURATION.

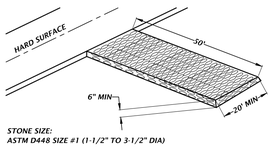
DANDY SACK™ DETAIL

NTS

NOTE:
1. CONSTRUCTION EXIT INSTALLATION SHALL REMOVE MUD/SOILS FROM TIRES TO PREVENT TRACKING ONTO PUBLIC ROADS.
2. TOP DRESSING WITH 2" STONE MAY BE REQ'D AS DETERMINED BY THE USE AND FUNCTION OF THE SYSTEM.
3. THE WASH PIT SHOULD BE INSTALLED IF THE STANDARD CONSTRUCTION EXIT DOES NOT SUFFICIENTLY REMOVE SOILS AND WASHING IS REQ'D.

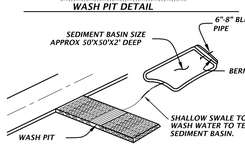


WASH PIT DETAIL



STONE SIZE:
ASTM D448 SIZE #1 (1-1/2" TO 3-1/2" DIA)

STANDARD CONSTRUCTION EXIT



CONSTRUCTION EXIT WITH WASH PIT

TEMPORARY CONSTRUCTION EXIT DETAIL

NTS

TYPICAL INSTALLATION SEQUENCE FOR SILT-SAVER FRAME AND FILTER

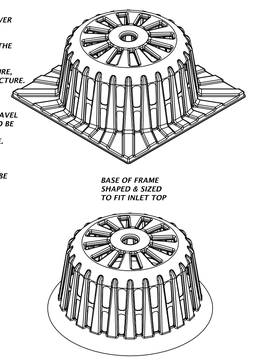
1. EXCAVATE APPROXIMATELY 4" TO 6" BELOW THE TOP OF THE INLET STRUCTURE.

2. PLACE THE FRAME ONTO THE INLET STRUCTURE, ENSURING PROPER SEATING OF FRAME TO STRUCTURE.

3. SLIDE THE FILTER OVER THE FRAME.

4. FILL THE FILTER POCKETS WITH SOIL, #57 GRAVEL OR EQUIVALENT. THE FILTER POCKETS SHOULD BE COMPLETELY FILLED TO ENSURE A GOOD SEAL BETWEEN THE GROUND AND INLET STRUCTURE.

5. BACK FILL AROUND THE FRAME AND FILTER ASSEMBLY IS NOT REQUIRED TO COMPLETE INSTALLATION; HOWEVER, BACK FILLING MAY BE NECESSARY TO COMPLETE EXCAVATION REQUIREMENTS FOR THE SITE.



SILT-SAVER® DETAIL

NTS

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(352) 331-1976
(352) 331-2476
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est. 1988 FLORIDA CA-9075
CHW
Professional Consultants

SCALE: 1"=20'
VERIFY SCALE
BAR IS ONE INCH ON ORIGINAL DRAWING
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0 1"

CONSTRUCTION BID REVISIONS

SUBMITTALS:
1/28/21 COA, SRWMD, AND FDOT
2/26/21 COA, SRWMD
3/18/21 FDOT
3/25/21 COA, SRWMD

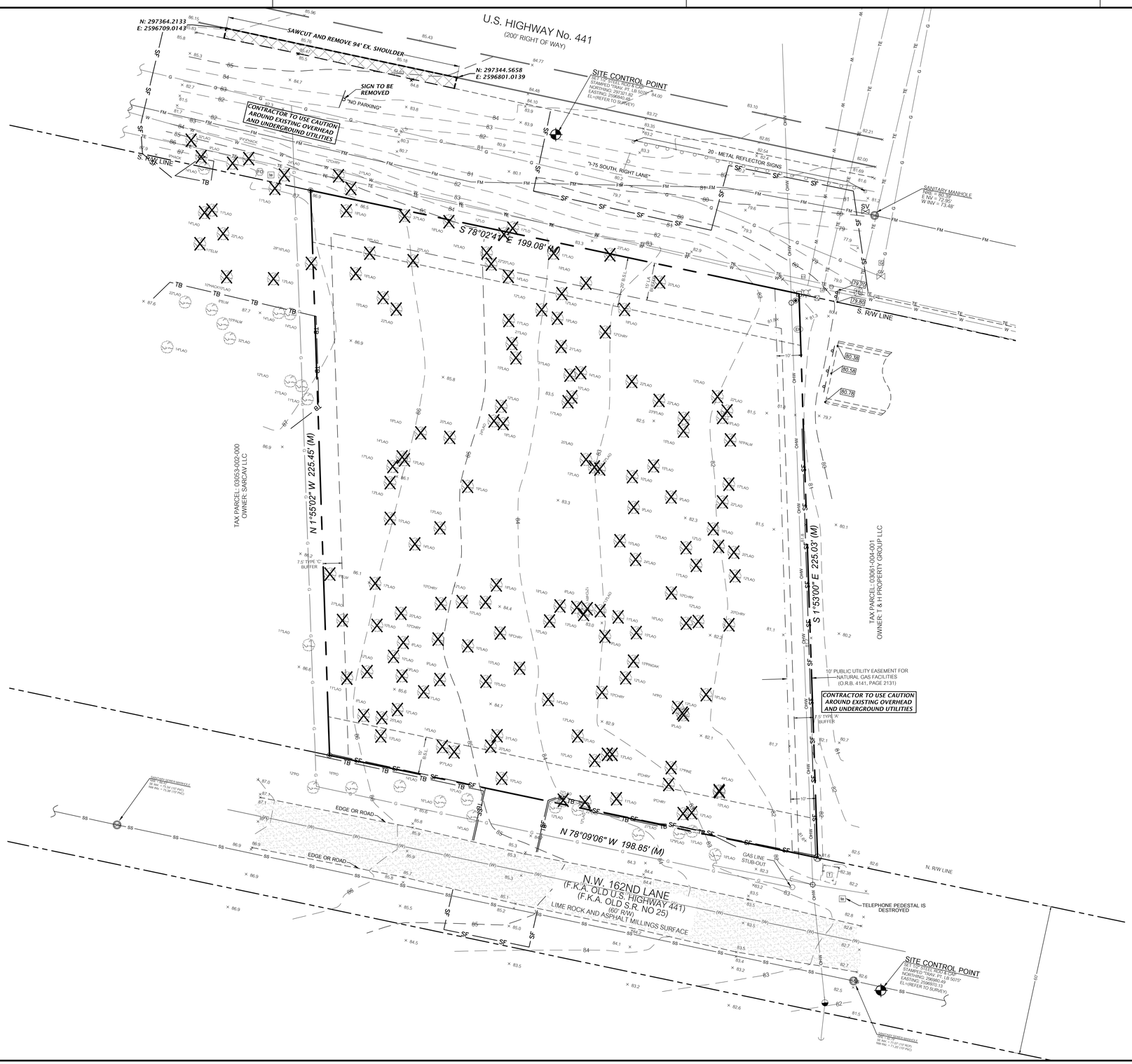
CLIENT: DE FOODS, LLC.
PROJECT: KFC ALACHUA, FL
SHEET TITLE: STORMWATER POLLUTION PREVENTION PLAN

TECHNICIAN: T.F. COWART
DESIGNER: C. MCKENNETT, E.I.
QUALITY CONTROL: D.H. YOUNG, P.E.
PROJECT NUMBER: 20-0113

DANIEL H. YOUNG
FL PE No. 70780

SHEET NO.: C0.21

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NOTE:

- CONTRACTOR SHALL HAVE ALL SILT FENCE & TREE BARRICADES INSTALLED PRIOR TO ANY SITE WORK. EXCEPT WITHIN LIMITS OF EXISTING IMPROVEMENTS (PAD AREA). THIS WILL BE INSTALLED ONCE DEMOLITION HAS BEEN COMPLETED.

- CONTRACTORS SHALL REPAIR/RESTORE ANY DISTURBED AREAS TO EXISTING OR BETTER CONDITION.

= EX. IMPROVEMENTS TO BE REMOVED

X = EX. TREE TO BE REMOVED

SF = PROPOSED SILT FENCE

TB = PROPOSED TREE BARRICADE

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(386) 418-1975
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1/28/21 COA, SRWMD, AND FDOT
3/08/21 COA, SRWMD
3/18/21 FDOT
3/25/21 COA, SRWMD

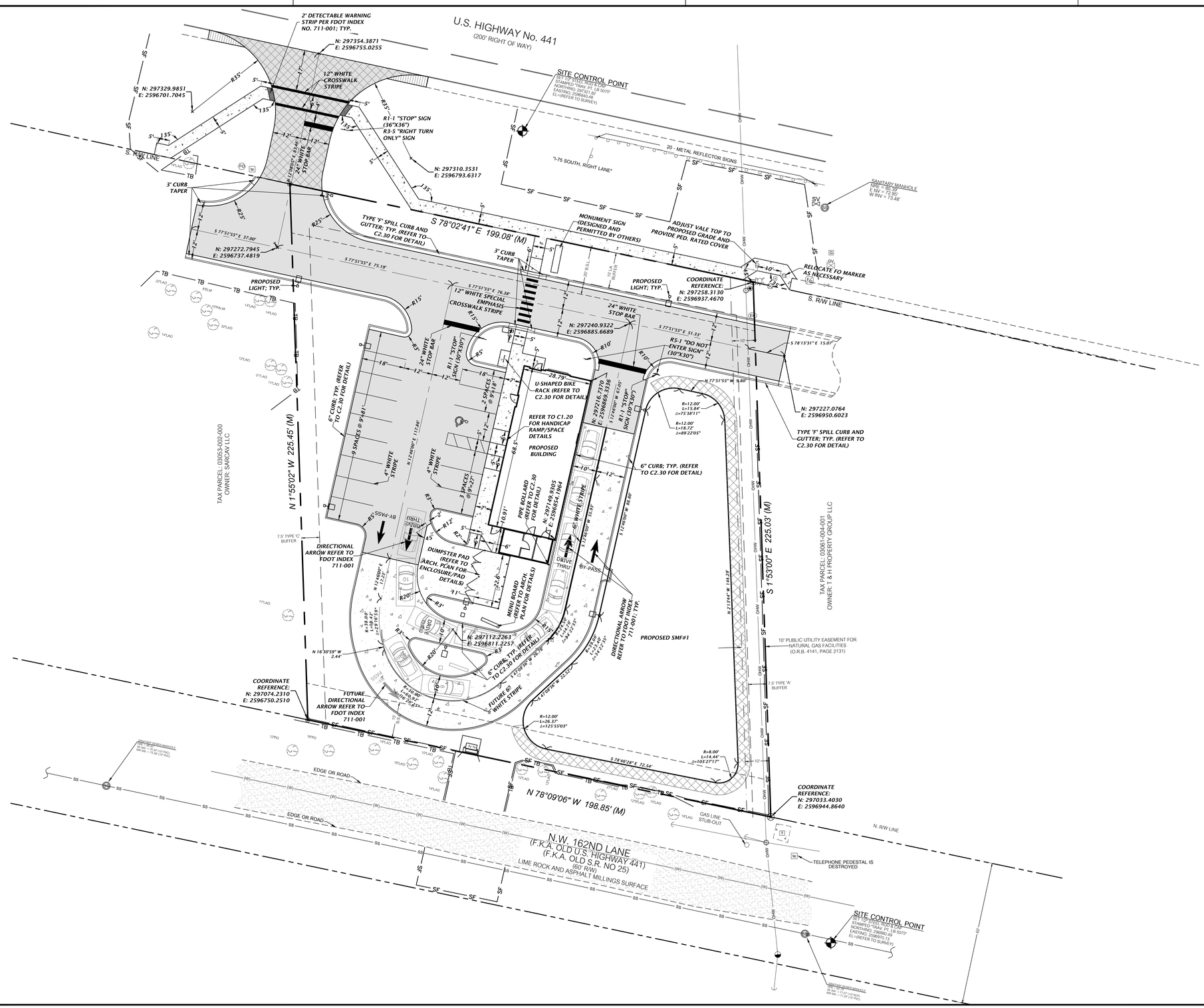
CLIENT: DE FOODS, LLC.
PROJECT: KFC
ALACHUA, FL
SHEET TITLE: DEMOLITION SITE PLAN

TECHNICIAN: T.F. COWART
DESIGNER: C. HEMENNETT, E.I.
QUALITY CONTROL: D.H. YOUNG, P.E.
PROJECT NUMBER: 20-0113

DANIEL H. YOUNG
FL PE No. 70780

SHEET NO.: C0.30

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0 1"

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SUBMITTALS:
1/28/21 COA, SRWMD, AND FDOT
2/8/21 COA, SRWMD
3/18/21 FDOT
3/25/21 COA, SRWMD

CLIENT: DE FOODS, LLC.

PROJECT: KFC
ALACHUA, FL

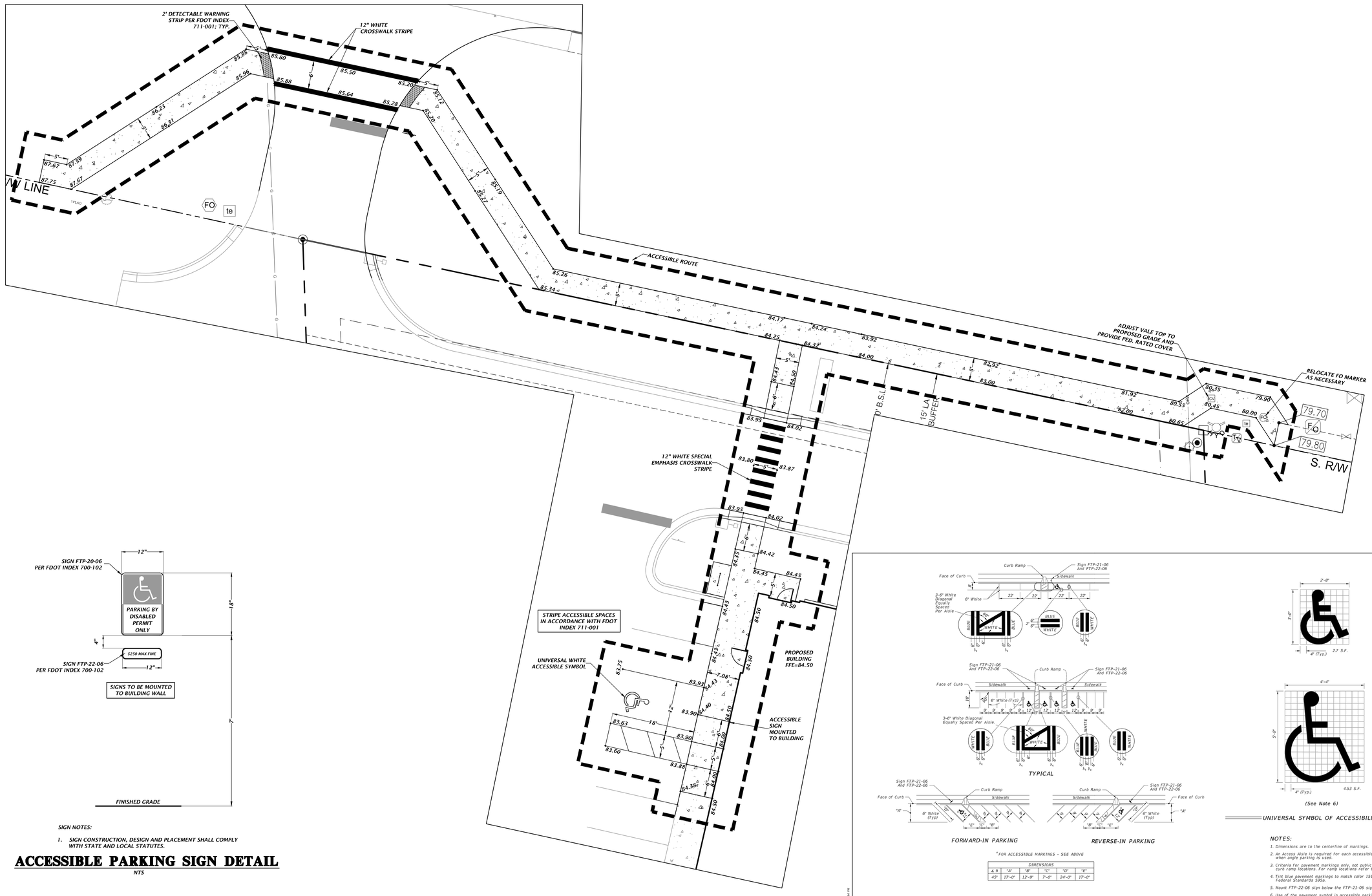
SHEET TITLE: DETAILED HORIZONTAL CONTROL AND SITE PLAN

TECHNICIAN: T.F. COWART
DESIGNER:
C. MEHMENETT, E.I.
CHECKED BY:
D.H. YOUNG, P.E.
PROJECT NUMBER: 20-0113

DANIEL H. YOUNG
FL PE No. 70780

SHEET NO.:
C1.10

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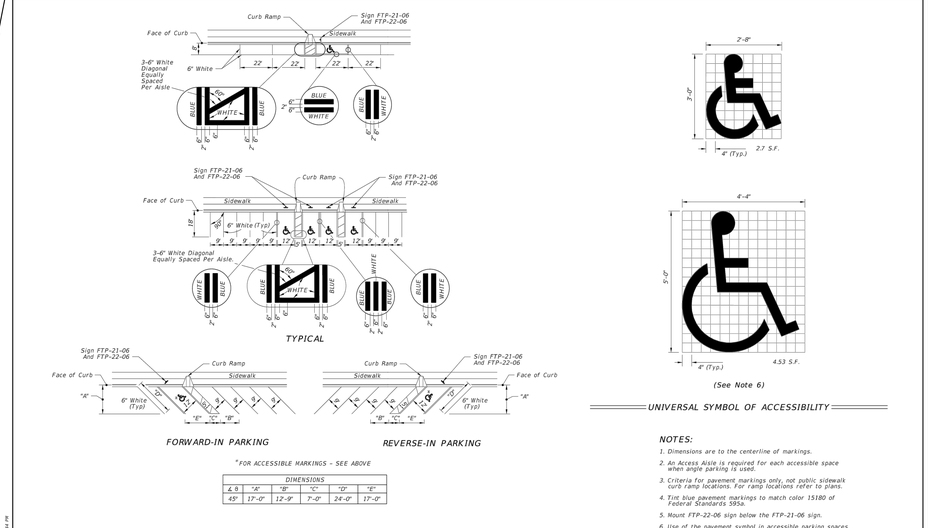


SIGN NOTES:

1. *SIGN CONSTRUCTION, DESIGN AND PLACEMENT SHALL COMPLY WITH STATE AND LOCAL STATUTES.*

ACCESSIBLE PARKING SIGN DETAIL

NTS



*"FOR ACCESSIBLE MARKINGS – SEE ABOVE

DIMENSIONS						
∠ θ	"A"	"B"	"C"	"D"	"E"
45°	17'-0"	12'-9"	7'-0"	24'-0"	17'-0"

PAVEMENT MARKING FOR PARKING

NOTES:

1. *Dimensions are to the centerline of markings.*
2. *An Access Aisle is required for each accessible space when angle parking is used.*
3. *Criteria for pavement markings only, not public sidewalk curb ramp locations. For ramp locations refer to plans.*
4. *Tint blue pavement markings to match color 15180 of Federal Standards 595a.*
5. *Mount FTP-22-06 sign below the FTP-21-06 sign.*
6. *Use of the pavement symbol in accessible parking spaces is optional. When pavement symbol is used, the symbol is either 3'-0" or 5'-0" high and white in color.*

LAST REVISION	DESCRIPTION:	FDOT FY 2020-21 STANDARD PLANS	PAVEMENT MARKINGS	INDEX	SHEET
11/01/19				711-001	12 of 13

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2/26/21 COA, SRWMD
3/18/21 FDOT
3/25/21 COA, SRWMD

CLIENT: DE FOODS, LLC
PROJECT: KFC ALACHUA, FL
SHEET TITLE: ACCESSIBLE SITE PLAN AND DETAILS

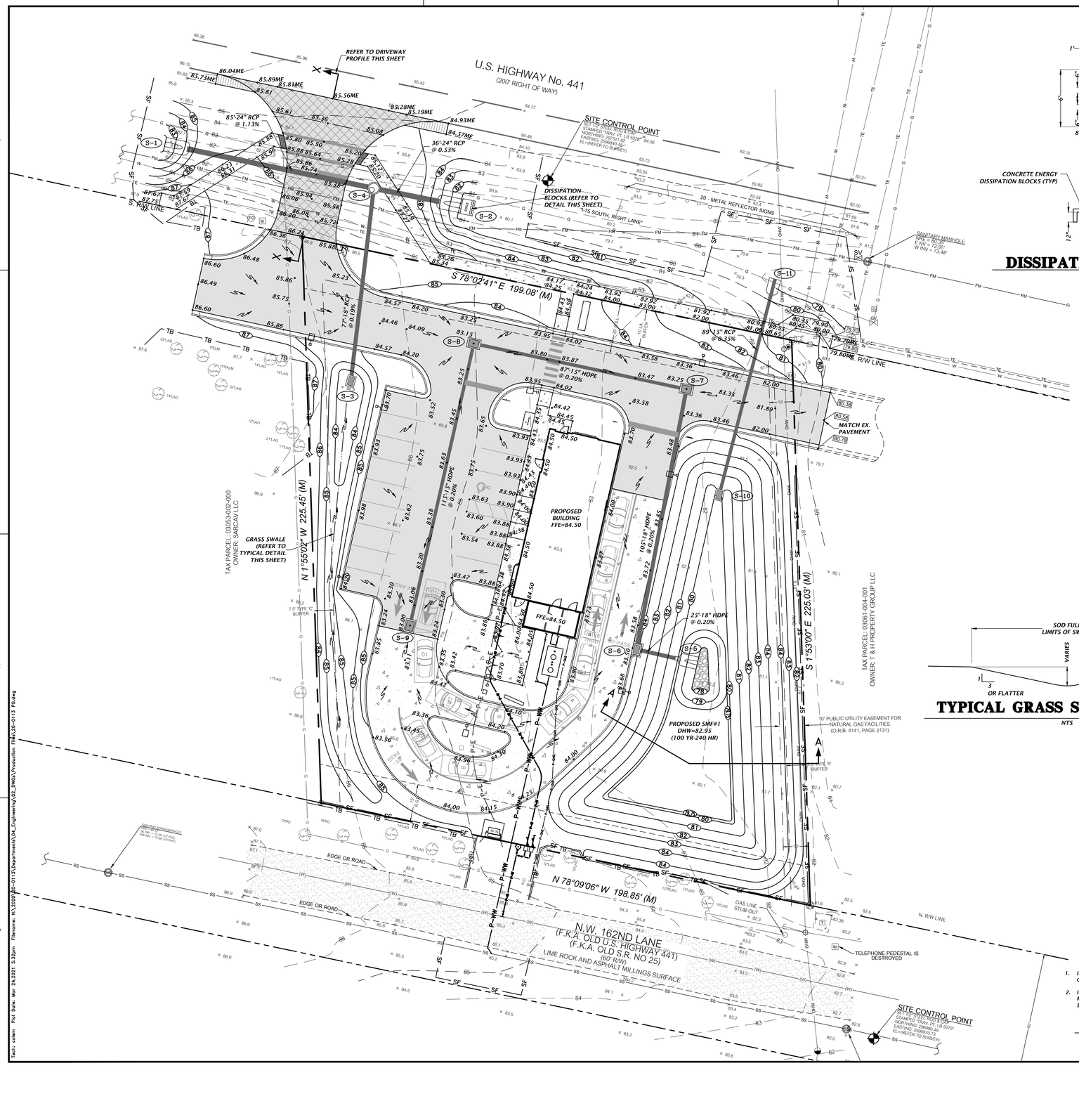
TECHNICIAN: T.F. COWART
DESIGNER: C. MCDONNETT, E.I.
QUALITY CONTROL: D.H. YOUNG, P.E.
PROJECT NUMBER: 20-0113

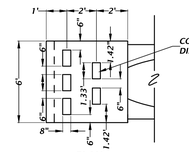
DANIEL H. YOUNG
FL PE No. 70780

SHEET NO: C1.20

CONSTRUCTION/BID REVISIONS:

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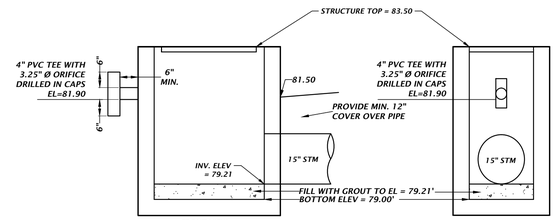


DISSIPATION BLOCK DETAIL

NTS

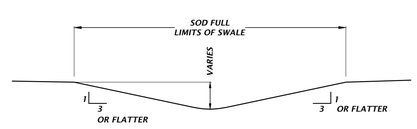
STORMWATER STRUCTURE TABLE				
STRUCTURE:	STRUCTURE TYPE	TOP	INVERT ELEV	NORTHING & EASTING
S-1	24" MITERED END SECTION PER FDOT INDEX 430-022	N/A	82.00' E	N: 297336.29 E: 2596688.13
S-2	24" MITERED END SECTION PER FDOT INDEX 430-022	N/A	80.85' W	N: 297311.68 E: 2596806.10
S-3	TYPE 'C' INLET PER FDOT INDEX 425-052	83.85'	81.18' N	N: 297242.02 E: 2596762.06
S-4	4'Ø MANHOLE PER FDOT INDEX 425-001 & 425-010	85.30'	81.04' S 81.04' W 81.04' E	N: 297318.74 E: 2596771.19
S-5	18" MITERED END SECTION PER FDOT INDEX 430-021	N/A	78.00' W	N: 297129.79 E: 2596899.96
S-6	TYPE C INLET PER FDOT INDEX 425-052	83.50'	78.05' E 78.05' N	N: 297135.28 E: 2596875.74
S-7	TYPE 'F' INLET PER FDOT INDEX 425-053	83.25'	78.26' S 78.26' W	N: 297238.78 E: 2596895.59
S-8	TYPE 'F' INLET PER FDOT INDEX 425-053	83.15'	78.44' E 78.44' S	N: 297256.99 E: 2596810.99
S-9	TYPE 'F' INLET PER FDOT INDEX 425-053	83.00'	78.67' N	N: 297144.64 E: 2596785.54
S-10	OUTFALL STRUCTURE (REFER TO DETAIL ON THIS SHEET	83.50'	79.21' N	N: 297196.74 E: 2596908.61
S-11	15" MITERED END SECTION PER FDOT INDEX 430-021	N/A	78.90' S	N: 297282.62 E: 2596930.87

- CHAIN INLET GRATES PER FDOT INDEX 425-001



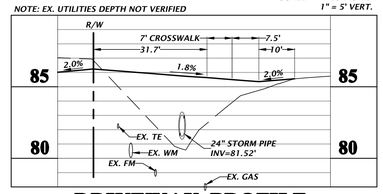
S-10; OUTFALL STRUCTURE
MODIFIED FDOT TYPE 'C' INLET PER FDOT INDEX 425-052

NTS

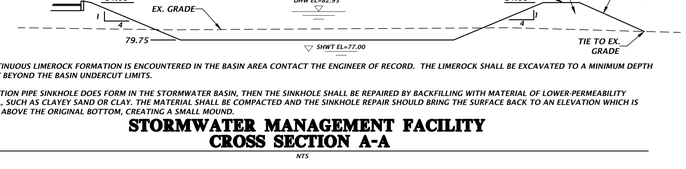


TYPICAL GRASS SWALE SECTION

NTS



DRIVEWAY PROFILE
X-X



1. IF A CONTINUOUS LIMEROCK FORMATION IS ENCOUNTERED IN THE BASIN AREA CONTACT THE ENGINEER OF RECORD. THE LIMEROCK SHALL BE EXCAVATED TO A MINIMUM DEPTH OF 3 FEET BEYOND THE BASIN UNDERCUT LIMITS.
2. IF A SOLUTION PIPE SINKHOLE DOES FORM IN THE STORMWATER BASIN, THEN THE SINKHOLE SHALL BE REPAIRED BY BACKFILLING WITH MATERIAL OF LOWER-PERMEABILITY MATERIAL, SUCH AS CLAYEY SAND OR CLAY. THE MATERIAL SHALL BE COMPACTED AND THE SINKHOLE REPAIR SHOULD BRING THE SURFACE BACK TO AN ELEVATION WHICH IS SLIGHTLY ABOVE THE ORIGINAL BOTTOM, CREATING A SMALL MOUND.

STORMWATER MANAGEMENT FACILITY
CROSS SECTION A-A

NTS

11801 Research Drive
Alachua, Florida 32615
(352) 331-1976
www.chw-inc.com
est. 1988 FLORIDA CA-9075
CHW
Professional Consultants

SCALE: 1"=20'

VERIFY SCALE BAR IS ONE INCH ON ORIGINAL DRAWING. IF NOT ONE INCH ON THIS SHEET ADJUST SCALES ACCORDINGLY.

CONSTRUCTION BID REVISIONS

SUBMITTALS:
1/28/21 COA, SRWMD, AND FDOT
2/16/21 COA, SRWMD
3/18/21 FDOT
3/25/21 COA, SRWMD

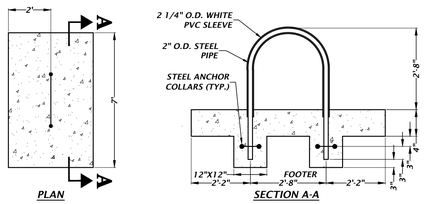
CLIENT: DE FOODS, LLC
PROJECT: I-FC ALACHUA, FL
SHEET TITLE: DETAILED GRADING AND DRAINAGE PLAN

TECHNICIAN: T.F. COWART
DESIGNER: C. MCDONNETT, E.I.
CHECKED BY: D.H. YOUNG, P.E.
PROJECT NUMBER: 20-0113

DANIEL H. YOUNG
FL PE No. 70780

SHEET NO: C2.10

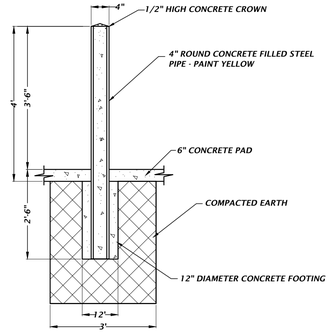
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PLAN SECTION A-A

U-SHAPE BIKE RACK

NTS

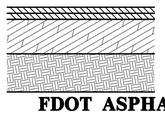


PIPE BOLLARD DETAIL

NTS

3" TYPE SP-12.5 TRAFFIC LEVEL D (PG 76-22) ASPHALTIC CONCRETE
10" LIMEROCK BASE (2-5" LIFTS) MIN. LBR 100 COMPACTION 98% AASHTO T-180
12" STABILIZED TYPE B SUBGRADE MIN. LBR 40 COMPACTION 98% AASHTO T-180

FDOT ASPHALT SHOULDER PAVMENT

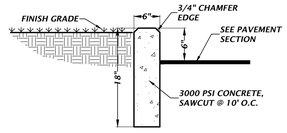


3" TYPE (2-1.5" LAYERS) SP-12.5 ASPHALTIC CONCRETE
10" LIMEROCK BASE MIN. LBR 100 COMPACTION 98% AASHTO T-180
12" STABILIZED TYPE B SUBGRADE MIN. LBR 40 COMPACTION 98% AASHTO T-180

FDOT ASPHALT PAVEMENT

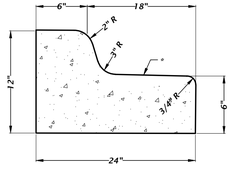
FDOT ASPHALT PAVEMENT DETAILS

NTS



TYPICAL 6" CURB DETAIL

NTS

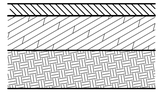


NOTE:

* WHEN USED ON THE HIGH SIDE OF ROADWAY, THE CROSS-SLOPE OF THE GUTTER SHALL MATCH THE CROSS-SLOPE OF THE ADJACENT PAVEMENT AND THE THICKNESS OF THE LIP SHALL BE 6".

TYPE 'F' SPILL CURB AND GUTTER DETAIL

NTS

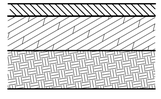


1.5" TYPE SP-12.5 ASPHALTIC CONCRETE
6" LIMEROCK BASE COMPACTION (98%) AASHTO T-180
12" STABILIZED TYPE B SUBGRADE MIN. LBR 40 COMPACTION 98% AASHTO T-180

NOTE: IF EXPANSIVE SOILS ARE ENCOUNTERED MIN. 24" SEPARATION FROM THE BASE COURSE, REFER TO GEOTECHNICAL REPORT FOR MEDIATION.

STANDARD DUTY ASPHALT PAVEMENT DETAIL

NTS

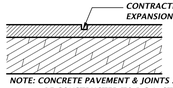


2" TYPE SP-12.5 ASPHALTIC CONCRETE
8" LIMEROCK BASE COMPACTION (98%) AASHTO T-180
12" STABILIZED TYPE B SUBGRADE MIN. LBR 40 COMPACTION 98% AASHTO T-180

NOTE: IF EXPANSIVE SOILS ARE ENCOUNTERED MIN. 24" SEPARATION FROM THE BASE COURSE, REFER TO GEOTECHNICAL REPORT FOR MEDIATION.

HEAVY DUTY ASPHALT PAVEMENT DETAIL

NTS



NOTE: CONCRETE PAVEMENT & JOINTS SHALL BE CONSTRUCTED TO P.C.A. STANDARDS.

CONCRETE PAVEMENT DETAIL

NTS

FLOW STREAMS

- P–W — POTABLE WATER
- WW– FM — WASTEWATER FORCE MAIN
- WW–GR — WASTEWATER GRAVITY

LINE LEGENDS

- TRACER WIRE
- SUBDIVISION BOUNDARY OR PROPERTY LOT BOUNDARY
- TRACER WIRE CONNECTOR
- GROUND WIRE

SYMBOLS

- APG — ACCESS POINT WITH PROVISIONS TO GROUND TRANSMITTER
- MH — MANHOLE
- APNG — GENERAL VALVE WITH VALVE BOX USED AS ACCESS POINT. (NON-GROUNDED)
- LS — LIFT STATION
- MB — METER BOX
- FH — FIRE HYDRANT

GENERAL NOTE(S):

1. RIGHT-OF-WAY AND PUBLIC UTILITY EASEMENTS NOT SHOWN.
2. THESE CONFIGURATION DETAILS DO NOT STAND ALONE. SEE RELATED TRACER WIRE DETAILS.
3. TRACER WIRE CONNECTORS SHALL BE PRO-LINE TRACER-LOCK CONNECTORS, PART #TL-LUG-SS.
4. TRACER WIRE REQUIRED FOR BOTH DUCTILE IRON AND PVC PIPE.

321A TRACER WIRE CONFIGURATION: LEGENDS AND ABBREVIATIONS

NTS

PAINT SYSTEM NO. 5

SURFACE PREPARATION -PRESSSURE WASH SURFACE TO BE PAINTED

OPTIMAL APPLICATION CONDITIONS

AMBIENT TEMPERATURE BETWEEN 55 AND 75 DEGREES F.

HUMIDITY BELOW 60 PERCENT

IF OUTSIDE THESE LIMITS, REQUEST PERMISSION TO APPLY PAINT FROM COA INSPECTOR OR SERVICES DURING CONSTRUCTION ENGINEER

APPLY (SPRAY) FIRST COAT

APPLY (SPRAY) SECOND COAT EITHER WITHIN TWO HOURS OR MORE THAN 48 HOURS AFTER APPLYING FIRST COAT.

PRODUCT

KRYLON SPAY PAINT

FUSION ALL-IN-ONE

PAINT PLUS PRIMER

COLORS

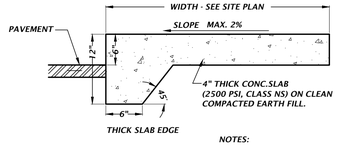
POTABLE WATER - GLOSS PATRIOTIC BLUE

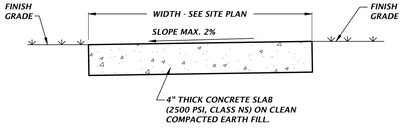
WASTEWATER - GLOSSY SPRING GREEN

RECLAIMED - GLOSSY ICY GRAPE

307 PAINT SYSTEM NO. 5

NTS





NOTES:

1. SAWCUT CONTROL JOINTS SHALL BE CONSTRUCTED 5 FEET ON CENTER
2. EXPANSION JOINTS WITH PREFORMED JOINT FILLER SHALL BE CONSTRUCTED BETWEEN ALL FIXED OBJECTS AND WALK AND AT CONSTRUCTION JOINTS.

ADJACENT TO PAVEMENT — NTS

NOT ADJACENT TO PAVEMENT — NTS

CONCRETE SIDEWALK DETAILS

11801 Research Drive
Alachua, Florida 32615
352.331.1976
www.chw-inc.com
est. 1988 FLORIDA CA-9075
Professional Consultants

SCALE: N/A

VERIFY SCALE
BAR IS ONE INCH ON ORIGINAL DRAWING
0 1"
IF NOT ONE INCH ON THIS SHEET ADJUST SCALES ACCORDINGLY

CONSTRUCTION/BID REVISIONS

SUBMITTALS
1/28/21 COA, SRWMD, AND FDOT
3/10/21 COA, SRWMD
3/18/21 FDOT
3/25/21 COA, SRWMD

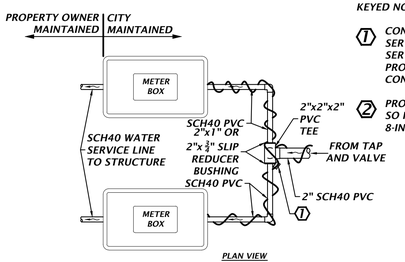
CLIENT: DE FOODS, LLC.
PROJECT: KFC ALACHUA, FL
SHEET TITLE: CONSTRUCTION DETAILS

TECHNICIAN: T.F. COWART
DESIGNER: C.MEECHENETT, E.I.
QUALITY CONTROL: D.H. YOUNG, P.E.
PROJECT NUMBER: 20-0113

DANIEL H. YOUNG
FL PE No. 70780

SHEET NO.: C2.30

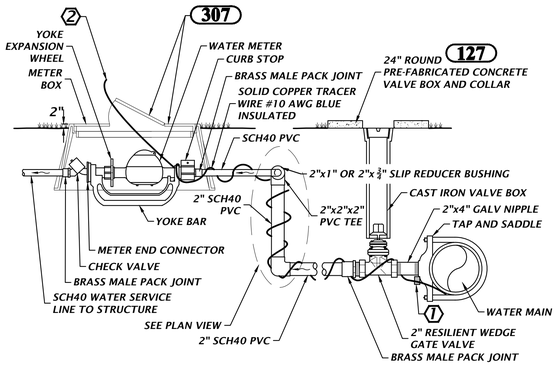
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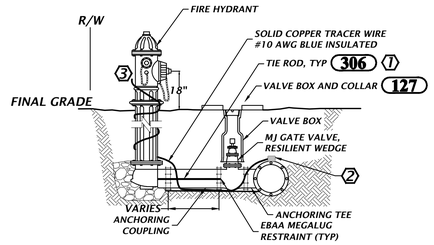
PLAN VIEW

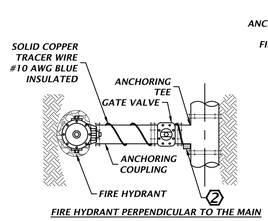
KEYED NOTE(S):

1. CONNECT TRACER WIRES FROM SERVICE TO MAIN AND FROM SERVICE TO SERVICE WITH PRO-LINE TRACER-LOCK CONNECTORS. PART # TL-LUG-SS.
2. PROVIDE ENOUGH TRACER WIRE SO IT CAN EXTEND AT LEAST 8-INCHES ABOVE METER BOX.

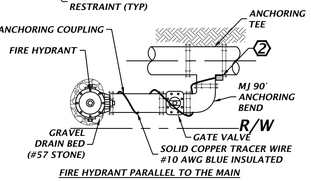


130 DUAL WATER SERVICE CONNECTION
NTS





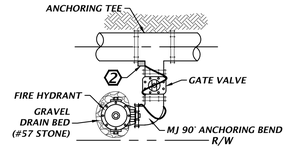
FIRE HYDRANT PERPENDICULAR TO THE MAIN



FIRE HYDRANT PARALLEL TO THE MAIN

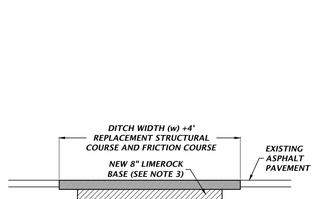
KEYED NOTE(S):

1. PROVIDE TIE RODS FOR DUAL RESTRAINT AND AS PER 306.
2. CONNECT TRACER WIRES FROM MAIN TO SERVICE WITH PRO-LINE TRACER-LOCK CONNECTORS. PART # TL-LUG-SS.
3. EXTEND TRACER WIRE AT LEAST 8-INCHES ABOVE GRADE.



FIRE HYDRANT PARALLEL TO THE MAIN

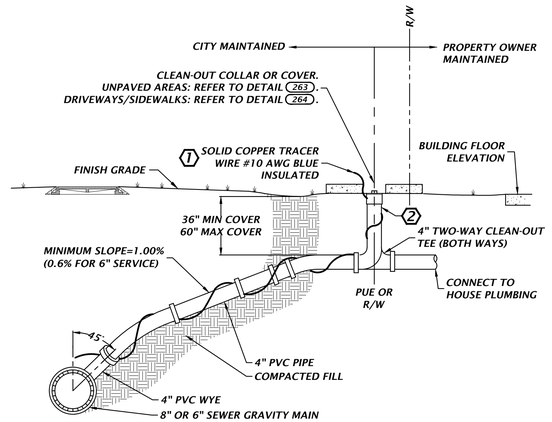
103 FIRE HYDRANT (PERPENDICULAR/ PARALLEL TO MAIN)
NTS



GENERAL NOTE(S):

1. SEE BEDDING AND TRENCHING DETAIL 303 FOR SPECIFICATIONS ON BACKFILL OVER A PIPE.
2. NEW 8" LIMEROCK BASE SHALL BE COMPACTED TO NOT LESS THAN 98% OF MAX DENSITY AS SPECIFIED BY THE FLORIDA DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATION 200-7.2. THE WIDTH OF THE LIMEROCK BASE SHALL BE THE WIDTH OF THE PIPE TRENCH PLUS 18" ON BOTH SIDES SEE DETAIL.
3. UNLESS OTHERWISE SPECIFIED, MATERIALS AND METHODS OF OPERATION REQUIRED TO INSTALL NEW AND REPLACEMENT PAVEMENT SHALL BE IN ACCORDANCE WITH THE LATEST APPLICABLE REQUIREMENTS OF THE FLORIDA DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION.
4. PAVEMENT SHALL BE REMOVED TO NEATLY SAWED STRAIGHT EDGES.
5. THE TYPE AND THICKNESS OF NEW SURFACE MATERIAL SHALL BE CONSISTENT WITH THAT OF THE EXISTING SURFACE, BUT IN ALL CASES SHALL MEET THE MINIMUM STANDARDS ESTABLISHED BY THE PLANS AND SPECIFICATIONS.
6. THE BACKFILLING AND PAVEMENT REPLACEMENT MUST BE DONE IN ACCORDANCE WITH FDOT INDEX.

300 BACKFILL OVER PIPE/PAVEMENT REPAIR DETAIL
NTS

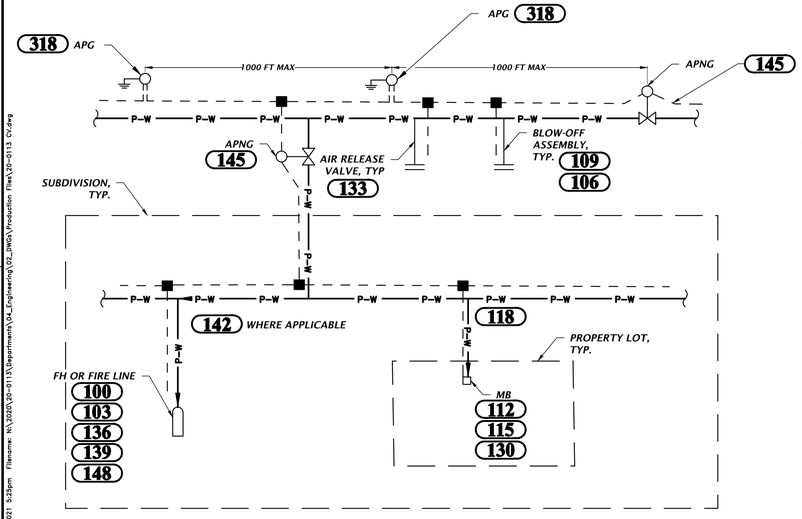


GENERAL NOTE(S):
A. SOLVENT WELD ALL PVC FITTINGS.

KEYED NOTE(S):

1. EXTEND TRACER WIRE 8-INCHES BEYOND TOP OF VALVE BOX COLLAR.
2. DO NOT CONNECT HOUSE PLUMBING TO STUB RISER.

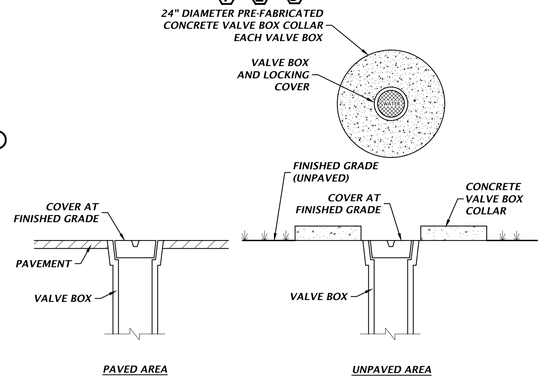
200 WASTEWATER SERVICE LATERAL
NTS



GENERAL NOTES: SEE 321A

321B POTABLE (OR RECLAIMED) TRACER WIRE CONFIGURATION
NTS

128 WHERE APPLICABLE
148 WHERE APPLICABLE



KEYED NOTE(S):

1. VALVE BOX COLLAR FEATURES:
   - 24" DIAMETER, 11" INNER OPENING
   - 4" THICK
   - TWO RINGS OF #3 REBAR
   - 4000 PSI TEST CONCRETE
2. SUPPLIER AND PRODUCT:
   - MASONRY SUPPLY, INC., SELMA, NC OR EQUAL
   - ITEM: 24" CONCRETE VALVE BOX COLLAR OR EQUAL
   - PART: MSVB-24 OR EQUAL
3. PAINT VALVE BOX COVER AND COLLAR AS PER 307

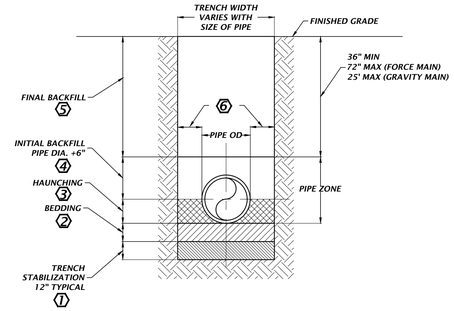
127 VALVE BOX COVER
NTS

GENERAL NOTE(S):

1. WATER SHALL NOT BE PERMITTED IN THE TRENCH DURING CONSTRUCTION.
2. ALL PIPE TO BE INSTALLED WITH BELL FACING UPSTREAM TO THE DIRECTION OF THE FLOW.
3. DEPTH FOR REMOVAL OF UNSUITABLE MATERIAL SHALL GOVERN DEPTH OF BEDDING ROCK BELOW THE PIPE. UTILITIES SHALL DETERMINE IN THE FIELD REQUIRED REMOVAL OF UNSUITABLE MATERIAL TO REACH SUITABLE FOUNDATION. THE BOTTOM OF THE TRENCH SHALL NOT BE EXCAVATED BELOW THE SPECIFIED GRADE. IF UNDERCUTTING OCCURS, THE BOTTOM OF THE TRENCH SHALL BE BROUGHT UP TO THE ORIGINAL GRADE WITH APPROVED MATERIAL AND THOROUGHLY COMPACTED TO A MINIMUM OF 95% MODIFIED PROCTOR.
4. FINAL RESTORATION IN IMPROVED AREAS SHALL BE IN COMPLIANCE WITH ALL APPLICABLE REGULATIONS OF GOVERNING AGENCIES.

KEYED NOTE(S):

1. TRENCH STABILIZATION SHALL BE PROVIDED TO A DEPTH OF 12-INCHES. THE MATERIAL SHALL BE NO. 57 STONE.
2. BEDDING SHALL BE A MINIMUM OF 6-INCHES AND COMPOSED OF IMPORTED GRANULAR FILL. COMPACT TO 95% STANDARD PROCTOR, AASHTO T-99.
3. HAUNCHING PORTION OF THE PIPE ZONE SHALL BE PLACED TO THE SPRINGLINE OF THE PIPE AND COMPOSED OF IMPORTED GRANULAR FILL. COMPACT TO 95% STANDARD PROCTOR ASSHTO T-99.
4. INITIAL BACKFILL: COMMON FILL COMPACTED TO 95% (98% UNDER PAVEMENT) OF THE MAXIMUM DENSITY AS PER AASHTO T-180. FROM THE SPRINGLINE OF THE PIPE TO TWO (2) FEET ABOVE THE PIPE THE SOIL SHALL BE CAREFULLY BACKFILLED IN 6-INCH LIFTS AND THE SOIL CONSOLIDATED WITH THE HAND OPERATED TAMPING MACHINE (OR AS REQUIRED BY CITY, COUNTY, OR STATE INSPECTORS).
5. FINAL BACKFILL: COMMON FILL COMPACTED TO 95% (98% UNDER PAVEMENT) OF THE MAXIMUM DENSITY AS PER AASHTO T-180. AFTER PLACEMENT AND COMPACTION OF THE INITIAL BACKFILL, THE BALANCE OF THE BACKFILL MATERIAL MAY BE MACHINE PLACED OR AS REQUIRED BY THE INSPECTOR AND SHALL NOT CONTAIN ANY ROCKS OR DEBRIS.
6. 12" MINIMUM



303 BEDDING AND TRENCHING DETAIL
NTS

11801 Research Drive
Alachua, Florida 32615
(386) 418-1976
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SCALE: N/A
VERIFY SCALE BAR IS ONE INCH ON ORIGINAL DRAWING. IF NOT ONE INCH ON THIS SHEET ADJUST SCALES ACCORDINGLY.
0 1"

CONSTRUCTION/BID REVISIONS

SUBMITTALS
1/28/21 COA, SRWMD, AND FDOT
2/26/21 COA, SRWMD
3/18/21 FDOT
3/25/21 COA, SRWMD

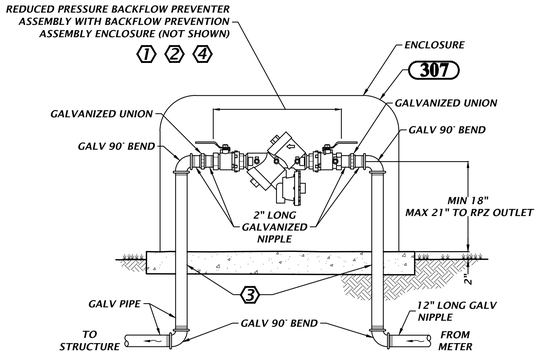
CLIENT: DE FOODS, LLC.
PROJECT: KFC ALACHUA, FL
SHEET TITLE: CONSTRUCTION DETAILS

TECHNICIAN: T.F. COWART
DESIGNER: C. MELISSA BENNETT, E.I.
QUALITY CONTROL: D.H. YOUNG, P.E.
PROJECT NUMBER: 20-0113

DANIEL H. YOUNG
FL PE No. 70780

SHEET NO.: C2.31

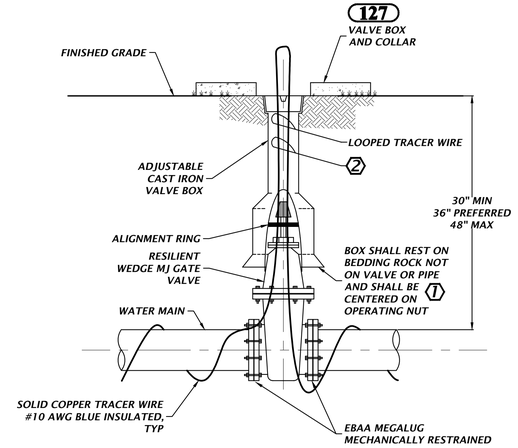
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KEYED NOTE(S):

1. PROVIDE BACKFLOW PREVENTION ASSEMBLY ENCLOSURE. COORDINATE WITH VENDOR TO SELECT ENCLOSURE SIZE SUITABLE FOR INSTALLED BACKFLOW PREVENTER ASSEMBLY.
2. ENCLOSURE VENDOR AND FEATURES:
   - MARINE GRADE ALUMINUM
   - 1.5 INCH INSULATION, 9.0 R VALUE
   - MEANS TO ACCESS ASSEMBLY
   - LOCKABLE
   - MOUNTING HARDWARE
   - COLOR: TAN, UNLESS OTHERWISE NOTED.
   - VENDORS: SAFE T COVER, NASHVILLE, TN; OR EQUAL
3. POUR CONCRETE PAD WITH DIMENSIONS AS PER VENDOR'S RECOMMENDATIONS. MINIMUM THICKNESS OF 4 INCHES.
   - PROVIDE SLEEVES AROUND PIPING
4. BACKFLOW PREVENTER FURNISHED, INSTALLED AND MAINTAINED BY PROPERTY OWNER, NOT COA.

121 3/4" TO 2" BACKFLOW PREVENTER AND ENCLOSURE
NTS



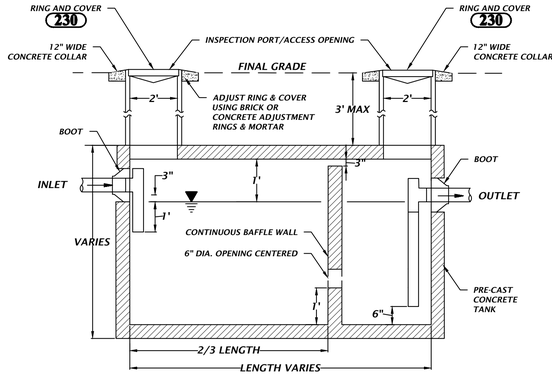
GENERAL NOTE(S):

1. PVC EXTENSIONS SHALL NOT BE USED ON VALVE BOX INSTALLATION.
2. VALVE BOX LID TO BE LETTERED WITH THE WORD "WATER" OR "RECLAIMED".

KEYED NOTE(S):

1. WHEN VALVE BOX IS TO BE INSTALLED IN ROADWAY OR OTHER TRAFFIC AREAS SET VALVE BOX ON FIVE (5) SOLID COMMON BRICKS.
2. VALVE BOX MAY SERVE AS ACCESS POINT FOR TRACER WIRES. PROVIDE ENOUGH LOOPED TRACER WIRE SO IT CAN EXTEND 8-INCHES ABOVE GRADE.

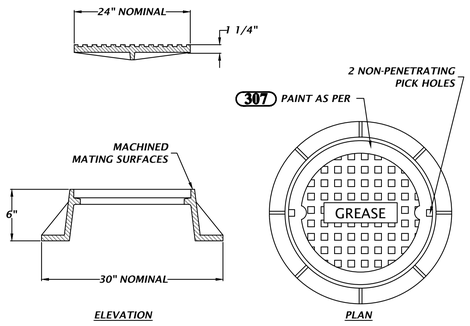
145 GATE VALVE FOR WATER AND RECLAIMED WATER MAINS 4" OR LARGER
NTS



GENERAL NOTE(S):

1. DESIGNER/OWNER SHALL SUBMIT TANK VOLUME CALCULATIONS. REQUIRED TANK VOLUME SHALL BE CONFIRMED BY CITY OF ALACHUA, 1000 GALLON MINIMUM TANK VOLUME. STRUCTURAL DESIGN SHALL BE THE RESPONSIBILITY OF THE TANK MANUFACTURER.
2. ONLY KITCHEN WASTE SHALL BE DISCHARGED INTO THE GREASE INTERCEPTOR. ALL DOMESTIC WASTE (i.e. RESTROOMS) SHALL BE CONNECTED DOWNSTREAM OF THE GREASE INTERCEPTOR.
3. MANHOLES THAT ARE NOT INSTALLED UNDER PAVEMENT SHALL HAVE A RIM ELEVATION AT LEAST 6" ABOVE FINISHED GRADE AND A 10:1 SLOPE TO FINISHED GRADE.
4. GREASE INTERCEPTOR ASSEMBLY TO BE H-20 TRAFFIC LOAD RATED - INCLUDING TANK, TANK LID, CHIMNEYS, AND MANHOLE RING & COVER.
5. NO CAPPED TEES.

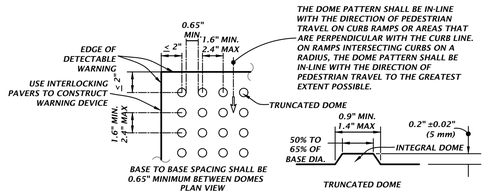
227 GREASE INTERCEPTOR
NTS



GENERAL NOTE:

1. MATERIAL: ASTM-A48 CLASS 30B GRAY IRON.
2. RING WT: 150 LBS. APP.
3. COVER WT: 130 LBS. APP.

230 RING AND COVER
NTS



1. PROVIDE DETECTABLE WARNINGS IN ACCORDANCE WITH THE AMERICANS WITH DISABILITIES ACT ACCESSIBILITY GUIDELINES (ADAAG) SECTION 705-DETECTABLE WARNINGS ON WALKING SURFACES AND THE 2017 FLORIDA BUILDING CODE, ACCESSIBILITY, CHAPTER 7, SECTION 705.
2. RAISED TRUNCATED DOMES SHALL HAVE A BASE DIAMETER FROM 0.9" MIN TO 1.4" MAX, A TOP DIAMETER FROM 50% TO 65% OF BASE DIAMETER, A HEIGHT OF 0.2 INCH (5 mm) NOMINAL, CENTER-TO-CENTER SPACING FROM 1.6" MIN TO 2.4" MAX, AND BASE-TO-BASE SPACING OF 0.65% MINIMUM, MEASURED BETWEEN THE MOST ADJACENT DOMES ON A SQUARE GRID.
3. SURFACES SHALL CONTRAST VISUALLY WITH ADJACENT WALKING SURFACES EITHER LIGHT-ON-DARK, OR DARK-ON-LIGHT.
4. ALL SIDEWALK CURB RAMPS SHALL HAVE DETECTABLE WARNING SURFACES THAT EXTEND THE FULL WIDTH OF THE RAMP AND IN THE DIRECTION OF TRAVEL 24 INCHES.
5. MATS ARE NOT ACCEPTABLE.
6. PROVIDE INTERLOCKING PAVERS AS FOLLOWS:
   A - DETECTABLE WARNING SURFACE FOR THE RAMPS SHALL CONSIST OF INTERLOCKING 4" X 8" ADA DETECTABLE WARNING SURFACE PAVERS HAVING A MINIMUM DEPTH OF 2". CONCRETE PAVERS ARE TO MEET ASTM C902 CLASS SX TYPE 1; AND BRICK PAVERS ARE TO MEET ASTM C55, GRADE N, SOLID BRICK COLOR TO MEET ADA CONTRAST REQUIREMENTS.
   B - ALL UNITS SHALL BE SOUND AND FREE OF DEFECTS THAT WOULD INTERFERE WITH THE APPEARANCE OF PROPER PLACEMENT OF THE UNIT OR IMPAIR THE STRENGTH OR LONGEVITY OF THE FINAL STRUCTURE. ANY UNITS THAT ARE STRUCTURALLY DAMAGED DURING THE WORK SHALL BE IMMEDIATELY REMOVED AND REPLACED. THE PAVERS ARE TO BE LAID IN A TWO BY TWO BASKETWEAVE PATTERN, FLUSH WITH THE FINISH GRADE OF THE RAMP SURFACE, AND HAVE GAPS BETWEEN 1/16 AND 1/8 INCH. CUT PAVERS (MASONRY SAW ONLY) SHALL BE NO SMALLER THAN ONE-THIRD OF A WHOLE PAVER.
   C - MODIFY FORMWORK OR PROVIDE FORMED GROUT INFILL BEHIND CURVED DROP CURB SECTIONS TO ENSURE STRAIGHT EDGE RESTRAINT FOR PAVERS.
   D - CONCRETE EDGE RESTRAINT FOR WARNING AREA. MAXIMUM GAP OF 1/8" BETWEEN PAVERS AND EDGE. USE ¼" RADIUS ALONG CONCRETE EDGES.
   E - WHEN PAVERS ABUT EITHER EARTH/DIRT OR PAVEMENT, A SIX INCH CONCRETE BAND SHALL BE INSTALLED. BAND SHALL BE WITHIN LIMITS OF THE WIDTH OF PROPOSED CONCRETE AND NOT EXTENDED AN ADDITIONAL SIX INCHES..
7. ALL MATERIALS/METHODS TO BE UTILIZED SHALL BE SUBMITTED FOR APPROVAL TO THE ENGINEER AND CITY OF ALACHUA PRIOR TO ORDERING THE MATERIALS/METHODS. FAILURE TO OBTAIN APPROVAL BEFORE ORDERING OR INSTALLATION MAY RESULT IN REMOVAL AND REPLACEMENT AT CONTRACTOR'S EXPENSE. ENGINEER'S APPROVAL DOES NOT RELIEVE THE CONTRACTOR'S RESPONSIBILITY FOR THE PERFORMANCE OF THE ITEM.

403 ADA DETECTABLE WARNING
NTS

11801 Research Drive
Alachua, Florida 32615
(352) 331-1976
www.chw-inc.com
est. 1988 FLORIDA CA-9075
CHW Professional Consultants

SCALE: N/A
VERIFY SCALE BAR IS ONE INCH ON ORIGINAL DRAWING. IF NOT ONE INCH ON THIS SHEET ADJUST SCALES ACCORDINGLY.
0 1"

CONSTRUCTION/BID REVISIONS

SUBMITTALS:
1/28/21 COA, SRWMD, AND FDOT
3/3/21 COA, SRWMD
3/18/21 FDOT
3/25/21 COA, SRWMD

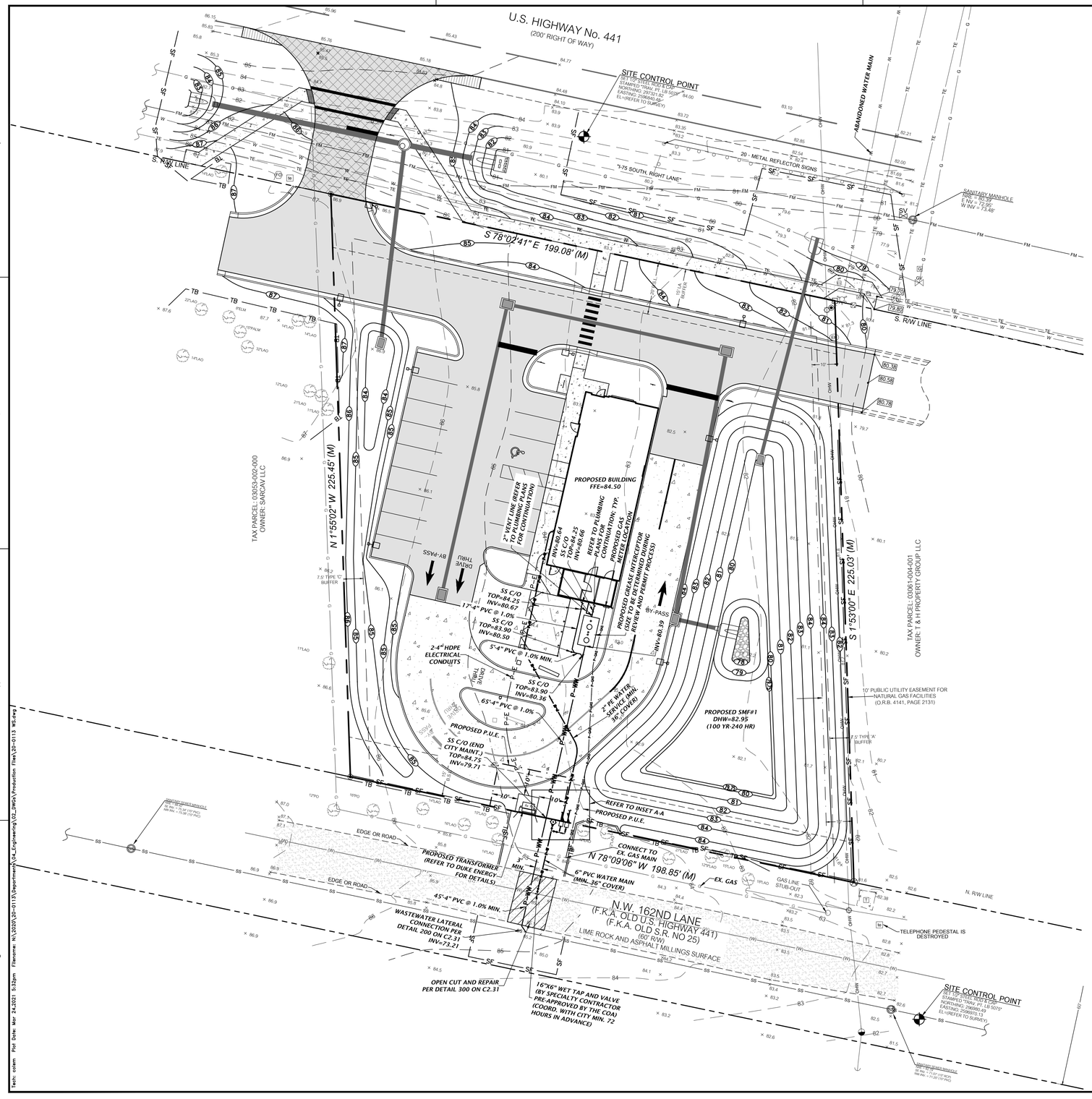
CLIENT: DE FOODS, LLC.
PROJECT: KFC ALACHUA, FL
SHEET TITLE: CONSTRUCTION DETAILS

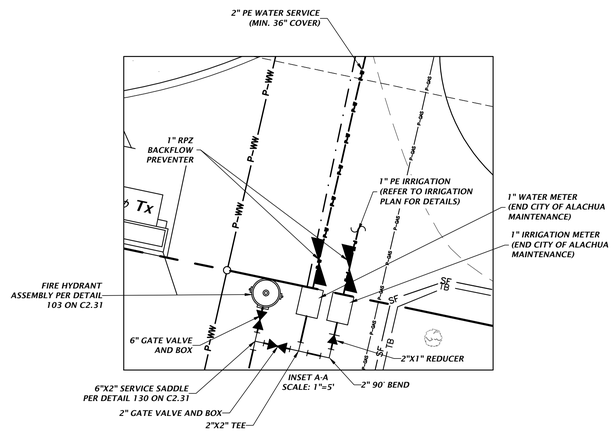
TECHNICIAN: T.F. COWART
DESIGNER: C. MEREDITH-NETT, E.I.
CONSTRUCTION OBS.: D.H. YOUNG, P.E.
PROJECT NUMBER: 20-0113

DANIEL H. YOUNG
FL PE No. 70780

SHEET NO.: C2.32

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BEFORE INSTALLING GREASE TRAP, SUBMIT SIZING CALCULATIONS, WHICH MUST BE APPROVED BY COA

GAS NOTES:
1) CONTACT GRU GAS, ERIC WILLIAMS, AT (352)393-1467 FOURTEEN (14) DAYS PRIOR TO DEMOLITION/CONSTRUCTION START.

2) MAINTAIN A MINIMUM 36" COVER OVER GAS MAINS DURING ALL PHASES OF CONSTRUCTION INCLUDING PAVEMENT AND DITCH WORK.

3) CONTACT GRU GAS FOURTEEN (14) DAYS IN ADVANCE FOR GAS SERVICE AND METER INSTALLATION.

4) DEVELOPER/CONTRACTOR IS RESPONSIBLE TO PROVIDE CASINGS AT A MINIMUM DEPTH OF 24" PRIOR TO PAVEMENT AND CONCRETE WORK.

CHW
Professional Consultants
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Alachua, Florida 32615
352.331.1976
352.331.2476
www.chw-inc.com
est. 1988 FLORIDA CA-9075

SCALE: 1"=20'
VERIFY SCALE BAR IS ONE INCH ON ORIGINAL DRAWING. IF NOT ONE INCH ON THIS SHEET ADJUST SCALES ACCORDINGLY.

CONSTRUCTION/BID REVISIONS:

SUBMITTALS:
1/28/21 COA, SRWMD, AND FDOT
3/4/21 COA, SRWMD
3/18/21 FDOT
3/25/21 COA, SRWMD

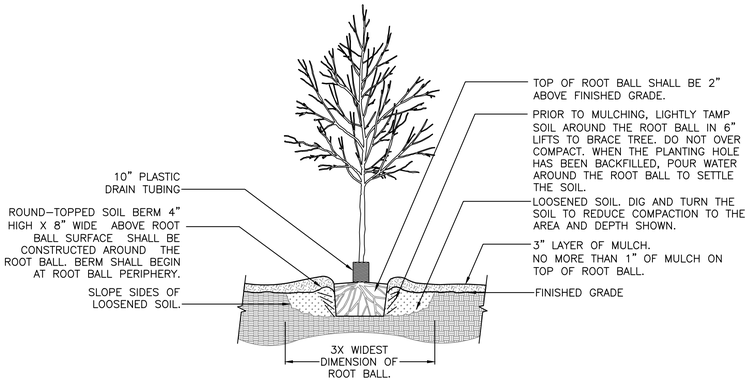
CLIENT: DE FOODS, LLC.
PROJECT: KFC ALACHUA, FL
SHEET TITLE: DETAILED UTILITY PLAN

TECHNICIAN: T.F. COWART
DESIGNER: C. MCKENNETT, E.I.
CHECKED BY: D.H. YOUNG, P.E.
PROJECT NUMBER: 20-0113

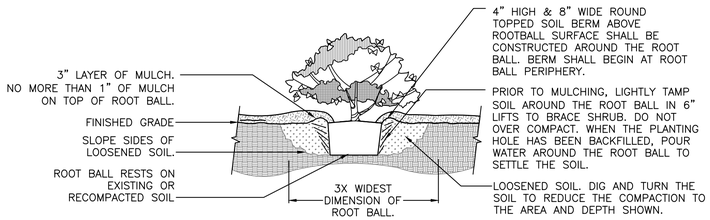
DANIEL H. YOUNG
FL PE No. 70780

SHEET NO.: C3.10

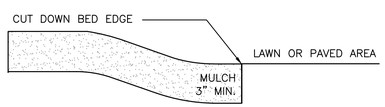
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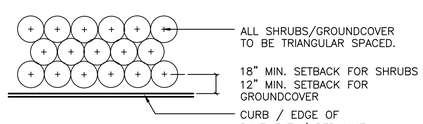
1 TREE PLANTING DETAIL
SCALE: N.T.S.



2 SHRUB PLANTING DETAIL
SCALE: N.T.S.



3 MULCH DETAIL
SCALE: N.T.S.



NOTE: THE PERIMETER OF ALL CURVED PLANTING BEDS SHALL BE PLANTED WITH A ROW OF SHRUBS AS SHOWN IN THE PLANS AND AT THE SPACING SHOWN IN THE PLANT LIST. INTERIOR PORTIONS OF EACH BED SHALL BE PLANTED AT APPROPRIATE SPACING ACCORDING TO THIS PLANT SPACING DETAIL.

4 SHRUB PLANTING DETAIL
SCALE: N.T.S.

LANDSCAPED AREA REQUIREMENT
PER CITY OF ALACHUA COMPREHENSIVE PLAN FUTURE LAND USE ELEMENT, POLICY 2.4.A

TOTAL SITE AREA	1.0 ACRES
TOTAL PROPOSED OPEN AREA	0.28 ACRES
OPEN AREA % OF TOTAL SITE AREA (MIN. 10%)	28%
TOTAL PROPOSED LANDSCAPE AREA (EXCLUSIVE OF OPEN AREA)	0.25 ACRES
LANDSCAPED AREA % OF TOTAL SITE AREA (MIN. 10%)	25%

TREE MITIGATION CALCULATIONS

CATEGORY	MITIGATION CALCULATIONS
INCH-FOR-INCH REPLACEMENT	HERITAGE TREES 44" LAO ... 44" 37" LAO ... 37" 31" LAO ... 31" 31" LAO ... 31" TOTAL INCHES ... 143" TOTAL INCHES TO BE MITIGATED AT INCH:INCH REPLACEMENT ... 143"
1:1 REPLACEMENT	TOTAL TREES TO BE MITIGATED AT 1:1 REPLACEMENT ... 126 TREES TOTAL TREES REQUIRED FOR MITIGATION @ 1:1 REPLACEMENT ... 126 TREES TOTAL INCHES REQUIRED FOR MITIGATION @ 1:1 REPLACEMENT (@ 1.5"/TREE) ... 189"
TOTAL REPLACEMENT	TOTAL MITIGATION REQUIRED ... 332" TOTAL MITIGATION PROVIDED ... 43.5" 29 TREES @ 1.5" = 43.5" TOTAL MITIGATION INCHES DEFICIT ... 288.5" TOTAL MITIGATION TREES DEFICIT (@ 1.5"/TREE) ... 193 TREES 193 REPLACEMENT TREES X $158.56/TREE* ... $30,602.08 *SEE ATTACHED TREE MITIGATION MEMO

PARKING AREA LANDSCAPE REQUIREMENTS [Sec. 6.2.2(D)(2)(a) & (b)]

DESCRIPTION	LANDSCAPE REQUIRED	LANDSCAPE PROVIDED
PARKING AREA PERIMETER LANDSCAPE	PARKING PERIMETER = 160 L.F. 4 CANOPY TREES PER 100 L.F. 2 UNDERSTORY/ORNAMENTAL PER 100 L.F. CONTINUOUS ROW OF SHRUBS 6 CANOPY TREES REQUIRED 3 UNDERSTORY TREES REQUIRED CONTINUOUS ROW OF SHRUBS	6 CANOPY TREES PROVIDED 3 UNDERSTORY TREES PROVIDED CONTINUOUS ROW OF SHRUBS PROVIDED
INTERIOR PARKING AREA LANDSCAPE	PARKING AREA = 11,960 S.F. 1 CANOPY TREE REQUIRED PER 1,800 S.F. 10 SHRUBS REQUIRED PER CANOPY TREE REQUIRED 7 CANOPY TREES REQUIRED 70 SHRUBS REQUIRED	7 CANOPY TREES PROVIDED 70 SHRUBS PROVIDED

SITE LANDSCAPE CALCULATIONS [Sec. 6.2.2(D)(1)(c)]

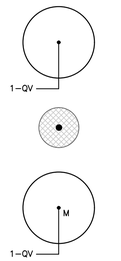
DESCRIPTION	TREE REQUIREMENTS	TREES PROVIDED
PRIMARY SIDE CANOPY TREES (NORTH SIDE)	3 TREES PER ACRE X 1.0 ACRES = 1 CANOPY TREES REQUIRED	3 CANOPY TREES PROVIDED
REAR SIDE CANOPY TREES (SOUTH SIDE)	2 TREES PER ACRE X 1.0 ACRES = 2 TREES REQUIRED	2 CANOPY TREES PROVIDED
EAST SIDE CANOPY TREES	2 TREES PER ACRE X 1.0 ACRES = 2 TREES REQUIRED	2 CANOPY TREES PROVIDED
WEST SIDE CANOPY TREES	2 TREES PER ACRE X 1.0 ACRES = 2 TREES REQUIRED	2 CANOPY TREES PROVIDED
SITE UNDERSTORY TREES	6 UNDERSTORY TREES PER ACRE X 1.0 ACRES = 6 TREES REQUIRED 50% IN FRONT = 3 TREES REQUIRED 25% ON EAST SIDE = 1 TREES REQUIRED 25% ON WEST SIDE = 2 TREES REQUIRED 6 UNDERSTORY TREES REQUIRED	FRONT SIDE 3 UNDERSTORY TREES PROVIDED EAST SIDE 1 UNDERSTORY TREES PROVIDED WEST SIDE 2 UNDERSTORY TREES PROVIDED 6 UNDERSTORY TREES PROVIDED
BUILDING FACADE	4 CANOPY TREES PER 150 LIN. FT. AND ROW OF SHRUBS REQUIRED ON EACH SIDE 30 LF NORTH FACADE = 1 TREE REQUIRED 30 LF SOUTH FACADE = 1 TREE REQUIRED 70 LF WEST FACADE = 2 TREES REQUIRED 70 LF EAST FACADE = 2 TREES REQUIRED	NORTH FACADE 1 CANOPY TREES PROVIDED SOUTH FACADE 1 CANOPY TREES PROVIDED WEST FACADE 2 CANOPY TREES PROVIDED EAST FACADE 2 CANOPY TREES PROVIDED ROW OF SHRUBS PROVIDED ON ALL SIDES

PERIMETER BUFFER LANDSCAPE REQUIREMENTS [Section 6.2.2(D)(3)]

LOCATION	BUFFER LENGTH & TYPE	LANDSCAPE REQUIRED	LANDSCAPE PROVIDED
NORTH/SOUTH/ EAST/WEST PERIMETER	810 L.F. BUFFER TYPE 'A'	OPTION 2 1 CANOPY TREE/60 LF & ONE HEDGE	14 CANOPY TREES & ONE HEDGE

KNOW WHAT'S BELOW
ALWAYS CALL 811 BEFORE YOU DIG
It's fast, it's free, it's the law.
Call 811 two business days before digging

LEGEND



GENERAL NOTES
ALL ASPHALT, LIMEROCK, AND CONSTRUCTION DEBRIS SHALL BE REMOVED FROM PLANTING BEDS AND SOD AREAS PRIOR TO LANDSCAPE INSTALLATION. IF ENCOUNTERED DURING CONSTRUCTION OR INSTALLATION, THESE SHALL BE EXCAVATED AND REMOVED FROM THE SITE. PLANTING DEPTH OF SOIL IN SUCH AREAS SHOULD BE AT LEAST 3'. IF FILL MUST BE ADDED, IT MUST BE FLORIDA CLEAN DEEP FILL (FREE OF WEED SEEDS) WITH pH 6.0 – 7.5.

ALL LANDSCAPED AREAS SHALL BE MULCHED WITH 3" THICKNESS OF MULCH. PINE BARK MULCH SHALL BE USED IN ALL AREAS.

NEW TREES OUTSIDE OF MULCHED LANDSCAPED BEDS WILL BE PROTECTED FROM MOWER AND STRING TRIMMER DAMAGE WITH 10" OF PLASTIC DRAIN TUBING.

LEADER SHOOTS AND MAIN STRUCTURAL LIMBS OF TREES SHALL NOT BE TOPPED OR PRUNED. TREES WILL BE STAKED AS NEEDED. GUYLINES WILL BE OF NON-SYNTHETIC BIODEGRADABLE MATERIAL.

REFER TO CIVIL PLANS FOR TREE BARRICADE DETAILS AND INFORMATION.

ALL PLANT MATERIAL HAS BEEN SELECTED IN ACCORDANCE WITH THE PRINCIPLES AND GUIDELINES AS DEFINED IN FLORIDA-FRIENDLY LANDSCAPING. PLANT MATERIAL SELECTION IS BASED ON SITE'S SOIL, LIGHT, WATER, AND CLIMATIC CONDITIONS AND WITH THE INTENT TO ATTRACT WILDLIFE. PLANTS WITH SIMILAR WATER NEEDS SHALL BE IN PROXIMITY OF EACH OTHER AND IRRIGATION SHALL BE ZONED APPROPRIATELY. FERTILIZER AND MULCH SHALL BE USED ACCORDING TO UF/IFAS RECOMMENDED RATES AND APPLICATION. LANDSCAPE DESIGN SHALL SEEK TO RETAIN AND USE RAINFALL AND IRRIGATION WATER WHERE POSSIBLE.

GRASSING
ALL DISTURBED AND UNPAVED AREAS TO BE GRASSED WITH SOD THAT IS FREE OF NOXIOUS WEEDS INCLUDING TROPICAL SODA APPLE OR SHALL BE SEEDED AND MULCHED. SEE CIVIL SITE PLANS FOR ADDITIONAL RELATED INFORMATION.

PLANT MATERIAL
ALL PLANT MATERIAL TO BE FLORIDA NO.1 OR BETTER, GRADED IN ACCORDANCE WITH THE STATE OF FLORIDA DEPARTMENT OF AGRICULTURE, DIVISION OF PLANT INDUSTRY'S 'GRADES AND STANDARDS FOR NURSERY PLANTS', CURRENT EDITION AT THE TIME PLANS ARE ISSUED. CALL THE CITY URBAN FORESTRY INSPECTOR AT 352-334-5023 AND LANDSCAPE ARCHITECT PRIOR TO PURCHASING ANY PLANT MATERIAL FOR AN ON SITE INSPECTION AND REVIEW OF LANDSCAPE PLAN.

PLANT QUANTITIES SHOWN ARE FOR PERMITTING PURPOSES ONLY. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO CONFIRM ALL QUANTITIES SHOWN ON THE PLAN.

10% OPEN SPACE REQUIREMENT
AS PER CITY OF ALACHUA LDR SECTION 6.7.3 (A), THE MINIMUM OPEN SPACE SET ASIDE SHALL BE 10% OF THE DEVELOPMENT SITE. REFER TO CIVIL PLANS FOR OPEN SPACE CALCULATIONS.

IRRIGATION
LANDSCAPE IRRIGATION TO BE PROVIDED BY AUTOMATIC IRRIGATION SYSTEM IN ACCORDANCE WITH CITY OF ALACHUA LDR SECTION 6.2.2(D)(6)(B)(VI).

A TEMPORARY OR PERMANENT AUTOMATIC IRRIGATION SYSTEM IS REQUIRED FOR ALL NEW REQUIRED LANDSCAPING INSTALLED [ALACHUA COUNTY LDC SEC. 407.46(A)]. TEMPORARY IRRIGATION SYSTEMS MUST BE REMOVED ONCE THE PLANTS ARE ESTABLISHED OR WITHIN TWO YEARS, WHICHEVER OCCURS FIRST. PRIOR TO THE INSTALLATION OF A NEW PERMANENT IRRIGATION SYSTEM OR SUBSTANTIAL MODIFICATION TO AN EXISTING SYSTEM, AN IRRIGATION PLAN MUST BE SUBMITTED TO AND APPROVED BY THE ALACHUA COUNTY ENVIRONMENTAL PROTECTION DEPARTMENT THROUGH THE ONLINE ALACHUA COUNTY IRRIGATION PROFESSIONAL PORTAL. THE CONTRACTOR SHALL BE RESPONSIBLE FOR SUBMITTING THE IRRIGATION PLAN THROUGH THE IRRIGATION PROFESSIONAL PORTAL. THE IRRIGATION SYSTEM SHALL COMPLY WITH THE LANDSCAPE IRRIGATION DESIGN AND MAINTENANCE STANDARDS FOUND IN ARTICLE II OF PART II, TITLE 7, CHAPTER 79 OF THE ALACHUA COUNTY CODE, WHICH, AMONG ADDITIONAL DESIGN STANDARDS, LIMITS HIGH VOLUME IRRIGATION TO 60% OF THE LANDSCAPED AREA. FOR MORE INFORMATION, CONTACT THE ALACHUA COUNTY ENVIRONMENTAL PROTECTION DEPARTMENT AT 352-264-6800.

ALL NEWLY PLANTED TREES SHALL HAVE INDIVIDUAL LOW FLOW OR MICRO-IRRIGATION SUPPLIES [407.46(A)3]. ALL IRRIGATION SYSTEMS ARE REQUIRED TO HAVE A MOISTURE SENSOR/AUTOMATIC RAINFALL SHUTOFF DEVICE [407.46(A)4].

11801 Research Drive
Alachua, Florida 32615
(352) 331-1976
www.chw-inc.com
est. 1988 FLORIDA LA0001663
CHW Professional Consultants

SCALE: N/A
VERIFY SCALE BAR IS ACCURATE FOR THIS PLOT
0 1" IF NOT ONE INCH ON THIS SHEET ADJUST SCALES ACCORDINGLY.

CONSTRUCTION/BID REVISIONS:

SUBMITTALS:
01/28/2021 - COA, SJRWMD, AND FDOT
02/17/2021 - COA, SJRWMD, AND FDOT
03/18/2021 - COA, SJRWMD, AND FDOT
03/25/2021 - COA, SJRWMD, AND FDOT

CLIENT: DE FOODS, LLC.
PROJECT: KFC ALACHUA, FL
SHEET TITLE: LANDSCAPE SCHEDULE, DETAILS, & CALCULATIONS

TECHNICIAN: L. ROSARIO
DESIGNER: L. ROSARIO
QUALITY CONTROL: C. TOLAR
PROJECT NUMBER: 20-0113

SHEET NO.: LS-1

Tech: lweber Plot Date: Mar 25,2021 8:54am Filename: K:\2020\20-0113\Departments\03_Design\02_DWGs\01_Permitting\20-0113 Design Base.dwg

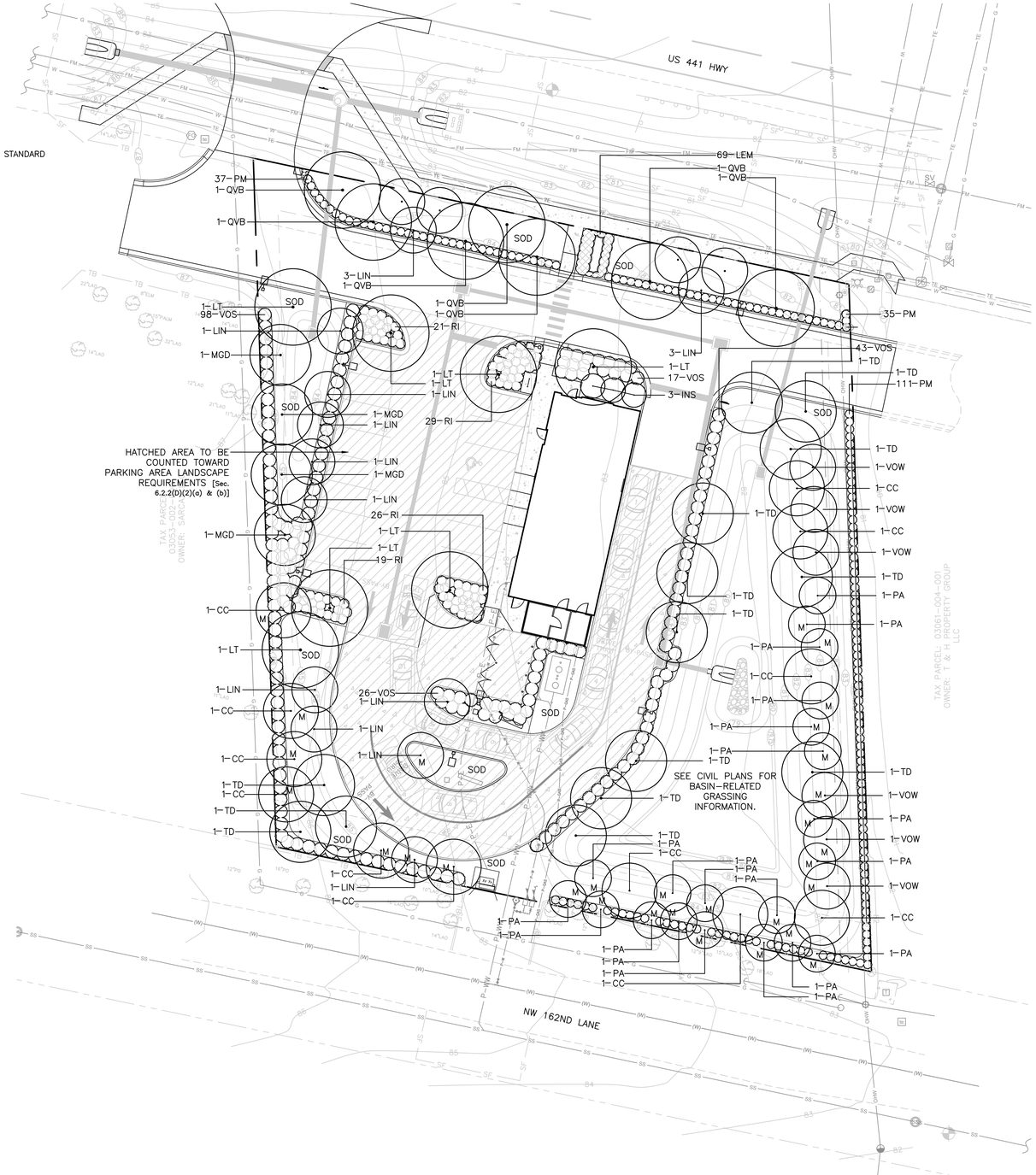
PLANT SCHEDULE

TREES	QTY	BOTANICAL NAME	COMMON NAME	SIZE
CC	12	CERCIS CANADENSIS	EASTERN REDBUD	15 GAL., 8' HT., 30" SPR., 1.5" CAL.
INS	3	ILEX X 'NELLIE R. STEVENS'	NELLIE R. STEVENS HOLLY	15 GAL, 6' HT, 30" SPR, 1.5" CAL
LIN	16	LAGERSTROEMIA INDICA 'NATCHEZ'	NATCHEZ CRAPE MYRTLE	15 GAL., 8' HT., 30" SPR., 1.5" CAL., STANDARD
LT	7	LIRIODENDRON TULIPIFERA	TULIP TREE	30 GAL, 11' HT, 3' SPR, 2" CAL
MGD	4	MAGNOLIA GRANDIFLORA 'DD BLANCHARD'	DD BLANCHARD MAGNOLIA	30 GAL, 12' HT, 42" SPR, 2" CAL
PA	20	PRUNUS ANGUSTIFOLIA	CHICKASAW PLUM	15 GAL, 6' HT, 30" SPR, 1.5" CAL
QVB	7	QUERCUS VIRGINIANA 'BOARDWALK'	BOARDWALK LIVE OAK	30 GAL, 12' HT, 54" SPR, 2" CAL
TD	14	TAXODIUM DISTICHUM	BALD CYPRESS	30 GAL, 12' HT, 42" SPR, 2" CAL
VOW	6	VIBURNUM OBOVATUM	WALTER'S VIBURNUM	15 GAL, 6' HT, 30" SPR, 1.5" CAL

SHRUBS	QTY	BOTANICAL NAME	COMMON NAME	SIZE
PM	183	PODOCARPUS MACROPHYLLUS 'PRINGLES DWARF'	DWARF PRINGLES PODOCARPUS	3 GAL, FULL
RI	95	RHAPHIOLEPIS INDICA 'ELEANOR TABOR'	ELEANOR TABOR INDIAN HAWTHORN	3 GAL., FULL
VOS	184	VIBURNUM OBOVATUM 'MRS. SCHILLER'S DELIGHT'	MRS. SCHILLERS DELIGHT VIBURNUM	3 GAL., FULL

GROUND COVERS	QTY	BOTANICAL NAME	COMMON NAME	SIZE
LEM	69	LIRIOPE MUSCARI 'EMERALD GODDESS'	EMERALD GODDESS LIRIOPE	1 GAL., FULL

SOD/SEED	BOTANICAL NAME	COMMON NAME	SIZE
SOD	PASPALUM NOTATUM 'ARGENTINE'	BAHIA GRASS	WEED FREE AND SAND GROWN SOD



GRAPHIC SCALE
0 10 20 40

N

11801 Research Drive
Alachua, FL 32615
(352) 331-1976
www.chw-inc.com
est. 1988 FLORIDA
LA0001663

CHW
Professional Consultants

SCALE: 1"=20'
VERIFY SCALE BAR IS CORRECT FOR PRINT SIZE
IF NOT ONE INCH ON THIS SHEET ADJUST SCALES ACCORDINGLY.

SUBMITTALS:
01/28/2021 - COA, SJRWMD, AND FDOT
02/09/2021 - COA, SJRWMD, AND FDOT
03/18/2021 - COA, SJRWMD, AND FDOT
03/25/2021 - COA, SJRWMD, AND FDOT

CONSTRUCTION/BID REVISIONS:

CLIENT: DE FOODS, LLC.
PROJECT: KFC ALACHUA, FL
SHEET TITLE: LANDSCAPE PLAN

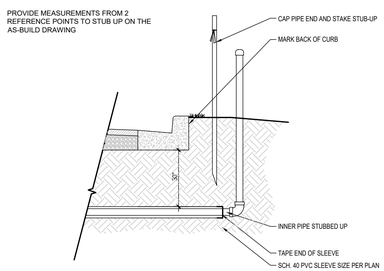
TECHNICIAN: L. ROSARIO
DESIGNER: L. ROSARIO
QUALITY CONTROL: C. TOLAR
PROJECT NUMBER: 20-0113

SHEET NO.: LS-2

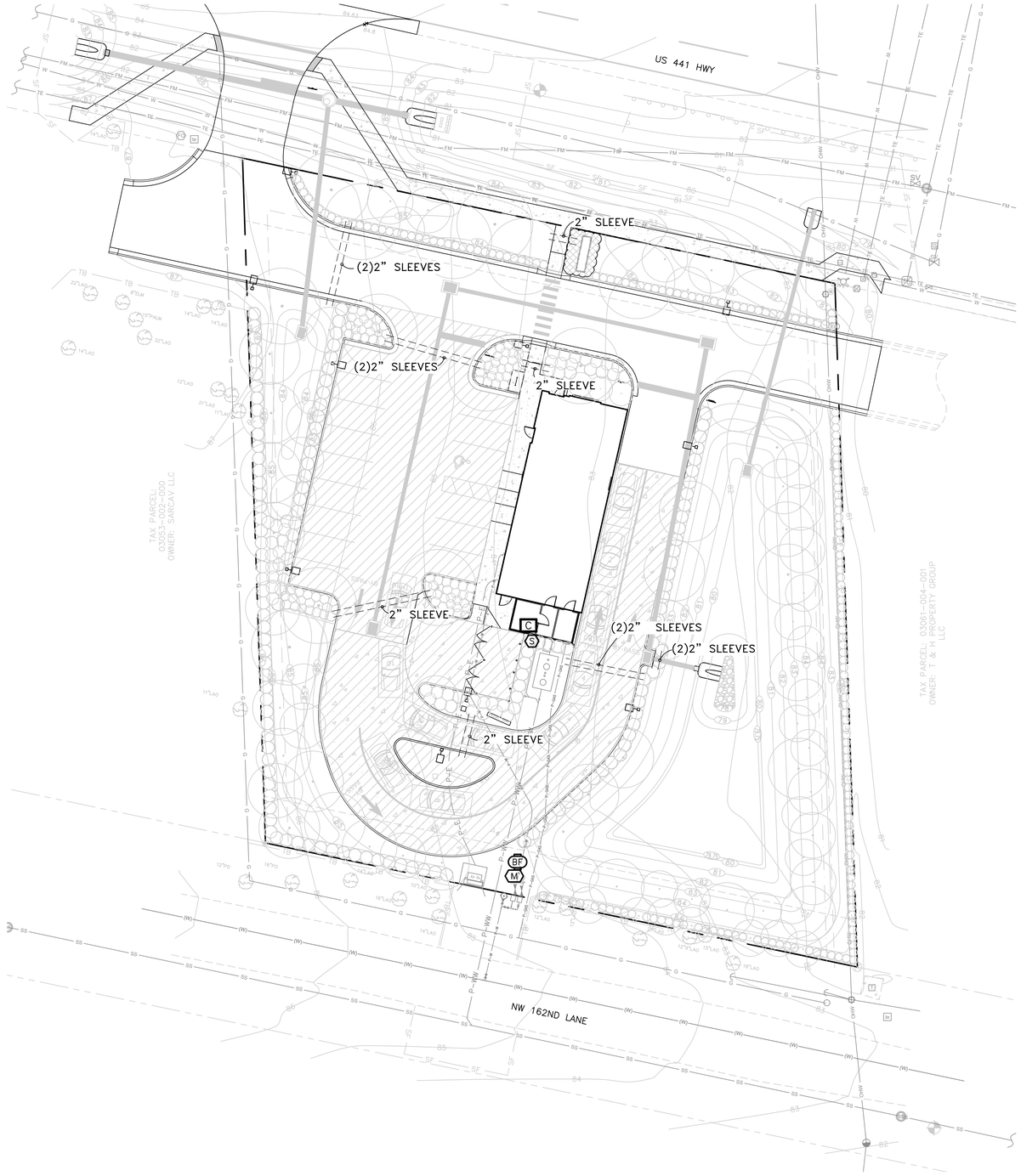
Tech: lsenor Plot Date: Mar 25,2021 8:54am Filename: H:\2020\20-0113\Department\03_Design\03_DWGs\01_Permitting\20-0113 Design Base.dwg

IRRIGATION_SCHEDULE

SYMBOL	MANUFACTURER/MODEL/DESCRIPTION
C	Hunter PC-600-PL 6 Station Controller
S	Hunter Solar-Sync Weather Sensor
M	Water Meter 1" (by others)
BF	Wilkins 975XL 1" Backflow Preventer
=========	Pipe Sleeve: PVC Schedule 40



1 SLEEVING ROUGH-IN DETAIL
N.T.S.



KNOW WHAT'S BELOW
ALWAYS CALL 811 BEFORE YOU DIG
It's fast, it's free, it's the law.
Call 811 two business days before digging

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Alachua, FL 32615
(352) 331-1976
www.chw-inc.com
est. 1988 FLORIDA
LA0001663
CHW
Professional Consultants

SCALE: 1"=20'
VERIFY SCALE
BAR IS ONE INCH ON ORIGINAL DRAWING
IF NOT ONE INCH ON THIS SHEET ADJUST SCALES ACCORDINGLY.
0 1"

CONSTRUCTION/BID REVISIONS:

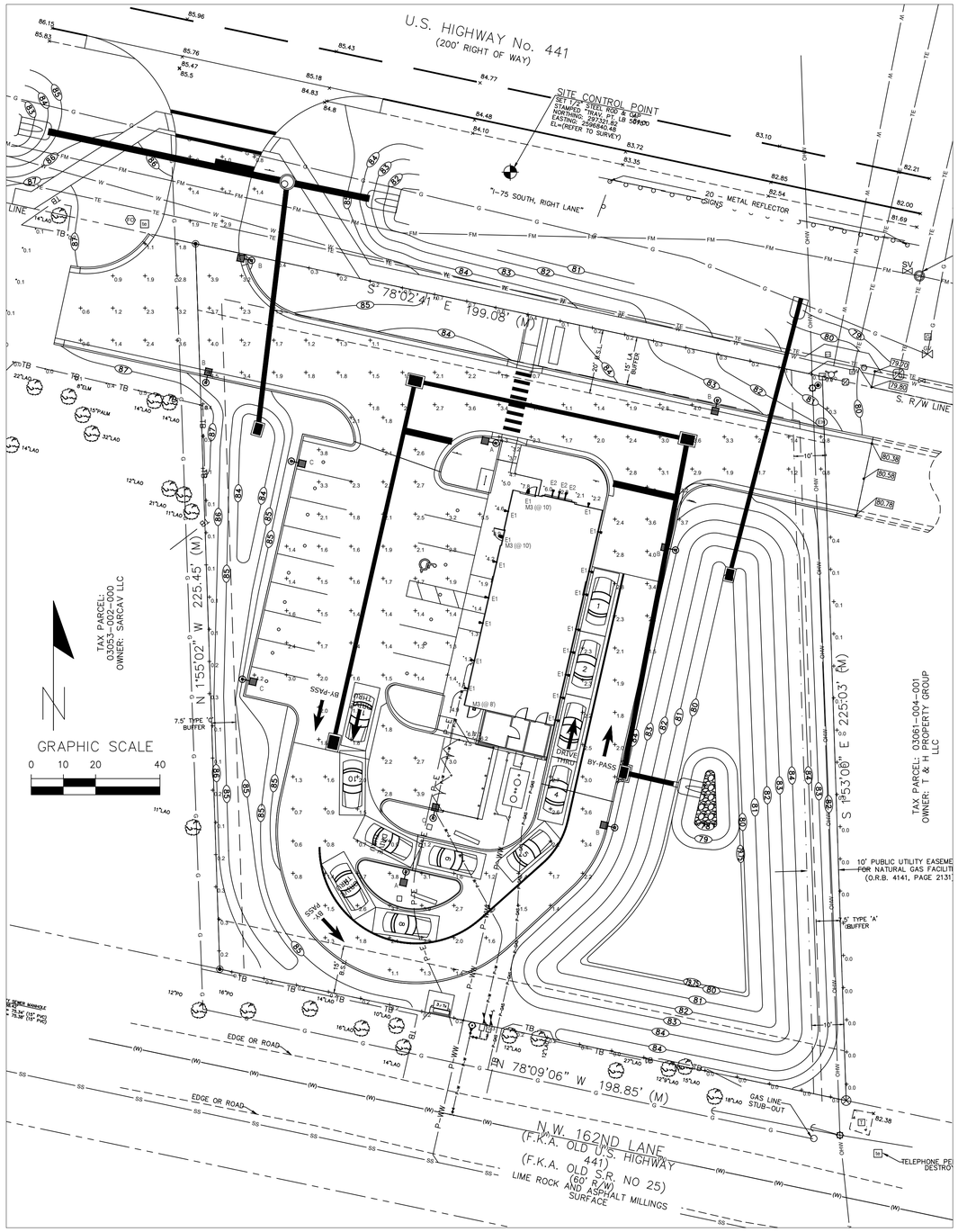
SUBMITTALS:
01/28/2021 - COA, SJRWMD, AND FDOT
02/09/2021 - COA, SJRWMD, AND FDOT
03/18/2021 - COA, SJRWMD, AND FDOT
03/25/2021 - COA, SJRWMD, AND FDOT

CLIENT: DE FOODS, LLC.
PROJECT: KFC ALACHUA, FL
SHEET TITLE: IRRIGATION DIAGRAM, SCHEDULE, & DETAIL

TECHNICIAN: L. ROSARIO
DESIGNED: L. ROSARIO
QUALITY CONTROL: C. TOLAR
PROJECT NUMBER: 20-0113

SHEET NO.: IR-1

Tech: kenor Plot Date: Mar 25,2021 8:34am Filename: N:\2020\20-0113\Departments\03_Design\01_Drwgs\01_Permitting\20-0113 Design Base-IR.dwg



Schedule

Symbol	Label	QTY	Manufacturer	Catalog Number	Description	Lamp	Number Lamps	Filename	Lumens per Lamp	Lumen Multiplier	LLF	Wattage	Distribution
	A	2	SIGNIFY GARDCO	ECF-S-32L-530-NW-G2-3	ECFORM AREA LED ECF - SMALL 32 LED's, 4000K CCT, TYPE 3 OPTIC,	2 LEDGINE SLD ARRAY(S) DRIVEN AT 530 mA	1	ECF-S-32L-530-NW-G2-3.IES	6715	1.0	1.0	55.7	III
	B	5	SIGNIFY GARDCO	ECF-S-32L-530-NW-G2-3-HIS	ECFORM AREA LED ECF - SMALL 32 LED's, 4000K CCT, TYPE 3 OPTIC, HOUSE-SIDE INTERNAL SHIELDING (HIS)	2 LEDGINE SLD ARRAY(S) DRIVEN AT 530 mA	1	ECF-S-32L-530-NW-G2-3-HIS.IES	5350	1.0	1.0	55.7	III HIS
	C	3	SIGNIFY GARDCO	ECF-S-32L-530-NW-G2-4-HIS	ECFORM AREA LED ECF - SMALL 32 LED's, 4000K CCT, TYPE 4 OPTIC, HOUSE-SIDE INTERNAL SHIELDING (HIS)	2 LEDGINE SLD ARRAY(S) DRIVEN AT 530 mA	1	ECF-S-32L-530-NW-G2-4-HIS.IES	5350	1.0	1.0	55.7	IV HIS
	E1	17	AMERLUX EXTERIOR	VARIETA FLC1-30 BLK ASSUMES 21 DEGREES AIMING TOWARDS WALL	SINGLE ARRAY ACCENT LED FLOODLIGHT, BLACK, 3000K WITH CUSTOM 36° BRACKET SEE ARCHITECTURAL PLANS	1 SINGLE COB LED	1	FLC1-30.IES	2151	1.0	1.0	24.1	WIDE SPOT
	E2	3	AMERLUX EXTERIOR	VARIETA FL1-HWS-BLK ASSUMES 21 DEGREES AIMING TOWARDS WALL	SINGLE ARRAY ACCENT LED FLOODLIGHT, BLACK, 3000K WITH CUSTOM 36° BRACKET SEE ARCHITECTURAL PLANS	SINGLE MODULE WITH 16 LEDS	1	FL1-HWS-30_AME429067 .IES	2302	1.0	1.0	31	HORIZONTAL WIDE SPOT
	M3	3	RAB LIGHTING INC	WPLED13Y	WALL SCONCE, BRONZE FINISH, 3000K, CLEAR FLAT GLASS CUTOFF FIXTURE SEE ARCHITECTURAL PLANS	SINGLE MULTICHIP LED	1	RAB02310.IES	1529	1.0	1.0	15.2	

Statistics							
Description	Symbol	Avg	Max	Min	Max/Min	Avg/Min	Applicable Code
PARKING	+	2.0 fc	5.5 fc	1.3 fc	6.0:1	2.3:1	NOTES C AND D
DRIVE, S AND E OF BUILDING	+	2.1 fc	4.0 fc	0.6 fc	6.7:1	3.5:1	NOTES C AND D
CONNECTOR ROAD	+	2.2 fc	4.5fc	0.6 fc	7.5:1	3.7:1	NOTES C AND D
PARCEL LINE BOUNDARY (SPILL)	+	N/A	0.7 fc	0.0 fc	N/A	N/A	NOTE E

NOTES:

A. THIS PLAN MEETS THE CITY OF ALACHUA, COA, DEVELOPMENT STANDARDS AND RECOMMENDED MAINTAINED HORIZONTAL FOOTCANDLE RECOMMENDATIONS OF THE ILLUMINATING ENGINEER SOCIETY OF NORTH AMERICA (IESNA) FOUND IN IESNA RP8-18 FOR PARKING LOT LIGHTING. SIDEWALKS AND AREA WITHIN US441 RIGHT OF WAY ARE NOT PART OF THIS PLAN.
B. ALL LUMINAIRES ARE FULL CUT-OFF AS PER COA DEVELOPMENT STANDARDS 6.4.4 (F)(1) .
C. LIGHTING UNIFORMITY RATIOS DO NOT EXCEED 10:1 AS PER COA 6.4.4(E)
D. MAXIMUM HORIZONTAL ILLUMINATION DOES NOT EXCEED 5 FOOTCANDLES AS PER COA 6.4.4(C)(2)
E. THE MAXIMUM LIGHT SPILL IS LESS THAN 1 FOOTCANDLE (FC) ON ADJACENT PROPERTIES. ROADWAY VALUES NOT INCLUDED.
F. THE MAXIMUM MOUNTING HEIGHT OF THE POLE MOUNTED FIXTURES IS 15' ABOVE FINISH GRADE AS PER COA 6.4.5.
G. LIGHT POLES ARE BLACK STEEL WITH 15' MOUNTING HEIGHT INCLUDING ABOVE GRADE FOUNDATION. POLE SIZE AND FOUNDATIONS MUST MEET FLORIDA BUILDING CODE WIND LOAD REQUIREMENTS. POLE LENGTH MUST TAKE INTO ACCOUNT THE HEIGHT OF THE CONCRETE FOUNDATION AND MAXIMUM MOUNTING HEIGHT ABOVE GRADE.
H. POLES ARE INSTALLED 3' FROM BACK OF CURB TO FACE OF POLE OR BACK OF SIDEWALK.
I. LIGHTS ARE CONTROLLED BY TIME CLOCK AND PHOTOCELL (INSTALLED AS PER ARCHITECTURAL BUILDING ELECTRICAL PLANS, NOT APART OF THIS PLAN).
J. FOOTCANDLE LEVELS SHOWN ARE INITIAL VALUES.
K. E1, E2 AND M3 ARE SPECIFIED IN THE ARHITECTURAL PLANS.
L. E1 AND E2 ARE BUILDING MOUNTED SPOTLIGHTS FOR ARHITECTURAL PURPOSES SO ARE NOT LIMITED TO THE 15' MAXIMUM HEIGHT. THEIR BRACKET DETAILS ARE FOUND IN THE ARCHITECTURAL PLANS.
M. CALCULATIONS ASSUME E1 &E2 FLOODS ARE TILTED AT 45 DEGREES TOWARDS WALLS.

GARDCO by Signify

Site & Area
EcoForm
ECF-S small area light



Gardco EcoForm Gen-2 combines economy with performance in an LED area luminaire. Capable of delivering up to 27,800 lumens or more in a compact, low profile LED luminaire, EcoForm offers a new level of customer value. EcoForm features an innovative retrofit arm kit, simplifying site conversions to LED by eliminating the need to drill additional holes in most existing poles. Integral control systems available for further energy savings. Includes Service Tag, our innovative way to provide assistance throughout the life of the product.

Project:
Location:
Cat.No:
Type:
Lamps: Qty:
Notes:

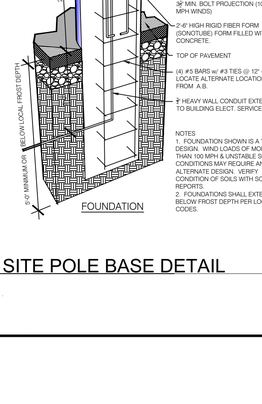
Ordering guide — example: ECF-S-64L-900-NW-G2-AR-S-120-HIS-MGY

Prefix	Number of LEDs	Drive Current	LED Color-Generation	Mounting	Distribution	Voltage	Options: Dimming controls	Motion sensing-less	Photo-sensing	Electrical	Luminaire	Finish
ECF-S												
ECF-S EcoForm site and area, small	32L 32 LEDs (2 modules)	365 365mA 530 530mA 700 700mA 1A 1050mA 1.2A 1200mA	WW-G2 Warm White 3000K, 70CRI Generation 2 NW-G2 Neutral White 4000K, 70CRI Generation 2 CW-G2 Cool White 5000K, 70CRI Generation 2	XAT Arm Mount standard[1] The following mounting kits must be ordered separately (See accessories): SF Slip Fitter Mount[1] (fits to 2⅜" O.D. tenon) WS Wall mount with surface conduit rear entry permitted RAM Retrofit arm mount kit[1]	Type 2 2 Type 2 2-90 Rotated left 90° 2-270 Rotated right 270° Type 3 3 Type 3 3-90 Rotated left 90° 3-270 Rotated right 270° Type 4 4 Type 4 4-90 Rotated left 90° 4-270 Rotated right 270° Type 5 5 Type 5 SW Type SW AFR Auto Front Row AFR-90 Auto Front Row, Rotated left 90° AFR-270 Auto Front Row, Rotated right 270°	120 120V 208 208V 240 240V 277 277V 347 347V 480 480V UNV 120-277V (50/60Hz) HVU 347-480V (50/60Hz)	DD 0-10V External dimming (by others)[1] DCC Dual Circuit Control[1,11] FAWS Field Adjustable Wattage Selector[1,2] SW Interface module for SiteWise[1,11] LLC Integral wireless module[1,6,9,11] BL Bi-level functionality[1,17] DynaDimmer: Automatic Profile Dimming CS50 Safety 50% Dimming, 7 hours[1,9] CM50 Median 50% Dimming, 8 hours[1,9] CS30 Safety 30% Dimming, 7 hours[1,9] CM30 Median 30% Dimming, 8 hours[1,9]	IMR3 Integral with #3 lens[16] IMR7 Integral with #7 lens[16]	PCB Photocontrol Button[14] TLR05 Twist Lock Receptacle 5 Pin[14] TLR07 Twist Lock Receptacle 7 Pin[14] TLRPC Twist Lock Receptacle w/ Photocell[14]	Fusing F1 Single (120, 277, 347VAC)[1] F2 Double (208, 240, 480VAC)[1] Pole Mount Fusing FP1 Single (120, 277, 347VAC)[1] FP2 Double (208, 240, 480VAC)[1] FP3 Canadian Double Pull (208, 240, 480VAC)[1] Surge Protection (SPA standard) SP2 Increased 20kA	Square Pole Adapter included in standard product TB Terminal Block[3] RPA Round Pole Adapter (fits to 3"-3.9" O.D. pole)[15] HIS Internal Housing Side Shield[16]	Textured BK Black WH White BZ Bronze DGY Dark Gray MGY Medium Gray Customer specified RAL Specify optional color or RAL (ex: RAL7024) CC Custom color (Must supply color chip for required factory quote)
	64L 64 LEDs (4 modules)	900 900mA 1A 1050mA										
	48L 48 LEDs (3 modules)	900 900mA 1A 1050mA 1.2A 1200mA										

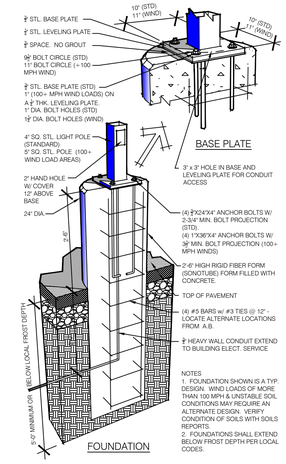
A&B
C
B&C

1. BL=IMR3/7 equipped with out-boarded sensor housing when voltage is HVU (347-480V)
2. Mounts to a 4" round pole with adapter included for square poles.
3. Limited to a maximum of 45 degrees aiming above horizontal.
4. Not available with other dimming control options.
5. Not available with motion sensor.
6. Not available with photocontrol.
7. Available only in 120 or 277V.
8. Not available in 347 or 480V
9. Must specify input voltage.
10. Dimming will not be connected to NEMA receptacle if ordering with other control options.
11. Not available in 480V. Order photocell separately with TLR05/7.
12. Not available with DCC.
13. Not available with SF and WS. RPAs provided with black finish standard.
14. HIS not available with Type 5 and SW optics.
15. Not available with DD, DCC, and FAWS dimming control options.
16. Not available with DD, DCC, FAWS and LLC dimming control options.
17. Must specify a motion sensor lens.

ECF-S_EcoForm_area_small 06/20 page 1 of 8



LIGHT POLE DETAIL
NOT TO SCALE



SITE POLE BASE DETAIL

WILLIAM T. STORMANT PE
ELECTRICAL ENGINEER
5304 NW 173 STREET
ALACHUA, FLORIDA 32615
STORMANTWT@COX.NET
(352) 665-1426

SCALE: VERIFY SCALE BAR IS ONE INCH ON ORIGINAL DRAWING. 0 1" IF NOT ONE INCH ON THIS SHEET ADJUST SCALES ACCORDINGLY.

SUBMITTALS: CONSTRUCTION/BID REVISIONS
3/18/21REVISIONS

CLIENT: ::
PROJECT: QSR IN ALACHUA
SHEET TITLE: PHOTOMETRICS

TECHNICIAN: WILLIAM T. STORMANT
DESIGNER: W STORMANT
QUALITY CONTROL: W STORMANT
PROJECT NUMBER: 20-22

FL PE No. 44156

SHEET NO.: P1

Tech: storm_l Plot Date: Mar 18,2021 3:29pm Filename: C:\Users\storm_l\Documents\QSR ALACHUA\QSR RPC Lighting 03-18-21.dwg

Varieta

Single COB LED Flood Light



FL01.x.x.BLK

amerlux
A Delta Group Company

Varieta (COB) Flood Light (FLC1)

Features

The Amerlux Varieta flood luminaire employs solid state technology and precision engineering to provide LED solutions for exterior signage, wall washing, and landscape layouts. All components are encapsulated inside a single attractive enclosure designed for superior performance in weather resistant applications. Offered in a choice of finishes, and color temperatures, three mounting options and glare shield are also available.

Product Overview

Type:	Exterior Flood Lighting
Wattage:	24W
Lumen Output :	2,500 lm
Color Temp:	3000K, 4000K, 5000K
Dimming:	0-10V

PROJECT:

TYPE:





Application:
Exterior flood lighting for signage, wall washing, and landscape layouts.

Construction:
- Die-cast aluminum faceplate
- Extruded aluminum housing
- IP67 sealed optical module

Optics:
- Lumen maintenance: 70% @ 60,000 hours
- Single COB LED is available in various color temperatures
- Lens provides NEMA 7x6 distribution pattern

Electrical:
- Integral driver
- Input voltage 120V-277V auto-sensing
- Power consumption 24W
- 1/2" NPT wire entry
- 0-10V dimming is standard

ETL listed, suitable for wet locations.

Mounting:
- Cast aluminum pivot
- 180° tilt, 360° rotation

Mounting Accessories:
- **GSP17** - Ground Stake
- **PMBS-2-18** - Pole Mount
- **WMS14** - Wall Mount

Accessories:
- **EGS** - External Glare Shield

Finish:
Premium quality thermoset polyester powdercoat for a durable finish.

Ambient Operating Range:
-30°C to 40°C

Weight:
5 lbs

EPA:
0.25

5 year limited warranty

Amerlux reserves the right to change details that do not affect overall function and performance.
Amerlux®, LLC - 178 Bauer Drive, Oakland, NJ 07436 - P: 973-882-5060 F: 973-882-3806 - amerlux.com

Varieta

Single Array LED Flood Light



FL1.x.x.BLK

amerlux
A Delta Group Company

Varieta Flood Lights (FL1)

Features

The Amerlux Varieta flood luminaire employs solid state technology and precision engineering to provide LED solutions for exterior signage, wall washing, and landscape layouts. All components are encapsulated inside a single attractive enclosure designed for superior performance in weather resistant applications. Offered in a choice of finishes, and color temperatures, three mounting options and glare shield are also available.

Product Overview

Type:	Exterior Flood Lighting
Wattage:	31W
Lumen Output :	3,170 lm
Color Temp:	3000K, 4000K, 5000K
Dimming:	0-10V

PROJECT:

TYPE:





Application:
Exterior flood lighting for signage, wall washing, and landscape layouts.

Construction:
- Die-cast aluminum faceplate
- Extruded aluminum housing
- IP67 sealed optical module

Optics:
- Lumen maintenance: 70% @ 60,000 hours
- 1 lens module containing 16 LEDs available in a number of symmetric or asymmetric axial beam spread patterns

Electrical:
- Integral driver
- Input voltage 120V-277V auto-sensing
- Power consumption 31W
- 1/2" NPT wire entry
- 0-10V dimming is standard

ETL listed, suitable for wet locations.

Mounting:
- Cast aluminum pivot
- 180° tilt, 360° rotation

Mounting Accessories:
(order separately)
- **GSP17** - Ground Stake
- **PMBS-2-18** - Pole Mount
- **WMS14** - Wall Mount
- **JBOX** - Junction Box
- **JCOV** - Junction Mount

Accessories:
- **EGS** - External Glare Shield

Finish:
Premium quality thermoset polyester powdercoat for a durable finish.

Ambient Operating Range:
-30°C to 40°C

Weight:
5 lbs

EPA:
0.25

5 year limited warranty

Amerlux reserves the right to change details that do not affect overall function and performance.
Amerlux®, LLC - 178 Bauer Drive, Oakland, NJ 07436 - P: 973-882-5060 F: 973-882-3806 - amerlux.com

Varieta TYPE E1

Single COB LED Flood Light

amerlux
A Delta Group Company

Varieta (COB) Flood Light (FLC1)

PROJECT:

TYPE:

Luminaire Ordering Information

FLC1 _____ . _____ . _____ . _____

1 **2** **3** **4**

1 Model
FLC1

2 Color Temp
30 - 3000K
40 - 4000K
50 - 5000K

3 Finish
BLK - Satin Black
CLB - Classic Bronze
GRN - Green
TBK - Textured Black
WHT - White
OSTM - Custom

4 Accessories
EGS - External Glare Shield

Varieta TYPE E2

Single Array LED Flood Light

amerlux
A Delta Group Company

Varieta Flood Light (FL1)

PROJECT:

TYPE:

Luminaire Ordering Information

FL1 _____ . _____ . _____ . _____ . _____

1 **2** **3** **4** **5**

1 Model
FL1

2 Light Distribution
NSP - Narrow Spot
HMS - Horizontal Medium Spot
VMS - Vertical Medium Spot
HWS - Horizontal Wide Spot
VWS - Vertical Wide Spot
SQS - Square Spot
MFL - Medium Flood
HMF - Horizontal Medium Flood
VMF - Vertical Medium Flood
HWF - Horizontal Wide Flood
VWF - Vertical Wide Flood
SQF - Square Flood

3 Color Temp
30 - 3000K
40 - 4000K
50 - 5000K

4 Finish
BLK - Satin Black
CLB - Classic Bronze
GRN - Green
TBK - Textured Black
WHT - White
OSTM - Custom

5 Accessories
EGS - External Glare Shield

WPLED13

RAB



Project:	Type:
Prepared By:	Date:

LED 10W & 13 Wall packs. patent-pending thermal management system. 100,000 hour L70 lifespan. 5-year, no-compromise warranty.

Color: Bronze Weight: 4.4 lbs

Driver Info

Type	Constant Current
120V	0.13A
208V	0.08A
240V	0.07A
277V	0.06A
Input Watts	15.2W

LED Info

Watts	13W
Color Temp	5000K (Cool)
Color Accuracy	73 CRI
L70 Lifespan	100,000 Hours
Lumens	1,530
Efficacy	100.7 lm/W

Technical Specifications

Compliance

UL Listed:
Suitable for Wet Locations as a Downlight. Suitable for Damp Locations as an Uplight. Wall Mount only. Suitable for Mounting within 4ft. of ground.

IESNA LM-79 & IESNA LM-80 Testing:
RAB LED luminaires and LED components have been tested by an independent laboratory in accordance with IESNA LM-79 and LM-80.

Performance

Lifespan:
100,000-Hour LED lifespan based on IES LM-80 results and TM-21 calculations

Construction

Cold Weather Starting:
Minimum starting temperature is -40°C (-40°F)

Maximum Ambient Temperature:
Suitable for use in up to 55°C (131°F)

Finish:
Formulated for high durability and long-lasting color

Green Technology:
Mercury and UV free. RoHS-compliant components.

Gaskets:
High-temperature Silicone

Electrical

Driver:
Multi-chip 13W high output long life LED Driver Constant Current, Class 2 100V - 277V, 50/60 Hz

THD:
10.4% at 120V, 16.7% at 277V

Power Factor:
98.8% at 120V, 92.6% at 277V

Surge Protection:
4kV

LED Characteristics

Color Temperature:
5000K

Lumen Maintenance:
The LED will deliver 70% of its initial lumens at 100,000 hours of operation

Need help? Tech help line: **(888) 722-1000** Email: **custserv@rablighting.com** Website: **www.rablighting.com**
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Schedule

Symbol	Label	QTY	Manufacturer	Catalog Number	Description	Lamp	Number Lamps	Filename	Lumens per Lamp	Lumen Multiplier	LLF	Wattage	Distribution
	A	2	SIGNIFY GARDCO	ECF-S-32L-530-NW-G2-3	ECFORM AREA LED ECF - SMALL 32 LED's, 4000K CCT, TYPE 3 OPTIC,	2 LEDGINE SLD ARRAY(S) DRIVEN AT 530 mA	1	ECF-S-32L-530-NW-G2-3.IES	6715	1.0	1.0	55.7	III
	B	5	SIGNIFY GARDCO	ECF-S-32L-530-NW-G2-3-HIS	ECFORM AREA LED ECF - SMALL 32 LED's, 4000K CCT, TYPE 3 OPTIC, HOUSE-SIDE INTERNAL SHIELDING (HIS)	2 LEDGINE SLD ARRAY(S) DRIVEN AT 530 mA	1	ECF-S-32L-530-NW-G2-3-HIS.IES	5350	1.0	1.0	55.7	III HIS
	C	3	SIGNIFY GARDCO	ECF-S-32L-530-NW-G2-4-HIS	ECFORM AREA LED ECF - SMALL 32 LED's, 4000K CCT, TYPE 4 OPTIC, HOUSE-SIDE INTERNAL SHIELDING (HIS)	2 LEDGINE SLD ARRAY(S) DRIVEN AT 530 mA	1	ECF-S-32L-530-NW-G2-4-HIS.IES	5350	1.0	1.0	55.7	IV HIS
	E1	17	AMERLUX EXTERIOR	VARIETA FLC1-30 BLK ASSUMES 21 DEGREES AIMING TOWARDS WALL	SINGLE ARRAY ACCENT LED FLOODLIGHT, BLACK, 3000K WITH CUSTOM 36" BRACKET SEE ARCHITECTURAL PLANS	1 SINGLE COB LED	1	FLC1-30.IES	2151	1.0	1.0	24.1	WIDE SPOT
	E2	3	AMERLUX EXTERIOR	VARIETA FL1-HWS-BLK ASSUMES 21 DEGREES AIMING TOWARDS WALL	SINGLE ARRAY ACCENT LED FLOODLIGHT, BLACK, 3000K WITH CUSTOM 36" BRACKET SEE ARCHITECTURAL PLANS	SINGLE MODULE WITH 16 LEDS	1	FL1-HWS-30_AME429067.IES	2302	1.0	1.0	31	HORIZONTAL WIDE SPOT
	M3	3	RAB LIGHTING INC	WPLED13Y	WALL SCONCE, BRONZE FINISH, 3000K, CLEAR FLAT GLASS CUTOFF FIXTURE SEE ARCHITECTURAL PLANS	SINGLE MULTICHIP LED	1	RAB02310.IES	1529	1.0	1.0	15.2	

WILLIAM T. STORMANT PE
ELECTRICAL ENGINEER
5304 NW 173 STREET
ALACHUA, FLORIDA 32615
STORMANTWT@COX.NET
(352) 665-1426

SCALE:
VERIFY SCALE
BAR IS ONE INCH ON
ORIGINAL DRAWING
0 1"
IF NOT ONE INCH ON
THIS SHEET, ADJUST
SCALE ACCORDINGLY

CONSTRUCTION/RFI REVISIONS:

SUBMITTALS:
3/18/21REVISIONS

CLIENT:
:

PROJECT:
OSR IN ALACHUA

SHEET TITLE:
PHOTOMETRICS

ELECTRICIAN:

DESIGNED:
W STORMANT

QUALITY CONTROL:
W STORMANT

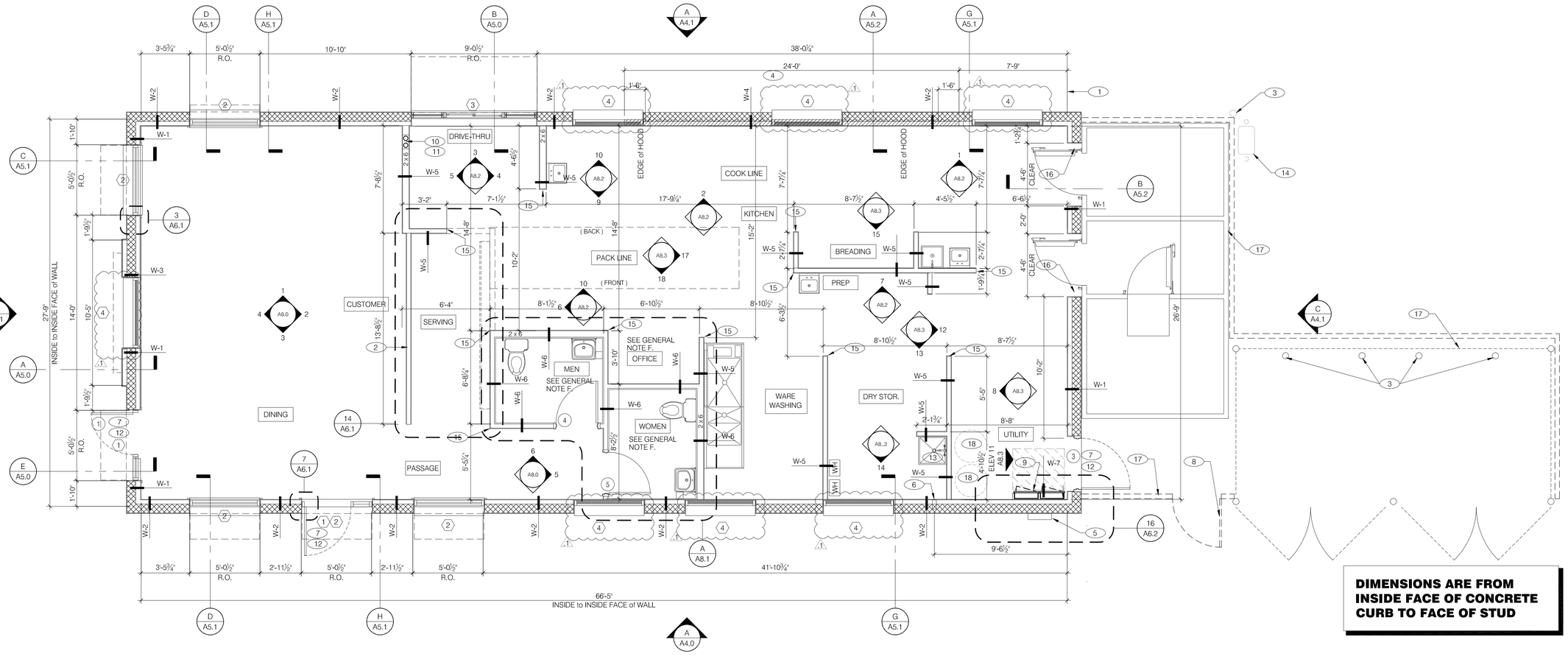
PROJECT NUMBER:
,

WILLIAM T. STORMANT

FL PE No. 44156

SHEET NO.:
P2

Tech: storm_J Plot Date: Mar 18,2021 3:27pm Filename: C:\Users\storm_J\Documents\OSR ALACHUA\OSR KFC Lighting 03-18-21.dwg



FLOOR PLAN 1/4"= 1'-0" | A

W-1 EXTERIOR SHEAR WALL:
8x8x16 (NOM.) CMU. GROUT CELLS SOLID IN ACCORDANCE WITH STRUCTURAL NOTES.
2x4 WOOD STUDS AT 16" O.C., SECURED TO CONCRETE FLOOR SLAB
1/2" EXTERIOR PLYWOOD SHEATHING AS SCHEDULED (SEE STRUCTURAL DRAWINGS).
3 ½" THICK RIGID BOARD INSULATION.
1/2" INTERIOR PLYWOOD SHEATHING AS SCHEDULED (SEE STRUCTURAL DRAWINGS).
SEE SHEETS A7.2, A8.0, A8.1, A8.2, A8.3 (INTERIOR FINISHES) / A4.0, A4.1, A4.2 (EXTERIOR FINISHES).

W-2 EXTERIOR WALL:
8x8x16 (NOM.) CMU. GROUT CELLS SOLID IN ACCORDANCE WITH STRUCTURAL NOTES.
2x4 WOOD STUDS AT 16" O.C., SECURED TO CONCRETE FLOOR SLAB and CMU wall
1/2" EXTERIOR PLYWOOD SHEATHING AS SCHEDULED (SEE STRUCTURAL DRAWINGS).
3 ½" THICK RIGID BOARD INSULATION.
1/2" GYPSUM BOARD INTERIOR FINISH (TYPICAL FOR DINING ROOM WALLS).
1/2" PLYWOOD INTERIOR FINISH (TYPICAL FOR KITCHEN WALLS).
1/2" CEMENT BOARD INTERIOR FINISH (TYPICAL FOR RESTROOM WALL BEHIND HOOD / TILE AREA).
1/2" GYPSUM BOARD INTERIOR FINISH (TYPICAL FOR RESTROOM WALLS AT NON-TILED WALL AREA).
SEE SHEETS A7.2, A8.0, A8.1, A8.2, A8.3 (INTERIOR FINISHES) / A4.0, A4.1, A4.2 (EXTERIOR FINISHES).

W-3 EXTERIOR TOWER WALL:
8x8x16 (NOM.) CMU. GROUT CELLS SOLID IN ACCORDANCE WITH STRUCTURAL NOTES.
2x4 WOOD STUDS AT 16" O.C., SECURED TO CONCRETE FLOOR SLAB
1/2" EXTERIOR PLYWOOD SHEATHING.
SEE SHEETS A4.0, A4.1, A4.2 FOR EXTERIOR FINISHES.

W-4 HOOD WALL:
AS APPLICABLE TO W-2 WITH 20 GA. S.S. PANEL OVER 5-1/2" METAL STUD FRAMING BEHIND HOOD.
SEE M3.0.

NOTE: REFER TO WALL BASE DETAILS, SHEET A6.2.

W-5 INTERIOR WALL:
2x4 (2x6 WHERE NOTED) WOOD STUDS AT 16" O.C. , SECURED TO SLAB.
1/2" CEMENT BOARD TO 6" A.F.F., BOTH SIDES.
1/2" GYPSUM BOARD FROM TOP OF CEMENT BOARD TO 2" ABOVE CEILING (DINING ROOM SIDE).
1/2" PLYWOOD FROM TOP OF CEMENT BOARD TO 2" ABOVE CEILING (KITCHEN SIDE).
SEE SHEETS A7.2, A8.0a, A8.0b, A8.1, A8.2, A8.3 (INTERIOR FINISHES).

W-6 RESTROOM WALL:
2x4 (2x6 WHERE NOTED) WOOD STUDS AT 16" O.C. , SECURED TO SLAB.
3-1/2" UN-FACED SOUND BATT INSULATION, FULL HEIGHT of FINISHED WALL.
1/2" CEMENT BOARD FULL HEIGHT BEHIND WALL TILE AREA; 1/2" GYPSUM BOARD AT NON-TILED WALL AREAS (RESTROOM SIDE).
1/2" CEMENT BOARD TO 6" A.F.F. (KITCHEN / DINING ROOM / OFFICE SIDE).
1/2" GYPSUM BOARD FROM TOP OF CEMENT BOARD TO 2" ABOVE CEILING (DINING ROOM SIDE).
1/2" PLYWOOD FROM TOP OF CEMENT BOARD TO 2" ABOVE CEILING (KITCHEN / OFFICE SIDE).
SEE SHEETS A7.2, A8.0a, A8.0b, A8.1, A8.2, A8.3 (INTERIOR FINISHES).

W-7 ELECTRICAL PANEL WALL:
2x6 WOOD STUDS AT 16" O.C., SECURED TO CONCRETE FLOOR SLAB.
½" PLYWOOD FROM FLOOR SLAB TO 2" ABOVE CEILING; ONE SIDE.
SEE SHEETS A7.2, A8.0a, A8.0b, A8.1, A8.2, A8.3 (INTERIOR FINISHES).

WALL TYPES | E

DIMENSIONS:
A. ALL DIMENSIONS ARE FROM INSIDE FACE OF CURB TO FACE OF STUD. REFER TO FOUNDATION PLAN FOR FACE OF CURB DIMENSIONS. SEE KEYNOTE 1, THIS SHEET.
B. DIMENSIONS NOTED AS "CLEAR" OR "HOLD" ARE MIN. REQ'D. NET CLEARANCE FROM FACE OF WALL FINISH.
C. VERIFY FINAL EQUIPMENT SIZES W/ VENDOR PRIOR TO INT. WALL FRAMING.

WINDOWS / DOORS:
A. SEE SHEET A1.1 FOR WINDOW TYPES AND DOOR SCHEDULE.
B. ALL DOOR AND WINDOW OPENING DIMENSIONS ARE TO ROUGH OPENING.

FINISH SUBSTRATES:
A. PROVIDE 1/2" THICK CEMENT BOARD FROM FLOOR SLAB TO 6' A.F.F., U.N.O.
B. ALL JOINTS, GAPS OR SPACES LEADING TO ALL HOLLOW OR INACCESSIBLE SPACES SHALL BE SEALED WITH "NSF INTERNATIONAL" APPROVED SEALANTS.
C. ALL BACK OF HOUSE AND OFFICE WALLS SHALL HAVE 1/2" CDX PLYWOOD SUBSTRATE, U.O.N.
D. PROVIDE SOLID BLOCKING FOR WALL SUPPORTED ITEMS.

DECOR:
A. SEE A2.0 FOR SEATING PLAN AND DETAILS.
B. SEE A7.0 FOR FLOOR FINISHES.
C. SEE A8.0, A8.1, A8.2, A8.3 FOR WALL FINISHES.
D. SEE A7.1 FOR CEILING FINISHES.

GENERAL:
A. PROVIDE THREE FIRE EXTINGUISHERS - (2) 10 lb. BC and (1) 10 lb. ABC - TO COMPLY WITH LOCAL FIRE CODE. LOCATE PER DIRECTION OF FIRE MARSHALL OR LOCAL AUTHORIZING AGENT.
B. DRAWINGS ARE BASED UPON WOOD FRAMING. UTILIZATION OF METAL STUDS ON NON-BEARING INTERIOR PARTITIONS, BULKHEADS AND SOFFITS IS ACCEPTABLE.
C. ALL ATTACHMENTS MADE THROUGH E.I.F.S. SHALL BE BUSHED TO PREVENT DAMAGE TO THE FINISH. SEE DETAIL 9 / A6.2.
D. ALL PENETRATIONS THROUGH E.I.F.S. SHALL BE SEALED USING MFR'S. APPROVED METHOD.
E. COORDINATE FRAMING ABOVE CEILING WITH HVAC DUCT WORK.
F. SEE SHEET A8.1 FOR ENLARGED RESTROOM AND OFFICE PLAN.

FLOOR PLAN NOTES | D

1. STARTING POINT; ALL SUB-TRADES SHALL USE THIS POINT OF ORIGIN FOR LAY-OUT (INSIDE CORNER OF CONCRETE CURB).
   INSIDE FACE OF CURB TO FACE OF CURB DIMENSIONS:
   > 26'-9" (SIDE to SIDE).
   > 66'-5" (FRONT to BACK).
2. CUSTOMER COUNTER KNEE-WALL BY G.C. SEE PLAN and SECTION, SHEET A6.1.
3. PIPE BOLLARD. REFER TO SITE DRAWINGS. COORDINATE WITH LOCATION OF GAS METER.
4. HOOD WALL, SEE WALL LEGEND. COORDINATE WITH HOOD LOCATION; SEE SHEET M2.0. REFER TO COOK LINE ELEVATION, SHEET A8.2 and HOOD SECTIONS 4 and 12, SHEET M3.0.
5. ELECTRICAL MAIN SWITCH BOARD. REFER TO ELECTRICAL DRAWINGS.
6. CO2 FILL BOX LOCATION.
7. METAL THRESHOLD.
8. GATE WITH PANIC HARDWARE
9. ELECTRICAL PANELS; REFER TO ELECTRICAL DRAWINGS.
10. SYRUP / FILTERED WATER TUBE BUNDLES IN CHASE WALL.
11. 8" x 12" HORIZONTAL OPENING FOR SYRUP TUBES. COORDINATE WALL OPENING with DRINK STATION EQUIPMENT SUPPLIER. SEE DETAIL 4 / A6.3. REFER TO ELEVATION 1 / A8.0 and 5 / A8.2.
12. PROVIDE HORIZONTAL DOOR SWEEPS AT ALL EXTERIOR DOORS. SEE SHEET A1.1.
13. COORDINATE WALL CLEARANCE WITH MOP SINK DIMENSIONS; SEE SHEET A2.0. REFER TO EQUIPMENT CUT SHEETS PROVIDED ON PLANS.YUM.COM WEBSITE.
14. GAS METER
15. S.S. CORNERGUARD / WALL CAP. TYPICAL AT ALL CORNERS FROM REAR WALL TO KITCHEN SIDE OF SERVICE COUNTER. SEE DETAIL 10 / A6.3
16. PROVIDE BACKER ROD AND SEALANT BETWEEN COLD STORAGE BOX AND ADJACENT WALL SURFACES. DISCONTINUE WALL COVERING BEHIND BOXES.
17. EXTERIOR FENCE BY OTHERS
18. RTI OIL TANK

KEY NOTES | B

GLMV Architecture
1525 E. Douglas Wichita, KS 67211
Tel: (316) 263-5297 Fax: (316) 263-5846
www.glmv.com

CONSULTANT'S SEAL

12127R20012

PLAN SET REVISIONS:

	REVISION	MARCH 2021
1		

CONTRACT DATE:	xx.xx.xxxx
BUILDING TYPE:	Kb 30
PLAN VERSION:	April 19.1
SITE NUMBER:	xxx - xxx
ENTITY NUMBER:	xxx - xxx
STORE NUMBER:	TBD

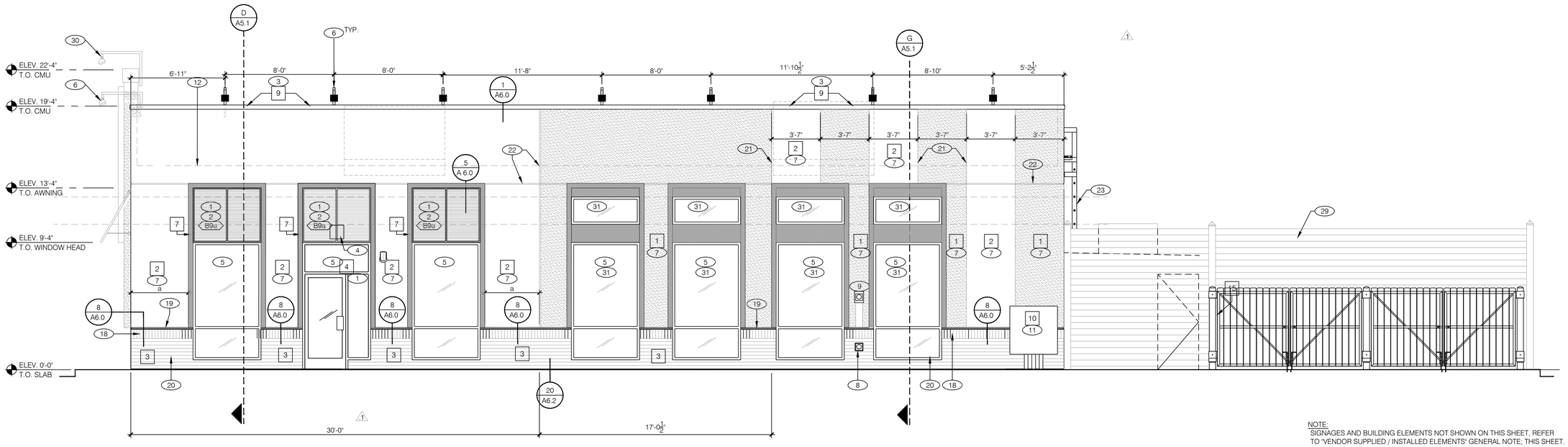
KFC
TBD
ALACHUA, FL. USA.

Kb 30-19.1

FLOOR PLAN

A1.0

PLOT DATE: 02.03.2021



NOTE:
SIGNAGES AND BUILDING ELEMENTS NOT SHOWN ON THIS SHEET, REFER TO "VENDOR SUPPLIED / INSTALLED ELEMENTS" GENERAL NOTE; THIS SHEET.

MAIN ENTRY ELEVATION 1/4"=1'-0" | **A**

SYMBOL	EXTERIOR WALL AREA	MANUFACTURER	COLOR	NOTES	CONTACT
1	EXTERIOR PAINT	BENJAMIN MOORE	AURA EXTERIOR PAINT EXOTIC RED 2086-10 LOW LUSTER (634)		KURT MCCLELLAND P: (800) 635-5147 / C: (502) 640-1608 kurt.mcclelland@benjaminmoore.com
2	EXTERIOR PAINT	BENJAMIN MOORE	REGAL SELECT EXTERIOR PAINT WEDDING VEIL 2125-70 LOW LUSTER (634)		KURT MCCLELLAND P: (800) 635-5147 / C: (502) 640-1608 kurt.mcclelland@benjaminmoore.com
3	BACK OF HOUSE WALLS/ WAINSCOT/ CURB	GLEN-GERY	AURA EXTERIOR PAINT BLACK HORIZON 2132-30 LOW LUSTER (634) MASONRY SUBSTRATE PRIMER: N068; GLAZED TILE SUBSTRATE PRIMER SXA-110	ALTERNATE COLOR: PER LOCAL/ MUNICIPAL REQUIREMENTS	
4	EXTERIOR PAINT	BENJAMIN MOORE	AURA EXTERIOR PAINT BLACK HORIZON 2132-30 LOW LUSTER (634)		KURT MCCLELLAND P: (800) 635-5147 / C: (502) 640-1608 kurt.mcclelland@benjaminmoore.com
5	LOUVERED AWNINGS	PRE-FINISHED	AURA EXTERIOR PAINT EXOTIC RED 2086-10 LOW LUSTER (634)		
6	BOLLARDS		PVC SLEEVE VERIFY COLOR w/ OWNER		
7	PAINTED 4" BORDER AROUND WINDOW, AWNING AND BEHIND AWNING (ABOVE THE WINDOW)		AURA EXTERIOR PAINT BLACK HORIZON 2132-30 LOW LUSTER (634)		KURT MCCLELLAND P: (800) 635-5147 / C: (502) 640-1608 kurt.mcclelland@benjaminmoore.com
8	STENCIL		BLACK		
9	EXTERIOR METAL	BENJAMIN MOORE	COROTECH POLYESTER URTHANE CUSTOM MATCH BENJAMIN MOORE 2125-70 WEDDING VEIL (V520)	COROTECH PRIMER (V132)	KURT MCCLELLAND P: (800) 635-5147 / C: (502) 640-1608 kurt.mcclelland@benjaminmoore.com
10	EXTERIOR METAL	BENJAMIN MOORE	COROTECH POLYESTER URTHANE CUSTOM MATCH BENJAMIN MOORE 2132-30 BLACK HORIZON (V520)	COROTECH PRIMER (V132)	KURT MCCLELLAND P: (800) 635-5147 / C: (502) 640-1608 kurt.mcclelland@benjaminmoore.com
11	EXTERIOR COMPOSITE SIDING	AZEK	COMPOSITE DECKING SQUARE SHOULDERED CAPPED, 5.5" w. X 1", HARVEST COLLECTION, ISLAND OAK	NAIL OR SCREW PVC PLANKS TO WOOD SUPPORTS, PER MANUF. REQUIREMENTS.	AZEK BUILDING PRODUCTS P: (877) 275-2935
12	EXTERIOR METAL	BENJAMIN MOORE	COROTECH POLYESTER URTHANE CUSTOM MATCH BENJAMIN MOORE 2086-10 EXOTIC RED (V520)		
13	EXTERIOR MASONRY	BENJAMIN MOORE	COROTECH POLYESTER URTHANE CUSTOM MATCH BENJAMIN MOORE 2132-30 BLACK HORIZON (V520)		

EXTERIOR FINISH SCHEDULE | **F**

IMAGE COMPONENTS SCHEDULE

SIGNAGE

TAG	QTY	ITEM DESCRIPTION	ELEC
S1a	1	VINYL LOGO/GRAPHICS FOR TOWER PANEL - FULL HT.	
S1b	0	VINYL LOGO/GRAPHICS FOR TOWER PANEL - HALF HT.	
S3a	0	KFC CHANNEL LETTERS - 30" RED	X
S3b	2	KFC CHANNEL LETTERS - 24" RED	X
S3c	0	KFC CHANNEL LETTERS - 30" WHITE	X
S3d	0	KFC CHANNEL LETTERS - 24" WHITE	X
S4a	1	"REAL MEALS TO GO" DIMENSIONAL LETTERS FOR 15' DT CANOPY	
S4b	0	"REAL MEALS TO GO" VINYL LETTERS FOR 8' DT CANOPY	
S5a	1	"WORLD FAMOUS CHICKEN" LETTERS - 16" DIMENSIONAL PIN MOUNT	
S5b	0	"WORLD FAMOUS CHICKEN" 16" LETTERS - PAINT	
S5c	0	"WORLD FAMOUS CHICKEN" 12" LETTERS - PAINT	
S5d	0	"WORLD FAMOUS CHICKEN" STACKED LETTERS - PAINT	
S6	0	"REAL MEALS" BUCKET SIGN	
S7	0	"REAL MEALS TO GO" BUCKET SIGN	
S8a	1	"HARD WAY" PAINT TEMPLATE - 57" ROUND	
S8d	1	"HARD WAY" PAINT TEMPLATE - HORIZONTAL	
S9	1	"HOT FRESH ARROW SIGN"	X
S15b	1	"HOT FRESH ARROW SIGN"	X
G9a	2	STORE HOURS - ENTRY DOOR	
G9b	1	STORE HOURS - DT WINDOW	
G9c	1	STORE HOURS - DT SPEAKER POST/CANOPY	

BUILDING EXTERIOR ELEMENTS

B1a	1	TOWER PANEL W/ HALF HT LOGO (NO GRAPHICS)- FULL HT W/ EXTENDER	
B1b	0	TOWER PANEL W/ LOGO AND GRAPHICS - HALF HT	
B2	1	TOWER LID CANOPY	
B3a	1	DT WINDOW CANOPY - 15'-0" WIDE	
B9a	5	SHUTTERED AWNINGS - 5'-0" WIDE, 4'-6" HT	
B9b	1	SHUTTERED AWNINGS - 6'-4" WIDE, 4'-6" HT	
B9a-DT	2	SHUTTERED AWNINGS - 5'-0" WIDE, 4'-0" HT	

IMAGE COMPONENTS SCHEDULE | **D**

MISCELLANEOUS:

A. SEE SHEET A1.1 "WINDOW TYPES" FOR WINDOW ELEVATIONS.
B. PAINT COLORS SHOWN ARE COLOR REFERENCES FOR THE . SUPPLIER.

SEALERS (REFER TO SPECS):

A. SEALANT AT ALL WALL AND ROOF PENETRATIONS.
B. SEALANT AT ALL WINDOW AND DOOR FRAMES AT HEAD AND JAMB. DO NOT SEAL SILL AT WINDOWS.

CRITICAL FINISH OPENINGS:

BUILDING FINISH OPENINGS THAT ARE DESIGNATED AS "CRITICAL" SHALL BE MAINTAINED TO PROVIDE FOR THE STANDARD INSTALLATION OF STANDARD SIGNAGE / BUILDING ELEMENTS. SEE GENERAL NOTE FOR 'VENDOR PROVIDED / VENDOR INSTALLED ELEMENTS'.

THIN BRICK INSTALLATION:

THE GENERAL CONTRACTOR TO ENSURE THAT THE BRICK JOINTS ARE PLUMB AND LEVEL, AND THE BRICK FACES ALIGNED AND FLUSH.

VENDOR SUPPLIED / INSTALLED ELEMENTS:

GC TO COORDINATE WITH VENDOR PROVIDED / VENDOR INSTALLED SIGNAGE AND BUILDING ELEMENTS.

GENERAL NOTES | **C**

1. PAINT WALL SURFACE BEHIND NEW AWNING
2. PRE-FINISHED METAL LOUVERED AWNINGS (AWNINGS ABOVE DOORS TO HAVE RAIN DIVERTERS)
3. PARAPET FASCIA CAP.
4. WALL PACK LIGHT
5. ALUMINUM STOREFRONT WINDOW / DOOR SYSTEM. SEE SHEET A1.1.
6. WALL LIGHTING - LIGHTING VENDOR SUPPLIED / GC INSTALLED. SEE SHEET E4.0.
7. STO CORP. OR EQUAL STO POWER WALL EXTRA SEAL DIRECT APPLIED STUCCO SYSTEM. WITH STO POWER FLEX ELASTOMERIC MEDIUM FINISH. REFER TO SPECIFICATIONS FOR MANUFACTURER'S ASSEMBLY DETAILS, FIELD SUPPORT SERVICES AND CONTACTS. REFER TO DETAILS, SHEET A6.3.
8. HOSE BIB - REFER TO DETAIL 5 / A6.2.
9. CO_2 FILLER VALVE & COVER. SEE DETAIL 11/A6.2
10. SCUPPER, COLLECTOR AND DOWNSPOUT 6" MIN.
11. SWITCHGEAR.
12. INDICATES TOP OF ROOF DECK.
13. DRIVE THRU WINDOW, SEE SHEET A2.1
14. GAS METER. DO NOT PAINT METER.
15. STEEL BOLLARD W/ PVC SLEEVE. SEE SITE DETAILS.
16. TOP OF RTU. COORDINATE WITH MANUFACTURER'S SPECIFICATIONS.
17. EXHAUST FANS ON ROOF.
18. SOLDIER COURSE
19. TRANSITION FROM THIN BRICK -to- STUCCO.
20. THIN BRICK RUNNING BOND
21. LINE OF PAINT COLOR CHANGE.
22. "V" GROOVE SEE DETAIL 14 / A6.2.
23. ROOF ACCESS LADDER. SEE DETAIL 14 & 15/ A6.3
24. THRU WALL OVERFLOW STORM DRAINAGE
25. STAINLESS STEEL CORNER GUARDS.
26. HANDICAP SIGNAGE; MOUNT AT 5'-0" A.F.F.
27. SECURITY DOOR. OWNER-SUPPLIED / G.C. INSTALL.
28. EMERGENCY LIGHT PER ELECTRIC
29. COMPOSITE PLANK WOOD SUPPORT FENCE SEE A6.3 DETAIL.
30. NEW ACCENT LIGHT FIXTURE, REFER TO LIGHT FIXTURE SCHEDULE
31. SPANDREL WINDOW. SEE SHEET A1.1.

NOTE: NOT ALL KEY NOTES APPLY TO THIS SHEET **KEY NOTES** | **B**

GLMV Architecture
1525 E. Douglas Wichita, KS 67211
Tel: (316) 265-5367 Fax: (316) 265-5646
www.glmv.com

ISSUE FOR BID

12127R20012

PLAN SET REVISIONS:

1 REVISION MARCH 2021
2 REVISION MARCH 25, 2021

CONTRACT DATE: xx.xx.xxxx
BUILDING TYPE: Kb 30
PLAN VERSION: April 19.1
SITE NUMBER: xxx - xxx
ENTITY NUMBER: xxx - xxx
STORE NUMBER: TBD

KFC
TBD
ALACHUA, FL. USA.

Kb 30-19.1

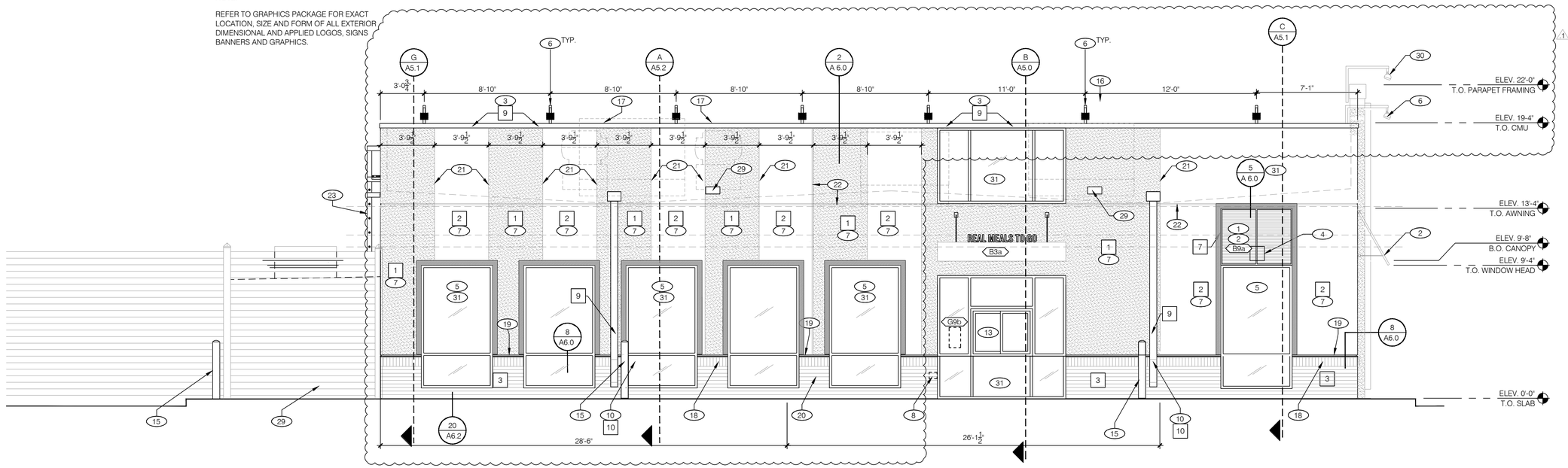
EXTERIOR ELEVATION

A4.0

PLOT DATE: 24.03.2021

NOTE:

REFER TO GRAPHICS PACKAGE FOR EXACT LOCATION, SIZE AND FORM OF ALL EXTERIOR DIMENSIONAL AND APPLIED LOGOS, SIGNS BANNERS AND GRAPHICS.

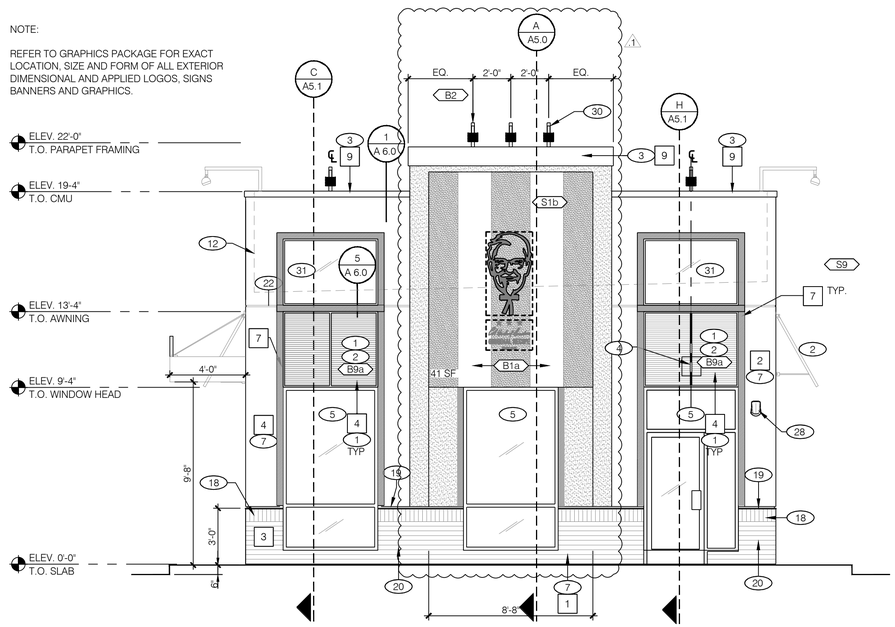


NOTE:
SIGNAGE AND BUILDING ELEMENTS ARE NOT SHOWN ON THIS SHEET, REFER TO "VENDOR SUPPLIED / INSTALLED ELEMENTS" GENERAL NOTE; THIS SHEET.

DRIVE-THRU ELEVATION 1/4"=1'-0" | **A**

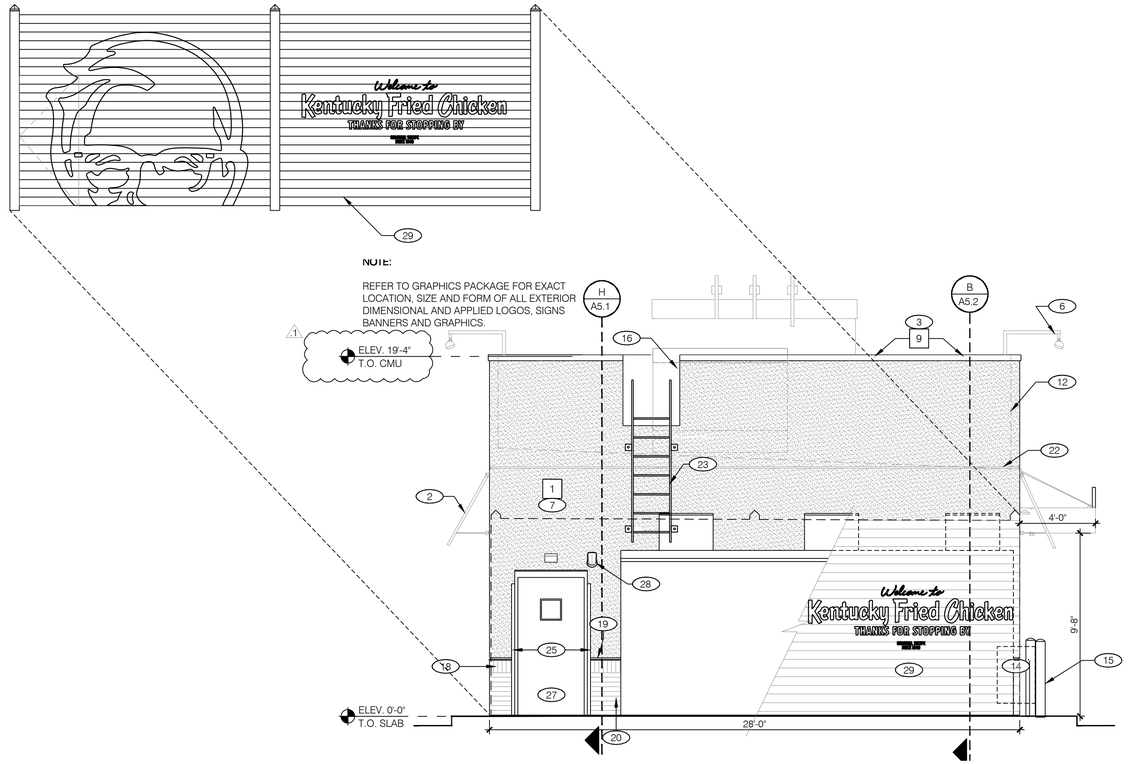
NOTE:

REFER TO GRAPHICS PACKAGE FOR EXACT LOCATION, SIZE AND FORM OF ALL EXTERIOR DIMENSIONAL AND APPLIED LOGOS, SIGNS BANNERS AND GRAPHICS.



XXX	QTY	ITEM DESCRIPTION	PROVIDED SQF
S1b	1	VINYL LOGO	41 SF
TOTAL SIGNAGE AREA FOR THIS ELEVATION:			41 SF

FRONT ELEVATION 1/4"=1'-0" | **B**



REAR ELEVATION 1/4"=1'-0" | **C**

GLMV Architecture
1525 E. Douglas Wichita, KS 67211
Tel: (316) 265-8387 Fax: (316) 265-5646
www.glmv.com

ISSUE FOR BID

12127R20012

PLAN SET REVISIONS:

	REVISION	MARCH 2021
1		

CONTRACT DATE:	xx.xx.xxxx
BUILDING TYPE:	Kb 30
PLAN VERSION:	April 19.1
SITE NUMBER:	xxx - xxx
ENTITY NUMBER:	xxx - xxx
STORE NUMBER:	TBD

KFC

TBD
ALACHUA, FL. USA.

Kb 30-19.1

EXTERIOR ELEVATIONS

A4.1

PLOT DATE: 24.03.2021

City of ALACHUA

THE GOOD LIFE COMMUNITY

FOR PLANNING USE ONLY
Case #: _____
Application Fee: $_____
Filing Date: _____
Acceptance Date: _____
Review Type: P&Z

Site Plan Application

Reference City of Alachua Land Development Regulations Article 2.4.9

A. PROJECT

1. Project Name: _____
2. Address of Subject Property: _____
3. Parcel ID Number(s): _____
4. Existing Use of Property: _____
5. Future Land Use Map Designation : _____
6. Zoning Designation: _____
7. Acreage: _____

B. APPLICANT

1. Applicant's Status ☐ Owner (title holder) ☐ Agent
2. Name of Applicant(s) or Contact Person(s): _____ Title: _____
   Company (if applicable): _____
   Mailing address: _____
   City: _____ State: _____ ZIP: _____
   Telephone: _____ FAX: _____ e-mail: _____
3. If the applicant is agent for the property owner*:
   Name of Owner (title holder): _____
   Mailing Address: _____
   City: _____ State: _____ ZIP: _____

   * Must provide executed Property Owner Affidavit authorizing the agent to act on behalf of the property owner.

C. ADDITIONAL INFORMATION

1. Is there any additional contact for sale of, or options to purchase, the subject property? ☐ Yes ☐ No
   If yes, list names of all parties involved: _____
   If yes, is the contract/option contingent or absolute? ☐ Contingent ☐ Absolute

D. ATTACHMENTS

1. Site Plan including but not limited to:
   a. Name, location, owner, and designer of the proposed development.
   b. Zoning of the subject property.
   c. Vicinity map - indicating general location of the site and all abutting streets and properties.
   d. Complete legal description.
   e. Statement of Proposed Uses.
   f. Location of the site in relation to adjacent properties, including the means of ingress and egress to such properties and any screening or buffers along adjacent properties.
   g. Date, north arrow, and graphic scale (not to exceed one (1) inch equal to fifty (50) feet.)
   h. Area and dimensions of site.
   i. Location of all property lines, existing right-of-way approaches, sidewalks, curbs, and gutters.
   j. Access and points of connection to utilities (electric, potable water, sanitary sewer, gas, etc.)
   k. Location and dimensions of all existing and proposed parking areas and loading areas.
   l. Location, size, and design of proposed landscaped areas (including existing trees and required landscaped buffer areas) with detail illustrating compliance with Section 6.2.2 of the Land Development Regulations.

m. Location and size of any lakes, ponds, canals, or other waters and waterways.
n. Structures and major features – fully dimensioned – including setbacks, distances between structures, floor area, width of driveways, parking spaces, property or lot lines, and floor area ratio.
o. Location of waste receptacles and detail of waste receptacle screening.
p. For development consisting of a nonresidential use, except for single tenant retail sales and services uses greater than or equal to 20,000 square feet in area and except for use types within the industrial services, manufacturing and production, warehouse freight and movement, waste-related services, and wholesale sales use categories:
    i. Architectural plans and dimension plans which demonstrate compliance with the design standards for business uses as provided in Section 6.8.2 of the LDRs, including:
        (a) Calculation of glazing of the front façade.
        (b) Calculation of the area of ground floor façades subject to glazing.
        (c) Detail on the architectural plans and dimension plans depicting façade massing and/or alternatives to required façade massing.
        (d) Sufficient plan detail and calculations of each material utilized in each façade.
q. For development consisting of a nonresidential use where a single tenant is greater than or equal to 20,000 square feet in area:
    i. Architectural plans and dimension plans which demonstrate compliance with the design standards for single tenant retail sales and service uses greater than or equal to 20,000 square feet in area as provided in Section 6.8.3 of the LDRs, including:
        (a) Calculation of glazing of the façades facing streets, residential uses, and vacant residential/agricultural land.
        (b) Calculation of the area of ground floor façades subject to glazing.
        (c) If glazing alternatives are used, calculation of area of alternative materials used.
        (d) Detail on the architectural plans and dimension plans depicting façade massing and/or alternatives to required façade massing.
        (e) Color architectural plans depicting the color of all materials used in the façade.
r. For development consisting of one or more of the following: Multi-family residential; Hotel; or Mobile Home Park:
    i. Tabulation of gross acreage.
    ii. Tabulation of density.
    iii. Number of dwelling units proposed.
    iv. Location and percent of total open space and recreation areas.
    v. Floor area of dwelling units.
    vi. Number of proposed parking spaces.
    vii. Street layout.
    viii. Layout of mobile home stands (for mobile home parks only).
    ix. City of Alachua Public School Student Generation Form.

Sheet Size: 24" X 36" with 3" left margin and ½" top, bottom, and right margins

2. Stormwater management plan - including the following:

    a. Existing contours at one (1) foot intervals based on U.S. Coastal and Geodetic Datum.
    b. Proposed finished floor elevation of each building site.
    c. Existing and proposed stormwater management facilities with size and grades.
    d. Proposed orderly disposal of surface water runoff.
    e. Centerline elevations along adjacent streets.
    f. Water Management District surfacewater management Statement of proposed uses on the site plan

3. Fire Department Access and Water Supply: The design criteria shall be Chapter 18 of the Florida Fire Prevention Code. Plans must be on separate sealed sheets and must be prepared by a professional Fire engineer licensed in the State of Florida. Fire flow calculations must be provided for each newly constructed building. When required, fire flow calculations shall be in accordance with the Guide for Determination of Required Fire Flow, latest edition, as published by the Insurance Service Office (ISO) and /or Chapter 18, Section 18.4 of the Florida Fire Prevention Code, whichever is greater. All calculations must be demonstrated and provided. All calculations and specifications must be on the plans and not on separate sheets. All fire protection plans are reviewed and approved by the Alachua County Fire Marshal.

4. Concurrency Impact Analysis showing the impact on public facilities, including potable water, sanitary sewer, transportation, solid waste, recreation, stormwater, and public schools in accordance with Article 2.4.14 of the Land Development Regulations.

5. Analysis of Consistency with the City of Alachua Comprehensive Plan (analysis must identify specific Goals, Objectives, and Policies and describe in detail how the application complies with the noted Goal, Objective, or Policy.)

For commercial project Applications:

a. In addition to submitting specific written information regarding your **commercial** development's compliance with the relevant Goals, Objectives, and Policies of the City of Alachua Comprehensive Plan, you must respond directly to the standards listed below. You should be specific in terms of how your commercial development will comply with these standards.

Policy 1.3.d Design and performance standards

The following criteria shall apply when evaluating commercial development proposals:

1. Integration of vehicular and non-vehicular access into the site and access management features of site in terms of driveway cuts and cross access between adjacent sites, including use of frontage roads and/or shared access;
2. Buffering from adjacent existing/potential uses;
3. Open space provisions and balance of proportion between gross floor area and site size;
4. Adequacy of pervious surface area in terms of drainage requirements;
5. Placement of signage;
6. Adequacy of site lighting and intrusiveness of lighting upon the surrounding area;
7. Safety of on-site circulation patterns (patron, employee and delivery vehicles), including parking layout and drive aisles, and points of conflict;
8. Landscaping, as it relates to the requirements of the Comprehensive Plan and Land Development Regulations;
9. Unique features and resources which may constrain site development, such as soils, existing vegetation and historic significance; and
10. Performance based zoning requirements, which may serve as a substitute for or accompany land development regulations in attaining acceptable site design.
11. Commercial uses shall be limited to an intensity of less than or equal to .50 floor area ratio for parcels 10 acres or greater, .50 floor area ratio for parcels less than 10 acres but 5 acres or greater, a .75 floor area ratio for parcels less than 5 acres but greater than 1 acre, and 1.0 floor area ratio to parcels 1 acre or less.

For industrial project Applications:

b. In addition to submitting specific written information regarding your **industrial** development's compliance with the relevant Goals, Objectives, and Policies of the City of Alachua Comprehensive Plan, you must respond directly to the standards listed below. You should be specific in terms of how your industrial development will comply with these standards.

Policy 1.5.d

The City shall develop performance standards for industrial uses in order to address the following:

1. Integration of vehicular and non-vehicular access into the site and access management features of site in terms of driveway cuts and cross access between adjacent sites, including use of frontage roads and/or shared access;
2. Buffering from adjacent existing/potential uses;
3. Open space provisions and balance of proportion between gross floor area and site size;
4. Adequacy of pervious surface area in terms of drainage requirements;
5. Placement of signage;
6. Adequacy of site lighting and intrusiveness of lighting upon the surrounding area;
7. Safety of on-site circulation patterns (patron, employee and delivery vehicles, trucks), including parking layout and drive aisles, and points of conflict;
8. Landscaping, as it relates to the requirements of the Comprehensive Plan and Land Development Regulations;
9. Unique features and resources which may constrain site development, such as soils, existing vegetation and historic significance; and
10. Performance based zoning requirements that may serve as a substitute for or accompany land development regulations in attaining acceptable site design.
11. Industrial uses shall be limited to an intensity of less than or equal to .50 floor area ratio for parcels 10 acres or greater, .50 floor area ratio for parcels less than 10 acres by 5 acres or greater, .75 floor area ratio for parcels less than 5 acres but greater than 1 acre, and 1.0 floor area ratio for parcels 1 acre or less.

6. *For Site Plans for Buildings Less than 80,000 Square Feet in Area:* One (1) set of labels for all property owners within 400 feet of the subject property boundaries – even if property within 400 feet falls outside of City limits (obtain from the Alachua County Property Appraiser's web site) – and all persons/organizations registered to receive notice of development applications.
   For Site Plans for Buildings Greater than or Equal to 80,000 Square Feet in Area: Two (2) sets of labels for all property owners within 400 feet of the subject property boundaries – even if property within 400 feet falls outside of City limits (obtain from the Alachua County Property Appraiser's web site) – and all persons/organizations registered to receive notice of development applications.

7. Neighborhood Meeting Materials, including:
   i. Copy of the required published notice (advertisement) – must be published a newspaper of general circulation, as defined in Article 10 of the City's Land Development Regulations
   ii. Copy of written notice (letter) sent to all property owners within 400 feet and to all persons/organizations registered with the City to receive notice, and mailing labels or list of those who received written notice
   iii. Written summary of meeting – must include (1) those in attendance; (2) a summary of the issues related to the development proposal discussed; (3) comments by those in attendance about the development proposal; and, (4) any other information deemed appropriate.

8. Legal description with tax parcel number, separate from all other documentation on 8.5" x 11" paper.

9. Proof of ownership (i.e., copy of deed.)

10. Proof of payment of taxes.

11. Environmental Resource Permit (or Letter of Exemption) from the Suwannee River Water Management District or Self-Certification for a Stormwater Management System in Uplands Serving Less than 10 Acres of Total Project Area and Less than 2 Acres of Impervious Surfaces from the Florida Department of Environmental Protection pursuant to Section 403.814(12), Florida Statutes.

12. If access is from a County Road, access management permit from Alachua County Public Works (or documentation providing evidence that a permit application has been submitted).

13. If access is from a State Road, access management permit from Florida Department of Transportation (or documentation providing evidence that a permit application has been submitted).

14. **Fee.** Please see fee schedule for fee determination. No application shall be accepted for processing until the required application fee is paid in full by the applicant. Any necessary technical review or additional reviews of the application beyond the initial engineering review fee will be billed to the applicant at the rate of the reviewing entity. The invoice shall be paid in full prior to any legislative and/or quasi-judicial action of any kind on the petition, appeal, or development application.

All 14 attachments are required for a complete application. A completeness review of the application will be conducted within five (5) business days of receipt. If the application is determined to be incomplete, the application will be returned to the applicant.

I/We certify and acknowledge that the information contained herein is true and correct to the best of my/our knowledge.

_____ _____

Signature of Applicant Signature of Co-applicant

_____ _____

Typed or printed name and title of applicant Typed or printed name of co-applicant

State of _____ County of _____

The foregoing application is acknowledged before me this _____ day of _____, 20__, by _____

_____, who is/are personally known to me, or who has/have produced _____

as identification.

NOTARY SEAL

_____

Signature of Notary Public, State of _____

April 6, 2021

Adam Hall, AICP
Planner
City of Alachua
Post Office Box 9
Alachua, Florida 32616

RE: KFC Alachua - Notice of Hearing

Dear Adam:

Please find attached 3 copies of the following items to address comments found in the City's Notice of Hearing letter dated March 31, 2021:

1. Site Plan Review Application;
2. Authorized Agent Affidavit;
3. Property Owner Affidavit(s);
4. Alachua County Property Appraiser Information;
5. Warranty Deed(s);
6. Proof of Tax Payment(s);
7. Division of Corporations Information;
8. Legal Description(s);
9. Neighborhood Workshop Materials;
10. Alachua County Stormwater Treatment Affidavit of Compliance;
11. Alachua County Wetland Self-Certification;
12. Signed and Sealed Fire Flow Calculations;
13. Signed and Sealed Meter Sizing Calculations;
14. Signed and Sealed Stormwater Report;
15. Revised Signed and Sealed Development Plans (7 sets);
16. Traffic Impact Analysis;
17. Tree Mitigation Memo;
18. FDOT Landscape Program Management Memo;
19. Comprehensive Plan Consistency Analysis;
20. Concurrency Impact Analysis;
21. Color Architectural Elevations; and
22. Hightail Link Including all PDFs

We trust you will find this submittal to be complete for review and approval. If you have any questions, or need additional information, please contact me at (352) 331-1976 or via email at daniely@chw-inc.com.

Sincerely,
CHW



Daniel Young, P.E.
Vice President, Engineering

City of ALACHUA

THE GOOD LIFE COMMUNITY

Authorized Agent Affidavit

A. PROPERTY INFORMATION

Address of Subject Property: TBD - Alachua, FL, 32615

Parcel ID Number(s): 03053-002-001

Acreage: ±1.00

B. PERSON PROVIDING AGENT AUTHORIZATION

Name: David E Evans Title: Managing Member

Company (if applicable): D.E. Foods, LLC

Mailing Address: 170 Olde Forge Road

City: Hanover State: Massachusetts ZIP: 02339

Telephone: Please use Agent information. FAX: e-mail: Please use Agent information.

C. AUTHORIZED AGENT

Name: Daniel Young, P.E. Title: Director of Engineering, Land Development

Company (if applicable): CHW

Mailing address: 11801 Research Drive

City: Alachua State: Florida ZIP: 32615

Telephone: (352) 331-1976 FAX: e-mail: daniely@chw-inc.com

D. REQUESTED ACTION:

Authorization to apply for and obtain permits for the QSR Alachua project.

I hereby certify that I am the property owner of record, or I have received authorization from the property owner of record to file an application for a development permit related to the property identified above. I authorize the agent listed above to act on my behalf for purposes of this application.

David E Evans

Signature of Applicant

Signature of Co-applicant

David E Evans

Typed or printed name and title of applicant

N/A

Typed or printed name of co-applicant

State of Florida County of ~~Alachua~~ LJC COLLIER

The foregoing application is acknowledged before me this 25TH day of ~~December~~ LJC JAN, 2021, by David E Evans

_____, who is/are personally known to me, or who has/have produced Driver License as identification.

NOTARY SEAL

Signature of Notary Public, State of FLORIDA

Notary Public State of Florida
Leslet Jean Charles
My Commission GG 306724
Expires 05/12/2023

PROPERTY OWNER AFFIDAVIT

CHW
Professional Consultants

Owner Name: T & H Property Group, LLC			
Address: 3580 North Main Street Gainesville, FL 32609	Phone: Please use Agent contact information below.		
Agent Name: CHW			
Address: 11801 Research Drive Alachua, FL 32615	Phone: (352) 331-1976		
Other: D.E. Foods, LLC			
Address: 170 Olde Forge Road Hanover, MA 02339	Phone: Please use Agent contact information above.		
Parcel No.: 03053-002-001			
Acreage: ±1.00	S: 9	T: 8	R: 18

Requested Action:

To obtain approvals from the City of Alachua, Suwannee River Water Management District, and Florida Department of Environmental Protection.

I hereby certify that:
I am the property owner of record. I authorize the above listed agent to act on my behalf for the purposes of this application.

Property owner signature: Kristi Hawley, member
Printed name: KRISTI HAWLEY
Date: 1·12·2021

The foregoing instrument was acknowledged before me by means of ☑ physical presence or ☐ online notarization, this 12th day of January, 2021, by Kristi Hawley.

Personally Known **OR** Produced Identification
Type of Identification Produced:

NOTARY SEAL

RYAN C. CURTIS
Commission # GG 923645
Expires January 20, 2024
Bonded Thru Troy Fain Insurance 800-385-7019

Signature of Notary Public, State of _____

To:	Kathy Winburn, AICP, City of Alachua Planning & Community Dev. Director
From:	Seth Wood, Project Planner
Date:	December 23, 2020
RE:	KFC Alachua – Comprehensive Plan Consistency Analysis

This Comprehensive Plan Consistency Analysis is submitted for KFC Alachua, on Alachua County Tax Parcel 03053-002-001. The parcel is located on the southern side of US 441, between NW 167th Boulevard and I-75. The site's Future Land Use (FLU) category is Commercial, and the Zoning District classification is Commercial, Intensive (CI). The proposed use is a ±1,900-square-foot quick service restaurant, with associated infrastructure and parking.

The following identifies specific City of Alachua Comprehensive Plan Goals, Objectives, and Policies and explains how this application is consistent with each. Comprehensive Plan text is provided in normal font, while consistency statements are provided in **bold**.

Future Land Use Element

Objective 1.3: Commercial

The City of Alachua shall establish three commercial districts: Community Commercial, Commercial, and Central Business District. These districts shall provide a broad range of retail sales and services, as well as office uses, in order to provide for the availability of goods and services, both to the citizens of Alachua and to the citizens of the North Central Florida region.

Policy 1.3.b: Commercial: The Commercial land use category is established to provide for general commercial uses, as well as more intense commercial and highway commercial uses. This is the land use category in which large-scale, regional commercial uses may locate. The following uses are allowed within the Commercial land use category:

1. Retail sales and services;
2. Personal services;
3. Financial Institutions;
4. Outdoor recreation and entertainment;
5. Tourist-related uses;
6. Hotels, motels;
7. Commercial shopping centers;
8. Auto-oriented uses;
9. Traditional Mixed-use Neighborhood Planned Developments;
10. Employment Center Planned Developments;
11. Commercial recreation centers;
12. Office/business parks;
13. Limited industrial services;
14. Eating establishments

t: (904) 619-6521 | 8563 Argyle Business Loop, Suite 3, Jacksonville, FL 32244
t: (352) 331-1976 | 11801 Research Drive, Alachua, FL 32615
t: (352) 414-4621 | 101 NE 1st Avenue, Ocala, FL 34470
www.chw-inc.com

The proposed use, an eating establishment, is allowed within the Commercia FLU category. This application is for development of an eating establishment with associated drive-through. Development will be consistent with standards set forth by the Commercial FLU category and CI Zoning District.

Policy 1.3.d: Design and performance standards: The following criteria shall apply when evaluating commercial development proposals:

1. Integration of vehicular and non-vehicular access into the site and access management features of site in terms of driveway cuts and cross access between adjacent sites, including use of frontage roads and/or shared access;

The application proposes a right-in/right-out ingress/egress point on US 441 that connects to a frontage road that serves the site as well as adjacent properties. This frontage road will promote improved access management on US 441, compared to traditional commercial development with driveways directly connecting to the highway. This access configuration is consistent with adjacent development and approved by the Florida Department of Transportation (FDOT). Patrons will benefit by the ability to access multiple businesses without relying on US 441 for access. A crosswalk within the frontage road ensures pedestrians can safely access the site from US 441.

2. Buffering from adjacent existing/potential uses;

The project site is bound by US 441 to the north and NW 162nd Lane to the south. Land to the east shares the project site's Commercial Future Land Use (FLU) designation and Commercial, Intensive (CI) Zoning designation, and is currently developed with a drive-through eating establishment. Land to the west also shares the site's Commercial FLU and CI Zoning but is currently undeveloped. Per City of Alachua Land Development Regulations (LDR) Table 6.2-2, a 7.5'-wide Type A buffer is required along the site's eastern boundary and a 7.5'-wide Type C buffer will be implemented along the site's western boundary, in consideration of the undeveloped western parcel's potential highest use classification—retail sales and service or eating establishment. Furthermore, per LDR §6.2.2(D)(2)(b), a parking area buffer is also required, and is located between the parking lot and the proposed cross-access.

3. Open space provisions and balance of proportion between gross floor area and site size;

City of Alachua LDR §6.7.3(A), Table 6.7-1. states that a minimum of 10% of the gross site area shall be designated as open space. The project proposes to designate 24% of the project site as open space, thus exceeding the 10% minimum requirement.

4. Adequacy of pervious surface area in terms of drainage requirements;

Onsite Stormwater Management Facilities (SMF) are proposed, as illustrated in the Stormwater Pollution Prevention Plan submitted as part of this application. The SMF, in combination with the onsite pervious surface area, will ensure drainage requirements are met.

5. Placement of signage;

All proposed signage will adhere to the policies stated in Section 6.5 of Article 6 of the City of Alachua LDR.

6. Adequacy of site lighting and potential impacts of lighting upon the surrounding area. Lighting should be designed to minimize impacts and

preserve the ambiance and quality of the nighttime sky by reducing light trespass and light pollution on adjacent properties by utilizing lighting at an appropriate intensity, direction and times to ensure light is not overused or impacting areas where it is not intended;

A photometric plan is submitted as part of this application, demonstrating compliance with City of Alachua lighting requirements. Onsite lighting is limited to ensure safe navigation of the site, and to deter any unlawful activity after hours. Light placement will not adversely affect surrounding properties.

7. Safety of on-site circulation patterns (patron, employee and delivery vehicles), including parking layout and drive aisles, and points of conflict;

The proposed on-site parking area is designed for safe onsite circulation with minimal vehicle and pedestrian conflicts. A crosswalk across the frontage road allows pedestrians to access the on-site pedestrian facilities without walking through the parking area or additional drive aisles. Areas for delivery and trash pickup are located to the rear of the site, away from anticipated pedestrian activity.

8. Landscaping, as it relates to the requirements of the Comprehensive Plan and Land Development Regulations;

Future Land Use Element (FLUE) Policy 2.4.a states that the minimum landscaped area shall be 10% of the development site, not inclusive of any designated open space areas. The submitted Landscape Plan designates 25% of the project site as landscaped area, meeting the 10% requirement.

9. Unique features and resources which may constrain site development, such as soils, existing vegetation and historic significance; and

There are no environmental features or significant resources on-site that warrant protection.

10. Performance based zoning requirements, which may serve as a substitute for or accompany land development regulations in attaining acceptable site design.

There are no performance-based zoning requirements being proposed for this site.

11. Commercial uses shall be limited to an intensity of less than or equal to .50 floor area ratio for parcels 10 acres or greater, .50 floor area ratio for parcels less than 10 acres but 5 acres or greater, a .75 floor area ratio for parcels less than 5 acres but greater than 1 acre, and 1.0 floor area ratio to parcels 1 acre or less.

The project site is 43,520 square feet, or ±1.0 acre, and the building proposed is a one-story, ±1,900 sq. ft. structure. This yields a Floor Area Ratio (FAR) of 0.04. The proposed development onsite will not exceed the permitted 1.0 FAR.

Policy 1.3.f: The City shall pursue the establishment of activity centers and similar criteria to guide the placement and design of commercial and business areas.

1. The City shall establish and maintain a Gateway Activity Center at the interchange of US 441 and Interstate 75. The boundaries of the Gateway Activity Center shall be based on the Generalized US 441/I-75 Activity Center Map provided in the Future Land Use Map Series. The intent of this

activity center is to welcome existing and future residents and visitors to the City, and to promote Alachua as an attractive, vibrant, and economically prosperous community.

2. The City shall establish and maintain the US 441 corridor as a Corporate Corridor Activity Center. The intent of establishing this activity center is to implement economic development objectives and to promote a coordinated development plan to maximize existing and future land use patterns and preserve the function of the US 441 corridor.

The project site is in the Gateway Activity Center, per the Generalized US 441/I-75 Activity Center Map provided in the Future Land Use Map Series. This development will provide an economically productive use on this currently vacant parcel located along a major corridor, and will help in promoting the attractiveness, vibrance, and economic prosperity of the City of Alachua. This project also aligns with the intent of the Corporate Corridor Activity Center, contributing to the City's economic development by adding job opportunities and business along the US 441 corridor.

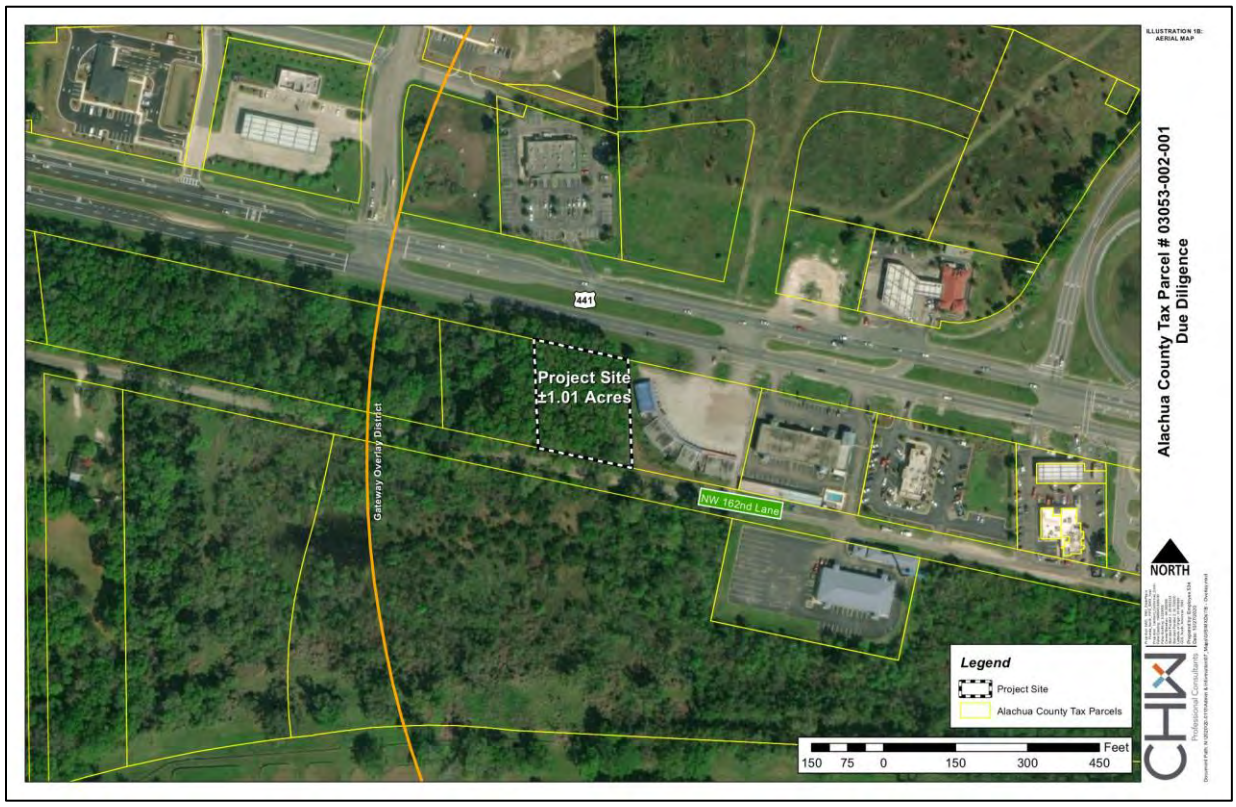


Figure 1: Project Site in Relation to Gateway Activity Center and Corporate Corridor Activity Center

Transportation Element

Objective 1.1: Level of Service

The City shall establish a safe, convenient and efficient level of service standard for all motorized and non-motorized transportation systems.

The proposed Fast Food Restaurant with Drive-through Window (Institute of Transportation Engineers (ITE) Land Use Code 934) will not result in a degradation of transportation Level of Service (LOS) standards, as is demonstrated in the complementary Concurrency Impact Analysis memorandum submitted as part of this application. Per LDR §2.4.14(H)(2)(a), affected roadway segments are those that fall within one-half (½) mile of the parcel's proposed

ingress/egress. These segments include Segments 1, 2, 5, 6, 7, 16, and 17, per the December 2020 Development Monitoring Report made available by City Staff.

According to the December 2020 Development Monitoring Report, there is more than enough capacity on the affected roadway segments to handle the increase in AADT created by the proposed application. This is demonstrated in the Concurrency Impact Analysis submitted as part of this application. Therefore, after build-out of the proposed development, there will continue to be a surplus of available trips.

Conservation and Open Space Element

Objective 1.5: Soils. The City shall protect soil resources through erosion and sedimentation control, by requiring proper design criteria on specific soils.

The site's soil types are Kendrick Sand, 2 to 5 percent slopes (hydro group: B), and Arredondo-Urban Land Complex, 0 to 5 percent slopes (hydro group: A). According to the NRCS soil database, the soil type is conducive to the proposed development, which is also demonstrated on adjacent sites with similar uses and soil types.

Objective 4.1 Infill development: Infill development shall be encouraged in order to protect the unique character of existing neighborhoods and commercial developments, provide for a safe urban environment, increase densities and intensities in a manner compatible with existing uses, provide open spaces, and restore or maintain economic vitality and cultural diversity.

The site's infill development creates new business and employment opportunities in the City. The infill development protects Alachua's unique character of existing neighborhoods by locating businesses in areas designated for commercial uses on the Future Land Use Map. Locating a quick service restaurant with drive-through in this location increases the intensity of the US 441 corridor in a manner that is compatible with existing development in the area. The proposed project will help maintain and enhance economic vitality within the City of Alachua by generating business and job opportunities.

Community Facilities Element

Policy 1.1.d: The City hereby establishes the following Level of Service standards for sanitary sewer facilities:

b. Quantity: System-wide wastewater collection and treatment will be sufficient to provide a minimum of 250 gallons per day per equivalent residential unit (ERU) on an average annual basis.

The proposed development will allow a net increase of ±1,900 sf of non-residential uses. Per the City of Alachua December 2020 Development Monitoring Report, there is a residual capacity of 679,702 gallons per day for Sanitary Sewer facilities. As is demonstrated in the Concurrency Impact Analysis submitted as part of this application, facility demands by the proposed non-residential use will not exceed this residual capacity.

Objective 2.1: Continue to ensure satisfactory and economical solid waste service for all City residents, with an emphasis on reuse and recycling.

Policy 2.1.a: The City hereby establishes the following level of service standards for solid waste disposal facilities:

FACILITY TYPE	LEVEL OF SERVICE STANDARD
Solid Waste Landfill	.73 tons per capita per year

The proposed development will allow a net increase of ±1,900 sf of non-residential uses. Per the City of Alachua December 2020 Development Monitoring Report, there is a 50-year build-out capacity for the solid waste disposal facility. As is demonstrated in the Concurrency Impact Analysis submitted as part of this application, facility demands by the proposed non-residential use will not exceed this build-out capacity.

Objective 4.1: Achieve and maintain acceptable levels of service for potable water quantity and quality.

Policy 4.1.c: The City establishes the following level of service standards for potable water:

2. Quantity: System-wide potable water distribution and treatment will be sufficient to provide a minimum of 275 gallons per day per equivalent residential unit (ERU) on an average annual basis.

The proposed development will allow a net increase of ±1,900 sf of non-residential uses. Per the City of Alachua December 2020 Development Monitoring Report, there is a residual capacity of 852,476 gallons per day for Potable Water facilities. As is demonstrated in the Concurrency Impact Analysis submitted as part of this application, facility demands by the proposed non-residential use will not exceed this residual capacity.

CONCURRENCY IMPACT ANALYSIS

KFC Alachua

20-0113

To: Kathy Winburn, AICP, City of Alachua Planning & Community Dev. Director

From: Seth Wood, Project Planner

Date: ~~December 23, 2020~~ March 9, 2021

RE: KFC Alachua – Concurrency Impact Analysis

This Concurrency Impact Analysis is submitted for KFC Alachua, on Alachua County Tax Parcel 03053-002-001. The parcel is located on the southern side of US 441, between NW 167th Boulevard and I-75. The site's Future Land Use (FLU) category is Commercial, and the Zoning District classification is Commercial, Intensive (CI). The proposed use is a ±1,904-square-foot quick service restaurant, with associated infrastructure and parking.

Trip generation figures are based on the Institute of Transportation Engineers (ITE) Trip Generation Manual, 10th Edition. The Concurrency Impact Analysis for this application is based on an **increase of ±1,904 square feet of Fast Food Restaurant with Drive-through Window space**, ITE Land Use Code 934.

The following analysis estimates potential impacts on City of Alachua public facilities that may result from the proposed development. The following tables include data from the City of Alachua December 2020 Development Monitoring Report, as well as data obtained within the City Comprehensive Plan and Florida Administrative Code (F.A.C.).

Roadways / Transportation

Table 1: Projected Trip Generation

Land Use	ITE LU Code	Variable (1000 SF)	Daily	PM Peak		
			Total	Total	In	Out
Fast Food Restaurant with Drive-Through Window	934	1.904	897	62	32	30
*Pass-by Trips, AM=49%, PM=50%, Weekday=49.5%			444	31	16	15
Net New Total Trips			453	31	16	15

Source: ITE Trip Generation 10th Edition

** The ITE provides a pass-by trip rate of 49 for the AM peak hour and 50 for the PM peak hour, but does not provide a pass-by rate for daily trips. The daily trip pass-by rate was estimated as the average of the AM and PM rates or 49.5%.*

Conclusion: Approval of this application may generate **453 new daily vehicle trips**. The proposed development *will not* negatively impact the adopted LOS for adjacent and nearby roadways, as demonstrated in Tables 1 and 2 of this report.

Per City LDR §2.4.14(H)(2), affected roadways for developments generating *less* than 1,000 external average daily trips (ADT) are those within one-half mile of the development's ingress/egress, or to the nearest intersecting major street, whichever is greater. Table 2 calculates projected impacts on City of Alachua roadways.

t: (904) 619-6521 | 8563 Argyle Business Loop, Suite 3, Jacksonville, FL 32244
t: (352) 331-1976 | 11801 Research Drive, Alachua, FL 32615
t: (352) 414-4621 | 101 NE 1st Avenue, Ocala, FL 34470
www.chw-inc.com

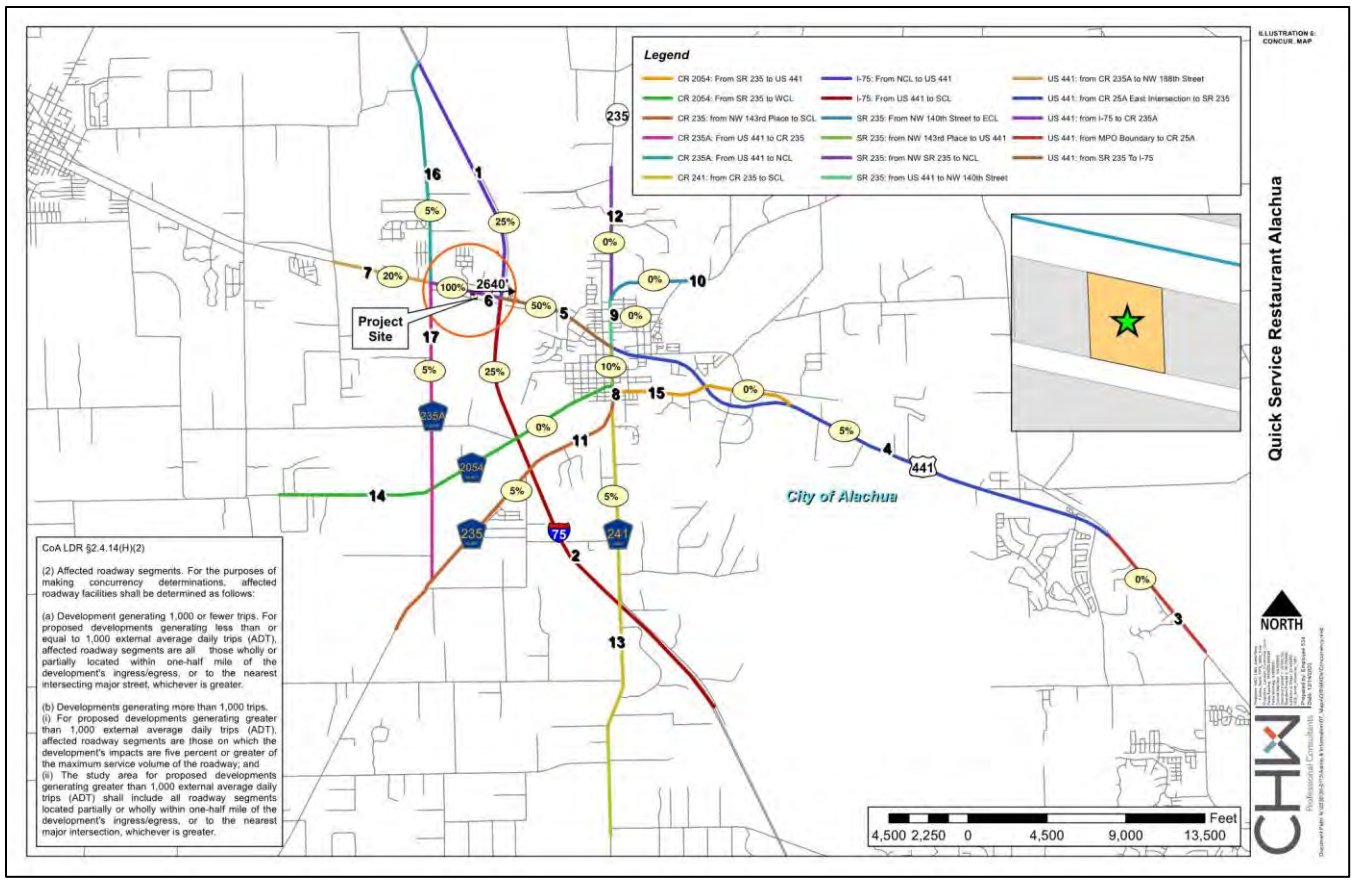


Figure 1: Anticipated Trip Distribution by Roadway Segment

Figure 1 illustrates the anticipated trip distribution. The project site is on US 441, between I-75 and NW 167th Boulevard. The center point used for the one-half mile radius measurement was the approximate ingress/egress location. Trips are anticipated to be generated predominately by customers and employees, who will likely live in the City of Alachua and surrounding communities.

Table 2: Projected Impacts on Roadways

	AADT	PM	AADT	PM	AADT	PM	AADT	PM	AADT	PM	AADT	PM	AADT	PM
Traffic System Category	**Segment 1:** I-75 From NCL to US 441		**Segment 2:** I-75 From US 441 to South City Limit		**Segment 5:** US Hwy 441 From SR 235 to I-75		**Segment 6:** US Hwy 441 From I-75 to CR 235A		**Segment 7:** US Hwy 441 From CR 235A to NW 188th Street		**Segment 16:** CR 235A From US 441 to NCL		**Segment 17:** CR 235A From US 441 to CR 235	
Maximum Service Volume[1]	91,600	8,250	91,600	8,250	39,000	3,510	39,000	3,510	43,000	3,870	15,120	1,359	15,120	1,359
Existing Traffic[1]	45,638	4,792	69,956	7,345	24,199	2,299	28,490	2,707	22,482	2,136	1,501	166	5,379	486
Reserved Trips[1]	508	38	508	38	3,286	290	3,286	290	3,286	290	809	84	191	20
Available Capacity[1]	45,454	3,420	21,136	867	11,515	921	7,224	513	17,232	1,444	12,810	1,109	9,550	853
Projected Trip Generation[2]	113	8	113	8	227	16	453	31	91	6	23	2	23	2
Available Capacity w/ Application approval	**45,341**	**3,412**	**21,023**	**859**	**11,289**	**906**	**6,771**	**482**	**17,141**	**1,438**	**12,787**	**1,107**	**9,527**	**851**

[1]*City of Alachua December 2020 Development Monitoring Report*
[2]*Note: Projected trip distribution percentage is estimated by 25% for Segment 1; 25% for Segment 2; 50% for Segment 5; 100% for Segment 6; 20% for Segment 7; 5% for Segment 16; and 5% for Segment 17.*

Conclusion: There is available capacity on all affected roadways with this application's approval. Therefore, this development *will not* cause the roadway segments to fall below the LOS standards.

Potable Water/Sanitary Sewer

Table 3: Projected Potable Water Impact

System Category	Gallons Per Day (GPD)
Current Permitted Capacity[1]	2,300,000
Less actual Potable Water Flows[1]	1,295,000
Reserved Capacity[1]	152,524
Residual Capacity[1]	852,476
Percentage of Permitted Design Capacity Utilized[1]	62.94%
Projected Potable Water Demand from Proposed Project[2]	1,523.2
Residual Capacity after Proposed Project	**850,952.8**

1. *Source: City of Alachua December 2020 Development Monitoring Report*
2. *Source: City of Alachua Comprehensive Plan & Chapter 64-E, F.A.C.*
   Formulas used: ITE 934 = ((1,904 sq. ft. x 60% dining space) / 15 sq. ft. per seat x 20 gpd)

Conclusion: There is residual capacity with this application's approval. Therefore, the proposed development *will not* cause the City's potable water facilities to fall below LOS standards.

Table 4: Projected Sanitary Sewer Impact

System Category	Gallons Per Day (GPD)
Treatment Plant Current Permitted Capacity[1]	1,500,000
Less actual Treatment Plant Flows[1]	675,000
Reserved Capacity[1]	145,298
Residual Capacity[1]	679,702
Percentage of Permitted Design Capacity Utilized[1]	54.69%
Projected Potable Water Demand from Proposed Project[2]	1,523.2
Residual Capacity after Proposed Project	**678,178.8**

1. *Source: City of Alachua December 2020 Development Monitoring Report*
2. *Source: City of Alachua Comprehensive Plan & Chapter 64-E, F.A.C.*
   Formulas used: ITE 934 = ((1,904 sq. ft. x 60% dining space) / 15 sq. ft. per seat x 20 gpd)

Conclusion: There is residual capacity with this application's approval. Therefore, the proposed development *will not* cause the City's sanitary sewer facilities to fall below LOS standards.

Solid Waste

Table 5: Projected Solid Waste Impact

System Category	LBs Per Day	Tons Per Year
Existing Demand[1]	41,192.00	7,517.54
Reserved Capacity[1]	9,953.23	1,816.46
New River Solid Waste Facility Capacity[1]	50 years	
Solid Waste Generated by Proposed Project[2]		**4.17**

1. *Source: City of Alachua December 2020 Development Monitoring Report.*
2. *Source: Sincero and Sincero; Environmental Engineering: A Design Approach. Prentice Hall, New Jersey, 1996.*
   Formula used: (((12 lbs. / 1,000 sq. ft./day x 1,904 sq. ft.) x 365)/2,000)

Conclusion: There is available capacity with this application's approval. Therefore, the proposed development *will not* cause the City's solid waste facilities to fall below LOS standards.

PROPERTY OWNER AFFIDAVIT

CHW
Professional Consultants

Owner Name: Sarcav, LLC			
Address: 2567 River Enclave Lane Jacksonville, Florida 32226	Phone: Contact CHW		
Agent Name: CHW			
Address: 11801 Research Drive Alachua, Florida 32615	Phone: (352) 331-1976		
Other: DE Foods, LLC			
Address: 101 Accord Park Drive Norwell, MA 02061	Phone: Contact CHW		
Parcel No.: 03053-002-000			
Acreage: ± 1.06	S: 09	T: 08	R: 18

Requested Action:

To submit and obtain approvals from the City of Alachua, Alachua County, Florida Department of Transportation, Florida Department of Environmental Protection, and Suwannee River Water Management District.

I hereby certify that:
I am the property owner of record. I authorize the above listed agent to act on my behalf for the purposes of this application.

Property owner signature: [signature]
Printed name: SILVIO CAVACEPPI
Date: 12/22/20

The foregoing instrument was acknowledged before me by means of ☐ physical presence or ☐ online notarization, this 22 day of December, 2020, by SILVIO CAVACEPPI.

Personally Known **OR** Produced Identification
Type of Identification Produced: FLDL

NOTARY SEAL: MIKE TONEY NOTARY My Comm. Expires Sept. 8, 2021 Comm. # GG 142022 PUBLIC STATE OF FLORIDA

[signature]
Signature of Notary Public, State of Florida

COUNTYWIDE STORMWATER TREATMENT CODE AFFIDAVIT OF COMPLIANCE

Instructions: Complete and submit this form for all activities that involve the construction of a stormwater management system that does not meet the exemptions identified in Chapter 77, Article III, Sec. 77.25 of the Alachua County Code. Please submit this form, along with the other required documents as listed in Sec. 77.28 of the Alachua County Code.

Part 1. Applicant Information	
Applicant/Owner:	Registered Professional:
Company:	Company:
Address:	Address:
Phone:	Phone:
Email:	Email:
Part 2. Project Information	
Project Name:	
Tax Parcel(s):	
☐ Unincorporated Alachua County ☐ Municipality: _____	Note: For projects located in Municipalities submittal of this form is a self-certification of compliance. An acknowledgement of receipt will be sent once all required documents are provided. This acknowledgement is not a review of the submitted materials. Projects in Unincorporated Alachua County will be reviewed pursuant to Sec. 77.27 of the Alachua County Code.
Part 3. Stormwater Discharge Information Please select all that apply:	
Watershed Name:	Waterbody Identification (WBID) #:
☐ Project discharges stormwater offsite as surface flow	☐ Project discharges stormwater to an Outstanding Florida Water (OFW)
☐ Project is in a WBID listed as impaired with nutrients as cause of impairment or where a nutrient TMDL has been established (net improvement criteria of Sec. 77.27 of the Alachua County Code applies).	
☐ Project stormwater infiltrates to groundwater	☐ Project is located in a Sensitive Karst Area
Please briefly describe the Best Management Practices used:	

Part 4. Exemption and Waiver Information
If applicable, Please list the exemption(s) this project qualifies for under Sec. 77.25 of the Alachua County Code. Supporting Documentation may be required.
If applicable, Please describe the waiver you are requesting under Sec. 77.26 of the Alachua County Code. Supporting documentation is required.

I authorize Alachua County, and its agents and contractors, to enter the property for the purpose of verifying compliance. If the property is sold and/or the entity responsible for operation and maintenance of the stormwater management system, the Property Owner will notify the Alachua County Environmental Protection Department within 30 calendar days of the sale or change in operation and maintenance entity. Failure to comply may result in enforcement action using the provisions of Alachua County Code Chapter 24 or any other remedy available by law or equity.

Applicant/Owner: _____ Signature: _____ Date: _____
(Printed Name)

I hereby certify that the above referenced project meets, or is exempt from, the requirements of the Alachua County Code Chapter 77, Article III Stormwater Treatment Code. I further certify that the Operation and Maintenance requirements have been provided to the owner and entity responsible for operation and maintenance of the stormwater management system.

Registered Professional: _____ Florida Registration Number: _____
(Printed Name)

Date: _____ Signature: _____

COUNTYWIDE WETLAND PROTECTION CODE SELF-CERTIFICATION FORM

Instructions: Submit this form to self-certify that the proposed activity complies with the surface water, wetland and buffer protection requirements outlined in Chapter 77, Article II of the Alachua County Code, or is exempt under Sec. 77.21. If you are unsure if your activity is in compliance or exempt, please fill out a Verification Request Form or contact staff at wetlands@alachuacounty.us for more information. Please note, you do not need to submit this form for bona-fide farm operations consistent with the provisions of F.S. 163.3162 or F.S. 823.14(6).

PART 1. APPLICANT INFORMATION	
Owner Name:	Applicant/Agent Name:
Phone:	Company:
Email:	Phone:
	Email:
PART 2. PROPERTY INFORMATION	
Tax Parcel Numbers:	Address:
City:	Zip:

PART 3. SELF-CERTIFICATION

I, _____ owner of the property or authorized agent for the owner, self-certify that the proposed activity complies with the surface water, wetland and buffer protection requirements found in Chapter 77, Article II of the Alachua County Code based on the following (select all that apply):

1. [ ] There are no wetlands, other surface waters, or buffers on or adjacent to the property (if you select this option, proceed to Acknowledgment and submit your self-certification)

2. [ ] There are wetlands, other surface waters, or buffers present on or adjacent to the property, but the proposed activity is outside of protected wetlands, surface waters and buffers (complete Part 4, Project Information)

3. [ ] There are wetlands, or other surface waters, or buffers present on or adjacent to the property, and there is work proposed in wetlands, other surface waters or buffers, but the proposed activity is exempt by Section 77.21(a) as follows (complete Part 4, Project Information)

4. [ ] There are wetlands, other surface waters, or buffers present on or adjacent to the property, but the proposed activity is exempt by Section 77.21(b) as an Urban Redevelopment Project (complete Part 5, Urban Redevelopment Projects).

PART 4. PROJECT INFORMATION

If you selected Self-certification [2] or [3] above, in addition to the information described in this form, your self-certification must also include:

- One set of plans, drawings, or sketches and other supporting documents that clearly and legibly depict and describe the proposed activities in sufficient detail to demonstrate that the work complies with protection requirements (buffer distances) or qualifies for the exemption.

Please answer the following:

Were wetlands and surface waters delineated by a qualified professional according to standards outlined in Florida Administrative Code Rule 62-340.300 for wetlands, and Rule 62-340.600 for surface waters?

☐ Yes ☐ No *Please note: wetland boundaries not delineated by a qualified professional are likely to be inaccurate. You may submit a Verification Request Form to verify compliance with protection requirements.*

Exemptions: Please list the exemption(s) you are requesting to use (refer to Sec. 77.21(a))

Describe in detail how the proposed work will comply with the terms and conditions of the above exemption:

Date Activity is proposed to commence _____ to be completed _____

PART 5. URBAN REDEVELOPMENT PROJECTS

Please submit the following:

- One set of plans and drawings, recent survey, environmental information, and other supporting documents and calculations clearly and legibly depicting that the existing site is 40%, or more, impervious surfaces.

ACKNOLWEDGMENT

I understand this notice is provided as self-certification of compliance with Chapter 77, Article II, Alachua County Code, or qualification to use exemption(s) found in Section 77.21.

_____ _____

Typed/Printed Name Signature Date

IMPORTANT: Applications for activities that do not comply with protection requirements found in Chapter 77, Article II nor any of the listed exemptions in Section 77.21, and are requesting to impact wetland and/or surface waters or their buffers must submit an ***Application Form***. Final Alachua County BOCC approval of the proposed impacts and Mitigation and Monitoring Plan must be secured prior to initiating the activity within wetlands or buffers.

Detail by Entity Name

Foreign Profit Corporation
C T CORPORATION SYSTEM

Filing Information

Document Number	804598
FEI/EIN Number	51-0006522
Date Filed	09/09/1936
State	DE
Status	ACTIVE
Last Event	CORPORATE MERGER
Event Date Filed	12/20/2013
Event Effective Date	12/31/2013

Principal Address

28 LIBERTY ST
NEW YORK, NY 10005

Changed: 04/24/2019

Mailing Address

28 LIBERTY ST
NEW YORK, NY 10005

Changed: 04/24/2019

Registered Agent Name & Address

THE CORPORATION COMPANY
1200 SOUTH PINE ISLAND RD.
PLANTATION, FL 33324

Name Changed: 09/01/2017

Address Changed: 09/01/2017

Officer/Director Detail

Name & Address

Title Member /Member Manager

Armstrong , Kimberly V
28 LIBERTY ST
NEW YORK, NY 10005

Annual Reports

Report Year	Filed Date
2020	03/18/2020
2020	04/22/2020
2020	05/20/2020

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09/09/1936 -- Domestic Profit	View image in PDF format
09/09/1936 -- RA CERTIFICATE	View image in PDF format

Detail by Entity Name

Foreign Limited Liability Company
D.E. FOODS, LLC

Filing Information

Document Number	M17000007229
FEI/EIN Number	27-4830609
Date Filed	08/21/2017
State	MA
Status	ACTIVE

Principal Address

170 OLDE FORGE ROAD
HANOVER, MA 02339

Mailing Address

170 OLDE FORGE ROAD
HANOVER, MA 02339

Registered Agent Name & Address

C T CORPORATION SYSTEM
1200 SOUTH PINE ISLAND ROAD

1200 SOUTH PINE ISLAND ROAD
PLANTATION, FL 33324

Authorized Person(s) Detail

Name & Address

Title MGR

EVANS, DAVID E
170 OLDE FORGE ROAD
HANOVER, MA 02339

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Report Year	Filed Date
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2019	04/02/2019
2020	01/22/2020

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04/06/2018 -- ANNUAL REPORT	View image in PDF format
08/21/2017 -- Foreign Limited	View image in PDF format

Detail by Entity Name

Florida Limited Liability Company
SARCAV, LLC

Filing Information

Document Number	L17000172317
FEI/EIN Number	82-5205337
Date Filed	08/11/2017
State	FL
Status	ACTIVE

Principal Address

2567 RIVER ENCLAVE LANE
JACKSONVILLE, FL 32226

Mailing Address

2567 RIVER ENCLAVE LANE
JACKSONVILLE, FL 32226

Registered Agent Name & Address

CAVACEPPI, SILVIO
2567 RIVER ENCLAVE LANE

2567 RIVER ENCLAVE LANE
JACKSONVILLE, FL 32226

Authorized Person(s) Detail

Name & Address

Title MGR

CAVACEPPI, SILVIO
2567 RIVER ENCLAVE LANE
JACKSONVILLE, FL 32226

Annual Reports

Report Year	Filed Date
2018	04/16/2018
2019	03/19/2019
2020	01/26/2020

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04/16/2018 -- ANNUAL REPORT	View image in PDF format
08/11/2017 -- Florida Limited Liability	View image in PDF format

Detail by Entity Name

Florida Limited Liability Company
T & H PROPERTY GROUP, LLC

Filing Information

Document Number	L18000059135
FEI/EIN Number	82-5056336
Date Filed	03/06/2018
State	FL
Status	ACTIVE
Last Event	LC AMENDMENT
Event Date Filed	03/22/2018
Event Effective Date	NONE

Principal Address

3580 NORTH MAIN ST
GAINESVILLE, FL 32609

Changed: 03/23/2018

Mailing Address

3580 NORTH MAIN ST
GAINESVILLE, FL 32609

Changed: 03/23/2018

Registered Agent Name & Address

HAWLEY, PHILLIP L
3580 N Main St
Gainesville, FL 32609

Address Changed: 03/26/2019

Authorized Person(s) Detail

Name & Address

Title AMBR

HAWLEY, KRISTI
300 SW 143RD ST.
JONESVILLE, FL 32669

Title AMBR

TOMLINSON, J D
3580 NORTH MAIN ST
GAINESVILLE, FL 32609

Title AMBR

TOMLINSON, TAMI
3580 NORTH MAIN ST
GAINESVILLE, FL 32609

Annual Reports

Report Year	Filed Date
2019	03/26/2019
2020	03/20/2020

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03/22/2018 -- LC Amendment	View image in PDF format
03/06/2018 -- Florida Limited Liability	View image in PDF format

Detail by Entity Name

Florida Profit Corporation
THE CORPORATION COMPANY

Filing Information

Document Number	017109
FEI/EIN Number	51-0099484
Date Filed	06/23/1925
State	FL
Status	ACTIVE
Last Event	AMENDMENT
Event Date Filed	12/31/1973
Event Effective Date	NONE

Principal Address

1200 S. PINE ISLAND ROAD
PLANTATION, FL 33324

Changed: 04/25/2019

Mailing Address

1200 S. PINE ISLAND ROAD
PLANTATION, FL 33324

Changed: 04/25/2019

Registered Agent Name & Address

C T CORPORATION SYSTEM
1200 S. PINE ISLAND ROAD
PLANTATION, FL 33324

Name Changed: 04/25/2019

Address Changed: 09/21/2018

Officer/Director Detail

Name & Address

Title President

WEBER, JOHN
28 LIBERTY ST 42ND FL
NEW YORK, NY 10005

Title EVP, Secretary

INGATO, ROBERT
28 LIBERTY ST 26TH FL
NEW YORK, NY 10005

Title Asst. Secretary

SANDERS, ERIN

2700 LAKE COOK ROAD
RIVERWOODS, IL 60015

Title Director, VP, Asst. Treasurer

BALNIUS, J. MICHELE
2700 LAKE COOK ROAD
RIVERWOODS, IL 60015

Title Treasurer, VP

FELDMAN, IRVING
2700 LAKE COOK ROAD
RIVERWOODS, IL 60015

Title Director

SATIN, ELIZABETH
28 LIBERTY ST 26TH FL
NEW YORK, NY 10005

Title Director

NESTOR, THOMAS J
28 LIBERTY ST 43RD FL
NEW YORK, NY 10005

Annual Reports

Report Year	Filed Date
2018	04/24/2018
2019	04/25/2019

2020	04/22/2020

Document Images

04/22/2020 -- ANNUAL REPORT	View image in PDF format
04/25/2019 -- ANNUAL REPORT	View image in PDF format
09/21/2018 -- AMENDED ANNUAL REPORT	View image in PDF format
04/24/2018 -- ANNUAL REPORT	View image in PDF format
04/03/2017 -- AMENDED ANNUAL REPORT	View image in PDF format
03/24/2017 -- ANNUAL REPORT	View image in PDF format
04/21/2016 -- ANNUAL REPORT	View image in PDF format
04/20/2015 -- ANNUAL REPORT	View image in PDF format
02/24/2014 -- ANNUAL REPORT	View image in PDF format
04/12/2013 -- ANNUAL REPORT	View image in PDF format
04/12/2012 -- ANNUAL REPORT	View image in PDF format
01/06/2011 -- ANNUAL REPORT	View image in PDF format
04/26/2010 -- ANNUAL REPORT	View image in PDF format
04/28/2009 -- ANNUAL REPORT	View image in PDF format
04/07/2008 -- ANNUAL REPORT	View image in PDF format
04/12/2007 -- ANNUAL REPORT	View image in PDF format
04/24/2006 -- ANNUAL REPORT	View image in PDF format
04/11/2005 -- ANNUAL REPORT	View image in PDF format
04/15/2004 -- ANNUAL REPORT	View image in PDF format
01/21/2003 -- ANNUAL REPORT	View image in PDF format
02/04/2002 -- ANNUAL REPORT	View image in PDF format
03/19/2001 -- ANNUAL REPORT	View image in PDF format
05/05/2000 -- ANNUAL REPORT	View image in PDF format
04/19/1999 -- ANNUAL REPORT	View image in PDF format
04/23/1998 -- ANNUAL REPORT	View image in PDF format

01/27/1997 -- ANNUAL REPORT	View image in PDF format
05/01/1996 -- ANNUAL REPORT	View image in PDF format
07/12/1995 -- Dom/For AR	View image in PDF format
06/22/1995 -- ANNUAL REPORT	View image in PDF format
05/01/1995 -- Dom/For AR	View image in PDF format
05/01/1994 -- Dom/For AR	View image in PDF format
05/01/1993 -- Dom/For AR	View image in PDF format
07/21/1992 -- Dom/For AR	View image in PDF format
01/31/1992 -- Reg. Agent Change	View image in PDF format
06/28/1991 -- Dom/For AR	View image in PDF format
06/29/1990 -- Dom/For AR	View image in PDF format
07/13/1989 -- Dom/For AR	View image in PDF format
07/08/1988 -- Dom/For AR	View image in PDF format
06/30/1987 -- Dom/For AR	View image in PDF format
06/30/1986 -- Dom/For AR	View image in PDF format
07/13/1983 -- Dom/For AR	View image in PDF format
08/03/1982 -- Dom/For AR	View image in PDF format
06/30/1980 -- Dom/For AR	View image in PDF format
07/19/1979 -- Dom/For AR	View image in PDF format
06/30/1978 -- Dom/For AR	View image in PDF format
06/30/1977 -- Dom/For AR	View image in PDF format
06/30/1975 -- Dom/For AR	View image in PDF format
07/28/1974 -- Dom/For AR	View image in PDF format
07/13/1974 -- Dom/For AR	View image in PDF format
04/05/1974 -- Amendment	View image in PDF format
06/18/1925 -- Domestic Profit	View image in PDF format

LEGAL DESCRIPTION

DATE: January 28, 2021
CLIENT: D.E. Foods, LLC
PROJECT NAME: QSR – KFC Alachua
PROJECT NO: 20-0113
DESCRIPTION FOR: 03053-002-000

A parcel of land situated in Section 9, Township 8 South, Range 18 East, Alachua County, Florida, being more particularly described as follows:

For a point of reference commence at the Southeast corner of said Section 9; thence North 03°37' 28" West, a distance of 286.74 feet to the North Right of Way line of Old S.R. No. 25 (Old US Highway 441); thence North 79°23' 05" West, along said North Right of Way line, a distance of 2552.76 feet to the Point of Beginnings thence continue North 79°23' 05“ West, along said North Right of Way line, a dietence of 198.88 feet to the Southwest corner of Parcel 2 of the lands described in Official Records Book 4233, Page 1337 of the Public Records of Alachua County, Florida; thence North 03°02' 54" West, along the West line of said lands, a distance of 226.49 feet to the Northwest corner of said lands, lying on the Southerly Right of Way line of New Highway No. 441 (200' wide Right of Way); thence South 79”05' 00" East, along said Southerly Right of Way line, a distance of 199.14 feet; thence departing said Southerly Right of Way line, South 03°02' 56" East, a distance of 225.41 feet to the Point of Beginning.

LEGAL DESCRIPTION

DATE: January 28, 2021
CLIENT: D.E. Foods, LLC
PROJECT NAME: QSR – KFC Alachua
PROJECT NO: 20-0113
DESCRIPTION FOR: 03053-002-001

A parcel of land situated in Section 9, Township 8 South, Range 18 East, Alachua County, Florida, being more particularly described as follows:

For a point of reference commence at the Southeast corner of said Section 9; thence North 03°37' 28" West, a distance of 286.74 feet to the North Right of Way line of Old S.R. No. 25 (Old US Highway 44t); thence North 79°23' 05" West, along said North Right of Way line, a distance of 2353.88 feet to the Southeast corner of Parcel 2 of the lands described in Official Records Book 4233, Page 1337 of the Public Records of Alachua County, Florida and the Point of Beginning; thence continue North 79°23’ 05" West, along said North Right of Way line, a distance of 198.88 feet; thence North 03°02' 56" West, a distance of 225.41 feet to the Southerly Right of Way line of New Highway No. 441 (200’ wide Right of Way); thence South 79“05' 00" East, along said Southerly Right of Way line, a distance of 199.13 feet to the Northeast corner of said Parcel 2; thence South 03°02' 54" East, along the East line of said Parcel 2, a distance of 224.33 feet to the Point of Beginning.

Account Summary

Real Estate Account #03053 002 000

Print this page

Owner:
SARCAV LLC

Situs:
UNASSIGNED LOCATION RE

ⓘ Parcel details

Get bills by email

Amount due

Your account is **paid in full**. There is nothing due at this time.
Your last payment was made on **11/07/2020** for **$7,951.35**. **Print paid bill (PDF)**

Apply for the 2021 installment payment plan

Account history

AMOUNT

BILL	DUE AMOUNT	STATUS		ACTION
BILL	DUE	STATUS		ACTION
2020 Annual Bill	$0.00	**Paid** $7,951.35	11/07/2020	**Receipt** #20-0006859	**Print (PDF)**
2019				
2019 Installment Bill #4	$0.00	**Paid** $1,696.33	11/27/2019	**Receipt** #19-0039890	**Print (PDF)**
2019 Installment Bill #3	$0.00	**Paid** $1,645.44	11/27/2019	**Receipt** #19-0039890	**Print (PDF)**
2019 Installment Bill #2	$0.00	**Paid** $2,569.93	11/27/2019	**Receipt** #19-0039890	**Print (PDF)**
2019 Installment Bill #1	$0.00	**Paid** $2,415.73	06/24/2019	**Receipt** #18-0198656	**Print (PDF)**
		Paid $8,327.43			
2018				
2018 Installment Bill #3	$0.00	**Paid** $2,523.70	11/29/2018	**Receipt** #18-0039340	**Print (PDF)**
2018 Installment Bill #4	$0.00	**Paid** $2,601.74	11/29/2018	**Receipt** #18-0039340	**Print (PDF)**
2018 Installment Bill #2	$0.00	**Paid** $2,423.90	09/12/2018	**Receipt** #17-0248648	**Print (PDF)**
2018 Installment Bill #1	$0.00	**Paid** $2,385.83	06/13/2018	**Receipt** #17-0191492	**Print (PDF)**
Total Amount Due	**$0.00**	**Paid $9,935.17**			

BILL	AMOUNT DUE	STATUS		ACTION
2017				
2017 Installment Bill #4	$0.00	**Paid** $3,415.52	03/21/2018	**Receipt** #17-0134926 **Print (PDF)**
2017 Installment Bill #3	$0.00	**Paid** $3,313.05	02/13/2018	**Receipt** #17-0107353 **Print (PDF)**
2017 Installment Bill #2	$0.00	**Paid** $1,585.99	09/25/2017	**Receipt** #16-0248335 **Print (PDF)**
2017 Installment Bill #1	$0.00	**Paid** $1,561.08	06/30/2017	**Receipt** #16-0196124 **Print (PDF)**
		Paid $9,875.64		
2016				
2016 Installment Bill #4	$0.00	**Paid** $1,628.80	03/31/2017	**Receipt** #16-0141640 **Print (PDF)**
2016 Installment Bill #3	$0.00	**Paid** $1,579.95	12/23/2016	**Receipt** #16-0066799 **Print (PDF)**
2016 Installment Bill #2	$0.00	**Paid** $1,616.47	09/16/2016	**Receipt** #15-0225794 **Print (PDF)**
2016 Installment Bill #1	$0.00	**Paid** $1,591.08	06/10/2016	**Receipt** #15-0168077 **Print (PDF)**
		Paid $6,416.30		
2015				
Total Amount Due	**$0.00**			

BILL	AMOUNT DUE	STATUS			ACTION
2015 Installment Bill #4	$0.00	**Paid** $1,714.09	03/31/2016	**Receipt** #15-0131669	**Print (PDF)**
2015 Installment Bill #3	$0.00	**Paid** $1,662.67	12/23/2015	**Receipt** #15-0059165	**Print (PDF)**
2015 Installment Bill #2	$0.00	**Paid** $1,595.99	09/22/2015	**Receipt** #14-0219220	**Print (PDF)**
2015 Installment Bill #1	$0.00	**Paid** $1,570.92	06/23/2015	**Receipt** #14-0169113	**Print (PDF)**
		Paid $6,543.67			
2014					
2014 Installment Bill #4	$0.00	**Paid** $1,671.71	03/31/2015	**Receipt** #14-0122314	**Print (PDF)**
2014 Installment Bill #3	$0.00	**Paid** $1,621.56	12/30/2014	**Receipt** #14-0062864	**Print (PDF)**
2014 Installment Bill #2	$0.00	**Paid** $1,595.48	09/30/2014	**Receipt** #13-0224207	**Print (PDF)**
2014 Installment Bill #1	$0.00	**Paid** $1,754.19	07/24/2014	**Receipt** #13-0185889	**Print (PDF)**
		Paid $6,642.94			
2013					
2013 Installment Bill #4	$0.00	**Paid** $1,673.24	03/31/2014	**Receipt** #13-0125272	**Print (PDF)**

Total Amount Due **$0.00**

BILL	AMOUNT DUE	STATUS		ACTION
2013 Installment Bill #3	$0.00	**Paid** $1,623.04	12/23/2013	**Receipt** #13-0056615	**Print (PDF)**
2013 Installment Bill #2	$0.00	**Paid** $1,668.08	10/14/2013	**Receipt** #12-0221286	**Print (PDF)**
2013 Installment Bill #1	$0.00	**Paid** $1,568.00	06/18/2013	**Receipt** #12-0156808	**Print (PDF)**
		Paid $6,532.36			
2012					
2012 Installment Bill #4	$0.00	**Paid** $1,631.38	04/01/2013	**Receipt** #12-0116837	**Print (PDF)**
2012 Installment Bill #3	$0.00	**Paid** $1,582.45	12/31/2012	**Receipt** #12-0065874	**Print (PDF)**
2012 Installment Bill #2	$0.00	**Paid** $1,704.77	12/31/2012	**Receipt** #12-0065874	**Print (PDF)**
2012 Installment Bill #1	$0.00	**Paid** $1,602.48	06/26/2012	**Receipt** #2011-1090635	**Print (PDF)**
		Paid $6,521.08			
2011 Annual Bill	$0.00	**Paid** $6,750.89	02/29/2012	**Receipt** #2011-1050052	**Print (PDF)**
2010 Annual Bill	$0.00	**Paid** $7,659.22	12/30/2010	**Receipt** #2010-1027865	**Print (PDF)**
2009 Annual Bill	$0.00	**Paid**	12/30/2009	**Receipt** #2009-	**Print**

Total Amount Due **$0.00**

		$7,621.96		9028949	**(PDF)**

BILL	AMOUNT DUE	STATUS			ACTION
2008 Annual Bill	$0.00	**Paid** $6,829.69	11/12/2008	**Receipt** #2008-8004375	**Print (PDF)**
2007 Annual Bill	$0.00	**Paid** $6,853.45	11/30/2007	**Receipt** #2007-7006662	**Print (PDF)**
2006 Annual Bill	$0.00	**Paid** $7,615.16	11/30/2006	**Receipt** #2006-6006248	**Print (PDF)**
2005 Annual Bill	$0.00	**Paid** $5,610.61	11/01/2005	**Receipt** #2005-5000043	**Print (PDF)**
2004 Annual Bill	$0.00	**Paid** $5,665.45	11/15/2004	**Receipt** #2004-4004457	**Print (PDF)**
2003					
2003 Installment Bill #4	$0.00	**Paid** $1,488.35	12/19/2003	**Receipt** #2003-3053174	**Print (PDF)**
2003 Installment Bill #3	$0.00	**Paid** $1,443.70	12/19/2003	**Receipt** #2003-3053174	**Print (PDF)**
2003 Installment Bill #2	$0.00	**Paid** $1,462.46	09/09/2003	**Receipt** #2002-0282834	**Print (PDF)**
2003 Installment Bill #1	$0.00	**Paid** $1,439.49	06/25/2003	**Receipt** #2002-0278206	**Print (PDF)**
		Paid $5,834.00			
2002					
2002 Installment Bill #4	$0.00	**Paid** $1,531.48	03/14/2003	**Receipt** #2002-0264841	**Print (PDF)**
Total Amount Due	**$0.00**				

BILL	AMOUNT DUE	STATUS			ACTION
2002 Installment Bill #3	$0.00	**Paid** $1,485.55	12/12/2002	**Receipt** #2002-0249273	**Print (PDF)**
2002 Installment Bill #2	$0.00	**Paid** $1,462.35	09/18/2002	**Receipt** #2001-0182562	**Print (PDF)**
2002 Installment Bill #1	$0.00	**Paid** $1,439.38	06/26/2002	**Receipt** #2001-0176999	**Print (PDF)**
		Paid $5,918.76			
Total Amount Due	**$0.00**				

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Account Summary

Real Estate Account #03053 002 001

Print this page

Owner:
T & H PROPERTY GROUP LLC

Situs:
UNASSIGNED LOCATION RE
Parcel details

Get bills by email

Amount due

Your account is **paid in full**. There is nothing due at this time.
Your last payment was made on **11/19/2020** for **$7,505.82**. **Print paid bill (PDF)**

Apply for the 2021 installment payment plan

Account history

AMOUNT

BILL	DUE AMOUNT	STATUS		ACTION
BILL	DUE	STATUS		ACTION
2020 Annual Bill ⓘ	$0.00	**Paid** $7,505.82	11/19/2020	**Receipt** #20-0025390	**Print (PDF)**
2019 Annual Bill ⓘ	$0.00	**Paid** $7,732.10	11/26/2019	**Receipt** #19-0036463	**Print (PDF)**
Total Amount Due	**$0.00**				

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AYESHA SOLOMON
ALACHUA COUNTY
PROPERTY APPRAISER
KNOWLEDGEABLE - COMMUNITY FOCUSED - COMMITTED

Parcel Summary

Parcel ID	03053-002-000
Prop ID	12665
Location Address	
Neighborhood/Area	ALACHUA COMMERCIAL (233200.50)
Subdivision	
Brief Legal Description*	COM SE COR SEC N 03 DEG W 286.74 FT TO N R/W OLD SR 25 N 79 DEG W 2552.76 FT POB N 79 DEG W 198.88 FT N 03 DEG W 226.49 FT TO S R/W US 441 S 79 DEG E ALG R/W 199.14 FT S 03 DEG E 225.41 FT POB OR 4645/1472 (Note: *The Description above is not to be used on legal documents.)
Property Use Code	VACANT COMM (01000)
Sec/Twp/Rng	09-08-18
Tax District	ALACHUA (District 1006)
Millage Rate	22.1970
Acreage	1.060
Homestead	N

View Map

Owner Information

SARCAV LLC
2567 RIVER ENCLAVE LN
JACKSONVILLE, FL 32226

Valuation

	2020 Certified Values	2019 Certified Values	2018 Certified Values	2017 Certified Values	2016 Certified Values
Improvement Value	$0	$0	$0	$0	$0
Land Value	$369,389	$369,389	$717,700	$717,700	$269,200
Land Agricultural Value	$0	$0	$0	$0	$0
Agricultural (Market) Value	$0	$0	$0	$0	$0
Just (Market) Value	**$369,389**	**$369,389**	**$717,700**	**$717,700**	**$269,200**
Assessed Value	$369,389	$369,389	$325,730	$296,120	$269,200
Exempt Value	$0	$0	$0	$0	$0
Taxable Value	$369,389	$369,389	$325,730	$296,120	$269,200
Maximum Save Our Homes Portability	$0	$0	$391,970	$421,580	$0

"Just (Market) Value" description - This is the value established by the Property Appraiser for ad valorem purposes. This value does not represent anticipated selling price.

TRIM Notice

2020 TRIM Notice (PDF)

Land Information

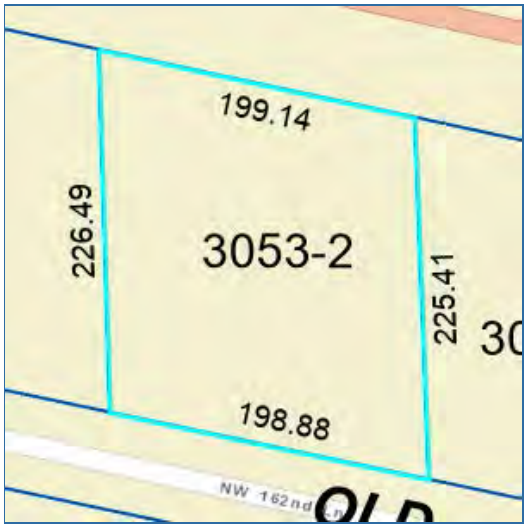
Land Use	Land Use Desc	Acres	Square Feet	Eff. Frontage	Depth	Zoning
1000	VACANT COMMERCIAL	1.06	46173.6	0	0	

Sales

Sale Date	Sale Price	Instrument	Book	Page	Qualified	Vacant/Improved	Grantor	Grantee	Link to Official Records
11/16/2018	$347,500	WD	4645	1472	37 - NOT EXPOSED TO OPEN MKT	Vacant			Link (Clerk)
10/7/2013	$220,000	MS	4233	1337	30 - AFFILIATED PARTIES	Improved	PATEL CHARU S	PATEL, INDIRA K	Link (Clerk)
11/6/2007	$50,000	WD	3703	699	U - UNQUALIFIED	Vacant	KANU S PATEL ////KANU S P	PATEL CHARU S	Link (Clerk)
1/12/2006	$50,000	WD	3298	1391	U - UNQUALIFIED	Vacant	PATEL KANU S	KANU S PATEL	Link (Clerk)
9/3/1992	$200,000	WD	1873	50	Q - QUALIFIED	Vacant	BILLIE SHARLEEN OWNBEY CAVACEP	PATEL KANU S	Link (Clerk)

Official Public Records information is provided by the Alachua County Clerk's Office. Clicking on these links will direct you to their web site displaying the document details for this specific transaction.

Map



No data available for the following modules: Building Information, Sub Area, Extra Features, Permits, Sketches, Photos.

This web application and the data herein is prepared for the inventory of real property found within Alachua County and is compiled from recorded deeds, plats, and other public records and data. Users of this web application and the data herein are hereby notified that the aforementioned public primary information sources should be consulted for verification of the information. Alachua County Property Appraiser's Office assumes no legal responsibility for the information contained herein.
User Privacy Policy
GDPR Privacy Notice

Developed by Schneider GEOSPATIAL

Last Data Upload: 3/9/2021, 5:19:11 AM

Version 2.3.110

AYESHA SOLOMON
ALACHUA COUNTY
PROPERTY APPRAISER
KNOWLEDGEABLE - COMMUNITY FOCUSED - COMMITTED

Parcel Summary

Parcel ID	03053-002-001
Prop ID	400000
Location Address	
Neighborhood/Area	ALACHUA COMMERCIAL (233200.50)
Subdivision	
Brief Legal Description*	COM SE COR SEC N 03 DEG W 286.74 FT TO N R/W OLD SR 25 N 79 DEG W 2353.88 FT POB N 79 DEG W 198.88 FT N 03 DEG W 225.41 FT TO S R/W US 441 S 79 DEG E ALG R/W 199.13 FT S 03 DEG E 224.33 FT POB OR 4645/1470 (Note: *The Description above is not to be used on legal documents.)
Property Use Code	VACANT COMM (01000)
Sec/Twp/Rng	09-08-18
Tax District	ALACHUA (District 1006)
Millage Rate	22.1970
Acreage	1.000
Homestead	N

View Map

Owner Information

T & H PROPERTY GROUP LLC
3580 N MAIN ST
GAINESVILLE, FL 32609

Valuation

	2020 Certified Values	2019 Certified Values
Improvement Value	$0	$0
Land Value	$348,480	$348,480
Land Agricultural Value	$0	$0
Agricultural (Market) Value	$0	$0
Just (Market) Value	**$348,480**	**$348,480**
Assessed Value	$348,480	$348,480
Exempt Value	$0	$0
Taxable Value	$348,480	$348,480
Maximum Save Our Homes Portability	$0	$0

"Just (Market) Value" description - This is the value established by the Property Appraiser for ad valorem purposes. This value does not represent anticipated selling price.

TRIM Notice

2020 TRIM Notice (PDF)

Land Information

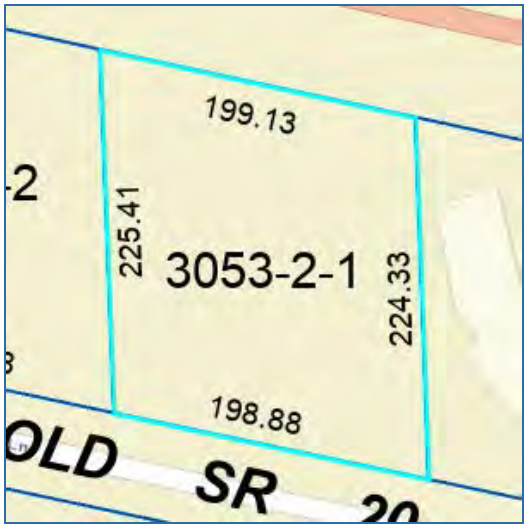
Land Use	Land Use Desc	Acres	Square Feet	Eff. Frontage	Depth	Zoning
1000	VACANT COMMERCIAL	1.00	43560	0	0	

Sales

Sale Date	Sale Price	Instrument	Book	Page	Qualified	Vacant/Improved	Grantor	Grantee	Link to Official Records
11/15/2018	$347,500	WD	4645	1470	37 - NOT EXPOSED TO OPEN MKT	Vacant			Link (Clerk)

Official Public Records information is provided by the Alachua County Clerk's Office. Clicking on these links will direct you to their web site displaying the document details for this specific transaction.

Map



No data available for the following modules: Building Information, Sub Area, Extra Features, Permits, Sketches, Photos.

This web application and the data herein is prepared for the inventory of real property found within Alachua County and is compiled from recorded deeds, plats, and other public records and data. Users of this web application and the data herein are hereby notified that the aforementioned public primary information sources should be consulted for verification of the information. Alachua County Property Appraiser's Office assumes no legal responsibility for the information contained herein.
User Privacy Policy
GDPR Privacy Notice

Developed by Schneider GEOSPATIAL

Last Data Upload: 3/9/2021, 5:19:11 AM

Version 2.3.110

AYESHA SOLOMON
ALACHUA COUNTY
PROPERTY APPRAISER
KNOWLEDGEABLE - COMMUNITY FOCUSED - COMMITTED

Parcel Summary

Parcel ID	03061-004-001
Prop ID	12743
Location Address	
	ALACHUA, FL 32615
Neighborhood/Area	ALACHUA COMMERCIAL (233200.50)
Subdivision	
Brief Legal Description*	THE W 281.82 FT OF COM SE COR OF SEC RUN N 3 DEG W 286.74 FT TO N R/W OLD SR 20 N 79 DEG W 1876.09 FT TO POB N 79 DEG W 477.79 FT N 3 DEG W 224.35 FT TO S R/W US 441 S 79 DEG E 531.82 FT S 10 DEG W 215.45 FT TO POB SHOPPING CENTER SITE OR 4581/0880
	(Note: *The Description above is not to be used on legal documents.)
Property Use Code	SH CTR CMMITY (01600)
Sec/Twp/Rng	09-08-18
Tax District	ALACHUA (District 1006)
Millage Rate	22.1970
Acreage	1.280
Homestead	N

View Map



Owner Information

T & H PROPERTY GROUP LLC
C/O BRAVO FLORIDA LLC
4220 EDISON LAKE PKWY
MISHAWAKA, IN 46545

Valuation

	2020 Certified Values	2019 Certified Values	2018 Certified Values	2017 Certified Values	2016 Certified Values
Improvement Value	$18,125	$18,125	$1,200	$230,700	$213,900
Land Value	$425,402	$425,402	$425,400	$531,800	$335,800
Land Agricultural Value	$0	$0	$0	$0	$0
Agricultural (Market) Value	$0	$0	$0	$0	$0
Just (Market) Value	**$443,527**	**$443,527**	**$426,600**	**$762,500**	**$549,700**
Assessed Value	$443,527	$443,527	$426,600	$493,190	$448,360
Exempt Value	$0	$0	$0	$0	$0
Taxable Value	$443,527	$443,527	$426,600	$493,190	$448,360
Maximum Save Our Homes Portability	$0	$0	$0	$269,310	$101,340

"Just (Market) Value" description - This is the value established by the Property Appraiser for ad valorem purposes. This value does not represent anticipated selling price.

TRIM Notice

2020 TRIM Notice (PDF)

Land Information

Land Use	Land Use Desc	Acres	Square Feet	Eff. Frontage	Depth	Zoning
1000		1.29	55974	0	0	

Building Information

Type	SOH MISC	**Heat**	
Total Area	31,004	**HC&V**	
Heated Area		**HVAC**	
Exterior Walls		**Bathrooms**	
Interior Walls		**Bedrooms**	
Roofing		**Total Rooms**	
Roof Type		**Stories**	1.0
Frame		**Actual Year Built**	0
Floor Cover		**Effective Year Built**	1973

Sub Area

Type	Description	Sq. Footage	Year	Quality	Imprv Use	Imprv Use Descr	Value
4420	LIGHTS	4	1973		C1	COMM	$300
4680	PAVING 1	31,000	1973		C1	COMM	$17,825

Sales

Sale Date	Sale Price	Instrument	Book	Page	Qualified	Vacant/Improved	Grantor	Grantee	Link to Official Records
3/8/2018	$499,000	SD	4581	0880	02 - CREDIBLE EVIDENCE	Improved		T & H PROPERTY GROUP LLC	Link (Clerk)
3/1/1983	$350,000	WD	1473	248	Q - QUALIFIED	Improved		CIRCLE K STORES INC	Link (Clerk)
2/1/1981	$0	WD	1334	895	U - UNQUALIFIED	Improved		ENERGY SPECIALTIES INC	Link (Clerk)

Official Public Records information is provided by the Alachua County Clerk's Office. Clicking on these links will direct you to their web site displaying the document details for this specific transaction.

Permits

Permit Number	Type	Primary	Active	Issue Date	Value
P10-20-7906	MISCELLANEOUS	Yes	Yes	10/29/2020	$0
P09-20-7827	SIGN PERMIT	Yes	Yes	10/14/2020	$62,900
P08-20-7770	COMMERCIAL BUILDING	Yes	Yes	9/15/2020	$480,012
6-19-6936	COMM REMODEL PERMIT	Yes	No	6/25/2019	$0
9-17-5707	MISCELLANEOUS	Yes	No	9/21/2017	$0
10-16-5096	SIGN PERMIT	Yes	No	10/10/2016	$2,400
A8-12-2731	SIGN PERMIT	Yes	No	9/4/2012	$3,450
A8-11-2288	COMM REMODEL PERMIT	Yes	No	8/8/2011	$48,956
A9-10-1959	MECHANICAL	Yes	No	9/10/2010	$3,571
07-10935	SIGN PERMIT	Yes	No	3/7/2007	$8,000
06-10406	SERVICE UPGRADE	Yes	No	4/13/2006	$0
06-10403	CHANGE OF USE-NO CONST	Yes	No	4/13/2006	$0
9064	SIGN PERMIT	Yes	No	1/12/2004	$785
9065	SIGN PERMIT	Yes	No	12/15/2003	$785
8871	SIGN PERMIT	Yes	No	7/29/2003	$450
8613	MISCELLANEOUS	Yes	No	3/5/2003	$7,000
7397	COMMERCIAL MECHANICAL	Yes	No	5/23/2000	$3,500
7393	REMODEL COMMERCIAL	Yes	No	5/22/2000	$10,000
7390	REMODEL COMMERCIAL	Yes	No	5/18/2000	$26,111
6561	SIGN PERMIT	Yes	No	12/22/1997	$400
06290	MISCELLANEOUS	Yes	No	4/1/1997	$7,000
06095	ROOFING	Yes	No	8/13/1996	$10,000

Our permitting information is pulled from the Alachua County Permitting Offices. Permitting information shown here is all the Property Appraiser has on file for this property. Any detailed questions about permits should be directed to the Permitting Offices.

Map



Photos

No data available for the following modules: Extra Features, Sketches.

This web application and the data herein is prepared for the inventory of real property found within Alachua County and is compiled from recorded deeds, plats, and other public records and data. Users of this web application and the data herein are hereby notified that the aforementioned public primary information sources should be consulted for verification of the information. Alachua County Property Appraiser's Office assumes no legal responsibility for the information contained herein.
User Privacy Policy
GDPR Privacy Notice

Developed by Schneider GEOSPATIAL

Last Data Upload: 3/9/2021, 5:19:11 AM

Version 2.3.110

RECORDED IN OFFICIAL RECORDS
INSTRUMENT # 3160280 2 PG(S)
11/26/2018 9:57 AM
BOOK 4645 PAGE 1472
J.K. JESS IRBY, ESQ.
Clerk of the Court, Alachua County, Florida
ERECORDED Receipt # 861830
Doc Stamp-Mort: $0.00
Doc Stamp-Deed: $2,432.50
Intang. Tax: $0.00

Prepared By and Return To: Kimberly G. Bosshardt, Esq.
Bosshardt Title Insurance Agency, LLC
5532 NW 43rd Street
Gainesville, FL 32653

For the issuance of title insurance, file #: 18-1213

[Space Above This Line For Recording Data]

WARRANTY DEED

This Warranty Deed made **November 16, 2018** between **Indira K. Patel,** whose address is 8706 Saddlehorn Drive, Irving, TX 75063, hereinafter called the grantor, to **SARCAV, LLC, a Florida Limited Liability Company,** whose post office address is: 2567 River Enclave Lane, Jacksonville, FL 32226, hereinafter called the grantee:

Witnesseth, that said grantor, for and in consideration of the sum of Ten Dollars and no/cents ($10.00) and other good and valuable considerations to said grantor in hand paid by said grantee, the receipt whereof is hereby acknowledged, has granted, bargained, and sold to the said grantee, and grantee's heirs and assigns forever, the following described land, situate, lying and being in **Alachua** County, **Florida** to-wit:

A parcel of land situated in Section 9, Township 8 South, Range 18 East, Alachua County, Florida, being more particularly described as follows:

For a point of reference commence at the Southeast corner of said Section 9; thence North 03°37' 28" West, a distance of 286.74 feet to the North Right of Way line of Old S.R. No. 25 (Old US Highway 441); thence North 79°23' 05" West, along said North Right of Way line, a distance of 2552.76 feet to the Point of Beginning; thence continue North 79°23' 05" West, along said North Right of Way line, a distance of 198.88 feet to the Southwest corner of Parcel 2 of the lands described in Official Records Book 4233, Page 1337 of the Public Records of Alachua County, Florida; thence North 03°02' 54" West, along the West line of said lands, a distance of 226.49 feet to the Northwest corner of said lands, lying on the Southerly Right of Way line of New Highway No. 441 (200' wide Right of Way); thence South 79°05' 00" East, along said Southerly Right of Way line, a distance of 199.14 feet; thence departing said Southerly Right of Way line, South 03°02' 56" East, a distance of 225.41 feet to the Point of Beginning.

A portion of Tax Parcel ID# 03053-002-000

Said property is not the homestead of the GRANTOR under the laws and constitution of the State of Florida in that neither GRANTOR nor any member of the household of GRANTOR reside thereon.

Together with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining

To Have and to Hold, the same in fee simple forever.

And the grantor hereby covenants with said grantee that the grantor is lawfully seized of said land in fee simple; that the grantor has good right and lawful authority to sell and convey said land; that the grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances, except taxes accruing subsequent to **December 31, 2017**.

WARRANTY DEED – Continued

In Witness Whereof, grantor has hereunto set grantor's hand and seal the day and year first above written.

Signed, sealed and delivered in our presence:

April Swim
1st Witness Signature

April Swim
1st Witness Printed Name

Kari L. Daelke
2nd Witness Signature

Kari L. Daelke
2nd Witness Printed Name

Indira K. Patel
Indira K. Patel

STATE OF Texas
COUNTY OF Dallas

This instrument was sworn to and acknowledged before me on this 15 day of November, 2018 by **Indira K. Patel**, ( ) who is personally known to me or (X) who produced a driver's license as identification.

My Commission Expires: (SEAL)

Notary Public

SHARLA M. TRUEBLOOD
Notary Public, State of Texas
My Commission Expires
May 18, 2019

RECORDED IN OFFICIAL RECORDS
INSTRUMENT # 3160279 2 PG(S)
11/26/2018 9:57 AM
BOOK 4645 PAGE 1470
J.K. JESS IRBY, ESQ.
Clerk of the Court, Alachua County, Florida
ERECORDED Receipt # 861829
Doc Stamp-Mort: $0.00
Doc Stamp-Deed: $2,432.50
Intang. Tax: $0.00

Prepared By and Return To: Kimberly G. Bosshardt, Esq.
Bosshardt Title Insurance Agency, LLC
5532 NW 43rd Street
Gainesville, FL 32653

For the issuance of title insurance, file #: 18-1213A

[Space Above This Line For Recording Data]

WARRANTY DEED

This Warranty Deed made **November 16, 2018** between **Indira K. Patel**, whose address is 8706 Saddlehorn Drive, Irving, TX 75063, hereinafter called the grantor, to **T & H Property Group, LLC, a Florida Limited Liability Company**, whose post office address is: 3580 North Main Street, Gainesville, FL 32609, hereinafter called the grantee:

Witnesseth, that said grantor, for and in consideration of the sum of Ten Dollars and no/cents ($10.00) and other good and valuable considerations to said grantor in hand paid by said grantee, the receipt whereof is hereby acknowledged, has granted, bargained, and sold to the said grantee, and grantee's heirs and assigns forever, the following described land, situate, lying and being in **Alachua** County, **Florida** to-wit:

A parcel of land situated in Section 9, Township 8 South, Range 18 East, Alachua County, Florida, being more particularly described as follows:

For a point of reference commence at the Southeast corner of said Section 9; thence North 03°37' 28" West, a distance of 286.74 feet to the North Right of Way line of Old S.R. No. 25 (Old US Highway 441); thence North 79°23' 05" West, along said North Right of Way line, a distance of 2353.88 feet to the Southeast corner of Parcel 2 of the lands described in Official Records Book 4233, Page 1337 of the Public Records of Alachua County, Florida and the Point of Beginning; thence continue North 79°23' 05" West, along said North Right of Way line, a distance of 198.88 feet; thence North 03°02' 56" West, a distance of 225.41 feet to the Southerly Right of Way line of New Highway No. 441 (200' wide Right of Way); thence South 79°05' 00" East, along said Southerly Right of Way line, a distance of 199.13 feet to the Northeast corner of said Parcel 2; thence South 03°02' 54" East, along the East line of said Parcel 2, a distance of 224.33 feet to the Point of Beginning.

A Portion of Tax Parcel ID: 03053-002-000

Said property is not the homestead of the GRANTOR under the laws and constitution of the State of Florida in that neither GRANTOR nor any member of the household of GRANTOR reside thereon.

Together with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining

To Have and to Hold, the same in fee simple forever.

And the grantor hereby covenants with said grantee that the grantor is lawfully seized of said land in fee simple; that the grantor has good right and lawful authority to sell and convey said land; that the grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances, except taxes accruing subsequent to **December 31, 2017**.

WARRANTY DEED – Continued

In Witness Whereof, grantor has hereunto set grantor's hand and seal the day and year first above written.

Signed, sealed and delivered in our presence:

April Swim
1st Witness Signature

April Swim
1st Witness Printed Name

Kari L. Daelke
2nd Witness Signature

Kari L. Daelke
2nd Witness Printed Name

Indira K. Patel

STATE OF Texas
COUNTY OF Dallas

This instrument was sworn to and acknowledged before me on this 15 day of November, 2018 by **Indira K. Patel**, ( ) who is personally known to me or (X) who produced a driver's license as identification.

My Commission Expires: (SEAL)

Notary Public

SHARLA M. TRUEBLOOD
Notary Public, State of Texas
My Commission Expires
May 18, 2019

RECORDED IN OFFICIAL RECORDS
INSTRUMENT # 3110696 5 PG(S)
3/12/2018 10:57 AM
BOOK 4581 PAGE 880
J.K.'JESS' IRBY
Clerk of the Court, Alachua County, Florida
ERECORDED Receipt # 817790
Doc Stamp-Mort: $0.00
Doc Stamp-Deed: $3,493.00
Intang. Tax: $0.00

CT-18-1023

Prepared by and Return to:
G. Preston Keyes, Esquire
Smith Hulsey & Busey, PA
225 Water Street, Suite 1800
Jacksonville, Florida 32202
(904) 359-7700

Property Appraiser's Parcel
Identification (Folio) No. 03061-004-001

Circle K Site No. 1119

Special Warranty Deed

This Special Warranty Deed is made on or as of the 9 day of March, 2018, by **CIRCLE K STORES INC.**, a Texas corporation, successor by merger to The Pantry, Inc., a Delaware corporation, successor by merger with Lil' Champ Food Stores, Inc., a Florida corporation ("Grantor"), the address of which is 1100 Situs Court, Suite 100, Raleigh, North Carolina 27606, to **T & H PROPERTY GROUP, LLC**, a Florida limited liability company ("Grantee"), the address of which is 300 SW 143RD Street, Jonesville, Florida 32669.

WITNESSETH:

THAT the Grantor, for and in consideration of the sum of TEN AND NO/100 DOLLARS ($10.00) and other valuable considerations, the receipt whereof is hereby acknowledged, hereby grants, bargains, sells, aliens, remises, releases, conveys and confirms unto Grantee, all that certain land situate in Alachua County, Florida, and more particularly described on Exhibit "A" (the "Property").

TOGETHER, with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

TO HAVE AND TO HOLD, the same in fee simple forever.

Subject, however, to the Permitted Exceptions more particularly set forth on Exhibit "B" (the "Permitted Exceptions"). In addition, the Property cannot be used: (a) for the sale of any food or food products, dairy products, and soft drinks, except as may be sold (i) through vending machines, or (ii) from a sit down or take-out restaurant (b) as a convenience store, (c) for retail sale of gasoline, (d) for retail sale of electronic cigarettes, cigarettes, and tobacco products, (e) for the retail sale of beer and wine for off-premises consumption, and (f) for any use ancillary (including, but not limited to, a parking lot, stormwater drainage retention pond or other stormwater purposes) to a business engaged in the restricted activities referred to in subclauses (a) through (e) above. As used herein, the phrase "sale of gasoline" shall include the sale or lease of any other energy source for motor vehicles as may hereafter be used in conjunction with or as an alternative to gasoline.

And Grantor hereby covenants with Grantee that Grantor is lawfully seized of the Property in fee simple subject to the Permitted Exceptions and as hereinabove described; that Grantor has good right and lawful authority to sell and convey the Property; and that Grantor does hereby warrant the title to the Property and will defend the same against the lawful claims of all persons claiming by, through or under Grantor, but against none others.

[The remainder of this page intentionally left blank. Signature page to follow]

[signature page to Special Warranty Deed]

IN WITNESS WHEREOF, the Grantor has caused this Special Warranty Deed to be executed the day and year first above written.

Signed, sealed, and delivered
in the presence of:

David Falk
Print Name: David Falk

Gay McDonald
Print Name: Gay McDonald

CIRCLE K STORES INC.,
a Texas corporation

By: Meredith W. Rice (SEAL)
Printed Name: Meredith W. Rice
Title: Vice President

STATE OF North Carolina
COUNTY OF Wake

The foregoing instrument was acknowledged before me this 8 day of March, 2018, by Meredith W. Rice, Jr., as Vice President of Circle K Stores Inc., a Texas corporation, on behalf of said corporation. Such person is ☒ personally known to me or ☐ has produced _____ as identification.

NOTARY PUBLIC LEE COUNTY, NC

Linda J. Bruce
Print Name: Linda J. Bruce
Notary Public, State of North Carolina
Commission No.: N/A
My Commission Expires: 7/9/18

[Affix Notary Stamp or Seal]

EXHIBIT "A"

Commence at the Southeast corner of Section Nine (9), Township Eight (8) South, Range Eighteen (18) East, and run North 3 degrees 37 minutes 28 seconds West 286.74 feet to the North right of way line of Old State Road No. 2 and 25; thence run North 79 degrees 23 minutes 05 seconds West along said right of way 2126.09 feet to the point of beginning; thence continue North 79 degrees 23 minutes 05 seconds West along said right of way 227.79 feet; thence run North 3 degrees 02 minutes 54 seconds West 224.35 feet to the South right of way of U.S. Highway No. 441; thence run South 79 degrees 06 minutes 35 seconds East along said right of way 281.82 feet; thence run South 10 degrees 53 minutes 25 seconds West 216.65 feet to the point of beginning. Being and lying in Section Nine (9), Township Eight (8) South, Range Eighteen (18) East, Alachua County, Florida.

EXHIBIT "B"
(Permitted Exceptions)

1. Taxes and assessments for the year 2018 and subsequent years, not due and payable, which Grantee assumes and agrees to pay.

2. All applicable laws (including zoning, building ordinances and land use regulations).

3. All matters which would be disclosed by a personal inspection or an accurate survey of the Property.

4. Use restrictive covenants as contained herein.

LEGAL DESCRIPTION

DATE: March 9, 2021
CLIENT: D.E. Foods, LLC
PROJECT NAME: QSR – KFC Alachua
PROJECT NO: 20-0113
DESCRIPTION FOR: 03061-004-001

Commence at the Southeast corner of Section Nine (9), Township Eight (8) South, Range Eighteen (18) last, and run North 3 degrees 37 minutes 28 seconds West 286.74 feet to the North right of way line of Old State Road No. 2 and 25; thence run North 79 degrees 23 minutes 05 seconds West along said right of way 2126.09 feet to the point of beginning; thence continue North 79 degrees 23 minutes 05 seconds West along said right of way 227.79 feet; thence run North 3 degrees 02 minutes 54 seconds West 224.35 feet to the South right of way of U.S. Highway No. 441; thence run South 79 degrees 06 minutes 35 seconds East along said right of way 281.82 feet; thence run South 10 degrees 53 minutes 25 seconds West 216.65 feet to the point of beginning. Being and lying in Section Nine (9), Township Eight (8) South, Range Eighteen (18) East, Alachua County, Florida.

ALATB15081F

John Power TAX COLLECTOR
Serving Alachua County

2020 PAID REAL ESTATE

12743

NOTICE OF AD VALOREM TAXES AND NON-AD VALOREM ASSESSMENTS

ACCOUNT NUMBER	ESCROW CD		MILLAGE CODE
03061 004 001		APPLICABLE VALUES AND EXEMPTIONS BELOW	1700

16130 NW US HWY 441

THE W 281.82 FT OF COM SE COR OF SEC
RUN N 3 DEG W 286.7
See Additional Legal on Tax Roll

T & H PROPERTY GROUP LLC
C/O BRAVO FLORIDA LLC
4220 EDISON LAKE PKWY
MISHAWAKA, IN 46545

AD VALOREM TAXES

TAXING AUTHORITY	MILLAGE RATE	ASSESSED VALUE	EXEMPTION(S)	TAXABLE VALUE	TAXES LEVIED
BOARD OF COUNTY COMMISSIONERS					
CNTY GENERAL	7.8935	443,527	0	443,527	3,500.98
ALACHUA CNTY LIBRARY DISTRICT					
LIBRARY GENERAL	1.1289	443,527	0	443,527	500.70
SCHOOL BOARD OF ALACHUA COUNTY					
SCHL CAP35 PROJECT (S01)	1.5000	443,527	0	443,527	665.29
SCHL DISCRNRY & CN (S01)	0.7480	443,527	0	443,527	331.76
SCHL GENERAL	3.6670	443,527	0	443,527	1,626.41
SCHOOL VOTED (S01)	1.0000	443,527	0	443,527	443.53
SUWANNEE RIVER WATER MGT DIST	0.3696	443,527	0	443,527	163.93
17 CITY OF ALACHUA	5.3900	443,527	0	443,527	2,390.61
CHILDREN'S TRUST	0.5000	443,527	0	443,527	221.76
TOTAL MILLAGE	22.1970		AD VALOREM TAXES		$9,844.97

John Power TAX COLLECTOR Serving Alachua County

WANT TO RECEIVE YOUR BILL ELECTRONICALLY NEXT YEAR? VISIT www.AlachuaCollector.com AND SIGN UP FOR E-BILLS!

PAY ONLINE WITH E-CHECK

SCAN TO PAY

PAY ONLY ONE AMOUNT.

NON-AD VALOREM ASSESSMENTS

LEVYING AUTHORITY		UNIT	RATE	AMOUNT
R803	803 BOCC SOLID WASTE MGMT	1.000	@ 124.9300	124.93
C550	550 COUNTY FIRE SERVICES	1.000	Varies	106.23
NON-AD VALOREM ASSESSMENTS				$231.16

COMBINED TAXES AND ASSESSMENTS	$10,076.13

If Paid By	Nov 30, 2020				
Please Pay	$0.00				

Please Retain this Portion for your Records. Receipt Available Online.

JOHN POWER, CFC
ALACHUA COUNTY TAX COLLECTOR

2020 PAID REAL ESTATE

12743

NOTICE OF AD VALOREM TAXES AND NON-AD VALOREM ASSESSMENTS

PLEASE PAY IN U.S. FUNDS (NO POSTDATED CHECKS) TO JOHN POWER, TAX COLLECTOR • 12 SE 1st ST, GAINESVILLE, FL 32601

ACCOUNT NUMBER	SITUS	MESSAGE
03061 004 001	16130 NW US HWY 441	

T & H PROPERTY GROUP LLC
C/O BRAVO FLORIDA LLC
4220 EDISON LAKE PKWY
MISHAWAKA, IN 46545

IF PAID BY	PLEASE PAY
☐ Nov 30, 2020	$0.00
☐	
☐	
☐	
☐	

Receipt # 20-0025393 $9,673.08 Paid 11/19/2020

PROJECT NAME: QSR Alachua
PROJECT No.: 20-0113 4:03 PM
FILE PATH: N:\2020\20-0113\Departments\04_Engineering\01_Regulatory Permitting\Utilities\Meter Sizing\Working Files\

ADF and ADF METER SIZING CALCULATIONS

Fixture Type	Quantity	Fixture Value (60 psi)	Total Fixture Value
Water closet	2	2	4
Lavatory	2	1.5	3
Hand Sink	3	1.5	4.5
3 Compartment Sink	1	3	3
Hose Bibs	3	2.5	7.5
Water Filter	2	1	2
Mop Sink	2	2.25	4.5
		Total	28.5

Peak Flow per AWWA M22 Manual Table 4-2. 39.0 gpm

Meter size per AWWA M22 Manual Table 6-1 1 inch

Use 1" Meter with 1" RPZ Backflow Preventer

Daniel Young, FL P.E. No. 70780

Date

Daniel H.Young, P.E.
State of Florida, Professional Engineer, License No. 70780

This item has been digitally signed and sealed by Daniel H. Young, P.E. on the date indicated here. 01/28/2021

Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

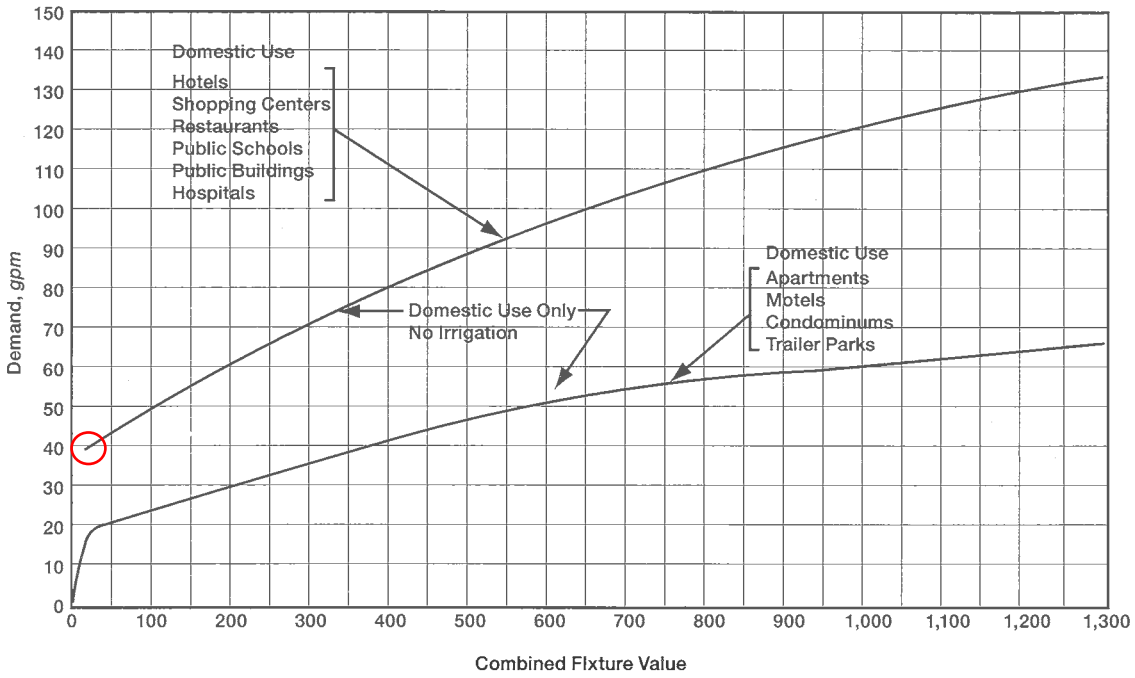


Figure 4-2 Water-flow demand per fixture value—enlarged scale from Figure 4-1

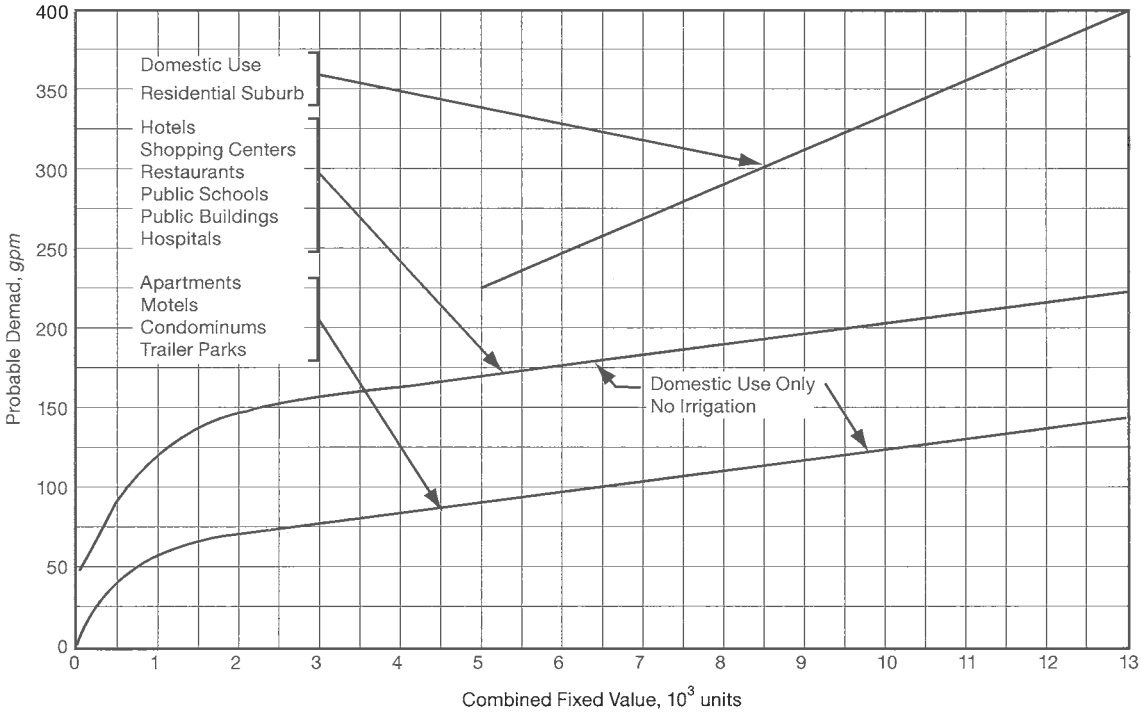


Figure 4-3 Water-flow demand per fixture value

Table 4-3 Example of fixture value adjustment for pressure (based on Figure 4-6)

Kitchen Faucet Fixture Value Adjustment			
Residual Pressure at Fixture Outlet, *psi*	Baseline Flow Rate at 60 *psi*	Actual Flow Rate at Residual Pressure (Fixture Value)	Pressure Adjustment Factor
15	1.8	1.0	0.56
20	1.8	1.1	0.61
25	1.8	1.2	0.67
30	1.8	1.3	0.72
35	1.8	1.4	0.78
40	1.8	1.5	0.83
50	1.8	1.7	0.94
60	1.8	1.8	1.00
70	1.8	1.8	1.00
80	1.8	1.8	1.00

Table 6-1 AWWA meter standards

Meter	Minimum Flow Rate, *gpm*	Low-Normal Flow Rate, *gpm*	Change-over Range (Compound Meters)	High-Normal Flow Rate, *gpm*	Maximum Flow Rate *gpm*	Head Loss at Maximum Flow, *psi*
Positive displacement						
½ in.	0.25	1	N/A	7.5	15	15
⅝ in.	0.25	1		10	20	15
¾ in.	0.50	2		15	30	15
1 in.	0.75	3		25	50	15
1½ in.	1.50	5		50	100	15
2 in.	2.00	8		80	160	15
Multijet						
⅝ in.	0.25	1	N/A	10	20	15
¾ in.	0.50	2		15	30	15
1 in.	0.75	3		25	50	15
1½ in.	1.50	5		50	100	15
2 in.	2.00	8		80	160	15
Turbine class II						
1½ in.	N/A	4	N/A	90	120	7
2 in.		4		160	190	7
3 in.		8		350	435	7
4 in.		15		650	750	7
6 in.		30		1,400	1,600	7
8 in.		50		2,400	2,800	7
10 in.		75		3,500	4,200	7
12 in.		120		4,400	5,300	7
16 in.		200		6,500	7,800	7
20 in.		300		10,000	12,000	7
Compound class II						
2 in.	0.25	1	13	80	160	15
3 in.	0.50	2	15	175	350	15
4 in.	0.75	3	18	300	600	15
6 in.	1.50	5	20	675	1,350	15
8 in.	2.00	16	35	900	1,600	15
Fire service, type II—compound						
3 in	* see note	2	30	250	350	12
4 in.		4	40	400	700	12
6 in.		5	90	900	1,600	12
8 in.		8	150	1,600	2,800	12
10 in.		8	200	2,200	4,400	12

Source: Data are drawn from AWWA Standards C700, C701, C702, C703, C704, C708, C710, C712, C713, and C714, latest revision.

N/A = not applicable.

* Minimum flow rate is per the applicable AWWA standard for the bypass meter employed.

(Table continued on next page.)

MEMORANDUM

Quick Service Restaurant – Alachua

20-0113

To: Fire Review

From: Daniel Young, PE

Date: January 27, 2021

RE: Quick Service Restaurant – KFC Alachua – Required Fire Flow

The following is a calculation for required fire flow for the proposed project based on the NFPA 1: Fire Code.

Building data is based on the information available from the architect at the time of this memo. Any changes to the building data will void the provided fire flow calculation and require a revised analysis to verify the building complies with the applicable fire protection criteria. The building will be not protected by an approved automatic fire sprinkler system.

Required Flow Calculations:

Building: **Quick-Service Restaurant**
Construction Type: V (000)
Fire Flow Area: ±1,904 SF

Required Fire Flow per NFPA Table 18.4.5.1.2: 1,500 gpm

Conclusions

Minimum Required Fire Flow to be provided: **1,500 gpm**

Available Fire Flow per Fire Hydrant Flow Test: **3,826 gpm**

Daniel H.Young, P.E.
State of Florida, Professional Engineer, License No. 70780

This item has been digitally signed and sealed by Daniel H. Young, P.E. on the date indicated here. 01/28/2021

Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Hydrant Flow Test Report

Test Date 11/04/2020 **Test Time** 4:00 PM

Location

Along 441 East of NW 167th Blvd.
Alachua Co Tax Parcel #03053-002-001
Alachua, FL. 32615

Tested by

Gator Fire Equipment Co., Inc.
Tested By: R. Weaver & A. Miller
Witnessed By: City of Alachua

Notes

Reading Hydrant East of NW167th Blvd along US441
Flowing Hydrant Further East

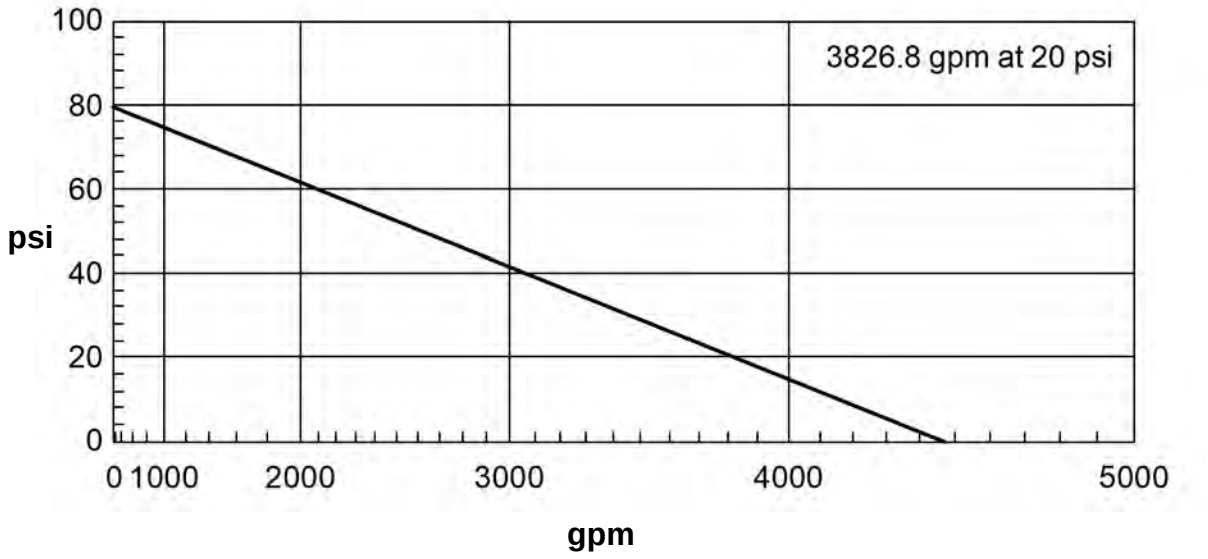
Read Hydrant

80 psi **static pressure**
50 psi **residual pressure**
0 ft **hydrant elevation**

Flow Hydrant(s)

Outlet	Elev	Size	C	Pitot Pressure	Flow
#1	0	2.5	.9	28	888 gpm
#2	0	2.5	.9	25	839 gpm
#3	0	2.5	.9	29	904 gpm
				Total	2631 gpm

Flow Graph



Created with the free hydrant flow test program from www.igneusinc.com

Table 18.4.5.1.2 Minimum Required Fire Flow and Flow Duration for Buildings

Fire Flow Area ft² (× 0.0929 for m²)					Fire Flow gpm[1] (× 3.785 for L/min)	Flow Duration (hours)
I(443), I(332), II(222)*	II(111), III(211)*	IV(2HH), V(111)*	II(000), III(200)*	V(000)*		
0–22,700	0–12,700	0–8200	0–5900	0–3600	1500	2
22,701–30,200	12,701–17,000	8201–10,900	5901–7900	3601–4800	1750	
30,201–38,700	17,001–21,800	10,901–12,900	7901–9800	4801–6200	2000	
38,701–48,300	21,801–24,200	12,901–17,400	9801–12,600	6201–7700	2250	
48,301–59,000	24,201–33,200	17,401–21,300	12,601–15,400	7701–9400	2500	
59,001–70,900	33,201–39,700	21,301–25,500	15,401–18,400	9401–11,300	2750	
70,901–83,700	39,701–47,100	25,501–30,100	18,401–21,800	11,301–13,400	3000	3
83,701–97,700	47,101–54,900	30,101–35,200	21,801–25,900	13,401–15,600	3250	
97,701–112,700	54,901–63,400	35,201–40,600	25,901–29,300	15,601–18,000	3500	
112,701–128,700	63,401–72,400	40,601–46,400	29,301–33,500	18,001–20,600	3750	
128,701–145,900	72,401–82,100	46,401–52,500	33,501–37,900	20,601–23,300	4000	4
145,901–164,200	82,101–92,400	52,501–59,100	37,901–42,700	23,301–26,300	4250	
164,201–183,400	92,401–103,100	59,101–66,000	42,701–47,700	26,301–29,300	4500	
183,401–203,700	103,101–114,600	66,001–73,300	47,701–53,000	29,301–32,600	4750	
203,701–225,200	114,601–126,700	73,301–81,100	53,001–58,600	32,601–36,000	5000	
225,201–247,700	126,701–139,400	81,101–89,200	58,601–65,400	36,001–39,600	5250	
247,701–271,200	139,401–152,600	89,201–97,700	65,401–70,600	39,601–43,400	5500	
271,201–295,900	152,601–166,500	97,701–106,500	70,601–77,000	43,401–47,400	5750	
Greater than 295,900	Greater than 166,500	106,501–115,800	77,001–83,700	47,401–51,500	6000	
		115,801–125,500	83,701–90,600	51,501–55,700	6250	
		125,501–135,500	90,601–97,900	55,701–60,200	6500	
		135,501–145,800	97,901–106,800	60,201–64,800	6750	
		145,801–156,700	106,801–113,200	64,801–69,600	7000	
		156,701–167,900	113,201–121,300	69,601–74,600	7250	
		167,901–179,400	121,301–129,600	74,601–79,800	7500	
		179,401–191,400	129,601–138,300	79,801–85,100	7750	
		Greater than 191,400	Greater than 138,300	Greater than 85,100	8000	

*Types of construction are based on NFPA 220.

[1]Measured at 20 psi (139.9 kPa).

Centennial FDOT 1915 ★ 2015

Florida Department of Transportation

RICK SCOTT
GOVERNOR

605 Suwannee Street
Tallahassee, FL 32399-0450

JIM BOXOLD
SECRETARY

PROGRAM MANAGEMENT MEMORANDUM 15-01
PROJECT MANAGEMENT MEMORANDUM NO. 15-01
DCE MEMORANDUM NO. 04-15
DME MEMORANDUM NO. 15-01

DATE: February 13, 2015

TO: Directors of Transportation Development, Directors of Transportation Operations, District Design Engineers, District Construction Engineers, District Maintenance Engineers, and District Program Management Engineers/Administrators

FROM: David A. Sadler, P.E., Director, Office of Construction
Tim Lattner, P.E., Director, Office of Maintenance
Trey Tillander, P.E., Manager, Office of Program Management
Bob Crim, P.E., Manager, Production Support Office
Kendra Sheffield, Manager, Work Program Development

COPIES: Brian Blanchard, Tom Byron, Duane Brautigam, Greg Davis, Daniel Scheer, Rudy Powell, Jeff Caster

SUBJECT: Implementation of Landscape Contracts

BACKGROUND

In 2013, the Executive Team established a new policy through Engineer and Operations (E&O) Memo 13-1 to program stand alone landscape projects using maintenance contract language and Phase 52 funds. This will allow landscape contracts to remain open throughout the plant establishment period and is expected to improve the Department's ability to enforce contract deliverables throughout the entire establishment period. All costs during the establishment period are incorporated as part of the capital cost of the plant material and will function as a warranty against loss.

REQUIREMENTS

Landscaping Included as Part of the Roadway Construction Project:

Landscape installations totaling the lesser of $100,000 or 10% of the preliminary cost estimate may continue to be included in a roadway construction project (including Design-Build roadway construction projects), as incidental landscape work.

There is no establishment period included in the specifications governing these roadway construction projects. Districts should determine who will be responsible for establishment and maintenance. If the party responsible for the landscape maintenance is someone other than the Department (i.e., Local Agency, etc.) a Maintenance Memorandum of Agreement must be fully executed during the project design and no later than the project letting.

Stand-alone Landscaping Projects:

The stand-alone landscaping projects will be programmed as either dependent or independent stand-alone projects for all fiscal years. Stand-alone landscaping projects will be advertised as Maintenance contracts and include Maintenance specifications which contain contract terms establishing an installation period and defined milestones for an establishment period of two years. No more than seventy-six percent of the contract amount will be paid prior to the start of the establishment period after which monthly payments to the contractor will be issued only upon satisfactory performance. The landscape installation Maintenance specification has been created to support this change (see Specification at: http://www.dot.state.fl.us/specificationsoffice/Maintenance/Jan15/default.shtm).

Dependent Stand-alone Landscaping Projects:

Landscaping associated with a roadway construction project will be programmed as a stand-alone landscape project in **the year in which roadway construction is anticipated for completion** and will be scheduled immediately following roadway construction. If the roadway construction project is federally funded, the landscaping work may also be eligible for the same federal funds.

Independent Stand-alone Landscaping Projects:

Landscaping not associated with a roadway construction project will be programmed as stand-alone landscape project in the year in which the landscape project is ready for production. Independent stand-alone landscaping projects will be state funded and let in the District Office.

Performance Bond:

The performance bond required in the Maintenance specifications, SP0030500 Award of Contract, is an annual renewable bond so that the contractor does not have to obtain a bond for 2+ years for the full contract value. The performance bond covers the installation period and the establishment period or the total contract time allowed.

Example: If the contract time is 930 days (2.55 years) with a hypothetical value of $5 million, take the $5 million and divide by the 2.55 years to arrive at the annual bond amount of $1,960,784. Therefore, the first 12 months of the contract will require a performance bond for $1,960,784; for the second 12 months, a performance bond for $1,960,784; and for the final 7 months a performance bond for $1,078,431 (which is the remaining contract amount $1,960,784x 2 = $3,921,568 minus $5,000,000 = $1,078,431).

Pay Items:
These contracts will be lump sum, and therefore there would only be one pay item (999-2). Seventy-six percent (76%) of the total contract value is payable upon completion of the planting, with the other twenty four-percent (24%) being paid monthly over the two year establishment period. The contractor is required to submit a schedule of values just as they would on a lump sum contract. Failure to perform will be a reduction in payment and not treated as retainage.

Contract Time:
Stand-alone landscaping contracts are to be designated in the AASHTO Projects (formerly Trns•port) Preconstruction system as Calendar Day Contracts. A Calendar Day Contract can be designated by selecting the "Time" tab on the "Proposal Summary" page and then selecting "CD-Calendar Days" on the "Type of Days" drop-down menu.

Incidental Work:
The intent is that these are landscape contracts, and will only include items of work that are necessary for the installation of the landscaping. Any items incidental to the landscaping would be exactly that, "incidental," and included in the lump sum landscape pay item. It is important to minimize the amount of work required which is not directly related to the installation of landscape plantings. For dependent landscape projects, all of the site work and any associated hardscape should be included in the highway construction contract. For independent landscape projects, it may be necessary to include some incidental items of work, but if this work amounts to a large portion of the contract work, consideration should be given to letting a separate construction project to accomplish the site work and non-landscape planting work necessary for the project.

Funding:
A Federal funding process for stand-alone landscaping has not been developed. Independent landscaping work by itself is no longer eligible for federal funds. For landscaping to be federally funded, it has to be part of the overall highway construction work authorized by FHWA. Going forward, FHWA has agreed to fund landscaping work that is a component of a highway construction project as a separate contract following the completion of the highway construction contract; with both contracts fulfilling the overall construction of the federally funded project. These are referred to as dependent stand-alone landscape projects.

Coordination:
The District Landscape Architects should be a common participant in all of these projects. For the stand-alone landscape packages, the preparer of the specifications package will need to coordinate closely with Maintenance staff and District contracting staff.

The District Landscape Architect is to be involved throughout all phases of landscape projects. Involvement of the District Landscape Architect during roadway and landscape design, construction and maintenance is necessary to fully comply with the Department Environmental Policy and Highway Beautification Policy.

Contract Administration:
The contract administration of the stand-alone landscape projects needs to be determined by each Director of Transportation Operations. The administration of stand-alone landscape projects can be performed by District Maintenance staff, District Construction staff, or a combination of these.

IMPLEMENTATION

This joint bulletin/memo is effective immediately and supersedes Estimates Bulletin 13-10, Project Management Memo 13-02, DCE Memorandum 13-21 and DMDH Memorandum 13-03.

The stand-alone Landscaping Specification is currently on Specs-on-the-Web as a Maintenance Specification (SS5800000) for use by the District Offices to execute a contract for all landscaping projects that meet the criteria described above.

The appropriate (Federal Aid or Non-Federal Aid) Construction landscaping Special Provision (SP5800000 Landscaping) will be used for any landscaping work that is to remain in the roadway construction contract starting with July 2015 lettings.

CONTACT

Jeff Caster, R.L.A.
State Transportation Landscape Architect
Florida Department of Transportation
605 Suwannee Street, MS 40
Tallahassee, FL 32399-0450
Phone (850)-414-5267
jeff.caster@dot.state.fl.us

Daniel Scheer, P.E.
State Specifications Engineer
Florida Department of Transportation
605 Suwannee Street, MS 75
Tallahassee, FL 32399-0450
Phone (850)-414-4130
daniel.scheer@dot.state.fl.us

Rudy Powell, P.E.
State Construction Engineer
Florida Department of Transportation
605 Suwannee Street, MS 31
Tallahassee, FL 32399-0450
Phone (850)-414-4280
rudy.powell@dot.state.fl.us

Melissa Hollis
Basis of Estimates Coordinator
Florida Department of Transportation
605 Suwannee Street, MS 75
Tallahassee, FL 32399-0450
Phone (850)-414-4182
melissa.hollis@dot.state.fl.us

Michael E. Sprayberry, P.E.
State Administrator for Maintenance - Contracting
Florida Department of Transportation
605 Suwannee Street, MS 52
Tallahassee, FL 32399-0450
Phone (850)-414-5757
mike.sprayberry@dot.state.fl.us

Kendra H. Sheffield.
Manager, Work Program Development
Florida Department of Transportation
605 Suwannee Street, MS 21
Tallahassee, FL 32399-0450
Phone (850)-414-4627
kendra.sheffield@dot.state.fl.us

DFB/DAS/TRL/VYT/KHS/dls

CHW
Professional Consultants

JACKSONVILLE | GAINESVILLE | OCALA
8563 Argyle Business Loop, Ste. 3, Jacksonville, Florida 32244
132 NW 76th Drive, Gainesville, Florida 32607
101 NE 1st Avenue, Ocala, Florida 34470
WWW.CHW-INC.COM

MEMORANDUM

To: **Justin Tabor, AICP**

From: **Caeli Tolar, PLA**

Date: **03/25/21**

RE: **KFC Alachua – Tree Mitigation Payment**

Good morning Justin,

The following is some research to determine a mitigation cost per replacement tree for KFC Alachua. As outlined in City of Alachua Code Sec. 6.2.1(D)(7):

(a) The City may allow off-site mitigation for required tree replacement that cannot be accommodated through on-site mitigation. Trees authorized for off-site mitigation shall be planted in City-owned properties and parks, City rights-of-way, and preservation or conservation areas owned by the City. The City may also plant trees within the medians and rights-of-way of State and County roads where an interlocal agreement authorizes such plantings.

(b) The City will establish a separate fund within the City's chart of accounts to be used exclusively for off-site tree mitigation payments. Funds withdrawn from this account shall be spent solely for the planting and maintenance of new trees in accordance with this section.

(c) The off-site mitigation formula shall be equal to the cost of the replacement tree, plus installation (labor and equipment), plus maintenance for one year, plus fund administration. This formula will be multiplied by the number of trees to be planted. The fee for off-site tree mitigation shall be adopted by the City Commission through resolution.

(d) Fees for off-site mitigation shall be paid to the City prior to the issuance of any tree removal permit or building permit for new construction with an approved site plan or plat. Receipts for payment will be specifically marked for the off-site mitigation account.

The following information provided outlines the process by which we have determined our mitigation payment calculations.

Description of Project Mitigation

Within this project, there are several trees proposed to be removed subject parcel. The majority of the trees to be removed are undesirable species such as Laurel Oaks. These trees present potential long-term safety and maintenance issues as they mature. Additionally, these trees present conflicts with proposed utilities along the throughout the site that are necessary to serve the proposed buildings. Therefore, most existing trees in site are proposed for removal.

Twenty-nine replacement trees have been proposed on the site. After the placement of these mitigation trees, there is still a deficit of mitigation trees required totaling 193 trees at 1.5”/tree. Due to the locations of proposed structures, parking, and code-required tree plantings, there is inadequate space on the site at the completion of construction to provide all 193 additional replacement trees for mitigation. Therefore, we are proposing a mitigation payment in lieu of on-site planting.

Determination of Replacement Tree Cost

At least 50% of the replacement trees must be shade trees, according to Sec.6.2.1(D)(4). According to Sec. 6.2.2(D)(8)(b)(ii), “Canopy trees shall be a minimum of eight feet in height with the caliper based on the tree type per Florida Nursery and Grade Standard.” The remaining 50% of the replacement trees shall be ornamental/understory trees, which, as outlined in Sec. 6.2.2(D)(8)(b)(iii), should be a minimum of 1 ½” caliper.

To establish this number, we have taken the given installed cost for a 15 gallon tree or the average, installed cost for 30 gallon trees provided to us on each of three recent and comparable projects to obtain the unit price per caliper inch. This number was multiplied by 1.5" caliper inches to get the unit price per 15 gallon/1.5" caliper tree. The calculations are below:

Project #1:
Given Bid Cost: 15 gal.,1.5" cal. =$109
Therefore, the given bid cost for a 15 gallon tree = $109.00

Project #2:
Given Bid Cost: 30 gal., 2" cal. =$179
$179 per 2" cal. = $89.50 per 1" cal.
$89.50 per 1" cal. x 1.5" cal. = $134.25
Therefore, the given bid cost for a 15 gallon tree = $134.25

Project #3:
Given Bid Cost: 30 gal., 2" cal. =$149
$149 per 2" cal. = $74.50 per 1" cal.
$74.50 per 1" cal. x 1.5" cal. = $111.75
Therefore, the given bid cost for a 15 gallon tree = $111.75

Overall Average:
15 gal. $118.33

Establishment/maintenance costs vary across projects, so we have done some research to estimate a standard cost. For the purposes of this project we will utilize the Florida Department of Transportation's standard, which is an additional 24% in establishment/maintenance costs (see Attachment 'A').

Therefore: $118.33 + and additional 24% = $146.73 per tree (installed + establishment/maintenance cost). The City of Alachua also requires an additional 10% fund administration as per Sec. 6.2.1(D)(7)(c), so our estimation of the cost per replacement tree is **$158.56 per tree.**

Mitigation Calculations

Heritage Trees (>30", at Inch-For Inch Replacement)
Total Inches Removed = 143"
Total Mitigation Inches Required = 143"

Regulated Trees (<30", >10", at 1:1 Replacement)
Total Regulated Trees Removed = 126 trees
Total Mitigation Trees Required = 126 Trees
Total Mitigation Inches Required
(@ 1.5" cal. per tree) = 189"

Total Mitigation Payment
Total Mitigation Required = 332"
Total Mitigation Provided = 43.5"
Total Mitigation Inches Deficit = 288.5"
Total Mitigation Tree Deficit
(@ 1.5" cal. per tree) = 193 Replacement Trees

193 replacement trees x $158.56 per tree = $30,602.08

Total Mitigation Payment Required = $30,602.08

I. LANDSCAPE MATERIALS					
KEY	**QUAN-TITY**	**COMMON NAME**	**SIZE**	**UNIT COST**	**AMOUNT**
		TREES			
AR	77	Florida Flame Red Maple - 11' Ht. x 3' Spr.; 2.5" Cal.	#30	149.00	11,473.00
BN	59	Dura Heat River Birch - 12' Ht. x 3.5' Spr.; 2.5" Cal.	#30	149.00	8,791.00
CC	5	Eastern Redbud - 10' Ht. x 4' Spr.; 2.5" Cal.	#30	149.00	745.00
CV	31	White Fringetree - 9' Ht. x 2.5' Spr.; 1.5" Cal.	#15	109.00	3,379.00
IC	25	Dahoon Holly - 7' Ht. x 3' Spr.; 2.5" Cal.	#30	149.00	3,725.00
LT	12	Tulip Tree - 11' Ht. x 3' Spr.; 2.5" Cal.	#30	149.00	1,788.00
MGB	54	Bracken's Brown Beauty Magnolia - 12' Ht. x 3.5' Spr.; 2.5" Cal.	#30	149.00	8,046.00
PA	29	Chickasaw Plum - 6' Ht. x 2.5' Spr.; 1.5" Cal.	#15	109.00	3,161.00
PO	7	Sycamore - 12' Ht. x 4.5' Spr.; 2.5" Cal.	#30	149.00	1,043.00
QN	39	Nuttall Oak - 12' Ht. x 3.5' Spr.; 2.5" Cal.	#30	149.00	5,811.00
QO	16	Basket Oak - 12' Ht. x 3.5' Spr.; 2.5" Cal.	#30	159.00	2,544.00
QS	15	Shumard Red Oak - 12' Ht. x 3.5' Spr.; 2.5" Cal.	#30	149.00	2,235.00
QV	44	Seedling Southern Live Oak - 12' Ht. x 4.5' Spr.; 2.5" Cal.	#30	179.00	7,876.00
QVC	34	Cathedral Live Oak - 14' Ht. x **5.**5' Spr.; 3.5" Cal.	#65	599.00	20,366.00
TD	129	Bald Cypress - 12' Ht. x 3.5' Spr.; 2.5" Cal.	#30	149.00	19,221.00
UA	33	Winged Elm - 12' Ht. x 6' Spr.; 3.5" Cal.	#65	599.00	19,767.00
VOW	78	Walter's Viburnum - 7' Ht. x 3' Spr.; 2.5" Cal.	#30	149.00	11,622.00
		SHRUBS			
HD	570	Cucumberleaf Sunflower – Full	#1	6.00	3,420.00
MU	3,262	Pink Muhly – Full	#1	6.00	19,572.00
SB	1,625	Sandcord Grass – Full	#1	6.00	9,750.00

Item No.	Item	Qty	Unit	Unit Cost	Cost	Description
A.	**Trees**					
1	Betula nigra 'Dura Heat'- Dura Heat River Birch	8	Ea	790.00	$ 6,320.00	65 gallon
2	Magnolia grandiflora 'DD Blanchard'	12	Ea	950.00	$ 11,400.00	100 gallon
3	Quecus Geminata	16	Ea	179.00	$ 2,864.00	30 gallon
4	Quercus virginiana - Southern Live Oak Seedling	84	Ea	179.00	$ 15,036.00	30 gallon
5	Quercus virginiana - Boardwalk Live Oak	20	Ea	179.00	$ 3,580.00	30 gallon
6	Quercus virginiana - Southern Live Oak Seedling	18	Ea	950.00	$ 17,100.00	100 gallon
7	Ulmus alata- Winged Elm	63	Ea	179.00	$ 11,277.00	30 gallon
8	Ulmus Parvifolia 'Allee'	25	Ea	179.00	$ 4,475.00	30 gallon
				Trees Subtotal	$ 72,052.00	
B.	**Palm Trees**					
9	Sabal Palmetto	44	Ea	360.00	$ 15,840.00	C.T. Varies
				Palms Subtotal	$ 15,840.00	
B.	**Shrubs & Groundcovers**					
10	Galphimia Gracius	17	Ea	49.00	$ 833.00	7 gallon
11	Podocarpus Macrophyllus	86	Ea	139.00	$ 11,954.00	15 gallon
12	Viburnum obovatum 'Mrs. Shiller's Delight' - Mrs. Shillers Delight Viburnum	506	Ea	9.00	$ 4,554.00	3 gallon
				Shrubs & Groundcovers Subtotal	$ 17,341.00	
C.	**Sod**					
13	Empire Zoysia Sod	247,282	SF	.56	$ 138,477.92	weed free and sand grown sod
14	Argentine Bahia Sod	140,060	SF	.31	$ 43,418.60	weed free and sand grown sod
15	Bermuda Grass Sod	213,804	SF	.56	$ 119,730.24	weed free and sand grown sod
				Sod Subtotal	$ 301,626.76	
D.	**Irrigation**					
16	Irrigation		LS		158,391.40	
				Irrigation Subtotal	$ 158,391.40	
	Project Subtotal				$ 565,251.16	
					$	
	PROJECT GRAND TOTAL				$ 628,918.01	

I. LANDSCAPE MATERIALS					
KEY	**QUAN-TITY**	**COMMON NAME**	**SIZE**	**UNIT COST**	**AMOUNT**
		TREES			
AR	6	Florida Flame Red Maple - 11' ht. 4' spr. x 2.5" cal.	#30	149.00	894.00
BN	23	Dura Heat River Birch - 12' ht. 4' spr. x 5" cal., Multi-Trunk	#30	149.00	3,427.00
INS	12	Nellie R. Stevens Holly – 13.5' ht. 5' spr. x 3.5" cal.	#65	790.00	9,480.00
JS	3	Southern Red Oak - 12' ht. x 42" spr. x 2" cal.	#30	149.00	447.00
MGD	22	DD Blanchard Magnolia - 12' ht. x 4' spr. x 2.5" cal.	#30	179.00	3,938.00
QG	24	Sand Live Oak - 12' ht. x 54" spr. x 2" cal.	#30	179.00	4,296.00
QS	15	Shumard Red Oak - 12' ht. x 4' spr. x 2.5" cal.	#30	149.00	2,235.00
QVB	47	Boardwalk Live Oak - 22' ht. x 10' spr. x 6" cal.	#200	1,650.00	77,550.00
QVH	11	Highrise Live Oak - 17' ht. x 6.5' spr. x 4" cal.	#100	850.00	9,350.00
TD	15	Bald Cypress - 12' ht. x 42" spr. x 2.5" cal., Full to Base	#30	149.00	2,235.00
UPE	8	Allee Elm - 10' ht. x 4' spr. x 2" cal.	#30	179.00	1,432.00
VC	9	Chaste Tree - 12' ht. x 6' spr. x 3.5" cal.	#65	510.00	4,590.00
VOW	7	Walter's Viburnum - 7' ht. x 3' spr. x 2" cal.	#30	149.00	1,043.00
		PALM TREES			
SP12	14	Sabal Palm - 12" min. DBH 12'-18' c.t.	FG	360.00	5,040.00
SP14	26	Sabal Palm - 12" min. DBH 12'-18' c.t.	FG	360.00	9,360.00
SP16	13	Sabal Palm - 12" min. DBH 12'-18' c.t.	FG	360.00	4,680.00
SP18	14	Sabal Palm - 12" min. DBH 12'-18' c.t.	FG	360.00	5,040.00
		SHRUBS			
GG	6	Rain of Gold (Thryallis) - 18" ht. x 18" spr.	#7	49.00	294.00

Mailed Memorandum

MEMORANDUM

Virtual Neighborhood Meeting

20-0113

To:	The Neighbors of Alachua County Tax Parcel 03053-002-001
From:	Daniel Young, PE, Director of Engineering, Land Development
Date:	5 November 2020
RE:	Virtual Neighborhood Meeting Public Notice

A virtual Neighborhood Meeting will be held via video conference to discuss a Site Plan application on ±1.01 acres (Alachua County Tax Parcel 03053-002-001). The project includes a ±2,200-square-foot quick service restaurant, associated infrastructure, and parking. The site is located on the southern side of US 441, between NW 167th Boulevard and I-75.

Date: 19 November 2020

Time: 6:00 p.m.

Link: bit.ly/QSR-3053-2-1

Contact: Daniel Young, PE
11801 Research Drive
Alachua, FL 32615
(352) 331-1976
DanielY@CHW-inc.com

This is not a public hearing. The purpose of the meeting is to inform the public about the nature of the proposal and seek their comments.

Directions to Attend Meeting: Using your web browser, visit the web address link above. Please note, the link is case-sensitive. For best performance, Google Chrome, Mozilla Firefox, Safari, or Microsoft Edge are recommended.

If you are unable to attend the meeting, please contact CHW using the information above and we will provide printed materials after the virtual meeting is held. Comments may also be submitted in writing to the address or email above or by phone at the phone number above. Printed copies of all materials discussed at the meeting will be made available upon request.

t: (904) 619-6521 | 8465 Merchants Way, Suite 102, Jacksonville, FL 32222
t: (352) 331-1976 | 11801 Research Drive, Alachua, FL 32615
t: (352) 414-4621 | 101 NE 1st Avenue, Ocala, FL 34470
www.chw-inc.com

Mailing Labels

ALACHUA A ONE LLC
15260 NW 147TH DR STE 100
ALACHUA, FL 32615

ALACHUA GATEWAY CENTER SURFACE
JAMES D SALTER ESQ
3940 NW 16TH BLVD BLDG B
GAINESVILLE, FL 32605

CAVACEPPI SHARLEEN O TRUSTEE
PO BOX 1325
ALACHUA, FL 32616

DWARKADHISH INVESTMENTS INC
1800 S ATLANTIC AVE
DAYTONA BEACH, FL 32118

SARCAV LLC
2567 RIVER ENCLAVE LN
JACKSONVILLE, FL 32226

T & H PROPERTY GROUP LLC
3580 N MAIN ST
GAINESVILLE, FL 32609

TALAL PROPERTIES LTD & TAREK PROF
1326 E LUMSDEN RD
BRANDON, FL 33511

TOMOKA HILLS FARMS INC
1301 DIXIANA DOMINO RD
LEXINGTON, KY 40511

Antoinette Endelicato
5562 NW 93rd Avenue
Gainesville FL 32653

Dan Rhine
288 Turkey Creek
Alachua FL 32615

Tom Gorman
9210 NW 59th Street
Alachua FL 32653

Richard Gorman
5716 NW 93rd Avenue
Alachua FL 32653

Peggy Arnold
410 Turkey Creek
Alachua FL 32615

David Forest
23 Turkey Creek
Alachua FL 32615
President
TCMOA
1000 Turkey Creek
Alachua FL 32615

Linda Dixon, AICP
Assistant Director Planning
PO Box 115050
Gainesville FL 32611

Craig Parenteau
FL Department of Environmental Protection
4801 Camp Ranch Road
Gainesville FL 32641

Jeannette Hinsdale
P.O. Box 1156
Alachua FL 32616

Lynn Coullias
7406 NW 126th Ave
Alachua FL 32615

Lynda Coon
7216 NW 126 Avenue
Alachua FL 32615

Tamara Robbins
PO Box 2317
Alachua FL 32616

Michele L. Lieberman
County Manager
12 SE 1st Street
Gainesville FL 32601

Bonnie Flynn
16801 NW 166th Drive
Alachua FL 32615

Hugh & Jean Calderwood
P.O. Box 2307
Alachua FL 32616

Newspaper Advertisement

PUBLIC NOTICE

A virtual Neighborhood Meeting will be held via video conference to discuss a Site Plan application on ±1.01 acres (Alachua County Tax Parcel 03053-002-001). The project includes a ±2,200-square-foot quick service restaurant, associated infrastructure, and parking. The site is located on the southern side of US 441, between NW 167th Boulevard and I-75.

This is not a public hearing. The purpose of this workshop is to inform the public about the nature of the proposal and seek their comments.

Time: 6:00 p.m. on November 19, 2020
Link: bit.ly/QSR-3053-2-1
Instructions: Type the link into the address bar of your web browser. If unable to participate in the meeting, please contact CHW for printed materials. Comments may also be submitted in writing to the address or email below, or by phone.

Contact: Daniel Young, PE
Address: 11801 Research Drive
Alachua, FL 32615
Phone Number: (352) 331-1976
Email: DanielY@CHW-inc.com

CHW
Professional Consultants

Young adults driving 71 new cases in Alachua County

Emily Mavrakis Gainesville Sun | USA TODAY NETWORK

Young adults continue to drive a majority of new COVID-19 cases in Alachua County.

Forty-six percent of the 71 cases reported by the state health department Wednesday were from the 18 to 24 age group. Another 17% of new cases were among those from ages 25 to 34 and 35 to 44.

No new resident deaths were reported Wednesday. The county's total case count now stands at 10,913 and the positivity rate for new cases was 6.9%.

Statewide, Florida added 4,423 cases, for a total of 821,123 coronavirus cases. The statewide positivity rate in Tuesday's update was 7.8%.

Alachua County public schools have reported 79 staff and 138 student cases since Aug. 17. There are 63 total active cases districtwide. Another 125 staff or students are in quarantine this week.

Active elementary cases include: Foster, one staff member; Glen Springs, one student; Idylwild, seven students and one staff member; Littlewood, one staff member; Meadowbrook, one student; Metcalfe, one student and one staff member; Talbot, one staff member; Terwilliger, one staff member; and Wiles, one staff member.

Middle school cases include: Bishop, one staff member; Hawthorne Jr./Sr. High, one staff member; Kanapaha, one student and one staff member; Lincoln, three students and one staff member; and Mebane, one staff member.

Current high school cases include: Buchholz, five students; Eastside: two staff members; Gainesville, 11 students and six staff members; and Newberry, two students, one staff member.

At the Sidney Lanier Center, one staff member has COVID-19, along with four staff members at A. Quinn Jones. One staff member at each the district office, Horizon Center, 2802 NE Eighth Ave; Manning Center, 1817 E. University Ave.; and Sivia Tech Support Center, 3700 NE 53rd Ave.; also have the coronavirus, along with a transportation worker.

The University of Florida's coronavirus testing and case dashboard includes positive case totals dating back to March and does not differentiate between active cases. Seventeen new cases among students or staff have been identified so far this month, according to the dashboard.

Statewide, Florida added 4,423 cases, for a total of 821,123 coronavirus cases. The statewide positivity rate in Tuesday's update was 7.8%.

The total number of positives so far since March is 507 for staff, 3,445 for students and 509 for employees of a UF affiliate organization. Another 712 total people affiliated with UF are in isolation or quarantine, according to the dashboard.

Tests conducted on campus at UF may not always be reflected in the county's health department report, depending on whether a student lists their primary address as an out-of-county city.

A total of 78 local residents have died from COVID-19 thus far, state data show, which includes 47 men and 31 women who ranged in age from 28 to 100. Thirty-four resident deaths are associated with long-term care facilities.

Within Alachua County's borders, the cases break down this way: Gainesville, 8,898; Alachua, 494; Newberry, 400; High Springs, 227; Archer, 143; Hawthorne, 167; Waldo, 48; Micanopy, 70; Santa Fe, 19; University of Florida: 83; Tioga, 20; LaCrosse, 14; Earleton, nine; Jonesville, two; Evinston, six; Cross Creek, two; Island Grove, one Another 187 cases don't have a specific area designation.

In surrounding counties: Bradford has had 1,246 cases and 15 deaths; Clay, 6,340 cases and 127 deaths; Putnam, 2,441 cases and 52 deaths; Marion, 11,089 cases and 350 deaths; Levy, 1,173 cases and 12 deaths; Gilchrist, 620 cases and 19 deaths; Columbia, 4,340 cases and 79 deaths; Union, 1,167 cases and 47 deaths.

In the region, Suwannee County has had 3,048 cases and 79 deaths, and Dixie County has had 915 cases and 11 deaths.

Cammack

Continued from Page 1A

preserve our constitutional principles and they see them how we do: timeless."

January, the 32-year-old will take over the open seat vacated by Yoho, who promised to limit his time in office to four terms.

Cammack was well funded, with a wave of endorsements from local, state and federal elected officials and organizations — but not Yoho. She also credits a focused and aggressive campaign leading up to Tuesday for her win.

"Anything can happen," she said. "Even though we're considered a safe Republican seat, we wanted to make sure we wanted to run a full campaign, and were very aggressive in the general election."



Cammack

In the last report from the Federal Elections Commission, she raised $938,361 and spent all but slightly more than $98,000 of it.

Christensen raised $186,623 and ended with $99,842 on hand.

Cammack, a local small business owner, campaigned on a heavily pro-President Donald Trump platform, appearing and speaking at his rally in Ocala in October. She told The Sun her national priorities include protecting individual amendment rights from federal-level infringements and bringing down the national debt.



Christensen

On a local level, she promised to focus on expanding broadband internet in the district's rural areas and to see through the completion of four VA clinics.

Upon the AP's call for the race, Christensen said he remained proud of his campaign team's work.

"We effectively did something no one else has ever seen," he said. "We bumped up turnout by 14%."

As the youngest Democrat running for Congress in the country, Christensen performed particularly well in Alachua County, drawing roughly 60% of the vote. In the northern part of Marion County, he earned 26,384 of the votes compared with Cammack's 43,704 votes.

Pitbull

Continued from Page 1A

The dognapper texted the victim over several hours, outlining plans for a money drop and threatening to harm Roz if the ransom wasn't paid.

In the meantime, the victim had made contact with sheriff's detectives.



Parris

On day two, detectives say Parris upped the ante, texting the victim that the ransom had been raised to $25,000 for Roz's safe return and threatening if the ransom wasn't paid, he would "send her (expletive deleted) paws in a box."

By then, detectives said, they had taken over the text-negotiations and arranged to leave a bag of money in bushes near the Brighton Beach Carwash at 11202 NW U.S. Highway 441, about six miles southeast of Alachua.

Detectives dropped an empty money bag at the location, and readied a surveillance team. They texted the photo of the bag.

Shortly after 5:42 p.m. Tuesday, they saw a man pull up in a pickup truck, grab the bag and leave. Minutes later, he texted again: "no money in that bag."

Deputies kept a tail on the truck until they were able to get ahead of it and place "stop sticks" in its path.

Deputies find Roz alive, huddled on the floorboard

Parris was stopped shortly thereafter — driving his wife's pickup — and deputies quickly found Roz alive, tucked between the driver's seat and the back seat, huddled on the floorboard. Also in the truck: the 24-year-old victim's $900 Louis Vuitton sweatshirt, $1,000 Louis Vuitton shoes and a leash.

A search warrant of Parris' home also recovered $14,940 of the stolen cash and packaging for a tracker device that had been placed on the victim's car.

Upon his arrest, Parris requested an attorney. His bail is set at $525,000.

According to the Florida Department of Corrections, Parris spent January 2007 until October 2009 in state prison on multiple burglary and grand theft-related charges out of Marion County.



Rosalyn the dog was found in the back of Jeffrey Parris' truck after being held $25,000 ransom, Alachua County sheriff's deputies said.
ALACHUA COUNTY SHERIFF'S OFFICE

PUBLIC NOTICE

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Instructions: Type the link into the address bar of your web browser. If unable to participate in the meeting, please contact CHW for printed materials. Comments may also be submitted in writing to the address or email below, or by phone.

Contact: Daniel Young, PE
Address: 11801 Research Drive
Alachua, FL 32615
Phone Number: (352) 331-1976
Email: DanielY@CHW-inc.com

CHW
Professional Consultants

NOTICE TO TAXPAYERS

CHAPTER 197.122 FLORIDA STATUTES PROVIDES IN PART: "ALL OWNERS OF PROPERTY SHALL BE HELD TO KNOW THAT TAXES ARE DUE AND PAYABLE ANNUALLY AND ARE CHARGED WITH THE DUTY OF ASCERTAINING THE AMOUNT OF CURRENT, AND DELINQUENT TAXES AND PAYING THEM BEFORE APRIL 1 OF THE YEAR FOLLOWING THE YEAR IN WHICH TAXES ARE ASSESSED."

UNDER CHAPTER 197.322, NOTICE IS HEREBY GIVEN THAT THE TAX ROLLS ARE NOW OPEN FOR PAYMENT OF 2020 LEVY COUNTY AND MUNICIPAL AD VALOREM TAXES AND NON-AD VALOREM ASSESSMENTS. PAYMENTS WILL BE ACCEPTED AT 310 SCHOOL STREET, BRONSON, FLORIDA; OR AT THE CHIEFLAND BRANCH OFFICE, WHICH IS LOCATED AT 226 NORTH MAIN STREET, CHIEFLAND, FLORIDA; OR AT THE WILLISTON BRANCH OFFICE, WHICH IS LOCATED AT 506 SW 1ST AVENUE, WILLISTON, FLORIDA, SUBJECT TO DISCOUNTS PROVIDED BY LAW.

ADDRESS

LEVY COUNTY ANNEX
310 SCHOOL STREET
BRONSON, FL 32621
PHONE NUMBER
(352-486-5172)

CHIEFLAND BRANCH OFFICE
226 NORTH MAIN ST.
CHIEFLAND, FL 32626
PHONE NUMBER
(352-486-5172)

WILLISTON BRANCH OFFICE
OPEN WEDNESDAY AND THURSDAY ONLY
506 SW 1ST AVENUE
WILLISTON, FL 32696
PHONE NUMBER
(352-486-5172)

OFFICES WILL BE CLOSED ON THE FOLLOWING HOLIDAY DATES:
(NOVEMBER 26 & 27 AND DECEMBER 24 & 25, 2020)

NOTE: THE 4% DISCOUNT WILL REMAIN VALID WITHIN 30 DAYS AFTER THE MAILING DATE OF THE ORIGINAL TAX NOTICE.

LINDA FUGATE, LEVY COUNTY TAX COLLECTOR

Free $10 Publix gift card.

Just get your flu shot at any Publix Pharmacy.

Restrictions apply. See pharmacy for details.



Workshop Presentation

Quick Service Restaurant

Site Plan Application

CHW Professional Consultants

City of Alachua
Neighborhood Meeting
November 19, 2020

20-0113 – QSR 3053-2-1

CHW

1

The purpose of the neighborhood meeting:

The City of Alachua requires Site Plan applicants to host a neighborhood meeting.

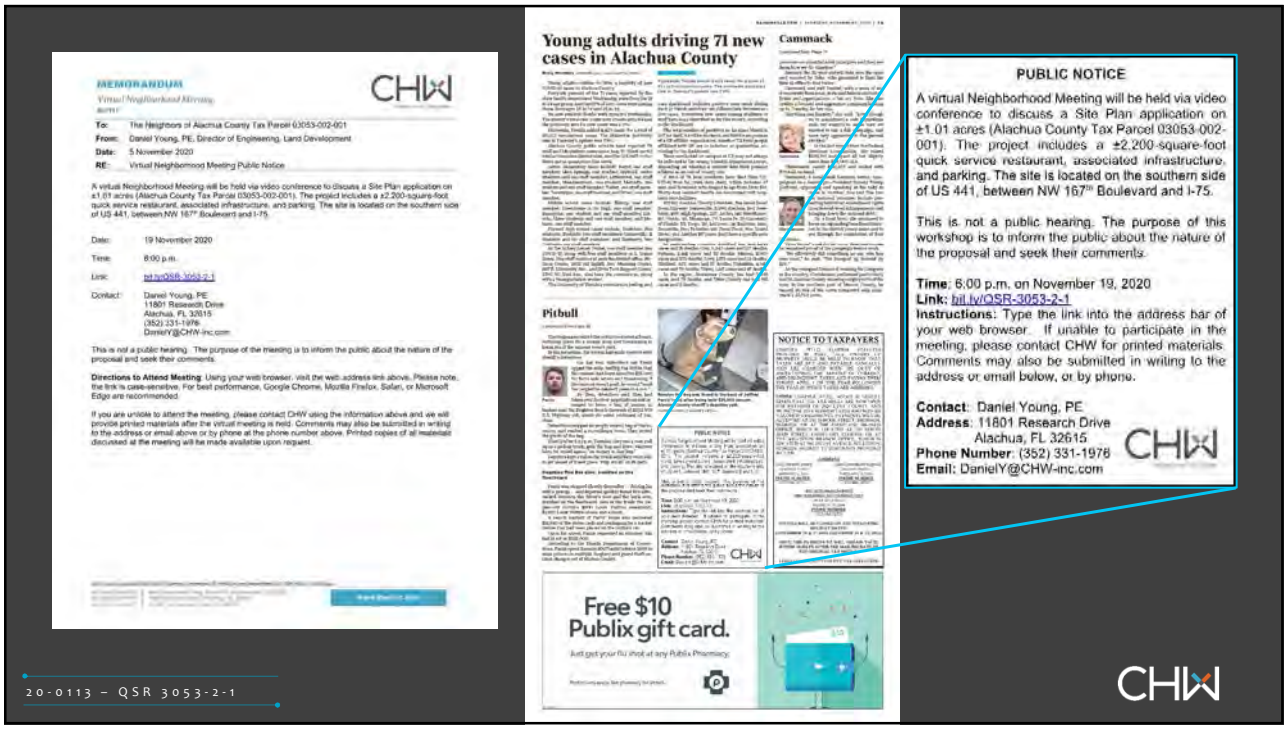
The purpose is to inform neighbors of the proposed development's nature and to get feedback early in the development process.

This meeting provides the applicant with an opportunity to mitigate concerns prior to the application's submission.



20-0113 – QSR 3053-2-1

2



3

REQUEST:
Site Plan application

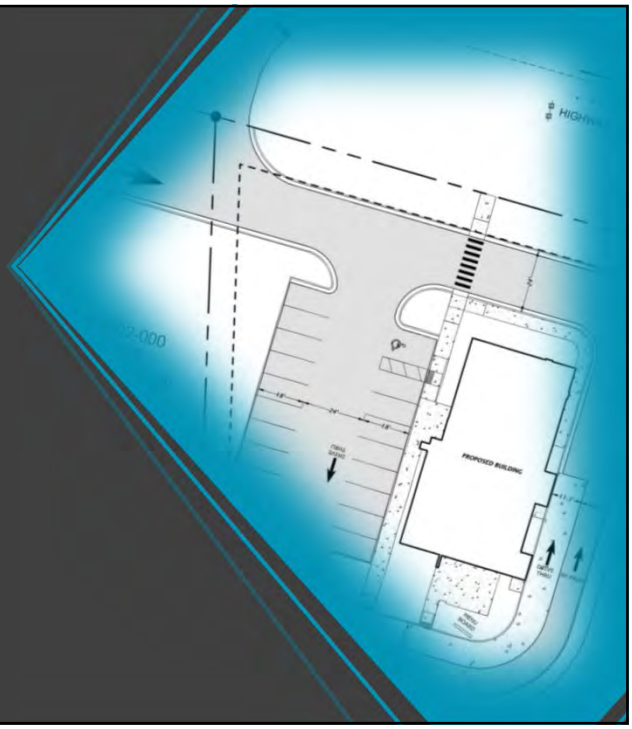


20-0113 – QSR 3053-2-1

4

INTENT:

Develop the site with a **quick service restaurant** consistent with City of Alachua Land Development Regulations.



20-0113 – QSR 3053-2-1

5

SITE PLAN TIMELINE

- Oct. 28, 2020 — Pre-application Meeting
- Nov. 19, 2020 — Neighborhood Meeting
- Dec. 2020 (tentative) — Submit Site Plan Application
- Jan.-Mar. 2020 (tentative) — Staff Review
- April 2021 (tentative) — Planning and Zoning Board
- April 2021 (tentative) — Final Site Plan Permit Issued

20-0113 – QSR 3053-2-1

CHW

6

Site Context



7

Perspectives





4

8

Aerial Map



9

Future Land Use Map

FLU: Commercial

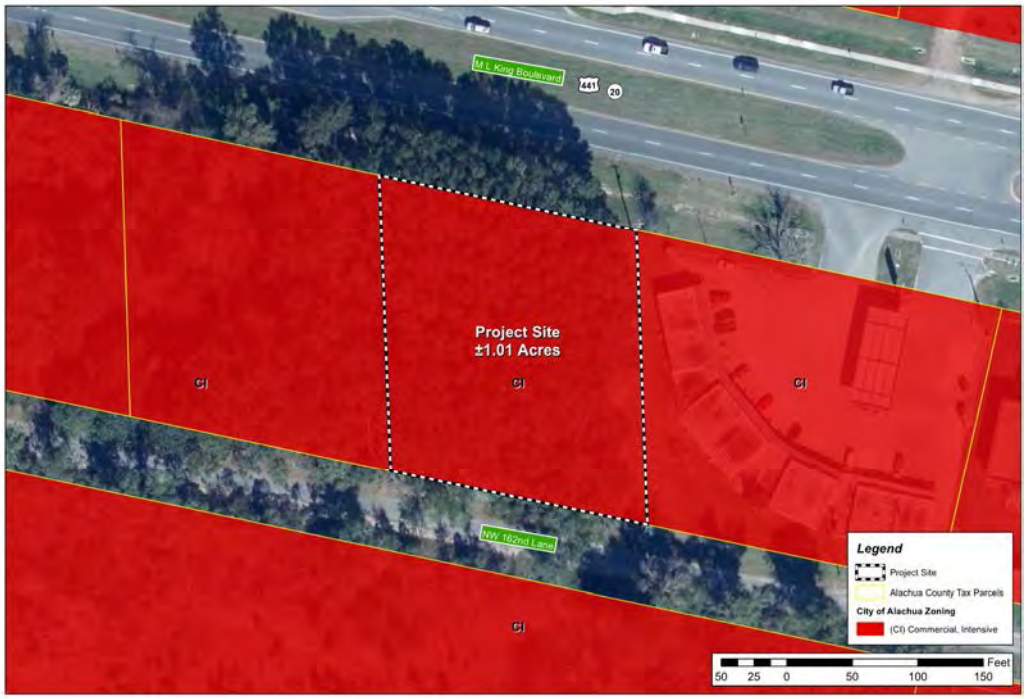


10

Zoning Map

Zoning:
Commercial Intensive (CI)

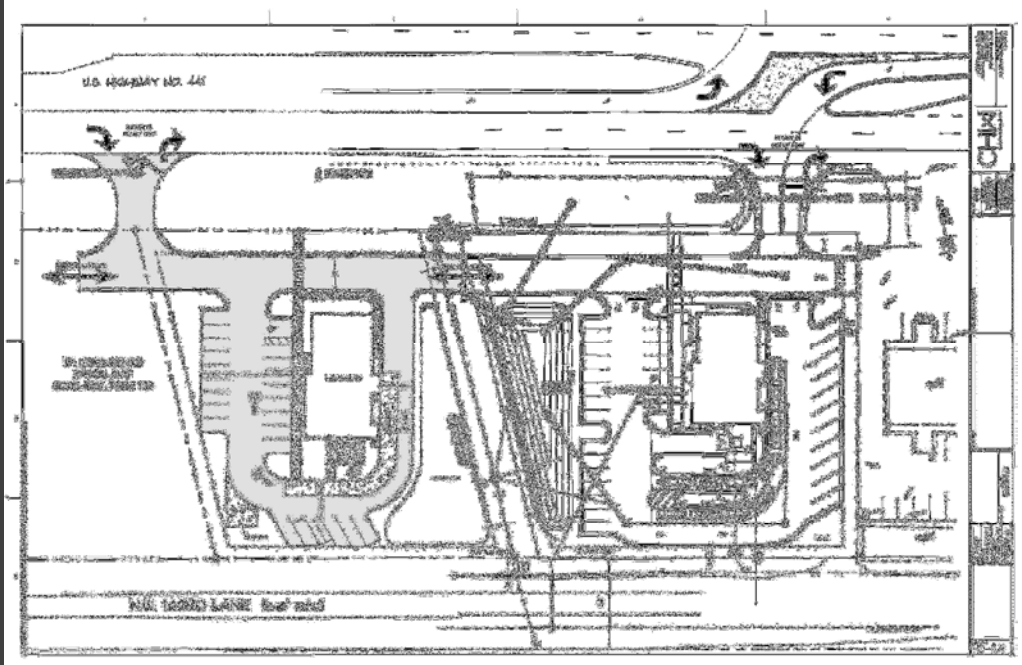
Restaurant with drive-through permitted by right in CI zoning district



11

Preliminary Concept Plan

- Access via US 441
- Buffering between parcels
- Drive-thru with by-pass option
- Stormwater management facility on-site
- Compatible with nearby development
- *Plan subject to change*



6

12

Preliminary Concept Plan in Context



CHW

13

Contact Information

Address: 11801 Research Drive
Alachua, FL, 32615
Phone: (352) 331-1976
Email: DanielY@chw-inc.com

CHW
Professional Consultants

14

Sign-in Sheet

SIGN-IN SHEET

Quick Service Restaurant Alachua

20-0113

Professional Consultants

Event:	Virtual Neighborhood Meeting
Date/Time:	November 19, 2020 @ 6:00 PM
Place:	bit.ly/QSR-3053-2-1
Re:	Quick Service Restaurant Alachua – Site Plan

	Attendee Name
1	Dan Whitney - Client
2	
3	
4	
5	
6	
7	
8	
9	
10	
11	
12	
13	

t: (904) 619-6521 | 8465 Merchants Way, Suite 102, Jacksonville, FL 32222
t: (352) 331-1976 | 11801 Research Drive, Alachua, FL 32615
t: (352) 414-4621 | 101 NE 1st Avenue, Ocala, FL 34470

www.chw-inc.com

Workshop Minutes

NEIGHBORHOOD MEETING MINUTES

Quick Service Restaurant Alachua

20-0113

Event:	Virtual Neighborhood Meeting
Date/Time:	November 19, 2020 @ 6:00 PM
Place:	https://bit.ly/QSR-3053-2-1
Re:	Quick Service Restaurant Alachua– Site Plan

CHW (Agent) Attendees:	Daniel Young, PE; Liberty Baker; Seth Wood
Applicant Attendees:	1
Public Attendees:	0

CHW hosted the required Neighborhood Meeting and presented the meeting's purpose, review process, the application's request and intent, maps illustrating the site's regulatory and physical characteristics, and a conceptual layout, in compliance with City design and development standards. There were no questions or comments.

The meeting was adjourned at approximately 6:10 p.m.

t: (904) 619-6521 | 8465 Merchants Way, Suite 102, Jacksonville, FL 32222
t: (352) 331-1976 | 11801 Research Drive, Alachua, FL 32615
t: (352) 414-4621 | 101 NE 1st Avenue, Ocala, FL 34470
www.chw-inc.com

Stormwater Management System Report



www.chw-inc.com

CHW
Professional Consultants

Engineer's Certification Statement

I hereby certify that the design of the stormwater management systems for the project known as Quick Service Restaurant – Alachua has been designed substantially in accordance with the City of Alachua, Suwannee River Water Management District, and FDOT applicable rules and regulations.

Daniel H. Young, FL PE No. 70780

Date

Causseaux, Hewett, & Walpole, Inc – Certificate of Authorization No. 0005075
11801 Research Drive, Alachua, Florida 32615 • Phone (352) 331-1976 • Fax (352) 331-2476 • www.chw-inc.com

Contents **Page**

Introduction 1

Design Criteria 1

Site Characteristics 2

Drainage Analysis 3

Summary and Conclusions 7

Figures

1 Project Location Map

2 USGS Quadrangle Map

3 Aerial Map

4 NRCS Soils Map

5 FEMA Flood Map

6 Pre-Development Drainage Map

7 Post-Development Drainage Map

Appendices

A. Drainage Calculations and Computer Model Output

B. Operation and Maintenance Requirements and Erosion and Sedimentation Control Requirements

C. Geotechnical Report

Introduction

The QSR Alachua project proposes the construction of a ±1,904 s.f. quick service restaurant with associated parking, drive aisles, driveways, stormwater management facility, and water and sewer services. The total proposed site area is ±1.06 acre, located southeast of the intersection of US 441 and NW 167th Boulevard in Alachua, Florida.

The project site is located on tax parcel #03053-002-001 and a portion of tax parcel #03053-002-000, according to the Alachua County Property Appraiser's website. Figure 1 provides a Location Map, and Figure 2 depicts the site on a portion of the High Springs USGS Quadrangle Map. The site is located in Section 9, Township 8 South, Range 18 East in Alachua County, Florida.

Refer to the accompanying engineering plans for details about the proposed construction regarding this project.

Design Criteria

The design criteria for the proposed Stormwater Management Facility (SMF) are based upon the criteria set forth by the City of Alachua (CoA), Suwannee River Water Management District (SRWMD), and Florida Department of Transportation (FDOT) for a dry retention system design in a closed watershed. The criteria are as follows:

1. Provide Peak Discharge Rate and Volume Attenuation: Attenuate the post-development peak discharge rates and cumulative discharge volumes to be less than the pre-development peak discharge rates and volumes for:
   a. The 100-year storm event with the 1-, 2-, 4-, 8-, 24-, 72-, 168-, and 240-hour durations (SRWMD).
   b. The 3-, 5-, 10-, 25-, 50-, and 100-year storm events, each with the 1-, 2-, 4-, 8-, 24-, 72-, 168-, and 240-hour durations (FDOT).

2. Provide Water Quality Treatment Volume (WQTV): The minimum stormwater treatment volume shall be the runoff from the first 2.0 inches of rainfall from the drainage area (SRWMD).

3. Provide Volume Recovery:
   a. Recover the required water quality treatment volume within 72 hours following the end of the design storm event (SRWMD).
   b. Retention systems must have one-half of the total volume available within 7 days following the end of the design storm event, and the total volume must be recovered within 30 days following the end of the design storm event (SRWMD & FDOT).

4. Freeboard: Stormwater ponds must have 1 foot of freeboard above the maximum pond stage in order to function properly during storms greater than the design storm (SRWMD).

CoA, SRWMD, and FDOT also require that best management practices be employed to control erosion and sedimentation and that an operation and maintenance entity be established.

Site Characteristics

Physical characteristics of the site are described in the following sections. Additional details are provided in the accompanying Engineering plans.

Site Topography

The existing QSR Alachua site is undeveloped with large trees and heavy underbrush. The site is bordered by US 441 to the north, NW 162nd Lane to the south, and a quick service restaurant to the east. The terrain gradually slopes from the west to the east, and elevations on site range from ±87' at the site's NW corner to ±81' at the site's NE corner. The site is located in a closed basin, and the receiving waterbody is Mill Creek Sink.

Pre-Development Drainage

Pre-development drainage on the site consists of one watershed. Pre-Development Watershed #1 (PRE-1) comprises the extents of proposed development within the undeveloped subject parcel. PRE-1 is ±0.78 acre in size and is all pervious. Stormwater runoff occurs via sheet flow and shallow concentrated flow and discharges offsite to the north and to the east.

PRE-1 consists of Type 'A' and Type 'B' soils per NRCS. Refer to Figure 6 for more information on the pre-development watershed.

Post-Development Drainage

Post-Development drainage on the site also consists of one watershed. Post-Development Watershed #1 (POST-1) consists of the extents of proposed onsite development within the subject parcel and inhabits the same ±0.78 acres as PRE-1. POST-1 includes the proposed building, parking and driveway areas, landscape areas, and the stormwater management facility. Stormwater runoff from POST-1 will flow via sheet flow and shallow concentrated flow to the stormwater pipe conveyance system and into the stormwater management facility (SMF-1). Refer to Figure 7 for more information on the post-development watershed.

SMF-1 is designed as a dry retention facility. The bottom of the pond is set at EL. 79.75' and the top of bank at EL. 84.00'. This facility's discharge structure will consist of a 3.25" orifice located at EL. 81.90'. The outfall structure will discharge stormwater to US 441 ROW via a conveyance pipe.

Soils Information

The National Resource Conservation Service (NRCS) Soil Survey for Alachua County describes the near surface soil profile for the project area as *Kendrick Sand (2 to 5% Slopes)* and *Arredondo-Urban Land Complex (0 to 5% Slopes)*. The onsite soil groups have a hydrologic soil group rating of 'B' and 'A,' respectively. Refer to Figure 4 for the NRCS Soils Map.

A site-specific soils investigation was conducted by ECS Florida, LLC in December 2020. Based on the Summary Report of Geotechnical Site Exploration, the following design parameters were

recommended for the stormwater management facility calculations. Refer to Appendix C for further details.

- Average ground elevation of borings within proposed SMF-1 area: 82.0'
- Base elevation of effective or mobilized aquifer (confining layer): 5.0' bls (SMF-1: 77.0')
- Average seasonal high groundwater table elevation: 5.0' bls (SMF-1: 77.0')
- Horizontal hydraulic conductivity: 18 feet per day (9 feet per day used in calculations)
- Unsaturated vertical infiltration rate: 12 feet per day (6 feet per day used in calculations)
- Specific yield (fillable porosity): 25%

A factor of safety of 2 was applied to the horizontal hydraulic conductivity and the unsaturated vertical infiltration rate for use in the calculations.

Drainage Analysis

The proposed stormwater management system (SMF-1) was designed to provide attenuation of the discharge rates and volumes for the 48 FDOT storm events. In addition, the stormwater management system provides for the required water quality treatment volume. SMF-1 discharges to a conveyance pipe.

Pre-development calculations were completed to determine the runoff rates and volumes for the existing conditions. Calculations for Post-Development Watershed #1 (POST-1) were completed to demonstrate that the required water quality treatment volume and peak discharge rate and volume attenuation were met. Also, the routing results were analyzed to ensure that the peak stage of each storm event satisfies the applicable freeboard requirement.

Appendix A contains details and calculations as well as a section for routing results, recovery analysis, hydraulic calculations, and general drainage calculations.

Analysis Methodology

The drainage analysis was conducted using the computer program PONDS (v3.3) to generate runoff hydrographs and route the runoff hydrographs through the proposed stormwater system with a groundwater mounding analysis. The required storm events were analyzed using FDOT rainfall amounts for the pre-development and post-development watersheds. The pre-development and post-development discharge rates, discharge volumes, and peak stages for all storm attenuations were established for SMF-1.

It is important to note that the proposed SMF-1 will be located adjacent to the existing Burger King SMF along the eastern bank. To account for this, horizontal infiltration was reduced. The length of the eastern bank was subtracted from the total perimeter of SMF-1. This adjusted perimeter was used to generate effective length and width values for the PONDS (v3.3) model, which represent horizontal infiltration through only the north, south, and west sides of SMF-1. Please refer to the Stage-Storage Table in Appendix A for more information regarding this calculation.

Unit Hydrograph Parameters

Unit hydrograph parameters required for the drainage analysis include run-off curve number (CN), time of concentration (Tc), and drainage area. Values used in the Analysis are summarized as follows:

Pre-Development Watershed #1 (PRE-1):
Watershed Area = 0.78 ac.
Impervious Area (Existing) = 0.00 ac.
Open Area (Good, Type 'B' Soil) = 0.22 ac.
Open Area (Good, Type 'A' Soil) = 0.55 ac.
CN = 45
Tc = 24 min.

Post-Development Watershed #1 (SMF-1):
Watershed Area = 0.78 ac.
Impervious Area (Proposed) = 0.52 ac.
Stormwater Management Facility = 0.18 ac.
Open Area (Good, Type 'B' Soil) = 0.00 ac.
Open Area (Good, Type 'A' Soil) = 0.08 ac.
CN = 92
Tc = 10 min.*

*Time of concentration was assumed to be a minimum of 10 minutes.

Basin Storage

Stage-storage values for the proposed stormwater management facility are provided in Appendix A.

Water Quality Treatment Volume (WQTV)

Per SRWMD, the required water quality treatment volume (WQTV) for a dry retention system is the runoff from 2.0 inches of rainfall calculated using the rational method. Table 1 contains information regarding the required WQTV for SMF-1. The required WQTV is ±4,971 cf.

Table 1: Post-Development Watershed Water Quality Treatment

Stormwater Management Facility	Required WQTV (cf)	Peak Elevation at WQTV (ft)	Time to Recover WQTV (days)
SMF-1	4,971	81.20	< 1

Run-off and Facility Routing Results

The routing results for SMF-1 are summarized on the next page in Table 2 which includes peak stage elevations and discharge rates and volumes for the analyzed storm events.

In all cases, total post-development discharge rates and volumes did not exceed the pre-development conditions. Over 1' of freeboard is provided for all storm events. Detailed results can be found in Appendix A.

Table 2: SMF-1 Peak Stage, Recovery, and Routing Results

Storm Event	Peak Stage (ft.)	Freeboard (Top = 84.00) (ft.)	1/2 Volume Recovery (days after storm event)	Full Recovery (days after storm event)	Discharge Rates (cfs)			Discharge Volumes (cf)		
					Pre	Post	Change	Pre	Post	Change
003yr - 001hr	81.05	2.95	N/A	< 1	0.00	0.00	0.00	6	0	-6
003yr - 002hr	81.10	2.90	N/A	< 1	0.02	0.00	-0.02	84	0	-84
003yr - 004hr	81.21	2.79	N/A	< 1	0.06	0.00	-0.06	284	0	-284
003yr - 008hr	81.22	2.78	N/A	2	0.08	0.00	-0.08	709	0	-709
003yr - 024hr	81.21	2.79	N/A	7	0.06	0.00	-0.06	2009	0	-2009
003yr - 072hr	81.77	2.23	N/A	14	0.12	0.00	-0.12	4344	0	-4344
003yr - 168hr	82.04	1.96	N/A	19	0.13	0.11	-0.02	7334	1863	-5471
003yr - 240hr	82.02	1.98	N/A	23	0.16	0.10	-0.06	9635	2911	-6724
005yr - 001hr	81.18	2.82	N/A	< 1	0.02	0.00	-0.02	34	0	-34
005yr - 002hr	81.28	2.72	N/A	< 1	0.05	0.00	-0.05	197	0	-197
005yr - 004hr	81.44	2.56	N/A	< 1	0.12	0.00	-0.12	548	0	-548
005yr - 008hr	81.42	2.58	N/A	3	0.12	0.00	-0.12	1081	0	-1081
005yr - 024hr	81.59	2.41	N/A	10	0.09	0.00	-0.09	2907	0	-2907
005yr - 072hr	81.91	2.09	N/A	16	0.14	0.03	-0.11	4931	178	-4753
005yr - 168hr	82.06	1.94	N/A	19	0.13	0.11	-0.02	7504	2075	-5429
005yr - 240hr	82.06	1.94	N/A	23	0.17	0.11	-0.06	10005	3340	-6665
010yr - 001hr	81.34	2.66	N/A	< 1	0.05	0.00	-0.05	101	0	-101
010yr - 002hr	81.50	2.50	N/A	< 1	0.09	0.00	-0.09	406	0	-406
010yr - 004hr	81.68	2.32	N/A	3	0.19	0.00	-0.19	918	0	-918
010yr - 008hr	81.74	2.26	N/A	6	0.19	0.00	-0.19	1839	0	-1839
010yr - 024hr	81.95	2.05	N/A	13	0.12	0.06	-0.06	4287	866	-3421
010yr - 072hr	82.05	1.95	N/A	16	0.17	0.11	-0.06	6337	2368	-3968
010yr - 168hr	82.23	1.77	N/A	21	0.16	0.16	0.00	10005	5136	-4869
010yr - 240hr	82.26	1.74	N/A	24	0.21	0.17	-0.04	12890	6626	-6265
025yr - 001hr	81.57	2.43	N/A	< 1	0.12	0.00	-0.12	260	0	-260
025yr - 002hr	81.74	2.26	N/A	2	0.15	0.00	-0.15	723	0	-723
025yr - 004hr	81.93	2.07	N/A	4	0.28	0.05	-0.23	1440	110	-1330
025yr - 008hr	81.96	2.04	N/A	8	0.29	0.07	-0.22	2554	196	-2359
025yr - 024hr	82.11	1.89	N/A	14	0.16	0.13	-0.03	5729	3299	-2430
025yr - 072hr	82.35	1.65	N/A	18	0.25	0.19	-0.06	10005	7629	-2376
025yr - 168hr	82.45	1.56	< 1	23	0.21	0.21	0.00	13892	9632	-4260
025yr - 240hr	82.57	1.43	< 1	25	0.28	0.23	-0.05	18069	12582	-5486
050yr - 001hr	81.77	2.23	N/A	< 1	0.21	0.00	-0.21	469	0	-469

050yr - 002hr	81.98	2.02	N/A	3	0.23	0.08	-0.15	1167	206	-961
050yr - 004hr	82.14	1.86	N/A	6	0.38	0.14	-0.24	2053	720	-1333
050yr - 008hr	82.17	1.83	N/A	10	0.42	0.15	-0.27	3513	1203	-2310
050yr - 024hr	82.27	1.73	N/A	15	0.22	0.17	-0.05	7504	6176	-1328
050yr - 072hr	82.50	1.50	< 1	18	0.28	0.22	-0.06	11907	10134	-1773
050yr - 168hr	82.52	1.48	< 1	23	0.23	0.22	-0.01	15949	11946	-4003
050yr - 240hr	82.70	1.30	< 1	26	0.31	0.25	-0.05	20245	14998	-5247
100yr - 001hr	81.96	2.04	N/A	2	0.31	0.07	-0.24	730	78	-652
100yr - 002hr	82.15	1.85	N/A	4	0.33	0.14	-0.19	1635	1061	-574
100yr - 004hr	82.38	1.62	N/A	6	0.51	0.20	-0.32	2894	1295	-1600
100yr - 008hr	82.31	1.69	N/A	11	0.51	0.18	-0.33	4173	2336	-1837
100yr - 024hr	82.53	1.47	< 1	15	0.31	0.22	-0.09	10173	10072	-101
100yr - 072hr	82.71	1.29	< 1	19	0.35	0.26	-0.10	15532	14648	-884
100yr - 168hr	82.66	1.35	< 1	24	0.28	0.25	-0.04	20245	16857	-3388
100yr - 240hr	82.95	1.06	< 1	27	0.36	0.29	-0.07	24740	19852	-4888

Summary and Conclusions

The proposed drainage system meets CoA, SRWMD, and FDOT criteria for a dry retention system design in a closed watershed. The criteria are as follows:

1. Provide Peak Discharge Rate and Volume Attenuation: The proposed SMF-1 attenuates the post-development peak discharge rates and cumulative discharge volumes to be less than the pre-development peak discharge rates and volumes for:
   c. The 100-year storm event with the 1-, 2-, 4-, 8-, 24-, 72-, 168-, and 240-hour durations (SRWMD).
   d. The 3-, 5-, 10-, 25-, 50-, and 100-year storm events, each with the 1-, 2-, 4-, 8-, 24-, 72-, 168-, and 240-hour durations (FDOT).

2. Provide Water Quality Treatment Volume (WQTV): SMF-1 was designed to provide treatment of the runoff from the first 2.0 inches of rainfall from the design storm (SRWMD).

3. Provide Volume Recovery:
   c. The required water quality treatment volume is recovered within 72 hours following the end of the design storm event (SRWMD).
   d. One-half of the total volume of SMF-1 is available within 7 days following the end of the design storm event, and the total volume is recovered within 30 days following the end of the design storm event (SRWMD & FDOT).

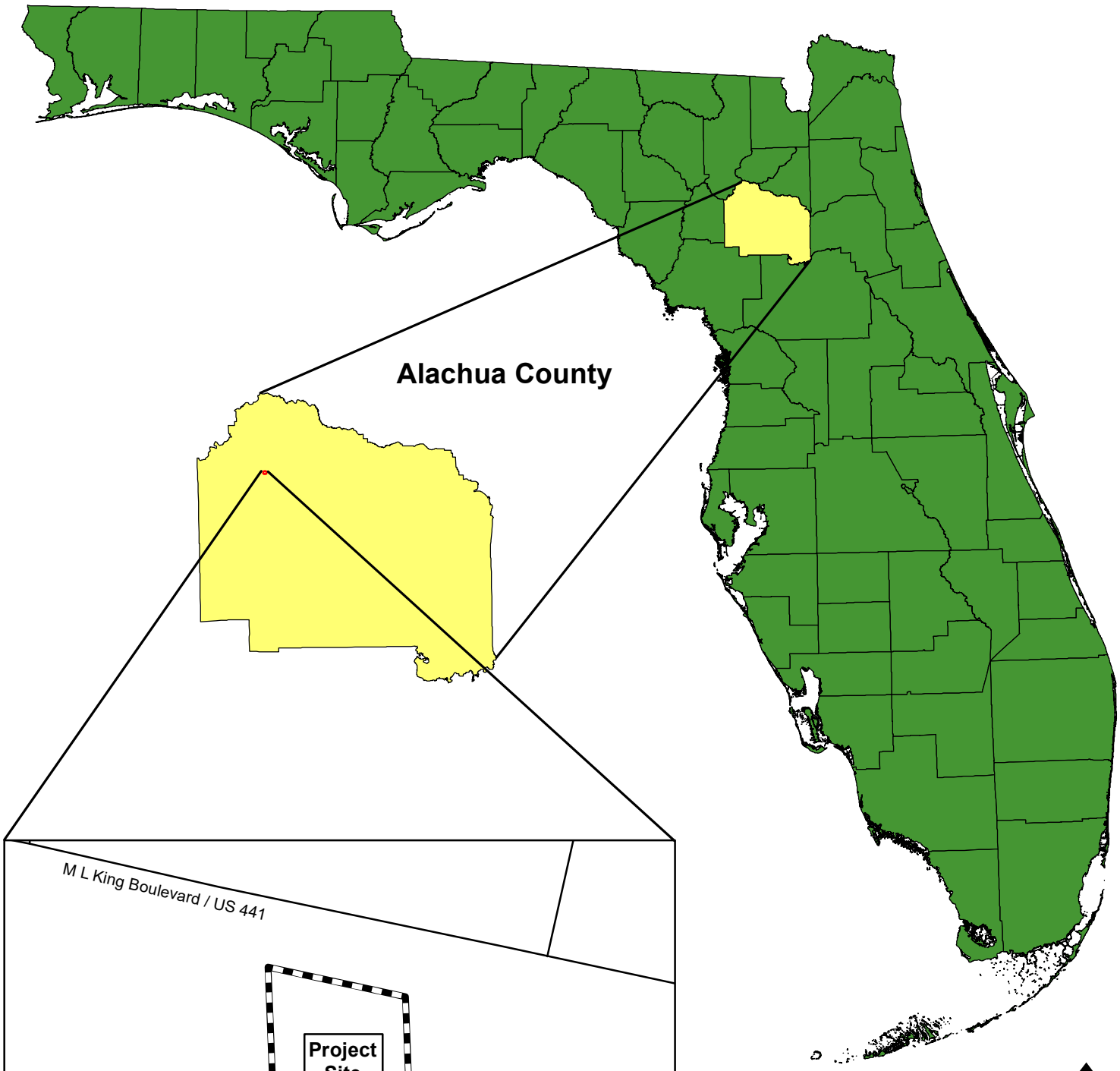
4. Freeboard: At least 1 foot of freeboard is provided for all storm events (SRWMD).

Based on the information provided, the project is eligible for approval by CoA, SRWMD, and FDOT.

Figure 1

Project Location Map

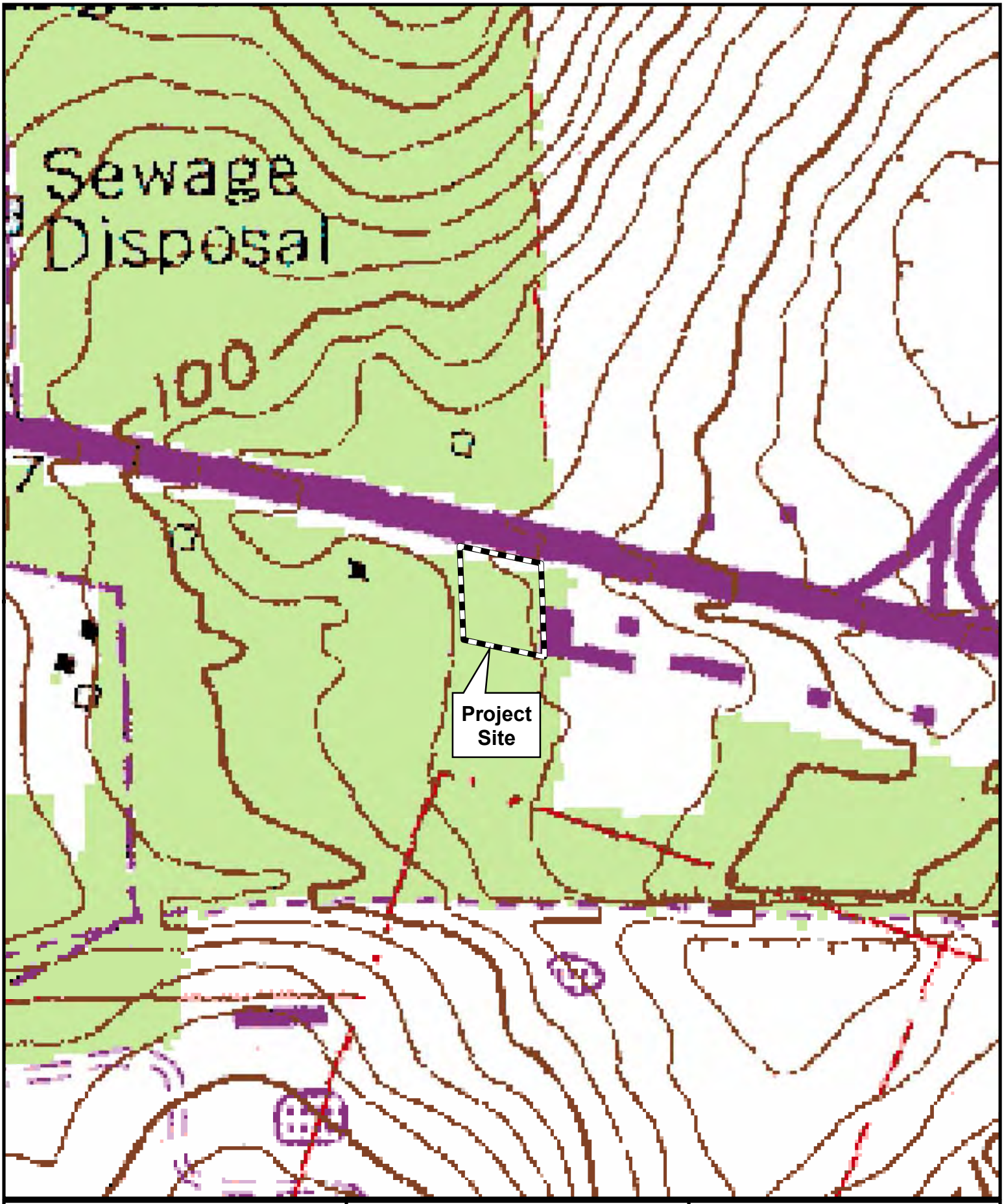
Project Location Map
QSR Alachua



CHW Professional Consultants
11801 Research Drive,
Alachua, Florida 32615
(352) 331-1976
www.chw-inc.com
est. 1988 FLORIDA
CA-5075

Figure 2

Quadrangle Map



CHW
Professional Consultants
11801 Research Drive,
Alachua, Florida 32615
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www.chw-inc.com
est. 1988 FLORIDA
CA-5075

QSR Alachua
Quad Map

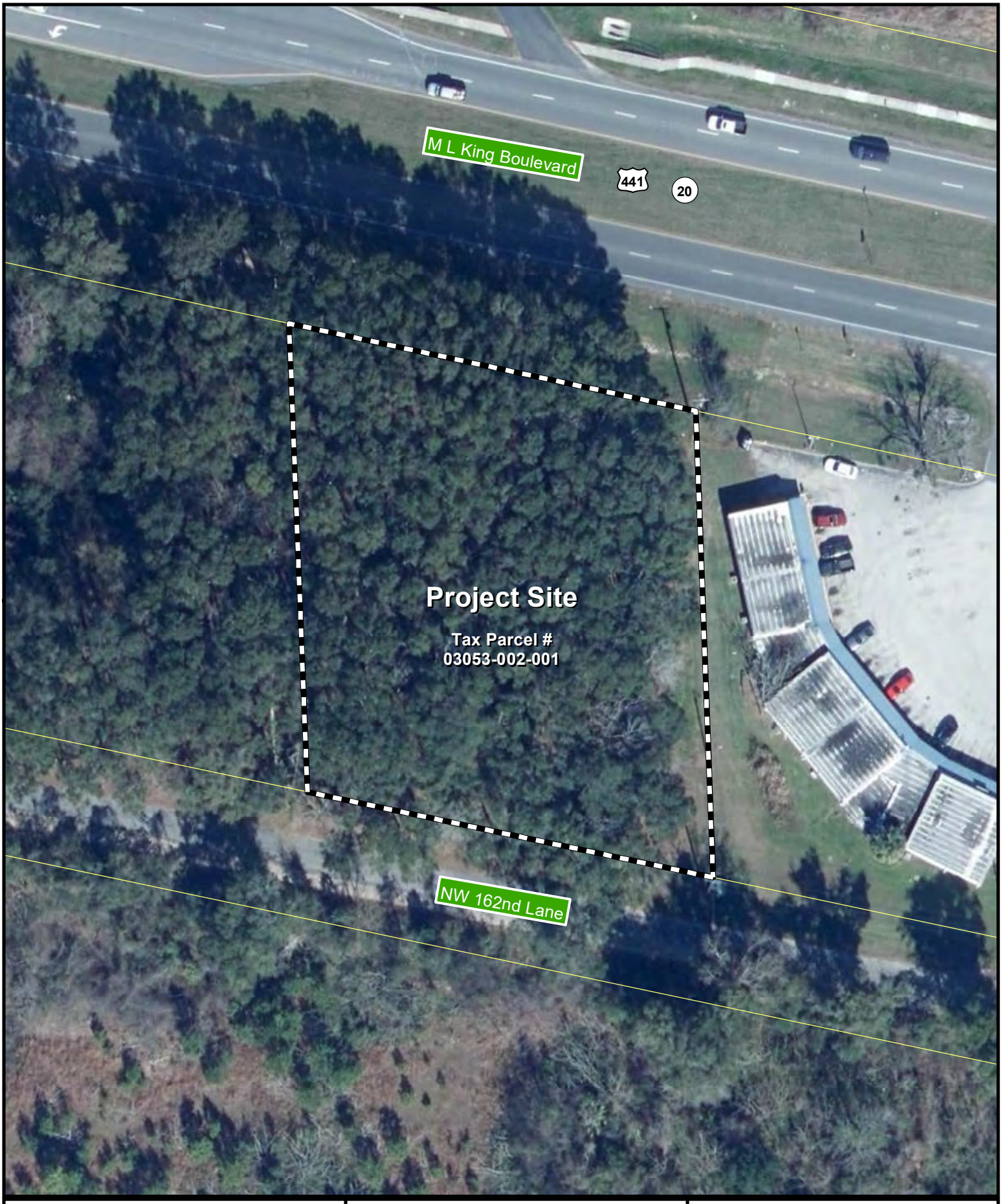
0 150 300
Feet

NORTH

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Figure 3

Aerial Map



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QSR Alachua
Aerial Map

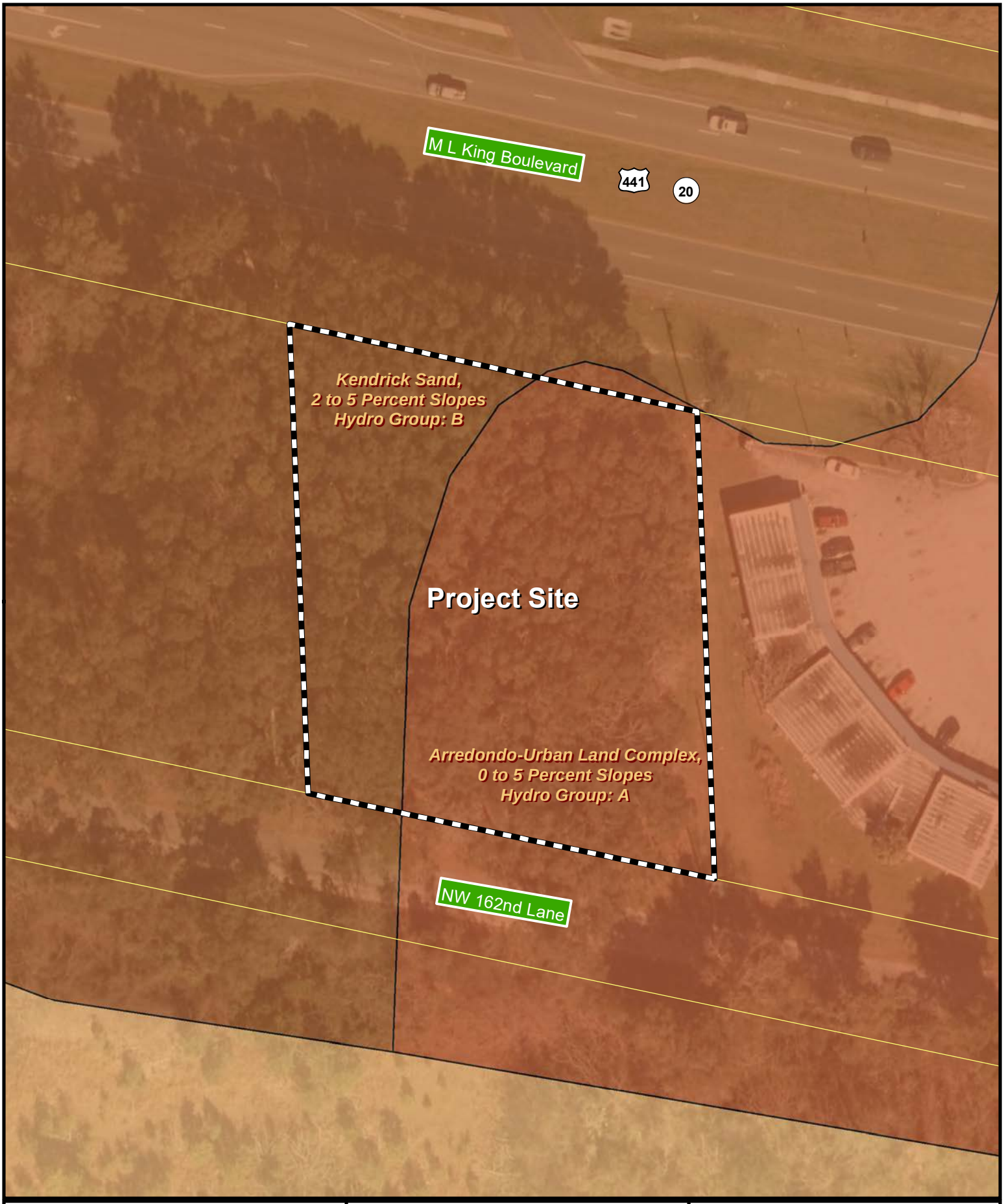
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Feet

NORTH

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Figure 4

NRCS Soils Map



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Soils Map

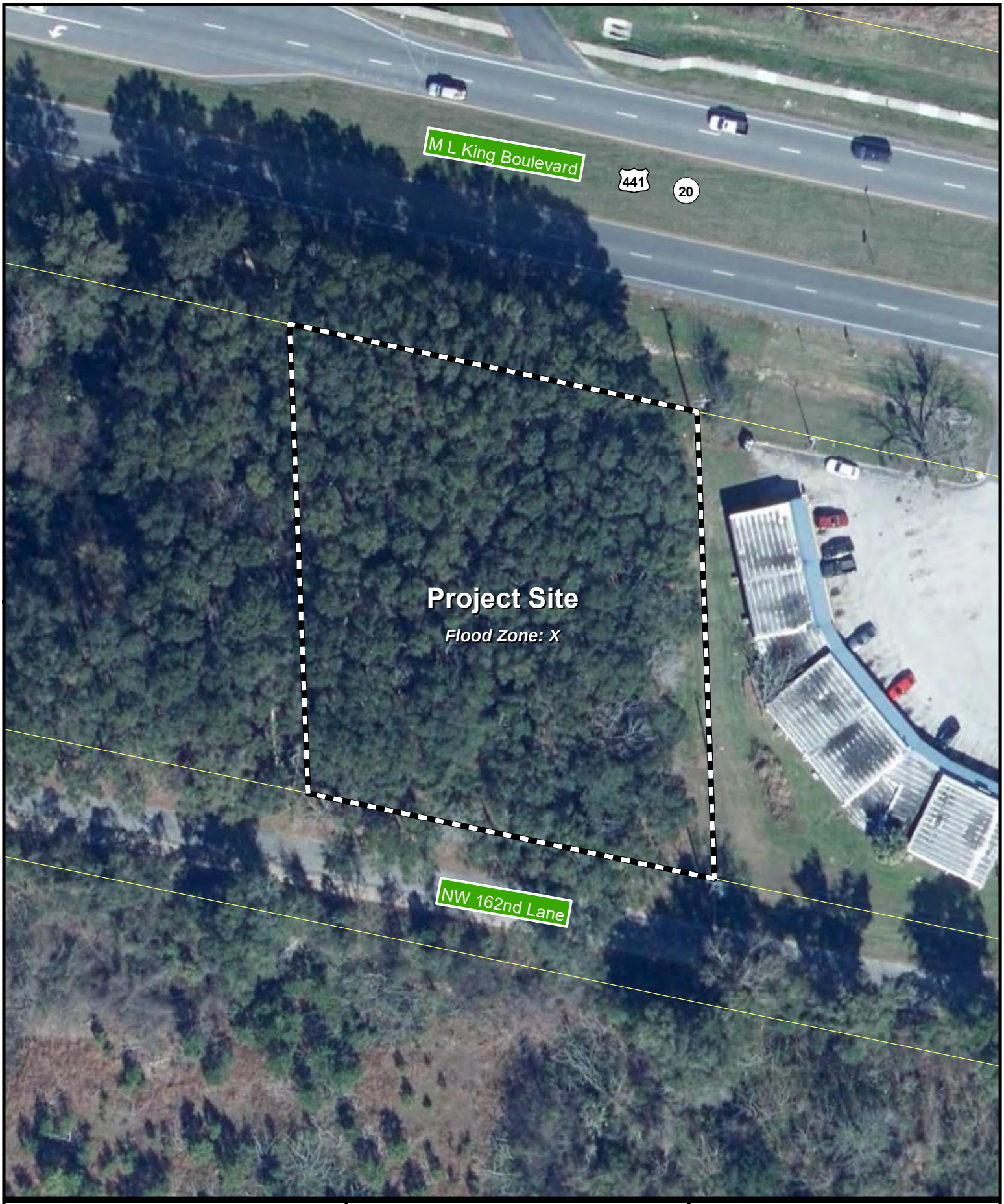
0 30 60
Feet

NORTH

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Figure 5

FEMA Flood Map



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QSR Alachua
FEMA Map

0 30 60
Feet

NORTH

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Figure 6

Pre-Development Drainage Map

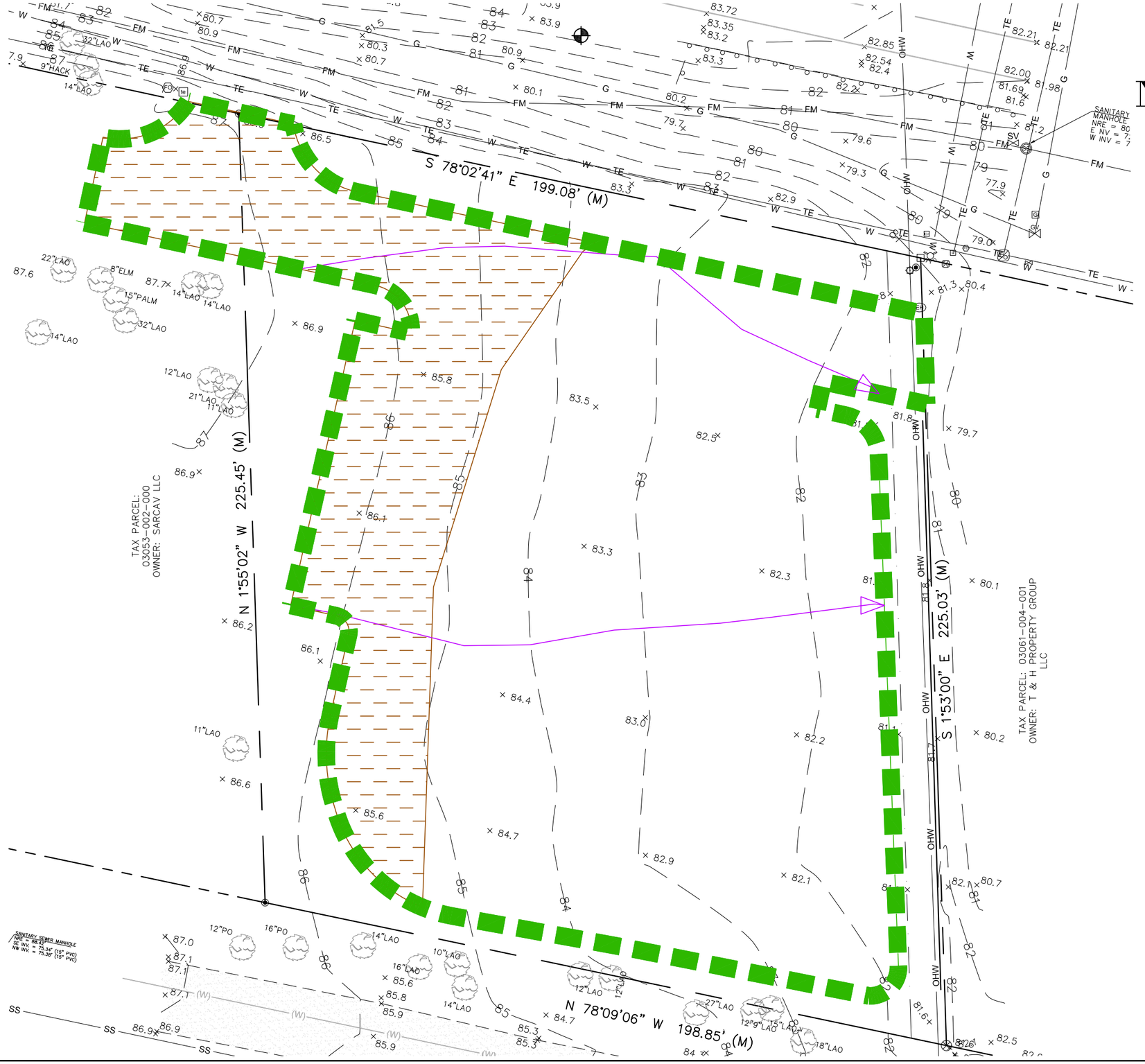
LEGEND

PRE-DEVELOPMENT WATERSHED:

PRE-DEVELOPMENT TIME OF CONCENTRATION:

TYPE 'B' SOILS:

NOTE: ALL SOILS ARE TYPE 'A' UNLESS OTHERWISE NOTED



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SCALE: 1"=60'

VERIFY SCALE BAR IS CORRECT FOR ORIGINAL DRAWING. IF NOT ONE INCH ON THIS SHEET ADJUST SCALES ACCORDINGLY.

CONSTRUCTION RECORD REVISIONS:

SUBMITTALS:

CLIENT: D.E. FOODS, LLC

PROJECT: QSR - ALACHUA

SHEET TITLE: PRE-DEVELOPMENT DRAINAGE MAP

TECHNICIAN:

DESIGNER:

QUALITY CONTROL:

PROJECT NUMBER: 20-0113

SHEET NO.: PRE

Figure 7

Post-Development Drainage Map

LEGEND

POST-DEVELOPMENT WATERSHED:

PROPOSED IMPERVIOUS AREA:

TYPE 'B' SOILS:

NOTE: ALL SOILS ARE TYPE 'A' UNLESS OTHERWISE NOTED



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SCALE: 1"=30'

VERIFY SCALE BAR IS CORRECT FOR ORIGINAL DRAWING

IF NOT ONE INCH ON THIS SHEET ADJUST SCALES ACCORDINGLY.

CONSTRUCTION RECORD REVISIONS:

SUBMITTALS:

CLIENT: D.E. FOODS, LLC

PROJECT: QSR - ALACHUA

SHEET TITLE: POST-DEVELOPMENT DRAINAGE MAP

TECHNICIAN:

DESIGNER:

QUALITY CONTROL:

PROJECT NUMBER: 20-0113

SHEET NO.: POST

Appendix A

Drainage Calculations and
Computer Model Output

Project Number: 20-0113
Project Name: QSR Alachua
Calculated by: LAB
Checked by: DHY
Date: 1/12/2021

CURVE NUMBER CALCULATIONS

Pre-Development Watershed #1 (PRE-1):

	Area (sf)	Area (ac)	Percent	CN	Area*CN
Total Area:	33,918	0.78	100%		
Impervious Area:	0	0.00	0.0%	98	0.00
Open Area (Good, Type 'B' Soil):	9,777	0.22	28.8%	61	13.69
Open Area (Good, Type 'A' Soil):	24,141	0.55	71.2%	39	21.61

Composite CN: **45**
Time of Concentration: 24 minutes

Open Space (lawns, parks, golf courses, cemeteries, etc.):

	A	B	C	D
Poor	68	79	86	89
Fair	49	69	79	84
Good	39	61	74	80

Impervious areas
Paved parking lots, roofs, driveways, etc. (excluding R/W):

A	B	C	D
98	98	98	98

Post-Development Watershed #1 (POST-1):

	Area (sf)	Area (ac)	Percent	CN	Area*CN
Total Area:	33,918	0.78	100%		
Impervious Area:	22,526	0.52	66.4%	98	50.68
Stormwater Management Facility:	7,688	0.18	22.7%	100	17.65
Open Area (Good, Type 'B' Soil):	27	0.00	0.1%	61	0.04
Open Area (Good, Type 'A' Soil):	3,677	0.08	10.8%	39	3.29

Composite CN: **92**
Time of Concentration: 10 minutes
(Time assumed to be 10 minutes)

WQTV CALCULATIONS

SMF-1 (Dry Retention):

SRWMD WQTV Calculations

2" x Drainage Area = 5,653 cf
C = 0.88
SRWMD WQTV: 4,971 cf
0.11 ac-ft

Runoff Coefficient Calculation

	Area (sf)	Coeff. C	C x A
Impervious Area	22526	0.95	21400
Open Space Area	3704	0.20	741
Pond Area	7688	1.00	7688
Total	*33918*		*29829*
		C =	0.88

Project Number: 20-0113
Project Name: QSR Alachua
Calculated by: LAB
Checked by: DHY
Date: 1/12/2021

Project No. 20-0113: QSR Alachua

	SHEET FLOW					SHALLOW CONCENTRATED FLOW					CHANNEL / PIPE FLOW							
BASIN	Manning's n (--)	Flow Length L (ft)	2-Year 24-Hour Rain, P2 (in)	Land Slope s (ft/ft)	Tt1 (hr)	Paved or Unpvd. (P or U)	Flow Length L (ft)	Water-course Slope, s (ft/ft)	Avg. Velocity V (ft/s)	Tt2 (hr)	Cross-Section Area, a (ft^2)	Wetted Perim. Pw (ft)	Hydraulic Radius r (ft)	Pipe Slope s (ft/ft)	Manning n (--)	Avg. Velocity V (ft/s)	Flow Length L (ft)	Tt3 (hr)
PRE-1	0.80	100	5.04	0.035	**0.40**	U	78	0.019	2.24	**0.01**	-	-	-	-	-	-	-	**0.00**

ID #	Tc (hr)	Tc (min)
PRE-1	0.41	**24**

TIME OF CONCENTRATION VALUES DETERMINED USING TR-55 METHODOLOGY.

SHEET FLOW:

$$Tt = \frac{.007\,(nL)^{0.8}}{(P2)^{0.5}s^{0.4}}$$

SHALLOW CONCENTRATED FLOW:

1. For slopes < 0.005 ft/ft
   Unpaved $V=16.1345\,s^{0.5}$
   Paved $V=20.3282\,s^{0.5}$

2. For slopes > 0.005 ft/ft
Velocity per Figure 3-1, TR-55

CHANNEL/PIPE FLOW:

$$V = \frac{1.49r^{2/3}s^{1/2}}{n}$$

$$Tt = \frac{L}{3600\,V}$$

Project Number: 20-0113
Project Name: QSR Alachua
Calculated by: LAB
Checked by: DHY
Date: 1/12/2021

Post-Development SMF-1: Stage-Storage Relationship				
ELEV.	**Area (SF)**	**Area (AC)**	**Volume (CF)**	**Volume (AC-FT)**
79.75	2,454	0.0563	0	0.000
80.00	2,774	0.0637	654	0.015
81.00	4,146	0.0952	4,114	0.094
82.00	5,650	0.1297	9,012	0.207
83.00	7,260	0.1667	15,467	0.355
84.00	8,970	0.2059	23,582	0.541

Post-Development - SMF-1 (Dry retention stormwater management facility):

Volume =	23,582	c.f.
Area =	8,970	s.f.
Perimeter* =	298	ft.
Depth=	4.3	ft.
Length =	**76**	**ft.**
Width =	**73**	**ft.**

*Actual perimeter of SMF-1 is 440 ft. The perimeter has been reduced to 298 ft for modeling purposes to compensate for lack of horizontal infiltration on the eastern side of SMF-1. 298 ft is equivalent to the length of the eastern bank subtracted from the total SMF-1 perimeter.

Project Number: 20-0113
Project Name: QSR Alachua
Calculated by: CCM
Checked by: DHY
Date: 3/9/2021

Project No. 20-0113: QSR Alachua

Structure No.		Invert Elev.		Length	Slope	Dia.		i	A	Q (cfs) Actual		Q Full	Pipe A	V - Full Flow	Pipe R	Minor Loss	Minor Loss	Loss	HGL		ToG/	F.B.
From	To	U.S.	D.S.	(ft)	(ft/foot)	(in)	C	(in/hr)	(ac)	Inc	Cumul	(cfs)	(sq-ft)	(fps)	(ft)	Coeff.	(ft)	(ft)	U.S.	D.S.	EoP	(in)
S-3	S-4	81.18	81.00	73.0	0.0025	18	0.20	6.4	0.11	0.15	0.15	5.65	1.8	3.2	0.38	1.0	0.00	0.00	82.50	82.50	83.85	16.2
S-9	S-8	78.67	78.44	115.0	0.0020	15	0.95	6.4	0.17	1.05	1.05	3.13	1.2	2.6	0.31	0.8	0.01	0.03	81.29	81.26	83.00	20.5
S-8	S-7	78.44	78.26	88.0	0.0020	15	0.95	6.4	0.17	1.04	1.04	3.17	1.2	2.6	0.31	0.8	0.01	0.02	81.26	81.23	83.15	22.7
S-7	S-6	78.26	78.05	105.0	0.0020	18	0.95	6.4	0.08	0.51	1.56	5.09	1.8	2.9	0.38	0.8	0.01	0.02	81.23	81.20	83.25	24.3
S-6	S-5	78.05	78.00	25.0	0.0020	18	0.95	6.4	0.11	0.66	2.22	5.09	1.8	2.9	0.38	1.0	0.02	0.01	81.23	81.20	83.25	24.2
S-10	S-11	79.21	78.90	89.0	0.0035	15	0.95	6.4	0.00	0.00	2.22	4.13	1.2	3.4	0.31	1.0	0.05	0.09	80.54	80.40	83.50	35.5

Notes
1. ToG = Top of Grate/EoP = Edge of Pavement
2. FB= Free Board
3. Rainfall intensity is based on the FDOT Zone 5 Rainfall Intensity-Duration-Frequency curve for the 3 YR - 10 min storm event (6.4 inches/hr)
4. The tailwater condition in the pond was set at the peak stage for the WQTV (EL. 81.20')

PONDS Version 3.3.0241
Retention Pond Recovery - Refined Method
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Devo Seereeram, Ph.D., P.E.

Project Data

Project Name:	QSR Alachua
Simulation Description:	PRE-1 SRWMD and FDOT storms
Project Number:	20-0113
Engineer :	Liberty Baker
Supervising Engineer:	Daniel Young, P.E.
Date:	01-05-2021

Scenario Input Data

Scenario 1 :: FDOT 1 Hour - 1 hr - 3 yr

Hydrograph Type:	Inline SCS
• **Modflow Routing:**	**Not routed**
Repetitions:	1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	24.0
DCIA (%)	0.0
Curve Number	45
Design Rainfall Depth (inches)	2.6
Design Rainfall Duration (hours)	1.0
Shape Factor	UHG 323
Rainfall Distribution	FDOT 1 Hour

Initial ground water level (ft datum) 77.00 (default)

No times after storm specified.

Scenario 2 :: FDOT 2 Hour - 2 hr - 3 yr

Hydrograph Type:	Inline SCS
• **Modflow Routing:**	**Not routed**
Repetitions:	1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	24.0
DCIA (%)	0.0
Curve Number	45
Design Rainfall Depth (inches)	3.1
Design Rainfall Duration (hours)	2.0
Shape Factor	UHG 323
Rainfall Distribution	FDOT 2 Hour

Initial ground water level (ft datum) 77.00 (default)

No times after storm specified.

Scenario 3 :: FDOT 4 Hour - 4 hr - 3 yr

Hydrograph Type:	Inline SCS
• **Modflow Routing:**	**Not routed**
Repetitions:	1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	24.0
DCIA (%)	0.0
Curve Number	45
Design Rainfall Depth (inches)	3.6
Design Rainfall Duration (hours)	4.0
Shape Factor	UHG 323
Rainfall Distribution	FDOT 4 Hour

Initial ground water level (ft datum) 77.00 (default)

No times after storm specified.

Scenario Input Data (cont'd.)

Scenario 4 :: FDOT 8 Hour - 8 hr - 3 yr

Hydrograph Type: Inline SCS
• **Modflow Routing: Not routed**
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	24.0
DCIA (%)	0.0
Curve Number	45
Design Rainfall Depth (inches)	4.3
Design Rainfall Duration (hours)	8.0
Shape Factor	UHG 323
Rainfall Distribution	FDOT 8 Hour

Initial ground water level (ft datum) 77.00 (default)

No times after storm specified.

Scenario 5 :: FDOT 24 Hour - 24 hr - 3 yr

Hydrograph Type: Inline SCS
• **Modflow Routing: Not routed**
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	24.0
DCIA (%)	0.0
Curve Number	45
Design Rainfall Depth (inches)	5.8
Design Rainfall Duration (hours)	24.0
Shape Factor	UHG 323
Rainfall Distribution	FDOT 24 Hour

Initial ground water level (ft datum) 77.00 (default)

No times after storm specified.

Scenario 6 :: FDOT 72 Hour - 72 hr - 3 yr

Hydrograph Type: Inline SCS
• **Modflow Routing: Not routed**
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	24.0
DCIA (%)	0.0
Curve Number	45
Design Rainfall Depth (inches)	7.6
Design Rainfall Duration (hours)	72.0
Shape Factor	UHG 323
Rainfall Distribution	FDOT 72 Hour

Initial ground water level (ft datum) 77.00 (default)

No times after storm specified.

Scenario Input Data (cont'd.)

Scenario 7 :: FDOT 168 Hour - 168 hr - 3 yr

Hydrograph Type:	Inline SCS
• **Modflow Routing:**	**Not routed**
Repetitions:	1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	24.0
DCIA (%)	0.0
Curve Number	45
Design Rainfall Depth (inches)	9.5
Design Rainfall Duration (hours)	168.0
Shape Factor	UHG 323
Rainfall Distribution	FDOT 168 Hour

Initial ground water level (ft datum) 77.00 (default)

No times after storm specified.

Scenario 8 :: FDOT 240 Hour - 240 hr - 3 yr

Hydrograph Type:	Inline SCS
• **Modflow Routing:**	**Not routed**
Repetitions:	1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	24.0
DCIA (%)	0.0
Curve Number	45
Design Rainfall Depth (inches)	10.8
Design Rainfall Duration (hours)	240.0
Shape Factor	UHG 323
Rainfall Distribution	FDOT 240 Hour

Initial ground water level (ft datum) 77.00 (default)

No times after storm specified.

Scenario 9 :: FDOT 1 Hour - 1 hr - 5 yr

Hydrograph Type:	Inline SCS
• **Modflow Routing:**	**Not routed**
Repetitions:	1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	24.0
DCIA (%)	0.0
Curve Number	45
Design Rainfall Depth (inches)	2.8
Design Rainfall Duration (hours)	1.0
Shape Factor	UHG 323
Rainfall Distribution	FDOT 1 Hour

Initial ground water level (ft datum) 77.00 (default)

No times after storm specified.

Scenario Input Data (cont'd.)

Scenario 10 :: FDOT 2 Hour - 2 hr - 5 yr

Hydrograph Type: Inline SCS
• **Modflow Routing: Not routed**
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	24.0
DCIA (%)	0.0
Curve Number	45
Design Rainfall Depth (inches)	3.4
Design Rainfall Duration (hours)	2.0
Shape Factor	UHG 323
Rainfall Distribution	FDOT 2 Hour

Initial ground water level (ft datum) 77.00 (default)

No times after storm specified.

Scenario 11 :: FDOT 4 Hour - 4 hr - 5 yr

Hydrograph Type: Inline SCS
• **Modflow Routing: Not routed**
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	24.0
DCIA (%)	0.0
Curve Number	45
Design Rainfall Depth (inches)	4.1
Design Rainfall Duration (hours)	4.0
Shape Factor	UHG 323
Rainfall Distribution	FDOT 4 Hour

Initial ground water level (ft datum) 77.00 (default)

No times after storm specified.

Scenario 12 :: FDOT 8 Hour - 8 hr - 5 yr

Hydrograph Type: Inline SCS
• **Modflow Routing: Not routed**
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	24.0
DCIA (%)	0.0
Curve Number	45
Design Rainfall Depth (inches)	4.8
Design Rainfall Duration (hours)	8.0
Shape Factor	UHG 323
Rainfall Distribution	FDOT 8 Hour

Initial ground water level (ft datum) 77.00 (default)

No times after storm specified.

Scenario Input Data (cont'd.)

Scenario 13 :: FDOT 24 Hour - 24 hr - 5 yr

Hydrograph Type:	Inline SCS
• **Modflow Routing:**	**Not routed**
Repetitions:	1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	24.0
DCIA (%)	0.0
Curve Number	45
Design Rainfall Depth (inches)	6.5
Design Rainfall Duration (hours)	24.0
Shape Factor	UHG 323
Rainfall Distribution	FDOT 24 Hour

Initial ground water level (ft datum) 77.00 (default)

No times after storm specified.

Scenario 14 :: FDOT 72 Hour - 72 hr - 5 yr

Hydrograph Type:	Inline SCS
• **Modflow Routing:**	**Not routed**
Repetitions:	1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	24.0
DCIA (%)	0.0
Curve Number	45
Design Rainfall Depth (inches)	8.0
Design Rainfall Duration (hours)	72.0
Shape Factor	UHG 323
Rainfall Distribution	FDOT 72 Hour

Initial ground water level (ft datum) 77.00 (default)

No times after storm specified.

Scenario 15 :: FDOT 168 Hour - 168 hr - 5 yr

Hydrograph Type:	Inline SCS
• **Modflow Routing:**	**Not routed**
Repetitions:	1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	24.0
DCIA (%)	0.0
Curve Number	45
Design Rainfall Depth (inches)	9.6
Design Rainfall Duration (hours)	168.0
Shape Factor	UHG 323
Rainfall Distribution	FDOT 168 Hour

Initial ground water level (ft datum) 77.00 (default)

No times after storm specified.

Scenario Input Data (cont'd.)

Scenario 16 :: FDOT 240 Hour - 240 hr - 5 yr

Hydrograph Type: Inline SCS
• **Modflow Routing: Not routed**
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	24.0
DCIA (%)	0.0
Curve Number	45
Design Rainfall Depth (inches)	11.0
Design Rainfall Duration (hours)	240.0
Shape Factor	UHG 323
Rainfall Distribution	FDOT 240 Hour

Initial ground water level (ft datum) 77.00 (default)

No times after storm specified.

Scenario 17 :: FDOT 1 Hour - 1 hr - 10 yr

Hydrograph Type: Inline SCS
• **Modflow Routing: Not routed**
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	24.0
DCIA (%)	0.0
Curve Number	45
Design Rainfall Depth (inches)	3.1
Design Rainfall Duration (hours)	1.0
Shape Factor	UHG 323
Rainfall Distribution	FDOT 1 Hour

Initial ground water level (ft datum) 77.00 (default)

No times after storm specified.

Scenario 18 :: FDOT 2 Hour - 2 hr - 10 yr

Hydrograph Type: Inline SCS
• **Modflow Routing: Not routed**
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	24.0
DCIA (%)	0.0
Curve Number	45
Design Rainfall Depth (inches)	3.8
Design Rainfall Duration (hours)	2.0
Shape Factor	UHG 323
Rainfall Distribution	FDOT 2 Hour

Initial ground water level (ft datum) 77.00 (default)

No times after storm specified.

Scenario Input Data (cont'd.)

Scenario 19 :: FDOT 4 Hour - 4 hr - 10 yr

Hydrograph Type: Inline SCS
• **Modflow Routing: Not routed**
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	24.0
DCIA (%)	0.0
Curve Number	45
Design Rainfall Depth (inches)	4.6
Design Rainfall Duration (hours)	4.0
Shape Factor	UHG 323
Rainfall Distribution	FDOT 4 Hour

Initial ground water level (ft datum) 77.00 (default)

No times after storm specified.

Scenario 20 :: FDOT 8 Hour - 8 hr - 10 yr

Hydrograph Type: Inline SCS
• **Modflow Routing: Not routed**
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	24.0
DCIA (%)	0.0
Curve Number	45
Design Rainfall Depth (inches)	5.6
Design Rainfall Duration (hours)	8.0
Shape Factor	UHG 323
Rainfall Distribution	FDOT 8 Hour

Initial ground water level (ft datum) 77.00 (default)

No times after storm specified.

Scenario 21 :: FDOT 24 Hour - 24 hr - 10 yr

Hydrograph Type: Inline SCS
• **Modflow Routing: Not routed**
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	24.0
DCIA (%)	0.0
Curve Number	45
Design Rainfall Depth (inches)	7.6
Design Rainfall Duration (hours)	24.0
Shape Factor	UHG 323
Rainfall Distribution	FDOT 24 Hour

Initial ground water level (ft datum) 77.00 (default)

No times after storm specified.

Scenario Input Data (cont'd.)

Scenario 22 :: FDOT 72 Hour - 72 hr - 10 yr

Hydrograph Type: Inline SCS
• **Modflow Routing: Not routed**
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	24.0
DCIA (%)	0.0
Curve Number	45
Design Rainfall Depth (inches)	8.9
Design Rainfall Duration (hours)	72.0
Shape Factor	UHG 323
Rainfall Distribution	FDOT 72 Hour

Initial ground water level (ft datum) 77.00 (default)

No times after storm specified.

Scenario 23 :: FDOT 168 Hour - 168 hr - 10 yr

Hydrograph Type: Inline SCS
• **Modflow Routing: Not routed**
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	24.0
DCIA (%)	0.0
Curve Number	45
Design Rainfall Depth (inches)	11.0
Design Rainfall Duration (hours)	168.0
Shape Factor	UHG 323
Rainfall Distribution	FDOT 168 Hour

Initial ground water level (ft datum) 77.00 (default)

No times after storm specified.

Scenario 24 :: FDOT 240 Hour - 240 hr - 10 yr

Hydrograph Type: Inline SCS
• **Modflow Routing: Not routed**
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	24.0
DCIA (%)	0.0
Curve Number	45
Design Rainfall Depth (inches)	12.5
Design Rainfall Duration (hours)	240.0
Shape Factor	UHG 323
Rainfall Distribution	FDOT 240 Hour

Initial ground water level (ft datum) 77.00 (default)

No times after storm specified.

Scenario Input Data (cont'd.)

Scenario 25 :: FDOT 1 Hour - 1 hr - 25 yr

Hydrograph Type:	Inline SCS
• **Modflow Routing:**	**Not routed**
Repetitions:	1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	24.0
DCIA (%)	0.0
Curve Number	45
Design Rainfall Depth (inches)	3.6
Design Rainfall Duration (hours)	1.0
Shape Factor	UHG 323
Rainfall Distribution	FDOT 1 Hour

Initial ground water level (ft datum) 77.00 (default)

No times after storm specified.

Scenario 26 :: FDOT 2 Hour - 2 hr - 25 yr

Hydrograph Type:	Inline SCS
• **Modflow Routing:**	**Not routed**
Repetitions:	1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	24.0
DCIA (%)	0.0
Curve Number	45
Design Rainfall Depth (inches)	4.3
Design Rainfall Duration (hours)	2.0
Shape Factor	UHG 323
Rainfall Distribution	FDOT 2 Hour

Initial ground water level (ft datum) 77.00 (default)

No times after storm specified.

Scenario 27 :: FDOT 4 Hour - 4 hr - 25 yr

Hydrograph Type:	Inline SCS
• **Modflow Routing:**	**Not routed**
Repetitions:	1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	24.0
DCIA (%)	0.0
Curve Number	45
Design Rainfall Depth (inches)	5.2
Design Rainfall Duration (hours)	4.0
Shape Factor	UHG 323
Rainfall Distribution	FDOT 4 Hour

Initial ground water level (ft datum) 77.00 (default)

No times after storm specified.

Scenario Input Data (cont'd.)

Scenario 28 :: FDOT 8 Hour - 8 hr - 25 yr

Hydrograph Type: Inline SCS
• **Modflow Routing: Not routed**
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	24.0
DCIA (%)	0.0
Curve Number	45
Design Rainfall Depth (inches)	6.2
Design Rainfall Duration (hours)	8.0
Shape Factor	UHG 323
Rainfall Distribution	FDOT 8 Hour

Initial ground water level (ft datum) 77.00 (default)

No times after storm specified.

Scenario 29 :: FDOT 24 Hour - 24 hr - 25 yr

Hydrograph Type: Inline SCS
• **Modflow Routing: Not routed**
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	24.0
DCIA (%)	0.0
Curve Number	45
Design Rainfall Depth (inches)	8.5
Design Rainfall Duration (hours)	24.0
Shape Factor	UHG 323
Rainfall Distribution	FDOT 24 Hour

Initial ground water level (ft datum) 77.00 (default)

No times after storm specified.

Scenario 30 :: FDOT 72 Hour - 72 hr - 25 yr

Hydrograph Type: Inline SCS
• **Modflow Routing: Not routed**
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	24.0
DCIA (%)	0.0
Curve Number	45
Design Rainfall Depth (inches)	11.0
Design Rainfall Duration (hours)	72.0
Shape Factor	UHG 323
Rainfall Distribution	FDOT 72 Hour

Initial ground water level (ft datum) 77.00 (default)

No times after storm specified.

Scenario Input Data (cont'd.)

Scenario 31 :: FDOT 168 Hour - 168 hr - 25 yr

Hydrograph Type: Inline SCS
• **Modflow Routing: Not routed**
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	24.0
DCIA (%)	0.0
Curve Number	45
Design Rainfall Depth (inches)	13.0
Design Rainfall Duration (hours)	168.0
Shape Factor	UHG 323
Rainfall Distribution	FDOT 168 Hour

Initial ground water level (ft datum) 77.00 (default)

No times after storm specified.

Scenario 32 :: FDOT 240 Hour - 240 hr - 25 yr

Hydrograph Type: Inline SCS
• **Modflow Routing: Not routed**
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	24.0
DCIA (%)	0.0
Curve Number	45
Design Rainfall Depth (inches)	15.0
Design Rainfall Duration (hours)	240.0
Shape Factor	UHG 323
Rainfall Distribution	FDOT 240 Hour

Initial ground water level (ft datum) 77.00 (default)

No times after storm specified.

Scenario 33 :: FDOT 1 Hour - 1 hr - 50 yr

Hydrograph Type: Inline SCS
• **Modflow Routing: Not routed**
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	24.0
DCIA (%)	0.0
Curve Number	45
Design Rainfall Depth (inches)	4.0
Design Rainfall Duration (hours)	1.0
Shape Factor	UHG 323
Rainfall Distribution	FDOT 1 Hour

Initial ground water level (ft datum) 77.00 (default)

No times after storm specified.

Scenario Input Data (cont'd.)

Scenario 34 :: FDOT 2 Hour - 2 hr - 50 yr

Hydrograph Type: Inline SCS
• **Modflow Routing: Not routed**
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	24.0
DCIA (%)	0.0
Curve Number	45
Design Rainfall Depth (inches)	4.9
Design Rainfall Duration (hours)	2.0
Shape Factor	UHG 323
Rainfall Distribution	FDOT 2 Hour

Initial ground water level (ft datum) 77.00 (default)

No times after storm specified.

Scenario 35 :: FDOT 4 Hour - 4 hr - 50 yr

Hydrograph Type: Inline SCS
• **Modflow Routing: Not routed**
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	24.0
DCIA (%)	0.0
Curve Number	45
Design Rainfall Depth (inches)	5.8
Design Rainfall Duration (hours)	4.0
Shape Factor	UHG 323
Rainfall Distribution	FDOT 4 Hour

Initial ground water level (ft datum) 77.00 (default)

No times after storm specified.

Scenario 36 :: FDOT 8 Hour - 8 hr - 50 yr

Hydrograph Type: Inline SCS
• **Modflow Routing: Not routed**
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	24.0
DCIA (%)	0.0
Curve Number	45
Design Rainfall Depth (inches)	7.0
Design Rainfall Duration (hours)	8.0
Shape Factor	UHG 323
Rainfall Distribution	FDOT 8 Hour

Initial ground water level (ft datum) 77.00 (default)

No times after storm specified.

Scenario Input Data (cont'd.)

Scenario 37 :: FDOT 24 Hour - 24 hr - 50 yr

Hydrograph Type:	Inline SCS
• **Modflow Routing:**	**Not routed**
Repetitions:	1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	24.0
DCIA (%)	0.0
Curve Number	45
Design Rainfall Depth (inches)	9.6
Design Rainfall Duration (hours)	24.0
Shape Factor	UHG 323
Rainfall Distribution	FDOT 24 Hour

Initial ground water level (ft datum) 77.00 (default)

No times after storm specified.

Scenario 38 :: FDOT 72 Hour - 72 hr - 50 yr

Hydrograph Type:	Inline SCS
• **Modflow Routing:**	**Not routed**
Repetitions:	1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	24.0
DCIA (%)	0.0
Curve Number	45
Design Rainfall Depth (inches)	12.0
Design Rainfall Duration (hours)	72.0
Shape Factor	UHG 323
Rainfall Distribution	FDOT 72 Hour

Initial ground water level (ft datum) 77.00 (default)

No times after storm specified.

Scenario 39 :: FDOT 168 Hour - 168 hr - 50 yr

Hydrograph Type:	Inline SCS
• **Modflow Routing:**	**Not routed**
Repetitions:	1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	24.0
DCIA (%)	0.0
Curve Number	45
Design Rainfall Depth (inches)	14.0
Design Rainfall Duration (hours)	168.0
Shape Factor	UHG 323
Rainfall Distribution	FDOT 168 Hour

Initial ground water level (ft datum) 77.00 (default)

No times after storm specified.

Scenario Input Data (cont'd.)

Scenario 40 :: FDOT 240 Hour - 240 hr - 50 yr

Hydrograph Type:	Inline SCS
• **Modflow Routing:**	**Not routed**
Repetitions:	1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	24.0
DCIA (%)	0.0
Curve Number	45
Design Rainfall Depth (inches)	16.0
Design Rainfall Duration (hours)	240.0
Shape Factor	UHG 323
Rainfall Distribution	FDOT 240 Hour

Initial ground water level (ft datum) 77.00 (default)

No times after storm specified.

Scenario 41 :: FDOT 1 Hour - 1 hr - 100 yr

Hydrograph Type:	Inline SCS
• **Modflow Routing:**	**Not routed**
Repetitions:	1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	24.0
DCIA (%)	0.0
Curve Number	45
Design Rainfall Depth (inches)	4.4
Design Rainfall Duration (hours)	1.0
Shape Factor	UHG 323
Rainfall Distribution	FDOT 1 Hour

Initial ground water level (ft datum) 77.00 (default)

No times after storm specified.

Scenario 42 :: FDOT 2 Hour - 2 hr - 100 yr

Hydrograph Type:	Inline SCS
• **Modflow Routing:**	**Not routed**
Repetitions:	1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	24.0
DCIA (%)	0.0
Curve Number	45
Design Rainfall Depth (inches)	5.4
Design Rainfall Duration (hours)	2.0
Shape Factor	UHG 323
Rainfall Distribution	FDOT 2 Hour

Initial ground water level (ft datum) 77.00 (default)

No times after storm specified.

Scenario Input Data (cont'd.)

Scenario 43 :: FDOT 4 Hour - 4 hr - 100 yr

Hydrograph Type:	Inline SCS
• **Modflow Routing:**	**Not routed**
Repetitions:	1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	24.0
DCIA (%)	0.0
Curve Number	45
Design Rainfall Depth (inches)	6.5
Design Rainfall Duration (hours)	4.0
Shape Factor	UHG 323
Rainfall Distribution	FDOT 4 Hour

Initial ground water level (ft datum) 77.00 (default)

No times after storm specified.

Scenario 44 :: FDOT 8 Hour - 8 hr - 100 yr

Hydrograph Type:	Inline SCS
• **Modflow Routing:**	**Not routed**
Repetitions:	1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	24.0
DCIA (%)	0.0
Curve Number	45
Design Rainfall Depth (inches)	7.5
Design Rainfall Duration (hours)	8.0
Shape Factor	UHG 323
Rainfall Distribution	FDOT 8 Hour

Initial ground water level (ft datum) 77.00 (default)

No times after storm specified.

Scenario 45 :: FDOT 24 Hour - 24 hr - 100 yr

Hydrograph Type:	Inline SCS
• **Modflow Routing:**	**Not routed**
Repetitions:	1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	24.0
DCIA (%)	0.0
Curve Number	45
Design Rainfall Depth (inches)	11.1
Design Rainfall Duration (hours)	24.0
Shape Factor	UHG 323
Rainfall Distribution	FDOT 24 Hour

Initial ground water level (ft datum) 77.00 (default)

No times after storm specified.

Scenario Input Data (cont'd.)

Scenario 46 :: FDOT 72 Hour - 72 hr - 100 yr

Hydrograph Type: Inline SCS
• **Modflow Routing: Not routed**
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	24.0
DCIA (%)	0.0
Curve Number	45
Design Rainfall Depth (inches)	13.8
Design Rainfall Duration (hours)	72.0
Shape Factor	UHG 323
Rainfall Distribution	FDOT 72 Hour

Initial ground water level (ft datum) 77.00 (default)

No times after storm specified.

Scenario 47 :: FDOT 168 Hour - 168 hr - 100 yr

Hydrograph Type: Inline SCS
• **Modflow Routing: Not routed**
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	24.0
DCIA (%)	0.0
Curve Number	45
Design Rainfall Depth (inches)	16.0
Design Rainfall Duration (hours)	168.0
Shape Factor	UHG 323
Rainfall Distribution	FDOT 168 Hour

Initial ground water level (ft datum) 77.00 (default)

No times after storm specified.

Scenario 48 :: FDOT 240 Hour - 240 hr - 100 yr

Hydrograph Type: Inline SCS
• **Modflow Routing: Not routed**
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	24.0
DCIA (%)	0.0
Curve Number	45
Design Rainfall Depth (inches)	18.0
Design Rainfall Duration (hours)	240.0
Shape Factor	UHG 323
Rainfall Distribution	FDOT 240 Hour

Initial ground water level (ft datum) 77.00 (default)

No times after storm specified.

Sort-By-Category Report

Scenarios Considered: 1 to 48

Discharge - Rate - Maximum Positive

Rank	Scenario Number	Maximum Positive Discharge Rate (ft^3/s)	Time (hours)	Description
1	44	0.513	4.160	FDOT 8 Hour - 8 hr - 100 yr
2	43	0.513	3.040	FDOT 4 Hour - 4 hr - 100 yr
3	36	0.422	4.160	FDOT 8 Hour - 8 hr - 50 yr
4	35	0.378	3.093	FDOT 4 Hour - 4 hr - 50 yr
5	48	0.362	184.000	FDOT 240 Hour - 240 hr - 100 yr
6	46	0.354	60.000	FDOT 72 Hour - 72 hr - 100 yr
7	42	0.327	1.227	FDOT 2 Hour - 2 hr - 100 yr
8	41	0.311	0.960	FDOT 1 Hour - 1 hr - 100 yr
9	45	0.309	12.107	FDOT 24 Hour - 24 hr - 100 yr
10	40	0.305	184.000	FDOT 240 Hour - 240 hr - 50 yr
11	28	0.289	4.160	FDOT 8 Hour - 8 hr - 25 yr
12	38	0.283	60.000	FDOT 72 Hour - 72 hr - 50 yr
13	47	0.281	160.000	FDOT 168 Hour - 168 hr - 100 yr
14	32	0.277	184.000	FDOT 240 Hour - 240 hr - 25 yr
15	27	0.277	3.093	FDOT 4 Hour - 4 hr - 25 yr
16	30	0.245	60.000	FDOT 72 Hour - 72 hr - 25 yr
17	39	0.233	160.000	FDOT 168 Hour - 168 hr - 50 yr
18	34	0.232	1.493	FDOT 2 Hour - 2 hr - 50 yr
19	37	0.219	12.107	FDOT 24 Hour - 24 hr - 50 yr
20	31	0.208	160.000	FDOT 168 Hour - 168 hr - 25 yr
21	24	0.208	184.000	FDOT 240 Hour - 240 hr - 10 yr
22	33	0.205	0.960	FDOT 1 Hour - 1 hr - 50 yr
23	20	0.191	4.160	FDOT 8 Hour - 8 hr - 10 yr
24	19	0.185	3.093	FDOT 4 Hour - 4 hr - 10 yr
25	16	0.168	184.000	FDOT 240 Hour - 240 hr - 5 yr
26	22	0.167	60.000	FDOT 72 Hour - 72 hr - 10 yr
27	8	0.163	184.000	FDOT 240 Hour - 240 hr - 3 yr
28	23	0.161	160.000	FDOT 168 Hour - 168 hr - 10 yr
29	29	0.160	15.093	FDOT 24 Hour - 24 hr - 25 yr
30	26	0.148	1.653	FDOT 2 Hour - 2 hr - 25 yr
31	14	0.135	60.000	FDOT 72 Hour - 72 hr - 5 yr
32	15	0.129	160.000	FDOT 168 Hour - 168 hr - 5 yr
33	7	0.126	160.000	FDOT 168 Hour - 168 hr - 3 yr
34	21	0.123	15.093	FDOT 24 Hour - 24 hr - 10 yr
35	6	0.121	60.000	FDOT 72 Hour - 72 hr - 3 yr
36	25	0.117	0.960	FDOT 1 Hour - 1 hr - 25 yr
37	11	0.116	3.147	FDOT 4 Hour - 4 hr - 5 yr
38	12	0.116	5.067	FDOT 8 Hour - 8 hr - 5 yr
39	18	0.089	1.760	FDOT 2 Hour - 2 hr - 10 yr
40	13	0.085	15.093	FDOT 24 Hour - 24 hr - 5 yr
41	4	0.077	5.120	FDOT 8 Hour - 8 hr - 3 yr
42	3	0.063	3.147	FDOT 4 Hour - 4 hr - 3 yr
43	5	0.060	15.093	FDOT 24 Hour - 24 hr - 3 yr
44	10	0.047	1.867	FDOT 2 Hour - 2 hr - 5 yr
45	17	0.046	0.960	FDOT 1 Hour - 1 hr - 10 yr
46	2	0.023	1.973	FDOT 2 Hour - 2 hr - 3 yr
47	9	0.015	1.013	FDOT 1 Hour - 1 hr - 5 yr
48	1	0.003	1.067	FDOT 1 Hour - 1 hr - 3 yr

Sort-By-Category Report (cont'd.)

Discharge - Cumulative Volume - Maximum Positive

Rank	Scenario Number	Maximum Positive Cumulative Discharge Volume (ft^3)	Time (hours)	Description
1	48	24739.5	242.027	FDOT 240 Hour - 240 hr - 100 yr
2	40	20244.5	242.080	FDOT 240 Hour - 240 hr - 50 yr
3	47	20244.5	170.080	FDOT 168 Hour - 168 hr - 100 yr
4	32	18068.7	242.027	FDOT 240 Hour - 240 hr - 25 yr
5	39	15948.8	170.080	FDOT 168 Hour - 168 hr - 50 yr
6	46	15532.1	74.027	FDOT 72 Hour - 72 hr - 100 yr
7	31	13892.1	170.027	FDOT 168 Hour - 168 hr - 25 yr
8	24	12890.1	242.080	FDOT 240 Hour - 240 hr - 10 yr
9	38	11907.4	74.027	FDOT 72 Hour - 72 hr - 50 yr
10	45	10172.5	26.080	FDOT 24 Hour - 24 hr - 100 yr
11	16	10005.0	242.080	FDOT 240 Hour - 240 hr - 5 yr
12	23	10005.0	170.080	FDOT 168 Hour - 168 hr - 10 yr
13	30	10005.0	74.080	FDOT 72 Hour - 72 hr - 25 yr
14	8	9635.4	242.027	FDOT 240 Hour - 240 hr - 3 yr
15	15	7504.1	170.080	FDOT 168 Hour - 168 hr - 5 yr
16	37	7504.1	26.080	FDOT 24 Hour - 24 hr - 50 yr
17	7	7333.7	170.080	FDOT 168 Hour - 168 hr - 3 yr
18	22	6336.6	74.080	FDOT 72 Hour - 72 hr - 10 yr
19	29	5729.2	26.080	FDOT 24 Hour - 24 hr - 25 yr
20	14	4930.5	74.027	FDOT 72 Hour - 72 hr - 5 yr
21	6	4343.8	74.027	FDOT 72 Hour - 72 hr - 3 yr
22	21	4286.6	26.080	FDOT 24 Hour - 24 hr - 10 yr
23	44	4172.8	10.080	FDOT 8 Hour - 8 hr - 100 yr
24	36	3512.9	10.080	FDOT 8 Hour - 8 hr - 50 yr
25	13	2906.9	26.080	FDOT 24 Hour - 24 hr - 5 yr
26	43	2894.4	6.080	FDOT 4 Hour - 4 hr - 100 yr
27	28	2554.3	10.080	FDOT 8 Hour - 8 hr - 25 yr
28	35	2052.8	6.080	FDOT 4 Hour - 4 hr - 50 yr
29	5	2009.3	26.080	FDOT 24 Hour - 24 hr - 3 yr
30	20	1839.0	10.080	FDOT 8 Hour - 8 hr - 10 yr
31	42	1634.5	4.107	FDOT 2 Hour - 2 hr - 100 yr
32	27	1439.8	6.080	FDOT 4 Hour - 4 hr - 25 yr
33	34	1166.7	4.107	FDOT 2 Hour - 2 hr - 50 yr
34	12	1081.0	10.080	FDOT 8 Hour - 8 hr - 5 yr
35	19	917.8	6.080	FDOT 4 Hour - 4 hr - 10 yr
36	41	729.9	3.093	FDOT 1 Hour - 1 hr - 100 yr
37	26	722.8	4.107	FDOT 2 Hour - 2 hr - 25 yr
38	4	708.6	10.080	FDOT 8 Hour - 8 hr - 3 yr
39	11	548.2	6.080	FDOT 4 Hour - 4 hr - 5 yr
40	33	468.9	3.093	FDOT 1 Hour - 1 hr - 50 yr
41	18	406.2	4.107	FDOT 2 Hour - 2 hr - 10 yr
42	3	283.5	6.080	FDOT 4 Hour - 4 hr - 3 yr
43	25	260.4	3.093	FDOT 1 Hour - 1 hr - 25 yr
44	10	196.8	4.107	FDOT 2 Hour - 2 hr - 5 yr
45	17	100.5	3.093	FDOT 1 Hour - 1 hr - 10 yr
46	2	83.8	4.107	FDOT 2 Hour - 2 hr - 3 yr
47	9	33.5	3.093	FDOT 1 Hour - 1 hr - 5 yr
48	1	5.6	3.093	FDOT 1 Hour - 1 hr - 3 yr

PONDS Version 3.3.0241
Retention Pond Recovery - Refined Method
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Devo Seereeram, Ph.D., P.E.

Project Data

Project Name:	QSR Alachua
Simulation Description:	POST-1 SRWMD and FDOT storms
Project Number:	20-0113
Engineer :	Liberty Baker
Supervising Engineer:	Daniel Young, P.E.
Date:	01-12-2021

Aquifer Data

Base Of Aquifer Elevation, [B] (ft datum):	77.00
Water Table Elevation, [WT] (ft datum):	77.00
Horizontal Saturated Hydraulic Conductivity, [Kh] (ft/day):	9.00
Fillable Porosity, [n] (%):	25.00
Unsaturated Vertical Infiltration Rate, [Iv] (ft/day):	6.0
Maximum Area For Unsaturated Infiltration, [Av] (ft^2):	7688.0

Geometry Data

Equivalent Pond Length, [L] (ft):	76.0
Equivalent Pond Width, [W] (ft):	73.0

Ground water mound is expected to intersect the pond bottom

Stage vs Area Data

Stage (ft datum)	Area (ft^2)
79.75	2454.0
80.00	2774.0
81.00	4146.0
82.00	5650.0
83.00	7260.0
84.00	8970.0

Discharge Structures

Discharge Structure #1 is active as orifice

Structure Parameters

Description: 3.25" orifice

Orifice elevation, (ft datum):	81.90
Orifice coefficient:	4.9
Orifice area, (ft²):	0.05761
Orifice exponent:	0.5

Tailwater

Description:	Top of Receiving Ditch
Flow Type:	Two directional, into/out of pond
Data Format:	Elapsed time
Units:	English

Elapsed Time (hrs)	Elevation (ft datum)
0	81
999	81

Discharge Structure #2 is inactive

Discharge Structure #3 is inactive

Scenario Input Data

Scenario 1 :: FDOT 1 Hour - 1 hr - 3 yr

Hydrograph Type: Inline SCS
Modflow Routing: Routed with infiltration
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	10.0
DCIA (%)	0.0
Curve Number	92
Design Rainfall Depth (inches)	2.6
Design Rainfall Duration (hours)	1.0
Shape Factor	UHG 484
Rainfall Distribution	FDOT 1 Hour

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.100	0.900	8.000	16.000	24.000
0.200	1.000	9.000	17.000	25.000
0.300	2.000	10.000	18.000	26.000
0.400	3.000	11.000	19.000	27.000
0.500	4.000	12.000	20.000	28.000
0.600	5.000	13.000	21.000	29.000
0.700	6.000	14.000	22.000	30.000
0.800	7.000	15.000	23.000	

Scenario 2 :: FDOT 2 Hour - 2 hr - 3 yr

Hydrograph Type: Inline SCS
Modflow Routing: Routed with infiltration
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	10.0
DCIA (%)	0.0
Curve Number	92
Design Rainfall Depth (inches)	3.1
Design Rainfall Duration (hours)	2.0
Shape Factor	UHG 484
Rainfall Distribution	FDOT 2 Hour

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.100	0.900	8.000	16.000	24.000
0.200	1.000	9.000	17.000	25.000
0.300	2.000	10.000	18.000	26.000
0.400	3.000	11.000	19.000	27.000
0.500	4.000	12.000	20.000	28.000
0.600	5.000	13.000	21.000	29.000
0.700	6.000	14.000	22.000	30.000
0.800	7.000	15.000	23.000	

Scenario Input Data (cont'd.)

Scenario 3 :: FDOT 4 Hour - 4 hr - 3 yr

Hydrograph Type:	Inline SCS
Modflow Routing:	Routed with infiltration
Repetitions:	1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	10.0
DCIA (%)	0.0
Curve Number	92
Design Rainfall Depth (inches)	3.6
Design Rainfall Duration (hours)	4.0
Shape Factor	UHG 484
Rainfall Distribution	FDOT 4 Hour

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.100	0.900	8.000	16.000	24.000
0.200	1.000	9.000	17.000	25.000
0.300	2.000	10.000	18.000	26.000
0.400	3.000	11.000	19.000	27.000
0.500	4.000	12.000	20.000	28.000
0.600	5.000	13.000	21.000	29.000
0.700	6.000	14.000	22.000	30.000
0.800	7.000	15.000	23.000	

Scenario 4 :: FDOT 8 Hour - 8 hr - 3 yr

Hydrograph Type:	Inline SCS
Modflow Routing:	Routed with infiltration
Repetitions:	1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	10.0
DCIA (%)	0.0
Curve Number	92
Design Rainfall Depth (inches)	4.3
Design Rainfall Duration (hours)	8.0
Shape Factor	UHG 484
Rainfall Distribution	FDOT 8 Hour

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.100	0.900	8.000	16.000	24.000
0.200	1.000	9.000	17.000	25.000
0.300	2.000	10.000	18.000	26.000
0.400	3.000	11.000	19.000	27.000
0.500	4.000	12.000	20.000	28.000
0.600	5.000	13.000	21.000	29.000
0.700	6.000	14.000	22.000	30.000
0.800	7.000	15.000	23.000	

Scenario Input Data (cont'd.)

Scenario 5 :: FDOT 24 Hour - 24 hr - 3 yr

Hydrograph Type: Inline SCS
Modflow Routing: Routed with infiltration
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	10.0
DCIA (%)	0.0
Curve Number	92
Design Rainfall Depth (inches)	5.8
Design Rainfall Duration (hours)	24.0
Shape Factor	UHG 484
Rainfall Distribution	FDOT 24 Hour

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.100	0.900	8.000	16.000	24.000
0.200	1.000	9.000	17.000	25.000
0.300	2.000	10.000	18.000	26.000
0.400	3.000	11.000	19.000	27.000
0.500	4.000	12.000	20.000	28.000
0.600	5.000	13.000	21.000	29.000
0.700	6.000	14.000	22.000	30.000
0.800	7.000	15.000	23.000	

Scenario 6 :: FDOT 72 Hour - 72 hr - 3 yr

Hydrograph Type: Inline SCS
Modflow Routing: Routed with infiltration
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	10.0
DCIA (%)	0.0
Curve Number	92
Design Rainfall Depth (inches)	7.6
Design Rainfall Duration (hours)	72.0
Shape Factor	UHG 484
Rainfall Distribution	FDOT 72 Hour

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.100	0.900	8.000	16.000	24.000
0.200	1.000	9.000	17.000	25.000
0.300	2.000	10.000	18.000	26.000
0.400	3.000	11.000	19.000	27.000
0.500	4.000	12.000	20.000	28.000
0.600	5.000	13.000	21.000	29.000
0.700	6.000	14.000	22.000	30.000
0.800	7.000	15.000	23.000	

Scenario Input Data (cont'd.)

Scenario 7 :: FDOT 168 Hour - 168 hr - 3 yr

Hydrograph Type: Inline SCS
Modflow Routing: Routed with infiltration
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	10.0
DCIA (%)	0.0
Curve Number	92
Design Rainfall Depth (inches)	9.5
Design Rainfall Duration (hours)	168.0
Shape Factor	UHG 484
Rainfall Distribution	FDOT 168 Hour

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.100	0.900	8.000	16.000	24.000
0.200	1.000	9.000	17.000	25.000
0.300	2.000	10.000	18.000	26.000
0.400	3.000	11.000	19.000	27.000
0.500	4.000	12.000	20.000	28.000
0.600	5.000	13.000	21.000	29.000
0.700	6.000	14.000	22.000	30.000
0.800	7.000	15.000	23.000	

Scenario 8 :: FDOT 240 Hour - 240 hr - 3 yr

Hydrograph Type: Inline SCS
Modflow Routing: Routed with infiltration
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	10.0
DCIA (%)	0.0
Curve Number	92
Design Rainfall Depth (inches)	10.8
Design Rainfall Duration (hours)	240.0
Shape Factor	UHG 484
Rainfall Distribution	FDOT 240 Hour

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.100	0.900	8.000	16.000	24.000
0.200	1.000	9.000	17.000	25.000
0.300	2.000	10.000	18.000	26.000
0.400	3.000	11.000	19.000	27.000
0.500	4.000	12.000	20.000	28.000
0.600	5.000	13.000	21.000	29.000
0.700	6.000	14.000	22.000	30.000
0.800	7.000	15.000	23.000	

Scenario Input Data (cont'd.)

Scenario 9 :: FDOT 1 Hour - 1 hr - 5 yr

Hydrograph Type: Inline SCS
Modflow Routing: Routed with infiltration
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	10.0
DCIA (%)	0.0
Curve Number	92
Design Rainfall Depth (inches)	2.8
Design Rainfall Duration (hours)	1.0
Shape Factor	UHG 484
Rainfall Distribution	FDOT 1 Hour

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.100	0.900	8.000	16.000	24.000
0.200	1.000	9.000	17.000	25.000
0.300	2.000	10.000	18.000	26.000
0.400	3.000	11.000	19.000	27.000
0.500	4.000	12.000	20.000	28.000
0.600	5.000	13.000	21.000	29.000
0.700	6.000	14.000	22.000	30.000
0.800	7.000	15.000	23.000	

Scenario 10 :: FDOT 2 Hour - 2 hr - 5 yr

Hydrograph Type: Inline SCS
Modflow Routing: Routed with infiltration
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	10.0
DCIA (%)	0.0
Curve Number	92
Design Rainfall Depth (inches)	3.4
Design Rainfall Duration (hours)	2.0
Shape Factor	UHG 484
Rainfall Distribution	FDOT 2 Hour

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.100	0.900	8.000	16.000	24.000
0.200	1.000	9.000	17.000	25.000
0.300	2.000	10.000	18.000	26.000
0.400	3.000	11.000	19.000	27.000
0.500	4.000	12.000	20.000	28.000
0.600	5.000	13.000	21.000	29.000
0.700	6.000	14.000	22.000	30.000
0.800	7.000	15.000	23.000	

Scenario Input Data (cont'd.)

Scenario 11 :: FDOT 4 Hour - 4 hr - 5 yr

Hydrograph Type: Inline SCS
Modflow Routing: Routed with infiltration
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	10.0
DCIA (%)	0.0
Curve Number	92
Design Rainfall Depth (inches)	4.1
Design Rainfall Duration (hours)	4.0
Shape Factor	UHG 484
Rainfall Distribution	FDOT 4 Hour

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.100	0.900	8.000	16.000	24.000
0.200	1.000	9.000	17.000	25.000
0.300	2.000	10.000	18.000	26.000
0.400	3.000	11.000	19.000	27.000
0.500	4.000	12.000	20.000	28.000
0.600	5.000	13.000	21.000	29.000
0.700	6.000	14.000	22.000	30.000
0.800	7.000	15.000	23.000	

Scenario 12 :: FDOT 8 Hour - 8 hr - 5 yr

Hydrograph Type: Inline SCS
Modflow Routing: Routed with infiltration
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	10.0
DCIA (%)	0.0
Curve Number	92
Design Rainfall Depth (inches)	4.8
Design Rainfall Duration (hours)	8.0
Shape Factor	UHG 484
Rainfall Distribution	FDOT 8 Hour

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.100	0.900	8.000	16.000	24.000
0.200	1.000	9.000	17.000	25.000
0.300	2.000	10.000	18.000	26.000
0.400	3.000	11.000	19.000	27.000
0.500	4.000	12.000	20.000	28.000
0.600	5.000	13.000	21.000	29.000
0.700	6.000	14.000	22.000	30.000
0.800	7.000	15.000	23.000	

Scenario Input Data (cont'd.)

Scenario 13 :: FDOT 24 Hour - 24 hr - 5 yr

Hydrograph Type: Inline SCS
Modflow Routing: Routed with infiltration
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	10.0
DCIA (%)	0.0
Curve Number	92
Design Rainfall Depth (inches)	6.5
Design Rainfall Duration (hours)	24.0
Shape Factor	UHG 484
Rainfall Distribution	FDOT 24 Hour

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.100	0.900	8.000	16.000	24.000
0.200	1.000	9.000	17.000	25.000
0.300	2.000	10.000	18.000	26.000
0.400	3.000	11.000	19.000	27.000
0.500	4.000	12.000	20.000	28.000
0.600	5.000	13.000	21.000	29.000
0.700	6.000	14.000	22.000	30.000
0.800	7.000	15.000	23.000	

Scenario 14 :: FDOT 72 Hour - 72 hr - 5 yr

Hydrograph Type: Inline SCS
Modflow Routing: Routed with infiltration
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	10.0
DCIA (%)	0.0
Curve Number	92
Design Rainfall Depth (inches)	8.0
Design Rainfall Duration (hours)	72.0
Shape Factor	UHG 484
Rainfall Distribution	FDOT 72 Hour

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.100	0.900	8.000	16.000	24.000
0.200	1.000	9.000	17.000	25.000
0.300	2.000	10.000	18.000	26.000
0.400	3.000	11.000	19.000	27.000
0.500	4.000	12.000	20.000	28.000
0.600	5.000	13.000	21.000	29.000
0.700	6.000	14.000	22.000	30.000
0.800	7.000	15.000	23.000	

Scenario Input Data (cont'd.)

Scenario 15 :: FDOT 168 Hour - 168 hr - 5 yr

Hydrograph Type:	Inline SCS
Modflow Routing:	Routed with infiltration
Repetitions:	1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	10.0
DCIA (%)	0.0
Curve Number	92
Design Rainfall Depth (inches)	9.6
Design Rainfall Duration (hours)	168.0
Shape Factor	UHG 484
Rainfall Distribution	FDOT 168 Hour

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.100	0.900	8.000	16.000	24.000
0.200	1.000	9.000	17.000	25.000
0.300	2.000	10.000	18.000	26.000
0.400	3.000	11.000	19.000	27.000
0.500	4.000	12.000	20.000	28.000
0.600	5.000	13.000	21.000	29.000
0.700	6.000	14.000	22.000	30.000
0.800	7.000	15.000	23.000	

Scenario 16 :: FDOT 240 Hour - 240 hr - 5 yr

Hydrograph Type:	Inline SCS
Modflow Routing:	Routed with infiltration
Repetitions:	1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	10.0
DCIA (%)	0.0
Curve Number	92
Design Rainfall Depth (inches)	11.0
Design Rainfall Duration (hours)	240.0
Shape Factor	UHG 484
Rainfall Distribution	FDOT 240 Hour

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.100	0.900	8.000	16.000	24.000
0.200	1.000	9.000	17.000	25.000
0.300	2.000	10.000	18.000	26.000
0.400	3.000	11.000	19.000	27.000
0.500	4.000	12.000	20.000	28.000
0.600	5.000	13.000	21.000	29.000
0.700	6.000	14.000	22.000	30.000
0.800	7.000	15.000	23.000	

Scenario Input Data (cont'd.)

Scenario 17 :: FDOT 1 Hour - 1 hr - 10 yr

Hydrograph Type: Inline SCS
Modflow Routing: Routed with infiltration
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	10.0
DCIA (%)	0.0
Curve Number	92
Design Rainfall Depth (inches)	3.1
Design Rainfall Duration (hours)	1.0
Shape Factor	UHG 484
Rainfall Distribution	FDOT 1 Hour

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.100	0.900	8.000	16.000	24.000
0.200	1.000	9.000	17.000	25.000
0.300	2.000	10.000	18.000	26.000
0.400	3.000	11.000	19.000	27.000
0.500	4.000	12.000	20.000	28.000
0.600	5.000	13.000	21.000	29.000
0.700	6.000	14.000	22.000	30.000
0.800	7.000	15.000	23.000	

Scenario 18 :: FDOT 2 Hour - 2 hr - 10 yr

Hydrograph Type: Inline SCS
Modflow Routing: Routed with infiltration
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	10.0
DCIA (%)	0.0
Curve Number	92
Design Rainfall Depth (inches)	3.8
Design Rainfall Duration (hours)	2.0
Shape Factor	UHG 484
Rainfall Distribution	FDOT 2 Hour

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.100	0.900	8.000	16.000	24.000
0.200	1.000	9.000	17.000	25.000
0.300	2.000	10.000	18.000	26.000
0.400	3.000	11.000	19.000	27.000
0.500	4.000	12.000	20.000	28.000
0.600	5.000	13.000	21.000	29.000
0.700	6.000	14.000	22.000	30.000
0.800	7.000	15.000	23.000	

Scenario Input Data (cont'd.)

Scenario 19 :: FDOT 4 Hour - 4 hr - 10 yr

Hydrograph Type: Inline SCS
Modflow Routing: Routed with infiltration
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	10.0
DCIA (%)	0.0
Curve Number	92
Design Rainfall Depth (inches)	4.6
Design Rainfall Duration (hours)	4.0
Shape Factor	UHG 484
Rainfall Distribution	FDOT 4 Hour

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.100	0.900	8.000	16.000	24.000
0.200	1.000	9.000	17.000	25.000
0.300	2.000	10.000	18.000	26.000
0.400	3.000	11.000	19.000	27.000
0.500	4.000	12.000	20.000	28.000
0.600	5.000	13.000	21.000	29.000
0.700	6.000	14.000	22.000	30.000
0.800	7.000	15.000	23.000	

Scenario 20 :: FDOT 8 Hour - 8 hr - 10 yr

Hydrograph Type: Inline SCS
Modflow Routing: Routed with infiltration
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	10.0
DCIA (%)	0.0
Curve Number	92
Design Rainfall Depth (inches)	5.6
Design Rainfall Duration (hours)	8.0
Shape Factor	UHG 484
Rainfall Distribution	FDOT 8 Hour

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.100	0.900	8.000	16.000	24.000
0.200	1.000	9.000	17.000	25.000
0.300	2.000	10.000	18.000	26.000
0.400	3.000	11.000	19.000	27.000
0.500	4.000	12.000	20.000	28.000
0.600	5.000	13.000	21.000	29.000
0.700	6.000	14.000	22.000	30.000
0.800	7.000	15.000	23.000	

Scenario Input Data (cont'd.)

Scenario 21 :: FDOT 24 Hour - 24 hr - 10 yr

Hydrograph Type: Inline SCS
Modflow Routing: Routed with infiltration
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	10.0
DCIA (%)	0.0
Curve Number	92
Design Rainfall Depth (inches)	7.6
Design Rainfall Duration (hours)	24.0
Shape Factor	UHG 484
Rainfall Distribution	FDOT 24 Hour

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.100	0.900	8.000	16.000	24.000
0.200	1.000	9.000	17.000	25.000
0.300	2.000	10.000	18.000	26.000
0.400	3.000	11.000	19.000	27.000
0.500	4.000	12.000	20.000	28.000
0.600	5.000	13.000	21.000	29.000
0.700	6.000	14.000	22.000	30.000
0.800	7.000	15.000	23.000	

Scenario 22 :: FDOT 72 Hour - 72 hr - 10 yr

Hydrograph Type: Inline SCS
Modflow Routing: Routed with infiltration
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	10.0
DCIA (%)	0.0
Curve Number	92
Design Rainfall Depth (inches)	8.9
Design Rainfall Duration (hours)	72.0
Shape Factor	UHG 484
Rainfall Distribution	FDOT 72 Hour

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.100	0.900	8.000	16.000	24.000
0.200	1.000	9.000	17.000	25.000
0.300	2.000	10.000	18.000	26.000
0.400	3.000	11.000	19.000	27.000
0.500	4.000	12.000	20.000	28.000
0.600	5.000	13.000	21.000	29.000
0.700	6.000	14.000	22.000	30.000
0.800	7.000	15.000	23.000	

Scenario Input Data (cont'd.)

Scenario 23 :: FDOT 168 Hour - 168 hr - 10 yr

Hydrograph Type:	Inline SCS
Modflow Routing:	Routed with infiltration
Repetitions:	1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	10.0
DCIA (%)	0.0
Curve Number	92
Design Rainfall Depth (inches)	11.0
Design Rainfall Duration (hours)	168.0
Shape Factor	UHG 484
Rainfall Distribution	FDOT 168 Hour

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.100	0.900	8.000	16.000	24.000
0.200	1.000	9.000	17.000	25.000
0.300	2.000	10.000	18.000	26.000
0.400	3.000	11.000	19.000	27.000
0.500	4.000	12.000	20.000	28.000
0.600	5.000	13.000	21.000	29.000
0.700	6.000	14.000	22.000	30.000
0.800	7.000	15.000	23.000	

Scenario 24 :: FDOT 240 Hour - 240 hr - 10 yr

Hydrograph Type:	Inline SCS
Modflow Routing:	Routed with infiltration
Repetitions:	1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	10.0
DCIA (%)	0.0
Curve Number	92
Design Rainfall Depth (inches)	12.5
Design Rainfall Duration (hours)	240.0
Shape Factor	UHG 484
Rainfall Distribution	FDOT 240 Hour

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.100	0.900	8.000	16.000	24.000
0.200	1.000	9.000	17.000	25.000
0.300	2.000	10.000	18.000	26.000
0.400	3.000	11.000	19.000	27.000
0.500	4.000	12.000	20.000	28.000
0.600	5.000	13.000	21.000	29.000
0.700	6.000	14.000	22.000	30.000
0.800	7.000	15.000	23.000	

Scenario Input Data (cont'd.)

Scenario 25 :: FDOT 1 Hour - 1 hr - 25 yr

Hydrograph Type: Inline SCS
Modflow Routing: Routed with infiltration
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	10.0
DCIA (%)	0.0
Curve Number	92
Design Rainfall Depth (inches)	3.6
Design Rainfall Duration (hours)	1.0
Shape Factor	UHG 484
Rainfall Distribution	FDOT 1 Hour

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.100	0.900	8.000	16.000	24.000
0.200	1.000	9.000	17.000	25.000
0.300	2.000	10.000	18.000	26.000
0.400	3.000	11.000	19.000	27.000
0.500	4.000	12.000	20.000	28.000
0.600	5.000	13.000	21.000	29.000
0.700	6.000	14.000	22.000	30.000
0.800	7.000	15.000	23.000	

Scenario 26 :: FDOT 2 Hour - 2 hr - 25 yr

Hydrograph Type: Inline SCS
Modflow Routing: Routed with infiltration
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	10.0
DCIA (%)	0.0
Curve Number	92
Design Rainfall Depth (inches)	4.3
Design Rainfall Duration (hours)	2.0
Shape Factor	UHG 484
Rainfall Distribution	FDOT 2 Hour

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.100	0.900	8.000	16.000	24.000
0.200	1.000	9.000	17.000	25.000
0.300	2.000	10.000	18.000	26.000
0.400	3.000	11.000	19.000	27.000
0.500	4.000	12.000	20.000	28.000
0.600	5.000	13.000	21.000	29.000
0.700	6.000	14.000	22.000	30.000
0.800	7.000	15.000	23.000	

Scenario Input Data (cont'd.)

Scenario 27 :: FDOT 4 Hour - 4 hr - 25 yr

Hydrograph Type: Inline SCS
Modflow Routing: Routed with infiltration
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	10.0
DCIA (%)	0.0
Curve Number	92
Design Rainfall Depth (inches)	5.2
Design Rainfall Duration (hours)	4.0
Shape Factor	UHG 484
Rainfall Distribution	FDOT 4 Hour

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.100	0.900	8.000	16.000	24.000
0.200	1.000	9.000	17.000	25.000
0.300	2.000	10.000	18.000	26.000
0.400	3.000	11.000	19.000	27.000
0.500	4.000	12.000	20.000	28.000
0.600	5.000	13.000	21.000	29.000
0.700	6.000	14.000	22.000	30.000
0.800	7.000	15.000	23.000	

Scenario 28 :: FDOT 8 Hour - 8 hr - 25 yr

Hydrograph Type: Inline SCS
Modflow Routing: Routed with infiltration
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	10.0
DCIA (%)	0.0
Curve Number	92
Design Rainfall Depth (inches)	6.2
Design Rainfall Duration (hours)	8.0
Shape Factor	UHG 484
Rainfall Distribution	FDOT 8 Hour

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.100	0.900	8.000	16.000	24.000
0.200	1.000	9.000	17.000	25.000
0.300	2.000	10.000	18.000	26.000
0.400	3.000	11.000	19.000	27.000
0.500	4.000	12.000	20.000	28.000
0.600	5.000	13.000	21.000	29.000
0.700	6.000	14.000	22.000	30.000
0.800	7.000	15.000	23.000	

Scenario Input Data (cont'd.)

Scenario 29 :: FDOT 24 Hour - 24 hr - 25 yr

Hydrograph Type: Inline SCS
Modflow Routing: Routed with infiltration
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	10.0
DCIA (%)	0.0
Curve Number	92
Design Rainfall Depth (inches)	8.5
Design Rainfall Duration (hours)	24.0
Shape Factor	UHG 484
Rainfall Distribution	FDOT 24 Hour

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.100	0.900	8.000	16.000	24.000
0.200	1.000	9.000	17.000	25.000
0.300	2.000	10.000	18.000	26.000
0.400	3.000	11.000	19.000	27.000
0.500	4.000	12.000	20.000	28.000
0.600	5.000	13.000	21.000	29.000
0.700	6.000	14.000	22.000	30.000
0.800	7.000	15.000	23.000	

Scenario 30 :: FDOT 72 Hour - 72 hr - 25 yr

Hydrograph Type: Inline SCS
Modflow Routing: Routed with infiltration
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	10.0
DCIA (%)	0.0
Curve Number	92
Design Rainfall Depth (inches)	11.0
Design Rainfall Duration (hours)	72.0
Shape Factor	UHG 484
Rainfall Distribution	FDOT 72 Hour

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.100	0.900	8.000	16.000	24.000
0.200	1.000	9.000	17.000	25.000
0.300	2.000	10.000	18.000	26.000
0.400	3.000	11.000	19.000	27.000
0.500	4.000	12.000	20.000	28.000
0.600	5.000	13.000	21.000	29.000
0.700	6.000	14.000	22.000	30.000
0.800	7.000	15.000	23.000	

Scenario Input Data (cont'd.)

Scenario 31 :: FDOT 168 Hour - 168 hr - 25 yr

Hydrograph Type:	Inline SCS
Modflow Routing:	Routed with infiltration
Repetitions:	1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	10.0
DCIA (%)	0.0
Curve Number	92
Design Rainfall Depth (inches)	13.0
Design Rainfall Duration (hours)	168.0
Shape Factor	UHG 484
Rainfall Distribution	FDOT 168 Hour

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.100	0.900	8.000	16.000	24.000
0.200	1.000	9.000	17.000	25.000
0.300	2.000	10.000	18.000	26.000
0.400	3.000	11.000	19.000	27.000
0.500	4.000	12.000	20.000	28.000
0.600	5.000	13.000	21.000	29.000
0.700	6.000	14.000	22.000	30.000
0.800	7.000	15.000	23.000	

Scenario 32 :: FDOT 240 Hour - 240 hr - 25 yr

Hydrograph Type:	Inline SCS
Modflow Routing:	Routed with infiltration
Repetitions:	1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	10.0
DCIA (%)	0.0
Curve Number	92
Design Rainfall Depth (inches)	15.0
Design Rainfall Duration (hours)	240.0
Shape Factor	UHG 484
Rainfall Distribution	FDOT 240 Hour

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.100	0.900	8.000	16.000	24.000
0.200	1.000	9.000	17.000	25.000
0.300	2.000	10.000	18.000	26.000
0.400	3.000	11.000	19.000	27.000
0.500	4.000	12.000	20.000	28.000
0.600	5.000	13.000	21.000	29.000
0.700	6.000	14.000	22.000	30.000
0.800	7.000	15.000	23.000	

Scenario Input Data (cont'd.)

Scenario 33 :: FDOT 1 Hour - 1 hr - 50 yr

Hydrograph Type: Inline SCS
Modflow Routing: Routed with infiltration
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	10.0
DCIA (%)	0.0
Curve Number	92
Design Rainfall Depth (inches)	4.0
Design Rainfall Duration (hours)	1.0
Shape Factor	UHG 484
Rainfall Distribution	FDOT 1 Hour

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.100	0.900	8.000	16.000	24.000
0.200	1.000	9.000	17.000	25.000
0.300	2.000	10.000	18.000	26.000
0.400	3.000	11.000	19.000	27.000
0.500	4.000	12.000	20.000	28.000
0.600	5.000	13.000	21.000	29.000
0.700	6.000	14.000	22.000	30.000
0.800	7.000	15.000	23.000	

Scenario 34 :: FDOT 2 Hour - 2 hr - 50 yr

Hydrograph Type: Inline SCS
Modflow Routing: Routed with infiltration
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	10.0
DCIA (%)	0.0
Curve Number	92
Design Rainfall Depth (inches)	4.9
Design Rainfall Duration (hours)	2.0
Shape Factor	UHG 484
Rainfall Distribution	FDOT 2 Hour

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.100	0.900	8.000	16.000	24.000
0.200	1.000	9.000	17.000	25.000
0.300	2.000	10.000	18.000	26.000
0.400	3.000	11.000	19.000	27.000
0.500	4.000	12.000	20.000	28.000
0.600	5.000	13.000	21.000	29.000
0.700	6.000	14.000	22.000	30.000
0.800	7.000	15.000	23.000	

Scenario Input Data (cont'd.)

Scenario 35 :: FDOT 4 Hour - 4 hr - 50 yr

Hydrograph Type: Inline SCS
Modflow Routing: Routed with infiltration
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	10.0
DCIA (%)	0.0
Curve Number	92
Design Rainfall Depth (inches)	5.8
Design Rainfall Duration (hours)	4.0
Shape Factor	UHG 484
Rainfall Distribution	FDOT 4 Hour

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.100	0.900	8.000	16.000	24.000
0.200	1.000	9.000	17.000	25.000
0.300	2.000	10.000	18.000	26.000
0.400	3.000	11.000	19.000	27.000
0.500	4.000	12.000	20.000	28.000
0.600	5.000	13.000	21.000	29.000
0.700	6.000	14.000	22.000	30.000
0.800	7.000	15.000	23.000	

Scenario 36 :: FDOT 8 Hour - 8 hr - 50 yr

Hydrograph Type: Inline SCS
Modflow Routing: Routed with infiltration
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	10.0
DCIA (%)	0.0
Curve Number	92
Design Rainfall Depth (inches)	7.0
Design Rainfall Duration (hours)	8.0
Shape Factor	UHG 484
Rainfall Distribution	FDOT 8 Hour

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.100	0.900	8.000	16.000	24.000
0.200	1.000	9.000	17.000	25.000
0.300	2.000	10.000	18.000	26.000
0.400	3.000	11.000	19.000	27.000
0.500	4.000	12.000	20.000	28.000
0.600	5.000	13.000	21.000	29.000
0.700	6.000	14.000	22.000	30.000
0.800	7.000	15.000	23.000	

Scenario Input Data (cont'd.)

Scenario 37 :: FDOT 24 Hour - 24 hr - 50 yr

Hydrograph Type: Inline SCS
Modflow Routing: Routed with infiltration
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	10.0
DCIA (%)	0.0
Curve Number	92
Design Rainfall Depth (inches)	9.6
Design Rainfall Duration (hours)	24.0
Shape Factor	UHG 484
Rainfall Distribution	FDOT 24 Hour

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.100	0.900	8.000	16.000	24.000
0.200	1.000	9.000	17.000	25.000
0.300	2.000	10.000	18.000	26.000
0.400	3.000	11.000	19.000	27.000
0.500	4.000	12.000	20.000	28.000
0.600	5.000	13.000	21.000	29.000
0.700	6.000	14.000	22.000	30.000
0.800	7.000	15.000	23.000	

Scenario 38 :: FDOT 72 Hour - 72 hr - 50 yr

Hydrograph Type: Inline SCS
Modflow Routing: Routed with infiltration
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	10.0
DCIA (%)	0.0
Curve Number	92
Design Rainfall Depth (inches)	12.0
Design Rainfall Duration (hours)	72.0
Shape Factor	UHG 484
Rainfall Distribution	FDOT 72 Hour

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.100	0.900	8.000	16.000	24.000
0.200	1.000	9.000	17.000	25.000
0.300	2.000	10.000	18.000	26.000
0.400	3.000	11.000	19.000	27.000
0.500	4.000	12.000	20.000	28.000
0.600	5.000	13.000	21.000	29.000
0.700	6.000	14.000	22.000	30.000
0.800	7.000	15.000	23.000	

Scenario Input Data (cont'd.)

Scenario 39 :: FDOT 168 Hour - 168 hr - 50 yr

Hydrograph Type:	Inline SCS
Modflow Routing:	Routed with infiltration
Repetitions:	1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	10.0
DCIA (%)	0.0
Curve Number	92
Design Rainfall Depth (inches)	14.0
Design Rainfall Duration (hours)	168.0
Shape Factor	UHG 484
Rainfall Distribution	FDOT 168 Hour

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.100	0.900	8.000	16.000	24.000
0.200	1.000	9.000	17.000	25.000
0.300	2.000	10.000	18.000	26.000
0.400	3.000	11.000	19.000	27.000
0.500	4.000	12.000	20.000	28.000
0.600	5.000	13.000	21.000	29.000
0.700	6.000	14.000	22.000	30.000
0.800	7.000	15.000	23.000	

Scenario 40 :: FDOT 240 Hour - 240 hr - 50 yr

Hydrograph Type:	Inline SCS
Modflow Routing:	Routed with infiltration
Repetitions:	1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	10.0
DCIA (%)	0.0
Curve Number	92
Design Rainfall Depth (inches)	16.0
Design Rainfall Duration (hours)	240.0
Shape Factor	UHG 484
Rainfall Distribution	FDOT 240 Hour

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.100	0.900	8.000	16.000	24.000
0.200	1.000	9.000	17.000	25.000
0.300	2.000	10.000	18.000	26.000
0.400	3.000	11.000	19.000	27.000
0.500	4.000	12.000	20.000	28.000
0.600	5.000	13.000	21.000	29.000
0.700	6.000	14.000	22.000	30.000
0.800	7.000	15.000	23.000	

Scenario Input Data (cont'd.)

Scenario 41 :: FDOT 1 Hour - 1 hr - 100 yr

Hydrograph Type: Inline SCS
Modflow Routing: Routed with infiltration
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	10.0
DCIA (%)	0.0
Curve Number	92
Design Rainfall Depth (inches)	4.4
Design Rainfall Duration (hours)	1.0
Shape Factor	UHG 484
Rainfall Distribution	FDOT 1 Hour

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.100	0.900	8.000	16.000	24.000
0.200	1.000	9.000	17.000	25.000
0.300	2.000	10.000	18.000	26.000
0.400	3.000	11.000	19.000	27.000
0.500	4.000	12.000	20.000	28.000
0.600	5.000	13.000	21.000	29.000
0.700	6.000	14.000	22.000	30.000
0.800	7.000	15.000	23.000	

Scenario 42 :: FDOT 2 Hour - 2 hr - 100 yr

Hydrograph Type: Inline SCS
Modflow Routing: Routed with infiltration
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	10.0
DCIA (%)	0.0
Curve Number	92
Design Rainfall Depth (inches)	5.4
Design Rainfall Duration (hours)	2.0
Shape Factor	UHG 484
Rainfall Distribution	FDOT 2 Hour

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.100	0.900	8.000	16.000	24.000
0.200	1.000	9.000	17.000	25.000
0.300	2.000	10.000	18.000	26.000
0.400	3.000	11.000	19.000	27.000
0.500	4.000	12.000	20.000	28.000
0.600	5.000	13.000	21.000	29.000
0.700	6.000	14.000	22.000	30.000
0.800	7.000	15.000	23.000	

Scenario Input Data (cont'd.)

Scenario 43 :: FDOT 4 Hour - 4 hr - 100 yr

Hydrograph Type: Inline SCS
Modflow Routing: Routed with infiltration
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	10.0
DCIA (%)	0.0
Curve Number	92
Design Rainfall Depth (inches)	6.5
Design Rainfall Duration (hours)	4.0
Shape Factor	UHG 484
Rainfall Distribution	FDOT 4 Hour

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.100	0.900	8.000	16.000	24.000
0.200	1.000	9.000	17.000	25.000
0.300	2.000	10.000	18.000	26.000
0.400	3.000	11.000	19.000	27.000
0.500	4.000	12.000	20.000	28.000
0.600	5.000	13.000	21.000	29.000
0.700	6.000	14.000	22.000	30.000
0.800	7.000	15.000	23.000	

Scenario 44 :: FDOT 8 Hour - 8 hr - 100 yr

Hydrograph Type: Inline SCS
Modflow Routing: Routed with infiltration
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	10.0
DCIA (%)	0.0
Curve Number	92
Design Rainfall Depth (inches)	7.5
Design Rainfall Duration (hours)	8.0
Shape Factor	UHG 484
Rainfall Distribution	FDOT 8 Hour

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.100	0.900	8.000	16.000	24.000
0.200	1.000	9.000	17.000	25.000
0.300	2.000	10.000	18.000	26.000
0.400	3.000	11.000	19.000	27.000
0.500	4.000	12.000	20.000	28.000
0.600	5.000	13.000	21.000	29.000
0.700	6.000	14.000	22.000	30.000
0.800	7.000	15.000	23.000	

Scenario Input Data (cont'd.)

Scenario 45 :: FDOT 24 Hour - 24 hr - 100 yr

Hydrograph Type: Inline SCS
Modflow Routing: Routed with infiltration
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	10.0
DCIA (%)	0.0
Curve Number	92
Design Rainfall Depth (inches)	11.1
Design Rainfall Duration (hours)	24.0
Shape Factor	UHG 484
Rainfall Distribution	FDOT 24 Hour

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.100	0.900	8.000	16.000	24.000
0.200	1.000	9.000	17.000	25.000
0.300	2.000	10.000	18.000	26.000
0.400	3.000	11.000	19.000	27.000
0.500	4.000	12.000	20.000	28.000
0.600	5.000	13.000	21.000	29.000
0.700	6.000	14.000	22.000	30.000
0.800	7.000	15.000	23.000	

Scenario 46 :: FDOT 72 Hour - 72 hr - 100 yr

Hydrograph Type: Inline SCS
Modflow Routing: Routed with infiltration
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	10.0
DCIA (%)	0.0
Curve Number	92
Design Rainfall Depth (inches)	13.8
Design Rainfall Duration (hours)	72.0
Shape Factor	UHG 484
Rainfall Distribution	FDOT 72 Hour

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.100	0.900	8.000	16.000	24.000
0.200	1.000	9.000	17.000	25.000
0.300	2.000	10.000	18.000	26.000
0.400	3.000	11.000	19.000	27.000
0.500	4.000	12.000	20.000	28.000
0.600	5.000	13.000	21.000	29.000
0.700	6.000	14.000	22.000	30.000
0.800	7.000	15.000	23.000	

Scenario Input Data (cont'd.)

Scenario 47 :: FDOT 168 Hour - 168 hr - 100 yr

Hydrograph Type: Inline SCS
Modflow Routing: Routed with infiltration
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	10.0
DCIA (%)	0.0
Curve Number	92
Design Rainfall Depth (inches)	16.0
Design Rainfall Duration (hours)	168.0
Shape Factor	UHG 484
Rainfall Distribution	FDOT 168 Hour

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.100	0.900	8.000	16.000	24.000
0.200	1.000	9.000	17.000	25.000
0.300	2.000	10.000	18.000	26.000
0.400	3.000	11.000	19.000	27.000
0.500	4.000	12.000	20.000	28.000
0.600	5.000	13.000	21.000	29.000
0.700	6.000	14.000	22.000	30.000
0.800	7.000	15.000	23.000	

Scenario 48 :: FDOT 240 Hour - 240 hr - 100 yr

Hydrograph Type: Inline SCS
Modflow Routing: Routed with infiltration
Repetitions: 1

Basin Area (acres)	0.780
Time Of Concentration (minutes)	10.0
DCIA (%)	0.0
Curve Number	92
Design Rainfall Depth (inches)	18.0
Design Rainfall Duration (hours)	240.0
Shape Factor	UHG 484
Rainfall Distribution	FDOT 240 Hour

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)	Time After Storm Event (days)
0.100	0.900	8.000	16.000	24.000
0.200	1.000	9.000	17.000	25.000
0.300	2.000	10.000	18.000	26.000
0.400	3.000	11.000	19.000	27.000
0.500	4.000	12.000	20.000	28.000
0.600	5.000	13.000	21.000	29.000
0.700	6.000	14.000	22.000	30.000
0.800	7.000	15.000	23.000	

Scenario Input Data (cont'd.)

Scenario 49 :: WQTV

Hydrograph Type: Slug Load
Modflow Routing: Routed with infiltration

Treatment Volume (ft^3) 4971

Initial ground water level (ft datum) 77.00 (default)

Time After Storm Event (days)	Time After Storm Event (days)
0.100	2.000
0.250	2.500
0.500	3.000
1.000	3.500
1.500	4.000

Sort-By-Category Report

Scenarios Considered: 1 to 49

Stage - Maximum

Rank	Scenario Number	Maximum Stage (ft datum)	Time (hours)	Description
1	48	82.945	184.111	FDOT 240 Hour - 240 hr - 100 yr
2	46	82.714	64.044	FDOT 72 Hour - 72 hr - 100 yr
3	40	82.697	184.111	FDOT 240 Hour - 240 hr - 50 yr
4	47	82.655	160.089	FDOT 168 Hour - 168 hr - 100 yr
5	32	82.573	184.111	FDOT 240 Hour - 240 hr - 25 yr
6	45	82.526	20.156	FDOT 24 Hour - 24 hr - 100 yr
7	39	82.517	160.089	FDOT 168 Hour - 168 hr - 50 yr
8	38	82.500	64.044	FDOT 72 Hour - 72 hr - 50 yr
9	31	82.445	160.089	FDOT 168 Hour - 168 hr - 25 yr
10	43	82.383	3.200	FDOT 4 Hour - 4 hr - 100 yr
11	30	82.347	64.044	FDOT 72 Hour - 72 hr - 25 yr
12	44	82.308	5.133	FDOT 8 Hour - 8 hr - 100 yr
13	37	82.272	21.089	FDOT 24 Hour - 24 hr - 50 yr
14	24	82.261	184.111	FDOT 240 Hour - 240 hr - 10 yr
15	23	82.231	160.089	FDOT 168 Hour - 168 hr - 10 yr
16	36	82.173	5.133	FDOT 8 Hour - 8 hr - 50 yr
17	42	82.153	1.911	FDOT 2 Hour - 2 hr - 100 yr
18	35	82.142	3.222	FDOT 4 Hour - 4 hr - 50 yr
19	29	82.106	21.111	FDOT 24 Hour - 24 hr - 25 yr
20	15	82.057	160.111	FDOT 168 Hour - 168 hr - 5 yr
21	16	82.055	184.133	FDOT 240 Hour - 240 hr - 5 yr
22	22	82.048	64.111	FDOT 72 Hour - 72 hr - 10 yr
23	7	82.042	160.111	FDOT 168 Hour - 168 hr - 3 yr
24	8	82.024	184.156	FDOT 240 Hour - 240 hr - 3 yr
25	34	81.978	1.933	FDOT 2 Hour - 2 hr - 50 yr
26	28	81.964	5.133	FDOT 8 Hour - 8 hr - 25 yr
27	41	81.957	1.022	FDOT 1 Hour - 1 hr - 100 yr
28	21	81.951	22.089	FDOT 24 Hour - 24 hr - 10 yr
29	27	81.932	3.267	FDOT 4 Hour - 4 hr - 25 yr
30	14	81.909	68.044	FDOT 72 Hour - 72 hr - 5 yr
31	6	81.774	68.178	FDOT 72 Hour - 72 hr - 3 yr
32	33	81.771	1.044	FDOT 1 Hour - 1 hr - 50 yr
33	26	81.744	1.978	FDOT 2 Hour - 2 hr - 25 yr
34	20	81.737	5.178	FDOT 8 Hour - 8 hr - 10 yr
35	19	81.680	3.356	FDOT 4 Hour - 4 hr - 10 yr
36	13	81.588	24.133	FDOT 24 Hour - 24 hr - 5 yr
37	25	81.570	1.044	FDOT 1 Hour - 1 hr - 25 yr
38	18	81.504	1.956	FDOT 2 Hour - 2 hr - 10 yr
39	11	81.442	3.289	FDOT 4 Hour - 4 hr - 5 yr
40	12	81.420	5.156	FDOT 8 Hour - 8 hr - 5 yr
41	17	81.343	1.022	FDOT 1 Hour - 1 hr - 10 yr
42	10	81.281	1.933	FDOT 2 Hour - 2 hr - 5 yr
43	4	81.219	5.133	FDOT 8 Hour - 8 hr - 3 yr
44	3	81.211	3.244	FDOT 4 Hour - 4 hr - 3 yr
45	5	81.208	22.311	FDOT 24 Hour - 24 hr - 3 yr
46	49	81.199	0.002	WQTV
47	9	81.182	1.022	FDOT 1 Hour - 1 hr - 5 yr
48	2	81.100	1.911	FDOT 2 Hour - 2 hr - 3 yr
49	1	81.050	1.022	FDOT 1 Hour - 1 hr - 3 yr

Sort-By-Category Report (cont'd.)

Discharge - Rate - Maximum Positive

Rank	Scenario Number	Maximum Positive Discharge Rate (ft^3/s)	Time (hours)	Description
1	48	0.289	184.111	FDOT 240 Hour - 240 hr - 100 yr
2	46	0.255	64.044	FDOT 72 Hour - 72 hr - 100 yr
3	40	0.252	184.111	FDOT 240 Hour - 240 hr - 50 yr
4	47	0.245	160.089	FDOT 168 Hour - 168 hr - 100 yr
5	32	0.232	184.111	FDOT 240 Hour - 240 hr - 25 yr
6	45	0.223	20.156	FDOT 24 Hour - 24 hr - 100 yr
7	39	0.222	160.089	FDOT 168 Hour - 168 hr - 50 yr
8	38	0.219	64.044	FDOT 72 Hour - 72 hr - 50 yr
9	31	0.208	160.089	FDOT 168 Hour - 168 hr - 25 yr
10	43	0.196	3.200	FDOT 4 Hour - 4 hr - 100 yr
11	30	0.189	64.044	FDOT 72 Hour - 72 hr - 25 yr
12	44	0.180	5.133	FDOT 8 Hour - 8 hr - 100 yr
13	37	0.172	21.089	FDOT 24 Hour - 24 hr - 50 yr
14	24	0.170	184.111	FDOT 240 Hour - 240 hr - 10 yr
15	23	0.162	160.089	FDOT 168 Hour - 168 hr - 10 yr
16	36	0.148	5.133	FDOT 8 Hour - 8 hr - 50 yr
17	42	0.142	1.911	FDOT 2 Hour - 2 hr - 100 yr
18	35	0.139	3.222	FDOT 4 Hour - 4 hr - 50 yr
19	29	0.128	21.111	FDOT 24 Hour - 24 hr - 25 yr
20	15	0.112	160.111	FDOT 168 Hour - 168 hr - 5 yr
21	16	0.111	184.133	FDOT 240 Hour - 240 hr - 5 yr
22	22	0.109	64.111	FDOT 72 Hour - 72 hr - 10 yr
23	7	0.107	160.111	FDOT 168 Hour - 168 hr - 3 yr
24	8	0.099	184.156	FDOT 240 Hour - 240 hr - 3 yr
25	34	0.079	1.933	FDOT 2 Hour - 2 hr - 50 yr
26	28	0.071	5.133	FDOT 8 Hour - 8 hr - 25 yr
27	41	0.067	1.022	FDOT 1 Hour - 1 hr - 100 yr
28	21	0.064	22.089	FDOT 24 Hour - 24 hr - 10 yr
29	27	0.050	3.267	FDOT 4 Hour - 4 hr - 25 yr
30	14	0.026	68.044	FDOT 72 Hour - 72 hr - 5 yr
31	1	None	N.A.	FDOT 1 Hour - 1 hr - 3 yr
32	2	None	N.A.	FDOT 2 Hour - 2 hr - 3 yr
33	3	None	N.A.	FDOT 4 Hour - 4 hr - 3 yr
34	4	None	N.A.	FDOT 8 Hour - 8 hr - 3 yr
35	5	None	N.A.	FDOT 24 Hour - 24 hr - 3 yr
36	6	None	N.A.	FDOT 72 Hour - 72 hr - 3 yr
37	9	None	N.A.	FDOT 1 Hour - 1 hr - 5 yr
38	10	None	N.A.	FDOT 2 Hour - 2 hr - 5 yr
39	11	None	N.A.	FDOT 4 Hour - 4 hr - 5 yr
40	12	None	N.A.	FDOT 8 Hour - 8 hr - 5 yr
41	13	None	N.A.	FDOT 24 Hour - 24 hr - 5 yr
42	17	None	N.A.	FDOT 1 Hour - 1 hr - 10 yr
43	18	None	N.A.	FDOT 2 Hour - 2 hr - 10 yr
44	19	None	N.A.	FDOT 4 Hour - 4 hr - 10 yr
45	20	None	N.A.	FDOT 8 Hour - 8 hr - 10 yr
46	25	None	N.A.	FDOT 1 Hour - 1 hr - 25 yr
47	26	None	N.A.	FDOT 2 Hour - 2 hr - 25 yr
48	33	None	N.A.	FDOT 1 Hour - 1 hr - 50 yr
49	49	None	N.A.	WQTV

Sort-By-Category Report (cont'd.)

Discharge - Cumulative Volume - Maximum Positive

Rank	Scenario Number	Maximum Positive Cumulative Discharge Volume (ft^3)	Time (hours)	Description
1	48	19852.0	240.289	FDOT 240 Hour - 240 hr - 100 yr
2	47	16856.7	175.778	FDOT 168 Hour - 168 hr - 100 yr
3	40	14997.5	240.222	FDOT 240 Hour - 240 hr - 50 yr
4	46	14648.2	79.778	FDOT 72 Hour - 72 hr - 100 yr
5	32	12582.4	240.178	FDOT 240 Hour - 240 hr - 25 yr
6	39	11946.3	173.378	FDOT 168 Hour - 168 hr - 50 yr
7	38	10134.2	77.378	FDOT 72 Hour - 72 hr - 50 yr
8	45	10071.9	31.778	FDOT 24 Hour - 24 hr - 100 yr
9	31	9632.1	173.378	FDOT 168 Hour - 168 hr - 25 yr
10	30	7629.3	74.978	FDOT 72 Hour - 72 hr - 25 yr
11	24	6625.5	240.111	FDOT 240 Hour - 240 hr - 10 yr
12	37	6175.7	29.378	FDOT 24 Hour - 24 hr - 50 yr
13	23	5136.0	170.978	FDOT 168 Hour - 168 hr - 10 yr
14	16	3340.0	216.600	FDOT 240 Hour - 240 hr - 5 yr
15	29	3299.1	26.978	FDOT 24 Hour - 24 hr - 25 yr
16	8	2911.0	216.556	FDOT 240 Hour - 240 hr - 3 yr
17	22	2368.4	72.133	FDOT 72 Hour - 72 hr - 10 yr
18	44	2335.9	10.978	FDOT 8 Hour - 8 hr - 100 yr
19	15	2074.7	168.356	FDOT 168 Hour - 168 hr - 5 yr
20	7	1862.9	168.333	FDOT 168 Hour - 168 hr - 3 yr
21	43	1294.8	4.578	FDOT 4 Hour - 4 hr - 100 yr
22	36	1203.0	7.289	FDOT 8 Hour - 8 hr - 50 yr
23	42	1060.5	4.978	FDOT 2 Hour - 2 hr - 100 yr
24	21	865.6	24.467	FDOT 24 Hour - 24 hr - 10 yr
25	35	720.0	4.533	FDOT 4 Hour - 4 hr - 50 yr
26	34	206.0	2.400	FDOT 2 Hour - 2 hr - 50 yr
27	28	195.8	5.911	FDOT 8 Hour - 8 hr - 25 yr
28	14	178.0	68.911	FDOT 72 Hour - 72 hr - 5 yr
29	27	110.3	3.800	FDOT 4 Hour - 4 hr - 25 yr
30	41	77.9	1.333	FDOT 1 Hour - 1 hr - 100 yr
31	1	None	N.A.	FDOT 1 Hour - 1 hr - 3 yr
32	2	None	N.A.	FDOT 2 Hour - 2 hr - 3 yr
33	3	None	N.A.	FDOT 4 Hour - 4 hr - 3 yr
34	4	None	N.A.	FDOT 8 Hour - 8 hr - 3 yr
35	5	None	N.A.	FDOT 24 Hour - 24 hr - 3 yr
36	6	None	N.A.	FDOT 72 Hour - 72 hr - 3 yr
37	9	None	N.A.	FDOT 1 Hour - 1 hr - 5 yr
38	10	None	N.A.	FDOT 2 Hour - 2 hr - 5 yr
39	11	None	N.A.	FDOT 4 Hour - 4 hr - 5 yr
40	12	None	N.A.	FDOT 8 Hour - 8 hr - 5 yr
41	13	None	N.A.	FDOT 24 Hour - 24 hr - 5 yr
42	17	None	N.A.	FDOT 1 Hour - 1 hr - 10 yr
43	18	None	N.A.	FDOT 2 Hour - 2 hr - 10 yr
44	19	None	N.A.	FDOT 4 Hour - 4 hr - 10 yr
45	20	None	N.A.	FDOT 8 Hour - 8 hr - 10 yr
46	25	None	N.A.	FDOT 1 Hour - 1 hr - 25 yr
47	26	None	N.A.	FDOT 2 Hour - 2 hr - 25 yr
48	33	None	N.A.	FDOT 1 Hour - 1 hr - 50 yr
49	49	None	N.A.	WQTV

Detailed Results :: *Scenario 1 :: FDOT 1 Hour - 1 hr - 3 yr*

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
0.000	0.0000	0.0000	77.000	0.00000	0.00000	0.0	0.0	0.0	N.A.
0.022	0.0000	0.0000	77.000	0.00000	0.00000	0.0	0.0	0.0	U
0.044	0.0000	0.0000	77.000	0.00000	0.00000	0.0	0.0	0.0	U
0.067	0.0000	0.0000	77.000	0.00000	0.00000	0.0	0.0	0.0	U
0.089	0.0000	0.0000	77.000	0.00000	0.00000	0.0	0.0	0.0	U
0.111	0.0000	0.0000	77.000	0.00000	0.00000	0.0	0.0	0.0	U
0.133	0.0000	0.0000	77.000	0.00000	0.00000	0.0	0.0	0.0	U
0.156	0.0000	0.0000	77.000	0.00000	0.00000	0.0	0.0	0.0	U
0.178	0.0000	0.0000	77.000	0.00017	0.00000	0.0	0.0	0.0	U
0.200	0.0007	0.0000	77.000	0.00216	0.00000	0.0	0.0	0.0	U
0.222	0.0073	0.0000	77.000	0.01127	0.00000	0.3	0.3	0.0	U
0.244	0.0298	0.0000	77.001	0.03728	0.00000	1.8	1.8	0.0	U
0.267	0.0822	0.0000	77.003	0.09150	0.00000	6.3	6.3	0.0	U
0.289	0.1718	0.0000	77.009	0.14905	0.00000	16.5	16.5	0.0	U
0.311	0.3030	0.0000	79.752	0.17142	0.00000	35.5	30.2	0.0	U/P
0.333	0.4841	0.0000	79.759	0.17237	0.00000	66.9	43.9	0.0	U/P
0.356	0.7271	0.0000	79.773	0.17399	0.00000	115.4	57.7	0.0	U/P
0.378	1.0325	0.0000	79.796	0.17643	0.00000	185.8	71.7	0.0	U/P
0.400	1.3771	0.0000	79.828	0.17977	0.00000	282.2	86.0	0.0	U/P
0.422	1.7316	0.0000	79.871	0.18397	0.00000	406.5	100.5	0.0	U/P
0.444	2.0736	0.0000	79.923	0.18895	0.00000	558.7	115.4	0.0	U/P
0.467	2.3859	0.0000	79.983	0.19475	0.00000	737.1	130.7	0.0	U/P
0.489	2.6587	0.0000	80.049	0.20127	0.00000	938.9	146.6	0.0	U/P
0.511	2.8886	0.0000	80.121	0.20818	0.00000	1160.8	162.9	0.0	U/P
0.533	3.0703	0.0000	80.195	0.21530	0.00000	1399.1	179.9	0.0	U/P
0.556	3.1952	0.0000	80.270	0.22247	0.00000	1649.7	197.4	0.0	U/P
0.578	3.2627	0.0000	80.345	0.22957	0.00000	1908.1	215.5	0.0	U/P
0.600	3.2900	0.0000	80.419	0.23650	0.00000	2170.2	234.1	0.0	U/P
0.622	3.2772	0.0000	80.491	0.24318	0.00000	2432.9	253.3	0.0	U/P
0.644	3.2165	0.0000	80.559	0.24954	0.00000	2692.6	273.0	0.0	U/P
0.667	3.0905	0.0000	80.624	0.25547	0.00000	2944.9	293.2	0.0	U/P
0.689	2.9177	0.0000	80.684	0.26093	0.00000	3185.2	313.9	0.0	U/P
0.711	2.7283	0.0000	80.738	0.26588	0.00000	3411.1	335.0	0.0	U/P
0.733	2.5349	0.0000	80.788	0.27036	0.00000	3621.6	356.4	0.0	U/P
0.756	2.3381	0.0000	80.833	0.27439	0.00000	3816.5	378.2	0.0	U/P
0.778	2.1481	0.0000	80.872	0.27799	0.00000	3996.0	400.3	0.0	U/P
0.800	1.9707	0.0000	80.908	0.28119	0.00000	4160.7	422.7	0.0	U/P
0.822	1.7965	0.0000	80.940	0.28403	0.00000	4311.4	445.3	0.0	U/P
0.844	1.6117	0.0000	80.968	0.28648	0.00000	4447.7	468.2	0.0	U/P
0.867	1.3986	0.0000	80.991	0.28859	0.00000	4568.1	491.2	0.0	U/P
0.889	1.1683	0.0000	81.010	0.29035	0.00000	4670.8	514.3	0.0	U/P
0.911	0.9462	0.0000	81.025	0.29167	0.00000	4755.4	537.6	0.0	U/P
0.933	0.7479	0.0000	81.036	0.29259	0.00000	4823.2	561.0	0.0	U/P
0.956	0.5810	0.0000	81.043	0.29318	0.00000	4876.3	584.4	0.0	U/P
0.978	0.4548	0.0000	81.047	0.29351	0.00000	4917.7	607.9	0.0	U/P
1.000	0.3604	0.0000	81.049	0.29366	0.00000	4950.4	631.4	0.0	U/P
1.022	0.2875	0.0000	81.050	0.29365	0.00000	4976.3	654.9	0.0	U/P
1.044	0.2284	0.0000	81.049	0.29353	0.00000	4996.9	678.4	0.0	U/P
1.067	0.1790	0.0000	81.047	0.29331	0.00000	5013.2	701.9	0.0	U/P
1.089	0.1369	0.0000	81.045	0.29300	0.00000	5025.8	725.3	0.0	U/P
1.111	0.1021	0.0000	81.041	0.29262	0.00000	5035.4	748.7	0.0	U/P
1.133	0.0742	0.0000	81.038	0.29219	0.00000	5042.5	772.1	0.0	U/P
1.156	0.0527	0.0000	81.033	0.29172	0.00000	5047.5	795.5	0.0	U/P
1.178	0.0371	0.0000	81.029	0.29122	0.00000	5051.1	818.8	0.0	U/P
1.200	0.0261	0.0000	81.024	0.29069	0.00000	5053.6	842.1	0.0	U/P
1.222	0.0183	0.0000	81.018	0.29015	0.00000	5055.4	865.3	0.0	U/P
1.244	0.0127	0.0000	81.013	0.28959	0.00000	5056.7	888.5	0.0	U/P
1.267	0.0086	0.0000	81.008	0.28903	0.00000	5057.5	911.7	0.0	U/P
1.289	0.0057	0.0000	81.002	0.28846	0.00000	5058.1	934.8	0.0	U/P
1.311	0.0036	0.0000	80.997	0.28790	0.00000	5058.4	957.8	0.0	U/P
1.333	0.0023	0.0000	80.991	0.28736	0.00000	5058.7	980.8	0.0	U/P
1.356	0.0016	0.0000	80.986	0.28684	0.00000	5058.8	1003.8	0.0	U/P
1.378	0.0011	0.0000	80.980	0.28631	0.00000	5059.0	1026.7	0.0	U/P
1.400	0.0007	0.0000	80.975	0.28578	0.00000	5059.0	1049.6	0.0	U/P
1.422	0.0005	0.0000	80.969	0.28525	0.00000	5059.1	1072.4	0.0	U/P
1.444	0.0003	0.0000	80.964	0.28472	0.00000	5059.1	1095.2	0.0	U/P
1.467	0.0002	0.0000	80.958	0.28419	0.00000	5059.1	1118.0	0.0	U/P
1.489	0.0001	0.0000	80.953	0.28366	0.00000	5059.1	1140.7	0.0	U/P
1.511	0.0000	0.0000	80.947	0.28313	0.00000	5059.1	1163.4	0.0	U/P
1.533	0.0000	0.0000	80.941	0.28260	0.00000	5059.1	1186.0	0.0	U/P
1.556	0.0000	0.0000	80.936	0.28207	0.00000	5059.1	1208.6	0.0	U/P
1.578	0.0000	0.0000	80.930	0.28155	0.00000	5059.1	1231.1	0.0	U/P
3.978	0.0000	0.0000	80.328	0.12686	0.00000	5059.1	3423.3	0.0	U/P
6.378	0.0000	0.0000	----	----	----	5059.1	5059.1	0.0	dry

Detailed Results (cont,d.) :: *Scenario 2 :: FDOT 2 Hour - 2 hr - 3 yr*

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
1.644	0.4548	0.0000	81.084	0.29737	0.00000	5727.6	1262.6	0.0	U/P
1.667	0.4412	0.0000	81.086	0.29765	0.00000	5763.5	1286.4	0.0	U/P
1.689	0.4237	0.0000	81.089	0.29789	0.00000	5798.1	1310.3	0.0	U/P
1.711	0.4055	0.0000	81.091	0.29810	0.00000	5831.2	1334.1	0.0	U/P
1.733	0.3889	0.0000	81.093	0.29828	0.00000	5863.0	1358.0	0.0	U/P
1.756	0.3751	0.0000	81.094	0.29843	0.00000	5893.6	1381.8	0.0	U/P
1.778	0.3652	0.0000	81.096	0.29856	0.00000	5923.2	1405.7	0.0	U/P
1.800	0.3584	0.0000	81.097	0.29868	0.00000	5952.1	1429.6	0.0	U/P
1.822	0.3518	0.0000	81.098	0.29878	0.00000	5980.5	1453.5	0.0	U/P
1.844	0.3429	0.0000	81.099	0.29886	0.00000	6008.3	1477.4	0.0	U/P
1.867	0.3292	0.0000	81.100	0.29892	0.00000	6035.2	1501.3	0.0	U/P
1.889	0.3115	0.0000	81.100	0.29894	0.00000	6060.8	1525.2	0.0	U/P
1.911	0.2931	0.0000	81.100	0.29893	0.00000	6085.0	1549.2	0.0	U/P
1.933	0.2764	0.0000	81.100	0.29889	0.00000	6107.8	1573.1	0.0	U/P
1.956	0.2625	0.0000	81.099	0.29882	0.00000	6129.3	1597.0	0.0	U/P
1.978	0.2525	0.0000	81.098	0.29873	0.00000	6149.9	1620.9	0.0	U/P
2.000	0.2456	0.0000	81.098	0.29863	0.00000	6169.9	1644.8	0.0	U/P
2.022	0.2372	0.0000	81.096	0.29851	0.00000	6189.2	1668.7	0.0	U/P
2.044	0.2229	0.0000	81.095	0.29836	0.00000	6207.6	1692.5	0.0	U/P
2.067	0.1978	0.0000	81.094	0.29816	0.00000	6224.4	1716.4	0.0	U/P
2.089	0.1641	0.0000	81.091	0.29789	0.00000	6238.9	1740.2	0.0	U/P
2.111	0.1286	0.0000	81.089	0.29757	0.00000	6250.6	1764.1	0.0	U/P
2.133	0.0959	0.0000	81.085	0.29717	0.00000	6259.6	1787.8	0.0	U/P
2.156	0.0686	0.0000	81.081	0.29673	0.00000	6266.2	1811.6	0.0	U/P
2.178	0.0489	0.0000	81.077	0.29625	0.00000	6270.9	1835.3	0.0	U/P
2.200	0.0353	0.0000	81.072	0.29574	0.00000	6274.2	1859.0	0.0	U/P
2.222	0.0255	0.0000	81.067	0.29521	0.00000	6276.7	1882.6	0.0	U/P
2.244	0.0182	0.0000	81.062	0.29467	0.00000	6278.4	1906.2	0.0	U/P
2.267	0.0130	0.0000	81.056	0.29411	0.00000	6279.7	1929.8	0.0	U/P
2.289	0.0093	0.0000	81.051	0.29355	0.00000	6280.5	1953.3	0.0	U/P
2.311	0.0066	0.0000	81.046	0.29298	0.00000	6281.2	1976.8	0.0	U/P
2.333	0.0047	0.0000	81.040	0.29241	0.00000	6281.6	2000.2	0.0	U/P
2.356	0.0033	0.0000	81.035	0.29184	0.00000	6281.9	2023.5	0.0	U/P
2.378	0.0023	0.0000	81.029	0.29126	0.00000	6282.2	2046.9	0.0	U/P
2.400	0.0016	0.0000	81.024	0.29068	0.00000	6282.3	2070.1	0.0	U/P
2.422	0.0011	0.0000	81.018	0.29011	0.00000	6282.4	2093.4	0.0	U/P
2.444	0.0007	0.0000	81.013	0.28953	0.00000	6282.5	2116.6	0.0	U/P
2.467	0.0004	0.0000	81.007	0.28895	0.00000	6282.6	2139.7	0.0	U/P
2.489	0.0002	0.0000	81.002	0.28837	0.00000	6282.6	2162.8	0.0	U/P
2.511	0.0001	0.0000	80.996	0.28780	0.00000	6282.6	2185.8	0.0	U/P
2.533	0.0000	0.0000	80.990	0.28727	0.00000	6282.6	2208.8	0.0	U/P
2.556	0.0000	0.0000	80.985	0.28674	0.00000	6282.6	2231.8	0.0	U/P
2.578	0.0000	0.0000	80.979	0.28621	0.00000	6282.6	2254.7	0.0	U/P
4.978	0.0000	0.0000	80.377	0.22978	0.00000	6282.6	4487.2	0.0	U/P
7.378	0.0000	0.0000	79.773	0.10059	0.00000	6282.6	6225.3	0.0	U/P
9.778	0.0000	0.0000	----	----	----	6282.6	6282.6	0.0	dry
12.178	0.0000	0.0000	----	----	----	6282.6	6282.6	0.0	dry
14.578	0.0000	0.0000	----	----	----	6282.6	6282.6	0.0	dry
16.978	0.0000	0.0000	----	----	----	6282.6	6282.6	0.0	dry
19.378	0.0000	0.0000	----	----	----	6282.6	6282.6	0.0	dry
21.778	0.0000	0.0000	----	----	----	6282.6	6282.6	0.0	dry
24.178	0.0000	0.0000	----	----	----	6282.6	6282.6	0.0	dry
26.578	0.0000	0.0000	----	----	----	6282.6	6282.6	0.0	dry
50.578	0.0000	0.0000	----	----	----	6282.6	6282.6	0.0	dry
74.578	0.0000	0.0000	----	----	----	6282.6	6282.6	0.0	dry
98.578	0.0000	0.0000	----	----	----	6282.6	6282.6	0.0	dry
122.578	0.0000	0.0000	----	----	----	6282.6	6282.6	0.0	dry
146.578	0.0000	0.0000	----	----	----	6282.6	6282.6	0.0	dry
170.578	0.0000	0.0000	----	----	----	6282.6	6282.6	0.0	dry
194.578	0.0000	0.0000	----	----	----	6282.6	6282.6	0.0	dry
218.578	0.0000	0.0000	----	----	----	6282.6	6282.6	0.0	dry
242.578	0.0000	0.0000	----	----	----	6282.6	6282.6	0.0	dry
266.578	0.0000	0.0000	----	----	----	6282.6	6282.6	0.0	dry
290.578	0.0000	0.0000	----	----	----	6282.6	6282.6	0.0	dry
314.578	0.0000	0.0000	----	----	----	6282.6	6282.6	0.0	dry
338.578	0.0000	0.0000	----	----	----	6282.6	6282.6	0.0	dry
362.578	0.0000	0.0000	----	----	----	6282.6	6282.6	0.0	dry
386.578	0.0000	0.0000	----	----	----	6282.6	6282.6	0.0	dry
410.578	0.0000	0.0000	----	----	----	6282.6	6282.6	0.0	dry
434.578	0.0000	0.0000	----	----	----	6282.6	6282.6	0.0	dry
458.578	0.0000	0.0000	----	----	----	6282.6	6282.6	0.0	dry
482.578	0.0000	0.0000	----	----	----	6282.6	6282.6	0.0	dry
506.578	0.0000	0.0000	----	----	----	6282.6	6282.6	0.0	dry
530.578	0.0000	0.0000	----	----	----	6282.6	6282.6	0.0	dry

Detailed Results (cont,d.) :: *Scenario 3 :: FDOT 4 Hour - 4 hr - 3 yr*

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
3.289	0.2901	0.0000	81.211	0.31049	0.00000	7205.8	2185.0	0.0	U/P
3.311	0.2845	0.0000	81.210	0.31044	0.00000	7228.8	2209.8	0.0	U/P
3.333	0.2806	0.0000	81.210	0.31039	0.00000	7251.4	2234.6	0.0	U/P
3.356	0.2777	0.0000	81.209	0.31033	0.00000	7273.7	2259.5	0.0	U/P
3.378	0.2757	0.0000	81.209	0.31026	0.00000	7295.9	2284.3	0.0	U/P
3.400	0.2742	0.0000	81.208	0.31020	0.00000	7317.9	2309.1	0.0	U/P
3.422	0.2731	0.0000	81.207	0.31013	0.00000	7339.8	2333.9	0.0	U/P
3.444	0.2724	0.0000	81.207	0.31006	0.00000	7361.6	2358.7	0.0	U/P
3.467	0.2718	0.0000	81.206	0.30998	0.00000	7383.4	2383.5	0.0	U/P
3.489	0.2714	0.0000	81.205	0.30991	0.00000	7405.1	2408.3	0.0	U/P
3.511	0.2699	0.0000	81.205	0.30984	0.00000	7426.7	2433.1	0.0	U/P
3.533	0.2648	0.0000	81.204	0.30975	0.00000	7448.1	2457.9	0.0	U/P
3.556	0.2530	0.0000	81.203	0.30964	0.00000	7468.8	2482.7	0.0	U/P
3.578	0.2337	0.0000	81.202	0.30949	0.00000	7488.3	2507.5	0.0	U/P
3.600	0.2102	0.0000	81.200	0.30931	0.00000	7506.1	2532.2	0.0	U/P
3.622	0.1867	0.0000	81.198	0.30908	0.00000	7521.9	2556.9	0.0	U/P
3.644	0.1658	0.0000	81.196	0.30881	0.00000	7536.0	2581.7	0.0	U/P
3.667	0.1495	0.0000	81.193	0.30851	0.00000	7548.7	2606.4	0.0	U/P
3.689	0.1379	0.0000	81.190	0.30820	0.00000	7560.1	2631.0	0.0	U/P
3.711	0.1297	0.0000	81.187	0.30786	0.00000	7570.9	2655.7	0.0	U/P
3.733	0.1238	0.0000	81.184	0.30751	0.00000	7581.0	2680.3	0.0	U/P
3.756	0.1195	0.0000	81.180	0.30716	0.00000	7590.7	2704.9	0.0	U/P
3.778	0.1164	0.0000	81.177	0.30680	0.00000	7600.2	2729.4	0.0	U/P
3.800	0.1142	0.0000	81.173	0.30643	0.00000	7609.4	2754.0	0.0	U/P
3.822	0.1126	0.0000	81.170	0.30607	0.00000	7618.5	2778.5	0.0	U/P
3.844	0.1114	0.0000	81.166	0.30570	0.00000	7627.4	2802.9	0.0	U/P
3.867	0.1106	0.0000	81.163	0.30533	0.00000	7636.3	2827.4	0.0	U/P
3.889	0.1100	0.0000	81.159	0.30496	0.00000	7645.1	2851.8	0.0	U/P
3.911	0.1096	0.0000	81.156	0.30458	0.00000	7653.9	2876.2	0.0	U/P
3.933	0.1092	0.0000	81.152	0.30421	0.00000	7662.7	2900.5	0.0	U/P
3.956	0.1090	0.0000	81.149	0.30384	0.00000	7671.4	2924.8	0.0	U/P
3.978	0.1088	0.0000	81.145	0.30347	0.00000	7680.1	2949.1	0.0	U/P
4.000	0.1087	0.0000	81.142	0.30309	0.00000	7688.8	2973.4	0.0	U/P
4.022	0.1070	0.0000	81.138	0.30272	0.00000	7697.4	2997.6	0.0	U/P
4.044	0.1020	0.0000	81.134	0.30233	0.00000	7705.8	3021.8	0.0	U/P
4.067	0.0912	0.0000	81.131	0.30192	0.00000	7713.5	3046.0	0.0	U/P
4.089	0.0761	0.0000	81.127	0.30149	0.00000	7720.2	3070.1	0.0	U/P
4.111	0.0598	0.0000	81.122	0.30102	0.00000	7725.6	3094.2	0.0	U/P
4.133	0.0446	0.0000	81.118	0.30053	0.00000	7729.8	3118.3	0.0	U/P
4.156	0.0319	0.0000	81.113	0.30001	0.00000	7732.9	3142.3	0.0	U/P
4.178	0.0228	0.0000	81.108	0.29948	0.00000	7735.1	3166.3	0.0	U/P
4.200	0.0164	0.0000	81.103	0.29893	0.00000	7736.6	3190.2	0.0	U/P
4.222	0.0119	0.0000	81.097	0.29837	0.00000	7737.8	3214.1	0.0	U/P
4.244	0.0085	0.0000	81.092	0.29781	0.00000	7738.6	3238.0	0.0	U/P
4.267	0.0061	0.0000	81.086	0.29724	0.00000	7739.2	3261.8	0.0	U/P
4.289	0.0044	0.0000	81.081	0.29667	0.00000	7739.6	3285.5	0.0	U/P
4.311	0.0031	0.0000	81.075	0.29609	0.00000	7739.9	3309.2	0.0	U/P
4.333	0.0022	0.0000	81.070	0.29552	0.00000	7740.1	3332.9	0.0	U/P
4.356	0.0016	0.0000	81.064	0.29494	0.00000	7740.3	3356.5	0.0	U/P
4.378	0.0011	0.0000	81.059	0.29436	0.00000	7740.4	3380.1	0.0	U/P
4.400	0.0008	0.0000	81.053	0.29378	0.00000	7740.4	3403.6	0.0	U/P
4.422	0.0005	0.0000	81.048	0.29320	0.00000	7740.5	3427.1	0.0	U/P
4.444	0.0003	0.0000	81.042	0.29262	0.00000	7740.5	3450.5	0.0	U/P
4.467	0.0002	0.0000	81.037	0.29204	0.00000	7740.6	3473.9	0.0	U/P
4.489	0.0001	0.0000	81.031	0.29146	0.00000	7740.6	3497.3	0.0	U/P
4.511	0.0000	0.0000	81.026	0.29088	0.00000	7740.6	3520.5	0.0	U/P
4.533	0.0000	0.0000	81.020	0.29030	0.00000	7740.6	3543.8	0.0	U/P
4.556	0.0000	0.0000	81.014	0.28972	0.00000	7740.6	3567.0	0.0	U/P
4.578	0.0000	0.0000	81.009	0.28917	0.00000	7740.6	3590.2	0.0	U/P
6.978	0.0000	0.0000	80.406	0.23257	0.00000	7740.6	5847.0	0.0	U/P
9.378	0.0000	0.0000	79.803	0.10958	0.00000	7740.6	7609.0	0.0	U/P
11.778	0.0000	0.0000	79.520	0.00761	0.00000	7740.6	7740.6	0.0	U/S
14.178	0.0000	0.0000	79.373	0.00000	0.00000	7740.6	7740.6	0.0	S
16.578	0.0000	0.0000	79.276	0.00000	0.00000	7740.6	7740.6	0.0	S
18.978	0.0000	0.0000	79.202	0.00000	0.00000	7740.6	7740.6	0.0	S
21.378	0.0000	0.0000	79.142	0.00000	0.00000	7740.6	7740.6	0.0	S
23.778	0.0000	0.0000	79.090	0.00000	0.00000	7740.6	7740.6	0.0	S
26.178	0.0000	0.0000	79.046	0.00000	0.00000	7740.6	7740.6	0.0	S
28.578	0.0000	0.0000	79.006	0.00000	0.00000	7740.6	7740.6	0.0	S
52.578	0.0000	0.0000	78.775	0.00000	0.00000	7740.6	7740.6	0.0	S
76.578	0.0000	0.0000	78.621	0.00000	0.00000	7740.6	7740.6	0.0	S
100.578	0.0000	0.0000	78.509	0.00000	0.00000	7740.6	7740.6	0.0	S
124.578	0.0000	0.0000	78.422	0.00000	0.00000	7740.6	7740.6	0.0	S
148.578	0.0000	0.0000	78.352	0.00000	0.00000	7740.6	7740.6	0.0	S

Detailed Results (cont,d.) :: *Scenario 4 :: FDOT 8 Hour - 8 hr - 3 yr*

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
8.222	0.0108	0.0000	80.839	0.27283	0.00000	9706.0	6243.2	0.0	U/P
8.244	0.0078	0.0000	80.833	0.27231	0.00000	9706.8	6265.0	0.0	U/P
8.267	0.0056	0.0000	80.828	0.27179	0.00000	9707.3	6286.8	0.0	U/P
8.289	0.0040	0.0000	80.822	0.27127	0.00000	9707.7	6308.5	0.0	U/P
8.311	0.0028	0.0000	80.817	0.27075	0.00000	9707.9	6330.2	0.0	U/P
8.333	0.0020	0.0000	80.811	0.27022	0.00000	9708.1	6351.9	0.0	U/P
8.356	0.0014	0.0000	80.806	0.26969	0.00000	9708.3	6373.5	0.0	U/P
8.378	0.0010	0.0000	80.800	0.26917	0.00000	9708.4	6395.0	0.0	U/P
8.400	0.0007	0.0000	80.795	0.26864	0.00000	9708.4	6416.5	0.0	U/P
8.422	0.0005	0.0000	80.789	0.26811	0.00000	9708.5	6438.0	0.0	U/P
8.444	0.0003	0.0000	80.784	0.26758	0.00000	9708.5	6459.4	0.0	U/P
8.467	0.0002	0.0000	80.778	0.26705	0.00000	9708.5	6480.8	0.0	U/P
8.489	0.0001	0.0000	80.773	0.26652	0.00000	9708.6	6502.1	0.0	U/P
8.511	0.0000	0.0000	80.767	0.26599	0.00000	9708.6	6523.4	0.0	U/P
8.533	0.0000	0.0000	80.762	0.26546	0.00000	9708.6	6544.7	0.0	U/P
8.556	0.0000	0.0000	80.756	0.26493	0.00000	9708.6	6565.9	0.0	U/P
8.578	0.0000	0.0000	80.750	0.26403	0.00000	9708.6	6587.1	0.0	U/P
10.978	0.0000	0.0000	80.266	0.11607	0.00000	9708.6	8269.8	0.0	U/S
13.378	0.0000	0.0000	80.160	0.03165	0.00000	9708.6	8592.8	0.0	S
15.778	0.0000	0.0000	80.084	0.02320	0.00000	9708.6	8816.7	0.0	S
18.178	0.0000	0.0000	80.022	0.01885	0.00000	9708.6	8993.7	0.0	S
20.578	0.0000	0.0000	79.968	0.01608	0.00000	9708.6	9142.4	0.0	S
22.978	0.0000	0.0000	79.921	0.01411	0.00000	9708.6	9271.5	0.0	S
25.378	0.0000	0.0000	79.877	0.01262	0.00000	9708.6	9386.2	0.0	S
27.778	0.0000	0.0000	79.837	0.01144	0.00000	9708.6	9489.6	0.0	S
30.178	0.0000	0.0000	79.800	0.01048	0.00000	9708.6	9584.0	0.0	S
32.578	0.0000	0.0000	79.765	0.00917	0.00000	9708.6	9670.7	0.0	S
56.578	0.0000	0.0000	79.433	0.00022	0.00000	9708.6	9708.6	0.0	S
80.578	0.0000	0.0000	79.207	0.00000	0.00000	9708.6	9708.6	0.0	S
104.578	0.0000	0.0000	79.043	0.00000	0.00000	9708.6	9708.6	0.0	S
128.578	0.0000	0.0000	78.917	0.00000	0.00000	9708.6	9708.6	0.0	S
152.578	0.0000	0.0000	78.815	0.00000	0.00000	9708.6	9708.6	0.0	S
176.578	0.0000	0.0000	78.731	0.00000	0.00000	9708.6	9708.6	0.0	S
200.578	0.0000	0.0000	78.660	0.00000	0.00000	9708.6	9708.6	0.0	S
224.578	0.0000	0.0000	78.598	0.00000	0.00000	9708.6	9708.6	0.0	S
248.578	0.0000	0.0000	78.543	0.00000	0.00000	9708.6	9708.6	0.0	S
272.578	0.0000	0.0000	78.495	0.00000	0.00000	9708.6	9708.6	0.0	S
296.578	0.0000	0.0000	78.452	0.00000	0.00000	9708.6	9708.6	0.0	S
320.578	0.0000	0.0000	78.412	0.00000	0.00000	9708.6	9708.6	0.0	S
344.578	0.0000	0.0000	78.377	0.00000	0.00000	9708.6	9708.6	0.0	S
368.578	0.0000	0.0000	78.344	0.00000	0.00000	9708.6	9708.6	0.0	S
392.578	0.0000	0.0000	78.314	0.00000	0.00000	9708.6	9708.6	0.0	S
416.578	0.0000	0.0000	78.286	0.00000	0.00000	9708.6	9708.6	0.0	S
440.578	0.0000	0.0000	78.260	0.00000	0.00000	9708.6	9708.6	0.0	S
464.578	0.0000	0.0000	78.235	0.00000	0.00000	9708.6	9708.6	0.0	S
488.578	0.0000	0.0000	78.212	0.00000	0.00000	9708.6	9708.6	0.0	S
512.578	0.0000	0.0000	78.191	0.00000	0.00000	9708.6	9708.6	0.0	S
536.578	0.0000	0.0000	78.171	0.00000	0.00000	9708.6	9708.6	0.0	S
560.578	0.0000	0.0000	78.152	0.00000	0.00000	9708.6	9708.6	0.0	S
584.578	0.0000	0.0000	78.134	0.00000	0.00000	9708.6	9708.6	0.0	S
608.578	0.0000	0.0000	78.117	0.00000	0.00000	9708.6	9708.6	0.0	S
632.578	0.0000	0.0000	78.100	0.00000	0.00000	9708.6	9708.6	0.0	S
656.578	0.0000	0.0000	78.085	0.00000	0.00000	9708.6	9708.6	0.0	S
680.578	0.0000	0.0000	78.070	0.00000	0.00000	9708.6	9708.6	0.0	S
704.578	0.0000	0.0000	78.056	0.00000	0.00000	9708.6	9708.6	0.0	S
728.578	0.0000	0.0000	78.042	----	----	9708.6	9708.6	0.0	N.A.

Detailed Results (cont,d.) :: *Scenario 5 :: FDOT 24 Hour - 24 hr - 3 yr*

Elapsed Time (hours)	Inflow Rate (ft^3/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft^3/s)	Overflow Discharge (ft^3/s)	Cumulative Inflow Volume (ft^3)	Cumulative Infiltration Volume (ft^3)	Cumulative Discharge Volume (ft^3)	Flow Type
34.178	0.0000	0.0000	80.914	0.02480	0.00000	13692.4	9929.0	0.0	S
36.578	0.0000	0.0000	80.864	0.02214	0.00000	13692.4	10130.4	0.0	S
38.978	0.0000	0.0000	80.818	0.02005	0.00000	13692.4	10311.7	0.0	S
41.378	0.0000	0.0000	80.775	0.01836	0.00000	13692.4	10476.9	0.0	S
43.778	0.0000	0.0000	80.735	0.01696	0.00000	13692.4	10629.0	0.0	S
46.178	0.0000	0.0000	80.698	0.01578	0.00000	13692.4	10770.1	0.0	S
48.578	0.0000	0.0000	80.662	0.01480	0.00000	13692.4	10901.8	0.0	S
72.578	0.0000	0.0000	80.407	0.00895	0.00000	13692.4	11795.3	0.0	S
96.578	0.0000	0.0000	80.203	0.00669	0.00000	13692.4	12449.0	0.0	S
120.578	0.0000	0.0000	80.031	0.00522	0.00000	13692.4	12951.1	0.0	S
144.578	0.0000	0.0000	79.885	0.00420	0.00000	13692.4	13350.7	0.0	S
168.578	0.0000	0.0000	79.756	0.00198	0.00000	13692.4	13677.2	0.0	S
192.578	0.0000	0.0000	79.593	0.00009	0.00000	13692.4	13692.4	0.0	S
216.578	0.0000	0.0000	79.460	0.00000	0.00000	13692.4	13692.4	0.0	S
240.578	0.0000	0.0000	79.348	0.00000	0.00000	13692.4	13692.4	0.0	S
264.578	0.0000	0.0000	79.253	0.00000	0.00000	13692.4	13692.4	0.0	S
288.578	0.0000	0.0000	79.170	0.00000	0.00000	13692.4	13692.4	0.0	S
312.578	0.0000	0.0000	79.097	0.00000	0.00000	13692.4	13692.4	0.0	S
336.578	0.0000	0.0000	79.032	0.00000	0.00000	13692.4	13692.4	0.0	S
360.578	0.0000	0.0000	78.973	0.00000	0.00000	13692.4	13692.4	0.0	S
384.578	0.0000	0.0000	78.920	0.00000	0.00000	13692.4	13692.4	0.0	S
408.578	0.0000	0.0000	78.871	0.00000	0.00000	13692.4	13692.4	0.0	S
432.578	0.0000	0.0000	78.826	0.00000	0.00000	13692.4	13692.4	0.0	S
456.578	0.0000	0.0000	78.784	0.00000	0.00000	13692.4	13692.4	0.0	S
480.578	0.0000	0.0000	78.746	0.00000	0.00000	13692.4	13692.4	0.0	S
504.578	0.0000	0.0000	78.710	0.00000	0.00000	13692.4	13692.4	0.0	S
528.578	0.0000	0.0000	78.676	0.00000	0.00000	13692.4	13692.4	0.0	S
552.578	0.0000	0.0000	78.645	0.00000	0.00000	13692.4	13692.4	0.0	S
576.578	0.0000	0.0000	78.615	0.00000	0.00000	13692.4	13692.4	0.0	S
600.578	0.0000	0.0000	78.587	0.00000	0.00000	13692.4	13692.4	0.0	S
624.578	0.0000	0.0000	78.561	0.00000	0.00000	13692.4	13692.4	0.0	S
648.578	0.0000	0.0000	78.536	0.00000	0.00000	13692.4	13692.4	0.0	S
672.578	0.0000	0.0000	78.513	0.00000	0.00000	13692.4	13692.4	0.0	S
696.578	0.0000	0.0000	78.491	0.00000	0.00000	13692.4	13692.4	0.0	S
720.578	0.0000	0.0000	78.469	0.00000	0.00000	13692.4	13692.4	0.0	S
744.578	0.0000	0.0000	78.449	----	----	13692.4	13692.4	0.0	N.A.

Detailed Results (cont,d.) *:: Scenario 6 :: FDOT 72 Hour - 72 hr - 3 yr*

Elapsed Time (hours)	Inflow Rate (ft^3/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft^3/s)	Overflow Discharge (ft^3/s)	Cumulative Inflow Volume (ft^3)	Cumulative Infiltration Volume (ft^3)	Cumulative Discharge Volume (ft^3)	Flow Type
72.356	0.0004	0.0000	81.761	0.02823	0.00000	18831.0	11127.2	0.0	S
72.378	0.0003	0.0000	81.761	0.02818	0.00000	18831.0	11129.5	0.0	S
72.400	0.0002	0.0000	81.760	0.02814	0.00000	18831.0	11131.7	0.0	S
72.422	0.0001	0.0000	81.760	0.02809	0.00000	18831.0	11134.0	0.0	S
72.444	0.0001	0.0000	81.759	0.02804	0.00000	18831.1	11136.2	0.0	S
72.467	0.0000	0.0000	81.759	0.02800	0.00000	18831.1	11138.5	0.0	S
72.489	0.0000	0.0000	81.758	0.02795	0.00000	18831.1	11140.7	0.0	S
72.511	0.0000	0.0000	81.758	0.02791	0.00000	18831.1	11142.9	0.0	S
72.533	0.0000	0.0000	81.758	0.02786	0.00000	18831.1	11145.2	0.0	S
72.556	0.0000	0.0000	81.757	0.02782	0.00000	18831.1	11147.4	0.0	S
72.578	0.0000	0.0000	81.757	0.02778	0.00000	18831.1	11149.6	0.0	S
74.978	0.0000	0.0000	81.715	0.02485	0.00000	18831.1	11369.9	0.0	S
77.378	0.0000	0.0000	81.674	0.02370	0.00000	18831.1	11579.0	0.0	S
79.778	0.0000	0.0000	81.635	0.02269	0.00000	18831.1	11779.4	0.0	S
82.178	0.0000	0.0000	81.598	0.02169	0.00000	18831.1	11971.1	0.0	S
84.578	0.0000	0.0000	81.561	0.02071	0.00000	18831.1	12154.2	0.0	S
86.978	0.0000	0.0000	81.526	0.01976	0.00000	18831.1	12328.9	0.0	S
89.378	0.0000	0.0000	81.492	0.01886	0.00000	18831.1	12495.6	0.0	S
91.778	0.0000	0.0000	81.459	0.01803	0.00000	18831.1	12654.8	0.0	S
94.178	0.0000	0.0000	81.428	0.01725	0.00000	18831.1	12807.1	0.0	S
96.578	0.0000	0.0000	81.397	0.01649	0.00000	18831.1	12953.0	0.0	S
120.578	0.0000	0.0000	81.158	0.01125	0.00000	18831.1	14043.4	0.0	S
144.578	0.0000	0.0000	80.956	0.00893	0.00000	18831.1	14896.8	0.0	S
168.578	0.0000	0.0000	80.782	0.00731	0.00000	18831.1	15587.2	0.0	S
192.578	0.0000	0.0000	80.629	0.00612	0.00000	18831.1	16159.9	0.0	S
216.578	0.0000	0.0000	80.493	0.00521	0.00000	18831.1	16644.0	0.0	S
240.578	0.0000	0.0000	80.369	0.00449	0.00000	18831.1	17059.4	0.0	S
264.578	0.0000	0.0000	80.257	0.00392	0.00000	18831.1	17420.0	0.0	S
288.578	0.0000	0.0000	80.153	0.00344	0.00000	18831.1	17736.0	0.0	S
312.578	0.0000	0.0000	80.058	0.00305	0.00000	18831.1	18015.2	0.0	S
336.578	0.0000	0.0000	79.969	0.00272	0.00000	18831.1	18263.3	0.0	S
360.578	0.0000	0.0000	79.886	0.00244	0.00000	18831.1	18485.2	0.0	S
384.578	0.0000	0.0000	79.809	0.00200	0.00000	18831.1	18684.6	0.0	S
408.578	0.0000	0.0000	79.730	0.00085	0.00000	18831.1	18831.1	0.0	S
432.578	0.0000	0.0000	79.632	0.00000	0.00000	18831.1	18831.1	0.0	S
456.578	0.0000	0.0000	79.547	0.00000	0.00000	18831.1	18831.1	0.0	S
480.578	0.0000	0.0000	79.473	0.00000	0.00000	18831.1	18831.1	0.0	S
504.578	0.0000	0.0000	79.406	0.00000	0.00000	18831.1	18831.1	0.0	S
528.578	0.0000	0.0000	79.345	0.00000	0.00000	18831.1	18831.1	0.0	S
552.578	0.0000	0.0000	79.289	0.00000	0.00000	18831.1	18831.1	0.0	S
576.578	0.0000	0.0000	79.238	0.00000	0.00000	18831.1	18831.1	0.0	S
600.578	0.0000	0.0000	79.190	0.00000	0.00000	18831.1	18831.1	0.0	S
624.578	0.0000	0.0000	79.146	0.00000	0.00000	18831.1	18831.1	0.0	S
648.578	0.0000	0.0000	79.105	0.00000	0.00000	18831.1	18831.1	0.0	S
672.578	0.0000	0.0000	79.066	0.00000	0.00000	18831.1	18831.1	0.0	S
696.578	0.0000	0.0000	79.029	0.00000	0.00000	18831.1	18831.1	0.0	S
720.578	0.0000	0.0000	78.995	0.00000	0.00000	18831.1	18831.1	0.0	S
744.578	0.0000	0.0000	78.962	0.00000	0.00000	18831.1	18831.1	0.0	S
768.578	0.0000	0.0000	78.931	0.00000	0.00000	18831.1	18831.1	0.0	S
792.578	0.0000	0.0000	78.902	----	----	18831.1	18831.1	0.0	N.A.

Detailed Results (cont,d.) :: *Scenario 7 :: FDOT 168 Hour - 168 hr - 3 yr*

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
167.733	0.0408	0.0000	81.904	0.02292	0.01786	24106.5	13800.4	1830.0	S
167.756	0.0408	0.0000	81.904	0.02291	0.01786	24109.8	13802.3	1831.4	S
167.778	0.0408	0.0000	81.904	0.02290	0.01786	24113.0	13804.1	1832.9	S
167.800	0.0408	0.0000	81.904	0.02290	0.01786	24116.3	13805.9	1834.3	S
167.822	0.0408	0.0000	81.904	0.02289	0.01787	24119.6	13807.8	1835.7	S
167.845	0.0408	0.0000	81.904	0.02288	0.01787	24122.8	13809.6	1837.1	S
167.867	0.0408	0.0000	81.904	0.02288	0.01787	24126.1	13811.4	1838.6	S
167.889	0.0408	0.0000	81.904	0.02287	0.01787	24129.4	13813.2	1840.0	S
167.911	0.0408	0.0000	81.904	0.02286	0.01788	24132.6	13815.1	1841.4	S
167.933	0.0408	0.0000	81.904	0.02286	0.01788	24135.9	13816.9	1842.9	S
167.956	0.0408	0.0000	81.904	0.02285	0.01788	24139.2	13818.7	1844.3	S
167.978	0.0408	0.0000	81.904	0.02284	0.01789	24142.4	13820.6	1845.7	S
168.000	0.0408	0.0000	81.904	0.02283	0.01789	24145.7	13822.4	1847.2	S
168.022	0.0402	0.0000	81.904	0.02281	0.01788	24148.9	13824.2	1848.6	S
168.044	0.0383	0.0000	81.904	0.02276	0.01784	24152.1	13826.0	1850.0	S
168.067	0.0343	0.0000	81.904	0.02268	0.01770	24155.0	13827.9	1851.4	S
168.089	0.0286	0.0000	81.904	0.02256	0.01741	24157.5	13829.7	1852.8	S
168.111	0.0225	0.0000	81.904	0.02243	0.01694	24159.5	13831.5	1854.2	S
168.133	0.0168	0.0000	81.903	0.02228	0.01626	24161.1	13833.3	1855.5	S
168.156	0.0120	0.0000	81.903	0.02214	0.01540	24162.2	13835.0	1856.8	S
168.178	0.0086	0.0000	81.903	0.02200	0.01436	24163.1	13836.8	1858.0	S
168.200	0.0062	0.0000	81.902	0.02187	0.01317	24163.7	13838.6	1859.1	S
168.222	0.0045	0.0000	81.902	0.02174	0.01182	24164.1	13840.3	1860.1	S
168.244	0.0032	0.0000	81.901	0.02159	0.01031	24164.4	13842.0	1861.0	S
168.267	0.0023	0.0000	81.901	0.02141	0.00858	24164.6	13843.8	1861.7	S
168.289	0.0016	0.0000	81.901	0.02111	0.00652	24164.8	13845.5	1862.3	S
168.311	0.0012	0.0000	81.900	0.02067	0.00373	24164.9	13847.1	1862.8	S
168.333	0.0008	0.0000	81.900	0.02136	0.00000	24165.0	13848.8	1862.9	S
168.356	0.0006	0.0000	81.900	0.02226	0.00000	24165.0	13850.5	1862.9	S
168.378	0.0004	0.0000	81.899	0.02224	0.00000	24165.1	13852.3	1862.9	S
168.400	0.0003	0.0000	81.899	0.02221	0.00000	24165.1	13854.1	1862.9	S
168.422	0.0002	0.0000	81.899	0.02218	0.00000	24165.1	13855.9	1862.9	S
168.444	0.0001	0.0000	81.898	0.02216	0.00000	24165.1	13857.7	1862.9	S
168.467	0.0001	0.0000	81.898	0.02213	0.00000	24165.1	13859.4	1862.9	S
168.489	0.0000	0.0000	81.898	0.02211	0.00000	24165.1	13861.2	1862.9	S
168.511	0.0000	0.0000	81.897	0.02209	0.00000	24165.1	13863.0	1862.9	S
168.533	0.0000	0.0000	81.897	0.02206	0.00000	24165.1	13864.7	1862.9	S
168.556	0.0000	0.0000	81.897	0.02204	0.00000	24165.1	13866.5	1862.9	S
168.578	0.0000	0.0000	81.896	0.02201	0.00000	24165.1	13868.3	1862.9	S
170.978	0.0000	0.0000	81.864	0.01995	0.00000	24165.1	14045.6	1862.9	S
173.378	0.0000	0.0000	81.833	0.01889	0.00000	24165.1	14212.9	1862.9	S
175.778	0.0000	0.0000	81.803	0.01800	0.00000	24165.1	14372.0	1862.9	S
178.178	0.0000	0.0000	81.775	0.01723	0.00000	24165.1	14524.0	1862.9	S
180.578	0.0000	0.0000	81.747	0.01655	0.00000	24165.1	14669.7	1862.9	S
182.978	0.0000	0.0000	81.721	0.01594	0.00000	24165.1	14809.9	1862.9	S
185.378	0.0000	0.0000	81.695	0.01539	0.00000	24165.1	14945.1	1862.9	S
187.778	0.0000	0.0000	81.670	0.01489	0.00000	24165.1	15075.8	1862.9	S
190.178	0.0000	0.0000	81.645	0.01443	0.00000	24165.1	15202.4	1862.9	S
192.578	0.0000	0.0000	81.621	0.01396	0.00000	24165.1	15325.1	1862.9	S
216.578	0.0000	0.0000	81.420	0.01049	0.00000	24165.1	16314.8	1862.9	S
240.578	0.0000	0.0000	81.243	0.00879	0.00000	24165.1	17137.0	1862.9	S
264.578	0.0000	0.0000	81.084	0.00749	0.00000	24165.1	17833.2	1862.9	S
288.578	0.0000	0.0000	80.941	0.00646	0.00000	24165.1	18430.9	1862.9	S
312.578	0.0000	0.0000	80.810	0.00564	0.00000	24165.1	18950.2	1862.9	S
336.578	0.0000	0.0000	80.691	0.00497	0.00000	24165.1	19405.8	1862.9	S
360.578	0.0000	0.0000	80.580	0.00441	0.00000	24165.1	19808.9	1862.9	S
384.578	0.0000	0.0000	80.477	0.00394	0.00000	24165.1	20168.0	1862.9	S
408.578	0.0000	0.0000	80.382	0.00354	0.00000	24165.1	20489.6	1862.9	S
432.578	0.0000	0.0000	80.292	0.00319	0.00000	24165.1	20779.1	1862.9	S
456.578	0.0000	0.0000	80.208	0.00289	0.00000	24165.1	21040.7	1862.9	S
480.578	0.0000	0.0000	80.130	0.00262	0.00000	24165.1	21277.9	1862.9	S
504.578	0.0000	0.0000	80.055	0.00239	0.00000	24165.1	21493.8	1862.9	S
528.578	0.0000	0.0000	79.985	0.00218	0.00000	24165.1	21690.9	1862.9	S
552.578	0.0000	0.0000	79.918	0.00200	0.00000	24165.1	21871.3	1862.9	S
576.578	0.0000	0.0000	79.855	0.00184	0.00000	24165.1	22037.1	1862.9	S
600.578	0.0000	0.0000	79.795	0.00153	0.00000	24165.1	22189.8	1862.9	S
624.578	0.0000	0.0000	79.733	0.00065	0.00000	24165.1	22302.2	1862.9	S
648.578	0.0000	0.0000	79.654	0.00000	0.00000	24165.1	22302.2	1862.9	S
672.578	0.0000	0.0000	79.586	0.00000	0.00000	24165.1	22302.2	1862.9	S
696.578	0.0000	0.0000	79.524	0.00000	0.00000	24165.1	22302.2	1862.9	S
720.578	0.0000	0.0000	79.467	0.00000	0.00000	24165.1	22302.2	1862.9	S
744.578	0.0000	0.0000	79.415	0.00000	0.00000	24165.1	22302.2	1862.9	S
768.578	0.0000	0.0000	79.367	0.00000	0.00000	24165.1	22302.2	1862.9	S
792.578	0.0000	0.0000	79.322	0.00000	0.00000	24165.1	22302.2	1862.9	S

Detailed Results (cont,d.) :: *Scenario 8 :: FDOT 240 Hour - 240 hr - 3 yr*

Elapsed Time (hours)	Inflow Rate (ft^3/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft^3/s)	Overflow Discharge (ft^3/s)	Cumulative Inflow Volume (ft^3)	Cumulative Infiltration Volume (ft^3)	Cumulative Discharge Volume (ft^3)	Flow Type
240.089	0.0104	0.0000	81.860	0.01548	0.00000	27821.2	16674.8	2911.0	S
240.111	0.0081	0.0000	81.860	0.01547	0.00000	27821.9	16676.1	2911.0	S
240.133	0.0061	0.0000	81.860	0.01545	0.00000	27822.5	16677.3	2911.0	S
240.156	0.0043	0.0000	81.860	0.01543	0.00000	27822.9	16678.5	2911.0	S
240.178	0.0031	0.0000	81.859	0.01541	0.00000	27823.2	16679.8	2911.0	S
240.200	0.0022	0.0000	81.859	0.01539	0.00000	27823.4	16681.0	2911.0	S
240.222	0.0016	0.0000	81.859	0.01536	0.00000	27823.5	16682.2	2911.0	S
240.244	0.0012	0.0000	81.859	0.01534	0.00000	27823.7	16683.5	2911.0	S
240.267	0.0008	0.0000	81.859	0.01532	0.00000	27823.7	16684.7	2911.0	S
240.289	0.0006	0.0000	81.858	0.01530	0.00000	27823.8	16685.9	2911.0	S
240.311	0.0004	0.0000	81.858	0.01528	0.00000	27823.8	16687.1	2911.0	S
240.333	0.0003	0.0000	81.858	0.01526	0.00000	27823.9	16688.4	2911.0	S
240.356	0.0002	0.0000	81.858	0.01524	0.00000	27823.9	16689.6	2911.0	S
240.378	0.0002	0.0000	81.858	0.01523	0.00000	27823.9	16690.8	2911.0	S
240.400	0.0001	0.0000	81.857	0.01521	0.00000	27823.9	16692.0	2911.0	S
240.422	0.0001	0.0000	81.857	0.01519	0.00000	27823.9	16693.2	2911.0	S
240.444	0.0000	0.0000	81.857	0.01518	0.00000	27823.9	16694.5	2911.0	S
240.467	0.0000	0.0000	81.857	0.01516	0.00000	27823.9	16695.7	2911.0	S
240.489	0.0000	0.0000	81.856	0.01515	0.00000	27823.9	16696.9	2911.0	S
240.511	0.0000	0.0000	81.856	0.01513	0.00000	27823.9	16698.1	2911.0	S
240.533	0.0000	0.0000	81.856	0.01512	0.00000	27823.9	16699.3	2911.0	S
240.556	0.0000	0.0000	81.856	0.01510	0.00000	27823.9	16700.5	2911.0	S
240.578	0.0000	0.0000	81.856	0.01509	0.00000	27823.9	16701.7	2911.0	S
242.978	0.0000	0.0000	81.833	0.01408	0.00000	27823.9	16825.7	2911.0	S
245.378	0.0000	0.0000	81.811	0.01359	0.00000	27823.9	16945.1	2911.0	S
247.778	0.0000	0.0000	81.789	0.01318	0.00000	27823.9	17060.6	2911.0	S
250.178	0.0000	0.0000	81.768	0.01282	0.00000	27823.9	17172.9	2911.0	S
252.578	0.0000	0.0000	81.747	0.01248	0.00000	27823.9	17282.1	2911.0	S
254.978	0.0000	0.0000	81.727	0.01217	0.00000	27823.9	17388.6	2911.0	S
257.378	0.0000	0.0000	81.707	0.01189	0.00000	27823.9	17492.5	2911.0	S
259.778	0.0000	0.0000	81.687	0.01162	0.00000	27823.9	17594.0	2911.0	S
262.178	0.0000	0.0000	81.668	0.01136	0.00000	27823.9	17693.2	2911.0	S
264.578	0.0000	0.0000	81.649	0.01109	0.00000	27823.9	17790.3	2911.0	S
288.578	0.0000	0.0000	81.483	0.00897	0.00000	27823.9	18619.3	2911.0	S
312.578	0.0000	0.0000	81.332	0.00784	0.00000	27823.9	19340.1	2911.0	S
336.578	0.0000	0.0000	81.193	0.00690	0.00000	27823.9	19973.3	2911.0	S
360.578	0.0000	0.0000	81.063	0.00612	0.00000	27823.9	20533.2	2911.0	S
384.578	0.0000	0.0000	80.944	0.00546	0.00000	27823.9	21031.1	2911.0	S
408.578	0.0000	0.0000	80.832	0.00489	0.00000	27823.9	21476.4	2911.0	S
432.578	0.0000	0.0000	80.728	0.00441	0.00000	27823.9	21876.6	2911.0	S
456.578	0.0000	0.0000	80.631	0.00398	0.00000	27823.9	22237.7	2911.0	S
480.578	0.0000	0.0000	80.539	0.00361	0.00000	27823.9	22564.8	2911.0	S
504.578	0.0000	0.0000	80.453	0.00329	0.00000	27823.9	22862.2	2911.0	S
528.578	0.0000	0.0000	80.372	0.00301	0.00000	27823.9	23133.4	2911.0	S
552.578	0.0000	0.0000	80.295	0.00275	0.00000	27823.9	23381.7	2911.0	S
576.578	0.0000	0.0000	80.222	0.00253	0.00000	27823.9	23609.4	2911.0	S
600.578	0.0000	0.0000	80.153	0.00233	0.00000	27823.9	23819.0	2911.0	S
624.578	0.0000	0.0000	80.087	0.00215	0.00000	27823.9	24012.3	2911.0	S
648.578	0.0000	0.0000	80.025	0.00199	0.00000	27823.9	24191.0	2911.0	S
672.578	0.0000	0.0000	79.965	0.00185	0.00000	27823.9	24356.4	2911.0	S
696.578	0.0000	0.0000	79.908	0.00172	0.00000	27823.9	24510.1	2911.0	S
720.578	0.0000	0.0000	79.853	0.00160	0.00000	27823.9	24653.0	2911.0	S
744.578	0.0000	0.0000	79.801	0.00149	0.00000	27823.9	24786.2	2911.0	S
768.578	0.0000	0.0000	79.751	0.00073	0.00000	27823.9	24910.5	2911.0	S
792.578	0.0000	0.0000	79.681	0.00001	0.00000	27823.9	24912.9	2911.0	S
816.578	0.0000	0.0000	79.618	0.00000	0.00000	27823.9	24912.9	2911.0	S
840.578	0.0000	0.0000	79.562	0.00000	0.00000	27823.9	24912.9	2911.0	S
864.578	0.0000	0.0000	79.510	0.00000	0.00000	27823.9	24912.9	2911.0	S
888.578	0.0000	0.0000	79.462	0.00000	0.00000	27823.9	24912.9	2911.0	S
912.578	0.0000	0.0000	79.418	0.00000	0.00000	27823.9	24912.9	2911.0	S
936.578	0.0000	0.0000	79.376	0.00000	0.00000	27823.9	24912.9	2911.0	S
960.578	0.0000	0.0000	79.336	----	----	27823.9	24912.9	2911.0	N.A.

Detailed Results :: *Scenario 9 :: FDOT 1 Hour - 1 hr - 5 yr*

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
0.000	0.0000	0.0000	77.000	0.00000	0.00000	0.0	0.0	0.0	N.A.
0.022	0.0000	0.0000	77.000	0.00000	0.00000	0.0	0.0	0.0	U
0.044	0.0000	0.0000	77.000	0.00000	0.00000	0.0	0.0	0.0	U
0.067	0.0000	0.0000	77.000	0.00000	0.00000	0.0	0.0	0.0	U
0.089	0.0000	0.0000	77.000	0.00000	0.00000	0.0	0.0	0.0	U
0.111	0.0000	0.0000	77.000	0.00000	0.00000	0.0	0.0	0.0	U
0.133	0.0000	0.0000	77.000	0.00000	0.00000	0.0	0.0	0.0	U
0.156	0.0000	0.0000	77.000	0.00003	0.00000	0.0	0.0	0.0	U
0.178	0.0001	0.0000	77.000	0.00053	0.00000	0.0	0.0	0.0	U
0.200	0.0019	0.0000	77.000	0.00409	0.00000	0.1	0.1	0.0	U
0.222	0.0125	0.0000	77.000	0.01789	0.00000	0.7	0.7	0.0	U
0.244	0.0447	0.0000	77.002	0.05392	0.00000	2.9	2.9	0.0	U
0.267	0.1138	0.0000	77.005	0.12462	0.00000	9.3	9.3	0.0	U
0.289	0.2262	0.0000	77.012	0.17065	0.00000	22.9	22.9	0.0	U
0.311	0.3842	0.0000	79.754	0.17176	0.00000	47.3	36.6	0.0	U/P
0.333	0.5962	0.0000	79.765	0.17302	0.00000	86.5	50.4	0.0	U/P
0.356	0.8748	0.0000	79.783	0.17506	0.00000	145.4	64.3	0.0	U/P
0.378	1.2203	0.0000	79.810	0.17800	0.00000	229.2	78.4	0.0	U/P
0.400	1.6063	0.0000	79.849	0.18192	0.00000	342.2	92.8	0.0	U/P
0.422	1.9998	0.0000	79.899	0.18678	0.00000	486.5	107.5	0.0	U/P
0.444	2.3761	0.0000	79.958	0.19255	0.00000	661.5	122.6	0.0	U/P
0.467	2.7163	0.0000	80.026	0.19922	0.00000	865.2	138.3	0.0	U/P
0.489	3.0105	0.0000	80.101	0.20652	0.00000	1094.3	154.5	0.0	U/P
0.511	3.2560	0.0000	80.180	0.21416	0.00000	1344.9	171.3	0.0	U/P
0.533	3.4476	0.0000	80.261	0.22197	0.00000	1613.1	188.8	0.0	U/P
0.556	3.5764	0.0000	80.343	0.22979	0.00000	1894.0	206.8	0.0	U/P
0.578	3.6422	0.0000	80.425	0.23750	0.00000	2182.8	225.5	0.0	U/P
0.600	3.6642	0.0000	80.505	0.24499	0.00000	2475.0	244.8	0.0	U/P
0.622	3.6426	0.0000	80.583	0.25220	0.00000	2767.3	264.7	0.0	U/P
0.644	3.5692	0.0000	80.657	0.25904	0.00000	3055.8	285.2	0.0	U/P
0.667	3.4246	0.0000	80.726	0.26541	0.00000	3335.5	306.2	0.0	U/P
0.689	3.2295	0.0000	80.790	0.27125	0.00000	3601.7	327.7	0.0	U/P
0.711	3.0168	0.0000	80.849	0.27655	0.00000	3851.5	349.6	0.0	U/P
0.733	2.8003	0.0000	80.902	0.28134	0.00000	4084.2	371.9	0.0	U/P
0.756	2.5809	0.0000	80.949	0.28564	0.00000	4299.5	394.6	0.0	U/P
0.778	2.3696	0.0000	80.992	0.28965	0.00000	4497.5	417.6	0.0	U/P
0.800	2.1726	0.0000	81.030	0.29339	0.00000	4679.2	440.9	0.0	U/P
0.822	1.9794	0.0000	81.064	0.29670	0.00000	4845.3	464.6	0.0	U/P
0.844	1.7751	0.0000	81.093	0.29956	0.00000	4995.4	488.4	0.0	U/P
0.867	1.5398	0.0000	81.119	0.30195	0.00000	5128.0	512.5	0.0	U/P
0.889	1.2860	0.0000	81.139	0.30384	0.00000	5241.1	536.7	0.0	U/P
0.911	1.0413	0.0000	81.155	0.30526	0.00000	5334.2	561.1	0.0	U/P
0.933	0.8229	0.0000	81.166	0.30625	0.00000	5408.7	585.6	0.0	U/P
0.956	0.6392	0.0000	81.174	0.30691	0.00000	5467.2	610.1	0.0	U/P
0.978	0.5003	0.0000	81.179	0.30729	0.00000	5512.8	634.7	0.0	U/P
1.000	0.3964	0.0000	81.181	0.30747	0.00000	5548.7	659.3	0.0	U/P
1.022	0.3161	0.0000	81.182	0.30749	0.00000	5577.2	683.9	0.0	U/P
1.044	0.2511	0.0000	81.182	0.30739	0.00000	5599.9	708.5	0.0	U/P
1.067	0.1969	0.0000	81.180	0.30718	0.00000	5617.8	733.1	0.0	U/P
1.089	0.1505	0.0000	81.178	0.30689	0.00000	5631.7	757.6	0.0	U/P
1.111	0.1122	0.0000	81.175	0.30653	0.00000	5642.2	782.2	0.0	U/P
1.133	0.0815	0.0000	81.171	0.30610	0.00000	5649.9	806.7	0.0	U/P
1.156	0.0579	0.0000	81.166	0.30564	0.00000	5655.5	831.1	0.0	U/P
1.178	0.0408	0.0000	81.162	0.30514	0.00000	5659.4	855.6	0.0	U/P
1.200	0.0287	0.0000	81.157	0.30461	0.00000	5662.2	880.0	0.0	U/P
1.222	0.0201	0.0000	81.152	0.30407	0.00000	5664.2	904.3	0.0	U/P
1.244	0.0139	0.0000	81.146	0.30352	0.00000	5665.5	928.6	0.0	U/P
1.267	0.0095	0.0000	81.141	0.30295	0.00000	5666.5	952.9	0.0	U/P
1.289	0.0062	0.0000	81.136	0.30239	0.00000	5667.1	977.1	0.0	U/P
1.311	0.0040	0.0000	81.130	0.30181	0.00000	5667.5	1001.2	0.0	U/P
1.333	0.0026	0.0000	81.125	0.30124	0.00000	5667.8	1025.4	0.0	U/P
1.356	0.0018	0.0000	81.119	0.30066	0.00000	5667.9	1049.4	0.0	U/P
1.378	0.0012	0.0000	81.114	0.30008	0.00000	5668.1	1073.5	0.0	U/P
1.400	0.0008	0.0000	81.108	0.29950	0.00000	5668.1	1097.5	0.0	U/P
1.422	0.0005	0.0000	81.103	0.29892	0.00000	5668.2	1121.4	0.0	U/P
1.444	0.0003	0.0000	81.097	0.29834	0.00000	5668.2	1145.3	0.0	U/P
1.467	0.0002	0.0000	81.091	0.29776	0.00000	5668.3	1169.1	0.0	U/P
1.489	0.0001	0.0000	81.086	0.29718	0.00000	5668.3	1192.9	0.0	U/P
1.511	0.0000	0.0000	81.080	0.29660	0.00000	5668.3	1216.7	0.0	U/P
1.533	0.0000	0.0000	81.075	0.29602	0.00000	5668.3	1240.4	0.0	U/P
1.556	0.0000	0.0000	81.069	0.29544	0.00000	5668.3	1264.0	0.0	U/P
1.578	0.0000	0.0000	81.064	0.29489	0.00000	5668.3	1287.7	0.0	U/P
3.978	0.0000	0.0000	80.461	0.23777	0.00000	5668.3	3590.0	0.0	U/P
6.378	0.0000	0.0000	79.858	0.10454	0.00000	5668.3	5396.3	0.0	U/P

Detailed Results (cont,d.) :: *Scenario 9 :: FDOT 1 Hour - 1 hr - 5 yr*

Elapsed Time (hours)	Inflow Rate (ft^3/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft^3/s)	Overflow Discharge (ft^3/s)	Cumulative Inflow Volume (ft^3)	Cumulative Infiltration Volume (ft^3)	Cumulative Discharge Volume (ft^3)	Flow Type
8.778	0.0000	0.0000	----	----	----	5668.3	5668.3	0.0	dry
11.178	0.0000	0.0000	----	----	----	5668.3	5668.3	0.0	dry
13.578	0.0000	0.0000	----	----	----	5668.3	5668.3	0.0	dry
15.978	0.0000	0.0000	----	----	----	5668.3	5668.3	0.0	dry
18.378	0.0000	0.0000	----	----	----	5668.3	5668.3	0.0	dry
20.778	0.0000	0.0000	----	----	----	5668.3	5668.3	0.0	dry
23.178	0.0000	0.0000	----	----	----	5668.3	5668.3	0.0	dry
25.578	0.0000	0.0000	----	----	----	5668.3	5668.3	0.0	dry
49.578	0.0000	0.0000	----	----	----	5668.3	5668.3	0.0	dry
73.578	0.0000	0.0000	----	----	----	5668.3	5668.3	0.0	dry
97.578	0.0000	0.0000	----	----	----	5668.3	5668.3	0.0	dry
121.578	0.0000	0.0000	----	----	----	5668.3	5668.3	0.0	dry
145.578	0.0000	0.0000	----	----	----	5668.3	5668.3	0.0	dry
169.578	0.0000	0.0000	----	----	----	5668.3	5668.3	0.0	dry
193.578	0.0000	0.0000	----	----	----	5668.3	5668.3	0.0	dry
217.578	0.0000	0.0000	----	----	----	5668.3	5668.3	0.0	dry
241.578	0.0000	0.0000	----	----	----	5668.3	5668.3	0.0	dry
265.578	0.0000	0.0000	----	----	----	5668.3	5668.3	0.0	dry
289.578	0.0000	0.0000	----	----	----	5668.3	5668.3	0.0	dry
313.578	0.0000	0.0000	----	----	----	5668.3	5668.3	0.0	dry
337.578	0.0000	0.0000	----	----	----	5668.3	5668.3	0.0	dry
361.578	0.0000	0.0000	----	----	----	5668.3	5668.3	0.0	dry
385.578	0.0000	0.0000	----	----	----	5668.3	5668.3	0.0	dry
409.578	0.0000	0.0000	----	----	----	5668.3	5668.3	0.0	dry
433.578	0.0000	0.0000	----	----	----	5668.3	5668.3	0.0	dry
457.578	0.0000	0.0000	----	----	----	5668.3	5668.3	0.0	dry
481.578	0.0000	0.0000	----	----	----	5668.3	5668.3	0.0	dry
505.578	0.0000	0.0000	----	----	----	5668.3	5668.3	0.0	dry
529.578	0.0000	0.0000	----	----	----	5668.3	5668.3	0.0	dry
553.578	0.0000	0.0000	----	----	----	5668.3	5668.3	0.0	dry
577.578	0.0000	0.0000	----	----	----	5668.3	5668.3	0.0	dry
601.578	0.0000	0.0000	----	----	----	5668.3	5668.3	0.0	dry
625.578	0.0000	0.0000	----	----	----	5668.3	5668.3	0.0	dry
649.578	0.0000	0.0000	----	----	----	5668.3	5668.3	0.0	dry
673.578	0.0000	0.0000	----	----	----	5668.3	5668.3	0.0	dry
697.578	0.0000	0.0000	----	----	----	5668.3	5668.3	0.0	dry
721.578	0.0000	0.0000	----	----	----	5668.3	5668.3	0.0	dry

Detailed Results (cont,d.) :: *Scenario 10 :: FDOT 2 Hour - 2 hr - 5 yr*

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
1.644	0.5105	0.0000	81.260	0.31582	0.00000	6575.9	1333.7	0.0	U/P
1.667	0.4951	0.0000	81.263	0.31615	0.00000	6616.1	1358.9	0.0	U/P
1.689	0.4754	0.0000	81.266	0.31644	0.00000	6654.9	1384.3	0.0	U/P
1.711	0.4550	0.0000	81.269	0.31670	0.00000	6692.1	1409.6	0.0	U/P
1.733	0.4363	0.0000	81.271	0.31692	0.00000	6727.8	1434.9	0.0	U/P
1.756	0.4209	0.0000	81.273	0.31711	0.00000	6762.1	1460.3	0.0	U/P
1.778	0.4098	0.0000	81.275	0.31728	0.00000	6795.3	1485.7	0.0	U/P
1.800	0.4021	0.0000	81.276	0.31744	0.00000	6827.8	1511.0	0.0	U/P
1.822	0.3946	0.0000	81.278	0.31758	0.00000	6859.6	1536.4	0.0	U/P
1.844	0.3847	0.0000	81.279	0.31770	0.00000	6890.8	1561.9	0.0	U/P
1.867	0.3692	0.0000	81.280	0.31779	0.00000	6920.9	1587.3	0.0	U/P
1.889	0.3494	0.0000	81.281	0.31784	0.00000	6949.7	1612.7	0.0	U/P
1.911	0.3288	0.0000	81.281	0.31787	0.00000	6976.8	1638.1	0.0	U/P
1.933	0.3100	0.0000	81.281	0.31785	0.00000	7002.4	1663.6	0.0	U/P
1.956	0.2944	0.0000	81.281	0.31781	0.00000	7026.5	1689.0	0.0	U/P
1.978	0.2832	0.0000	81.280	0.31775	0.00000	7049.6	1714.4	0.0	U/P
2.000	0.2754	0.0000	81.280	0.31767	0.00000	7072.0	1739.8	0.0	U/P
2.022	0.2660	0.0000	81.279	0.31757	0.00000	7093.6	1765.2	0.0	U/P
2.044	0.2500	0.0000	81.278	0.31745	0.00000	7114.3	1790.6	0.0	U/P
2.067	0.2218	0.0000	81.276	0.31727	0.00000	7133.1	1816.0	0.0	U/P
2.089	0.1840	0.0000	81.274	0.31702	0.00000	7149.4	1841.4	0.0	U/P
2.111	0.1442	0.0000	81.272	0.31671	0.00000	7162.5	1866.8	0.0	U/P
2.133	0.1075	0.0000	81.268	0.31633	0.00000	7172.6	1892.1	0.0	U/P
2.156	0.0769	0.0000	81.264	0.31589	0.00000	7179.9	1917.4	0.0	U/P
2.178	0.0549	0.0000	81.260	0.31541	0.00000	7185.2	1942.6	0.0	U/P
2.200	0.0396	0.0000	81.255	0.31491	0.00000	7189.0	1967.8	0.0	U/P
2.222	0.0285	0.0000	81.250	0.31438	0.00000	7191.7	1993.0	0.0	U/P
2.244	0.0204	0.0000	81.245	0.31384	0.00000	7193.7	2018.1	0.0	U/P
2.267	0.0146	0.0000	81.240	0.31329	0.00000	7195.1	2043.2	0.0	U/P
2.289	0.0104	0.0000	81.235	0.31272	0.00000	7196.1	2068.3	0.0	U/P
2.311	0.0074	0.0000	81.229	0.31216	0.00000	7196.8	2093.3	0.0	U/P
2.333	0.0052	0.0000	81.224	0.31159	0.00000	7197.3	2118.2	0.0	U/P
2.356	0.0037	0.0000	81.218	0.31101	0.00000	7197.7	2143.1	0.0	U/P
2.378	0.0026	0.0000	81.213	0.31044	0.00000	7197.9	2168.0	0.0	U/P
2.400	0.0018	0.0000	81.207	0.30986	0.00000	7198.1	2192.8	0.0	U/P
2.422	0.0012	0.0000	81.202	0.30928	0.00000	7198.2	2217.5	0.0	U/P
2.444	0.0008	0.0000	81.196	0.30870	0.00000	7198.3	2242.3	0.0	U/P
2.467	0.0005	0.0000	81.191	0.30812	0.00000	7198.3	2266.9	0.0	U/P
2.489	0.0002	0.0000	81.185	0.30754	0.00000	7198.4	2291.6	0.0	U/P
2.511	0.0001	0.0000	81.180	0.30696	0.00000	7198.4	2316.1	0.0	U/P
2.533	0.0000	0.0000	81.174	0.30638	0.00000	7198.4	2340.7	0.0	U/P
2.556	0.0000	0.0000	81.168	0.30580	0.00000	7198.4	2365.2	0.0	U/P
2.578	0.0000	0.0000	81.163	0.30524	0.00000	7198.4	2389.6	0.0	U/P
4.978	0.0000	0.0000	80.560	0.24728	0.00000	7198.4	4775.2	0.0	U/P
7.378	0.0000	0.0000	79.957	0.10922	0.00000	7198.4	6662.6	0.0	U/P
9.778	0.0000	0.0000	----	----	----	7198.4	7198.4	0.0	dry
12.178	0.0000	0.0000	----	----	----	7198.4	7198.4	0.0	dry
14.578	0.0000	0.0000	----	----	----	7198.4	7198.4	0.0	dry
16.978	0.0000	0.0000	----	----	----	7198.4	7198.4	0.0	dry
19.378	0.0000	0.0000	----	----	----	7198.4	7198.4	0.0	dry
21.778	0.0000	0.0000	----	----	----	7198.4	7198.4	0.0	dry
24.178	0.0000	0.0000	----	----	----	7198.4	7198.4	0.0	dry
26.578	0.0000	0.0000	----	----	----	7198.4	7198.4	0.0	dry
50.578	0.0000	0.0000	----	----	----	7198.4	7198.4	0.0	dry
74.578	0.0000	0.0000	----	----	----	7198.4	7198.4	0.0	dry
98.578	0.0000	0.0000	----	----	----	7198.4	7198.4	0.0	dry
122.578	0.0000	0.0000	----	----	----	7198.4	7198.4	0.0	dry
146.578	0.0000	0.0000	----	----	----	7198.4	7198.4	0.0	dry
170.578	0.0000	0.0000	----	----	----	7198.4	7198.4	0.0	dry
194.578	0.0000	0.0000	----	----	----	7198.4	7198.4	0.0	dry
218.578	0.0000	0.0000	----	----	----	7198.4	7198.4	0.0	dry
242.578	0.0000	0.0000	----	----	----	7198.4	7198.4	0.0	dry
266.578	0.0000	0.0000	----	----	----	7198.4	7198.4	0.0	dry
290.578	0.0000	0.0000	----	----	----	7198.4	7198.4	0.0	dry
314.578	0.0000	0.0000	----	----	----	7198.4	7198.4	0.0	dry
338.578	0.0000	0.0000	----	----	----	7198.4	7198.4	0.0	dry
362.578	0.0000	0.0000	----	----	----	7198.4	7198.4	0.0	dry
386.578	0.0000	0.0000	----	----	----	7198.4	7198.4	0.0	dry
410.578	0.0000	0.0000	----	----	----	7198.4	7198.4	0.0	dry
434.578	0.0000	0.0000	----	----	----	7198.4	7198.4	0.0	dry
458.578	0.0000	0.0000	----	----	----	7198.4	7198.4	0.0	dry
482.578	0.0000	0.0000	----	----	----	7198.4	7198.4	0.0	dry
506.578	0.0000	0.0000	----	----	----	7198.4	7198.4	0.0	dry
530.578	0.0000	0.0000	----	----	----	7198.4	7198.4	0.0	dry

Detailed Results (cont,d.) :: *Scenario 11 :: FDOT 4 Hour - 4 hr - 5 yr*

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
3.289	0.3317	0.0000	81.442	0.33466	0.00000	8438.9	2345.7	0.0	U/P
3.311	0.3253	0.0000	81.442	0.33465	0.00000	8465.2	2372.5	0.0	U/P
3.333	0.3208	0.0000	81.442	0.33462	0.00000	8491.1	2399.2	0.0	U/P
3.356	0.3175	0.0000	81.442	0.33460	0.00000	8516.6	2426.0	0.0	U/P
3.378	0.3152	0.0000	81.441	0.33456	0.00000	8541.9	2452.8	0.0	U/P
3.400	0.3135	0.0000	81.441	0.33453	0.00000	8567.1	2479.5	0.0	U/P
3.422	0.3123	0.0000	81.441	0.33449	0.00000	8592.1	2506.3	0.0	U/P
3.444	0.3114	0.0000	81.440	0.33445	0.00000	8617.0	2533.0	0.0	U/P
3.467	0.3107	0.0000	81.440	0.33441	0.00000	8641.9	2559.8	0.0	U/P
3.489	0.3102	0.0000	81.439	0.33436	0.00000	8666.8	2586.6	0.0	U/P
3.511	0.3086	0.0000	81.439	0.33432	0.00000	8691.5	2613.3	0.0	U/P
3.533	0.3027	0.0000	81.438	0.33426	0.00000	8716.0	2640.0	0.0	U/P
3.556	0.2892	0.0000	81.438	0.33418	0.00000	8739.6	2666.8	0.0	U/P
3.578	0.2671	0.0000	81.437	0.33406	0.00000	8761.9	2693.5	0.0	U/P
3.600	0.2402	0.0000	81.436	0.33390	0.00000	8782.2	2720.2	0.0	U/P
3.622	0.2133	0.0000	81.434	0.33369	0.00000	8800.3	2746.9	0.0	U/P
3.644	0.1895	0.0000	81.432	0.33344	0.00000	8816.4	2773.6	0.0	U/P
3.667	0.1708	0.0000	81.429	0.33316	0.00000	8830.9	2800.3	0.0	U/P
3.689	0.1576	0.0000	81.426	0.33285	0.00000	8844.0	2826.9	0.0	U/P
3.711	0.1483	0.0000	81.423	0.33253	0.00000	8856.2	2853.5	0.0	U/P
3.733	0.1415	0.0000	81.420	0.33220	0.00000	8867.8	2880.1	0.0	U/P
3.756	0.1366	0.0000	81.417	0.33186	0.00000	8878.9	2906.7	0.0	U/P
3.778	0.1330	0.0000	81.414	0.33151	0.00000	8889.7	2933.2	0.0	U/P
3.800	0.1305	0.0000	81.410	0.33116	0.00000	8900.3	2959.7	0.0	U/P
3.822	0.1286	0.0000	81.407	0.33080	0.00000	8910.6	2986.2	0.0	U/P
3.844	0.1273	0.0000	81.403	0.33045	0.00000	8920.9	3012.7	0.0	U/P
3.867	0.1264	0.0000	81.400	0.33009	0.00000	8931.0	3039.1	0.0	U/P
3.889	0.1257	0.0000	81.397	0.32973	0.00000	8941.1	3065.5	0.0	U/P
3.911	0.1252	0.0000	81.393	0.32937	0.00000	8951.1	3091.8	0.0	U/P
3.933	0.1248	0.0000	81.390	0.32901	0.00000	8961.1	3118.2	0.0	U/P
3.956	0.1246	0.0000	81.386	0.32865	0.00000	8971.1	3144.5	0.0	U/P
3.978	0.1244	0.0000	81.383	0.32829	0.00000	8981.1	3170.8	0.0	U/P
4.000	0.1242	0.0000	81.379	0.32793	0.00000	8991.0	3197.0	0.0	U/P
4.022	0.1223	0.0000	81.376	0.32756	0.00000	9000.9	3223.2	0.0	U/P
4.044	0.1165	0.0000	81.372	0.32718	0.00000	9010.4	3249.4	0.0	U/P
4.067	0.1042	0.0000	81.369	0.32679	0.00000	9019.3	3275.6	0.0	U/P
4.089	0.0869	0.0000	81.365	0.32636	0.00000	9026.9	3301.7	0.0	U/P
4.111	0.0683	0.0000	81.360	0.32590	0.00000	9033.1	3327.8	0.0	U/P
4.133	0.0510	0.0000	81.356	0.32541	0.00000	9037.9	3353.8	0.0	U/P
4.156	0.0365	0.0000	81.351	0.32490	0.00000	9041.4	3379.9	0.0	U/P
4.178	0.0261	0.0000	81.346	0.32437	0.00000	9043.9	3405.8	0.0	U/P
4.200	0.0188	0.0000	81.341	0.32382	0.00000	9045.7	3431.8	0.0	U/P
4.222	0.0136	0.0000	81.336	0.32326	0.00000	9047.0	3457.6	0.0	U/P
4.244	0.0097	0.0000	81.330	0.32270	0.00000	9047.9	3483.5	0.0	U/P
4.267	0.0070	0.0000	81.325	0.32213	0.00000	9048.6	3509.3	0.0	U/P
4.289	0.0050	0.0000	81.319	0.32156	0.00000	9049.1	3535.0	0.0	U/P
4.311	0.0036	0.0000	81.314	0.32099	0.00000	9049.4	3560.7	0.0	U/P
4.333	0.0025	0.0000	81.308	0.32041	0.00000	9049.6	3586.4	0.0	U/P
4.356	0.0018	0.0000	81.303	0.31983	0.00000	9049.8	3612.0	0.0	U/P
4.378	0.0013	0.0000	81.297	0.31926	0.00000	9049.9	3637.6	0.0	U/P
4.400	0.0009	0.0000	81.292	0.31868	0.00000	9050.0	3663.1	0.0	U/P
4.422	0.0006	0.0000	81.286	0.31810	0.00000	9050.1	3688.5	0.0	U/P
4.444	0.0004	0.0000	81.281	0.31752	0.00000	9050.1	3714.0	0.0	U/P
4.467	0.0002	0.0000	81.275	0.31694	0.00000	9050.1	3739.3	0.0	U/P
4.489	0.0001	0.0000	81.270	0.31636	0.00000	9050.2	3764.7	0.0	U/P
4.511	0.0000	0.0000	81.264	0.31577	0.00000	9050.2	3790.0	0.0	U/P
4.533	0.0000	0.0000	81.258	0.31519	0.00000	9050.2	3815.2	0.0	U/P
4.556	0.0000	0.0000	81.253	0.31461	0.00000	9050.2	3840.4	0.0	U/P
4.578	0.0000	0.0000	81.247	0.31405	0.00000	9050.2	3865.5	0.0	U/P
6.978	0.0000	0.0000	80.645	0.25144	0.00000	9050.2	6323.0	0.0	U/P
9.378	0.0000	0.0000	80.066	0.12616	0.00000	9050.2	8210.4	0.0	U/S
11.778	0.0000	0.0000	79.961	0.02862	0.00000	9050.2	8503.1	0.0	S
14.178	0.0000	0.0000	79.886	0.02089	0.00000	9050.2	8705.0	0.0	S
16.578	0.0000	0.0000	79.824	0.01690	0.00000	9050.2	8864.0	0.0	S
18.978	0.0000	0.0000	79.772	0.01077	0.00000	9050.2	8997.1	0.0	S
21.378	0.0000	0.0000	79.708	0.00307	0.00000	9050.2	9050.2	0.0	S
23.778	0.0000	0.0000	79.640	0.00000	0.00000	9050.2	9050.2	0.0	S
26.178	0.0000	0.0000	79.581	0.00000	0.00000	9050.2	9050.2	0.0	S
28.578	0.0000	0.0000	79.528	0.00000	0.00000	9050.2	9050.2	0.0	S
52.578	0.0000	0.0000	79.224	0.00000	0.00000	9050.2	9050.2	0.0	S
76.578	0.0000	0.0000	79.021	0.00000	0.00000	9050.2	9050.2	0.0	S
100.578	0.0000	0.0000	78.874	0.00000	0.00000	9050.2	9050.2	0.0	S
124.578	0.0000	0.0000	78.760	0.00000	0.00000	9050.2	9050.2	0.0	S
148.578	0.0000	0.0000	78.669	0.00000	0.00000	9050.2	9050.2	0.0	S

Detailed Results (cont,d.) :: *Scenario 12 :: FDOT 8 Hour - 8 hr - 5 yr*

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
8.222	0.0121	0.0000	81.055	0.29400	0.00000	11028.1	6682.6	0.0	U/P
8.244	0.0087	0.0000	81.050	0.29344	0.00000	11029.0	6706.1	0.0	U/P
8.267	0.0062	0.0000	81.045	0.29287	0.00000	11029.6	6729.5	0.0	U/P
8.289	0.0045	0.0000	81.039	0.29230	0.00000	11030.0	6752.9	0.0	U/P
8.311	0.0032	0.0000	81.034	0.29173	0.00000	11030.3	6776.3	0.0	U/P
8.333	0.0023	0.0000	81.028	0.29115	0.00000	11030.5	6799.6	0.0	U/P
8.356	0.0016	0.0000	81.023	0.29057	0.00000	11030.7	6822.9	0.0	U/P
8.378	0.0011	0.0000	81.017	0.29000	0.00000	11030.8	6846.1	0.0	U/P
8.400	0.0008	0.0000	81.012	0.28942	0.00000	11030.8	6869.3	0.0	U/P
8.422	0.0005	0.0000	81.006	0.28884	0.00000	11030.9	6892.4	0.0	U/P
8.444	0.0003	0.0000	81.000	0.28826	0.00000	11030.9	6915.5	0.0	U/P
8.467	0.0002	0.0000	80.995	0.28770	0.00000	11031.0	6938.5	0.0	U/P
8.489	0.0001	0.0000	80.989	0.28717	0.00000	11031.0	6961.5	0.0	U/P
8.511	0.0000	0.0000	80.984	0.28664	0.00000	11031.0	6984.5	0.0	U/P
8.533	0.0000	0.0000	80.978	0.28611	0.00000	11031.0	7007.4	0.0	U/P
8.556	0.0000	0.0000	80.973	0.28558	0.00000	11031.0	7030.3	0.0	U/P
8.578	0.0000	0.0000	80.967	0.28411	0.00000	11031.0	7053.1	0.0	U/P
10.978	0.0000	0.0000	80.624	0.09882	0.00000	11031.0	8381.2	0.0	U/S
13.378	0.0000	0.0000	80.517	0.03730	0.00000	11031.0	8760.7	0.0	S
15.778	0.0000	0.0000	80.440	0.02754	0.00000	11031.0	9025.7	0.0	S
18.178	0.0000	0.0000	80.376	0.02252	0.00000	11031.0	9236.6	0.0	S
20.578	0.0000	0.0000	80.321	0.01932	0.00000	11031.0	9414.9	0.0	S
22.978	0.0000	0.0000	80.273	0.01705	0.00000	11031.0	9570.6	0.0	S
25.378	0.0000	0.0000	80.228	0.01531	0.00000	11031.0	9709.4	0.0	S
27.778	0.0000	0.0000	80.187	0.01393	0.00000	11031.0	9835.1	0.0	S
30.178	0.0000	0.0000	80.149	0.01280	0.00000	11031.0	9950.1	0.0	S
32.578	0.0000	0.0000	80.113	0.01189	0.00000	11031.0	10056.3	0.0	S
56.578	0.0000	0.0000	79.863	0.00564	0.00000	11031.0	10744.6	0.0	S
80.578	0.0000	0.0000	79.629	0.00166	0.00000	11031.0	11031.0	0.0	S
104.578	0.0000	0.0000	79.412	0.00000	0.00000	11031.0	11031.0	0.0	S
128.578	0.0000	0.0000	79.249	0.00000	0.00000	11031.0	11031.0	0.0	S
152.578	0.0000	0.0000	79.121	0.00000	0.00000	11031.0	11031.0	0.0	S
176.578	0.0000	0.0000	79.016	0.00000	0.00000	11031.0	11031.0	0.0	S
200.578	0.0000	0.0000	78.928	0.00000	0.00000	11031.0	11031.0	0.0	S
224.578	0.0000	0.0000	78.851	0.00000	0.00000	11031.0	11031.0	0.0	S
248.578	0.0000	0.0000	78.785	0.00000	0.00000	11031.0	11031.0	0.0	S
272.578	0.0000	0.0000	78.726	0.00000	0.00000	11031.0	11031.0	0.0	S
296.578	0.0000	0.0000	78.673	0.00000	0.00000	11031.0	11031.0	0.0	S
320.578	0.0000	0.0000	78.626	0.00000	0.00000	11031.0	11031.0	0.0	S
344.578	0.0000	0.0000	78.583	0.00000	0.00000	11031.0	11031.0	0.0	S
368.578	0.0000	0.0000	78.544	0.00000	0.00000	11031.0	11031.0	0.0	S
392.578	0.0000	0.0000	78.507	0.00000	0.00000	11031.0	11031.0	0.0	S
416.578	0.0000	0.0000	78.474	0.00000	0.00000	11031.0	11031.0	0.0	S
440.578	0.0000	0.0000	78.443	0.00000	0.00000	11031.0	11031.0	0.0	S
464.578	0.0000	0.0000	78.414	0.00000	0.00000	11031.0	11031.0	0.0	S
488.578	0.0000	0.0000	78.387	0.00000	0.00000	11031.0	11031.0	0.0	S
512.578	0.0000	0.0000	78.361	0.00000	0.00000	11031.0	11031.0	0.0	S
536.578	0.0000	0.0000	78.337	0.00000	0.00000	11031.0	11031.0	0.0	S
560.578	0.0000	0.0000	78.315	0.00000	0.00000	11031.0	11031.0	0.0	S
584.578	0.0000	0.0000	78.293	0.00000	0.00000	11031.0	11031.0	0.0	S
608.578	0.0000	0.0000	78.273	0.00000	0.00000	11031.0	11031.0	0.0	S
632.578	0.0000	0.0000	78.254	0.00000	0.00000	11031.0	11031.0	0.0	S
656.578	0.0000	0.0000	78.235	0.00000	0.00000	11031.0	11031.0	0.0	S
680.578	0.0000	0.0000	78.218	0.00000	0.00000	11031.0	11031.0	0.0	S
704.578	0.0000	0.0000	78.201	0.00000	0.00000	11031.0	11031.0	0.0	S
728.578	0.0000	0.0000	78.186	----	----	11031.0	11031.0	0.0	N.A.

Detailed Results (cont,d.) :: *Scenario 13 :: FDOT 24 Hour - 24 hr - 5 yr*

Elapsed Time (hours)	Inflow Rate (ft^3/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft^3/s)	Overflow Discharge (ft^3/s)	Cumulative Inflow Volume (ft^3)	Cumulative Infiltration Volume (ft^3)	Cumulative Discharge Volume (ft^3)	Flow Type
34.178	0.0000	0.0000	81.319	0.02876	0.00000	15838.3	10327.7	0.0	S
36.578	0.0000	0.0000	81.268	0.02581	0.00000	15838.3	10561.9	0.0	S
38.978	0.0000	0.0000	81.221	0.02346	0.00000	15838.3	10773.7	0.0	S
41.378	0.0000	0.0000	81.177	0.02156	0.00000	15838.3	10967.4	0.0	S
43.778	0.0000	0.0000	81.136	0.01998	0.00000	15838.3	11146.2	0.0	S
46.178	0.0000	0.0000	81.098	0.01864	0.00000	15838.3	11312.6	0.0	S
48.578	0.0000	0.0000	81.061	0.01751	0.00000	15838.3	11468.3	0.0	S
72.578	0.0000	0.0000	80.797	0.01079	0.00000	15838.3	12539.2	0.0	S
96.578	0.0000	0.0000	80.583	0.00817	0.00000	15838.3	13333.1	0.0	S
120.578	0.0000	0.0000	80.404	0.00645	0.00000	15838.3	13950.8	0.0	S
144.578	0.0000	0.0000	80.250	0.00526	0.00000	15838.3	14448.3	0.0	S
168.578	0.0000	0.0000	80.114	0.00438	0.00000	15838.3	14859.3	0.0	S
192.578	0.0000	0.0000	79.993	0.00371	0.00000	15838.3	15205.2	0.0	S
216.578	0.0000	0.0000	79.883	0.00319	0.00000	15838.3	15500.7	0.0	S
240.578	0.0000	0.0000	79.783	0.00195	0.00000	15838.3	15756.1	0.0	S
264.578	0.0000	0.0000	79.664	0.00048	0.00000	15838.3	15838.3	0.0	S
288.578	0.0000	0.0000	79.549	0.00000	0.00000	15838.3	15838.3	0.0	S
312.578	0.0000	0.0000	79.451	0.00000	0.00000	15838.3	15838.3	0.0	S
336.578	0.0000	0.0000	79.366	0.00000	0.00000	15838.3	15838.3	0.0	S
360.578	0.0000	0.0000	79.290	0.00000	0.00000	15838.3	15838.3	0.0	S
384.578	0.0000	0.0000	79.222	0.00000	0.00000	15838.3	15838.3	0.0	S
408.578	0.0000	0.0000	79.160	0.00000	0.00000	15838.3	15838.3	0.0	S
432.578	0.0000	0.0000	79.104	0.00000	0.00000	15838.3	15838.3	0.0	S
456.578	0.0000	0.0000	79.053	0.00000	0.00000	15838.3	15838.3	0.0	S
480.578	0.0000	0.0000	79.006	0.00000	0.00000	15838.3	15838.3	0.0	S
504.578	0.0000	0.0000	78.962	0.00000	0.00000	15838.3	15838.3	0.0	S
528.578	0.0000	0.0000	78.921	0.00000	0.00000	15838.3	15838.3	0.0	S
552.578	0.0000	0.0000	78.883	0.00000	0.00000	15838.3	15838.3	0.0	S
576.578	0.0000	0.0000	78.848	0.00000	0.00000	15838.3	15838.3	0.0	S
600.578	0.0000	0.0000	78.814	0.00000	0.00000	15838.3	15838.3	0.0	S
624.578	0.0000	0.0000	78.783	0.00000	0.00000	15838.3	15838.3	0.0	S
648.578	0.0000	0.0000	78.753	0.00000	0.00000	15838.3	15838.3	0.0	S
672.578	0.0000	0.0000	78.725	0.00000	0.00000	15838.3	15838.3	0.0	S
696.578	0.0000	0.0000	78.698	0.00000	0.00000	15838.3	15838.3	0.0	S
720.578	0.0000	0.0000	78.673	0.00000	0.00000	15838.3	15838.3	0.0	S
744.578	0.0000	0.0000	78.649	----	----	15838.3	15838.3	0.0	N.A.

Detailed Results (cont,d.) :: *Scenario 14 :: FDOT 72 Hour - 72 hr - 5 yr*

Elapsed Time (hours)	Inflow Rate (ft^3/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft^3/s)	Overflow Discharge (ft^3/s)	Cumulative Inflow Volume (ft^3)	Cumulative Infiltration Volume (ft^3)	Cumulative Discharge Volume (ft^3)	Flow Type
72.356	0.0004	0.0000	81.890	0.02828	0.00000	19952.2	11374.0	178.0	S
72.378	0.0003	0.0000	81.890	0.02822	0.00000	19952.2	11376.3	178.0	S
72.400	0.0002	0.0000	81.889	0.02817	0.00000	19952.3	11378.5	178.0	S
72.422	0.0001	0.0000	81.889	0.02811	0.00000	19952.3	11380.8	178.0	S
72.444	0.0001	0.0000	81.889	0.02806	0.00000	19952.3	11383.0	178.0	S
72.467	0.0001	0.0000	81.888	0.02801	0.00000	19952.3	11385.3	178.0	S
72.489	0.0000	0.0000	81.888	0.02796	0.00000	19952.3	11387.5	178.0	S
72.511	0.0000	0.0000	81.887	0.02791	0.00000	19952.3	11389.8	178.0	S
72.533	0.0000	0.0000	81.887	0.02786	0.00000	19952.3	11392.0	178.0	S
72.556	0.0000	0.0000	81.887	0.02781	0.00000	19952.3	11394.2	178.0	S
72.578	0.0000	0.0000	81.886	0.02777	0.00000	19952.3	11396.4	178.0	S
74.978	0.0000	0.0000	81.845	0.02539	0.00000	19952.3	11620.4	178.0	S
77.378	0.0000	0.0000	81.805	0.02438	0.00000	19952.3	11835.2	178.0	S
79.778	0.0000	0.0000	81.766	0.02341	0.00000	19952.3	12041.7	178.0	S
82.178	0.0000	0.0000	81.729	0.02241	0.00000	19952.3	12239.7	178.0	S
84.578	0.0000	0.0000	81.693	0.02142	0.00000	19952.3	12429.0	178.0	S
86.978	0.0000	0.0000	81.657	0.02047	0.00000	19952.3	12609.8	178.0	S
89.378	0.0000	0.0000	81.624	0.01956	0.00000	19952.3	12782.6	178.0	S
91.778	0.0000	0.0000	81.591	0.01872	0.00000	19952.3	12947.9	178.0	S
94.178	0.0000	0.0000	81.559	0.01794	0.00000	19952.3	13106.1	178.0	S
96.578	0.0000	0.0000	81.529	0.01716	0.00000	19952.3	13257.8	178.0	S
120.578	0.0000	0.0000	81.289	0.01180	0.00000	19952.3	14398.5	178.0	S
144.578	0.0000	0.0000	81.087	0.00942	0.00000	19952.3	15296.4	178.0	S
168.578	0.0000	0.0000	80.911	0.00774	0.00000	19952.3	16025.8	178.0	S
192.578	0.0000	0.0000	80.756	0.00650	0.00000	19952.3	16633.1	178.0	S
216.578	0.0000	0.0000	80.617	0.00555	0.00000	19952.3	17148.2	178.0	S
240.578	0.0000	0.0000	80.492	0.00480	0.00000	19952.3	17591.5	178.0	S
264.578	0.0000	0.0000	80.377	0.00420	0.00000	19952.3	17977.4	178.0	S
288.578	0.0000	0.0000	80.272	0.00370	0.00000	19952.3	18316.5	178.0	S
312.578	0.0000	0.0000	80.174	0.00329	0.00000	19952.3	18616.8	178.0	S
336.578	0.0000	0.0000	80.083	0.00294	0.00000	19952.3	18884.4	178.0	S
360.578	0.0000	0.0000	79.999	0.00263	0.00000	19952.3	19124.0	178.0	S
384.578	0.0000	0.0000	79.920	0.00238	0.00000	19952.3	19339.7	178.0	S
408.578	0.0000	0.0000	79.845	0.00215	0.00000	19952.3	19534.5	178.0	S
432.578	0.0000	0.0000	79.776	0.00139	0.00000	19952.3	19711.1	178.0	S
456.578	0.0000	0.0000	79.691	0.00037	0.00000	19952.3	19774.2	178.0	S
480.578	0.0000	0.0000	79.605	0.00000	0.00000	19952.3	19774.2	178.0	S
504.578	0.0000	0.0000	79.529	0.00000	0.00000	19952.3	19774.2	178.0	S
528.578	0.0000	0.0000	79.462	0.00000	0.00000	19952.3	19774.2	178.0	S
552.578	0.0000	0.0000	79.401	0.00000	0.00000	19952.3	19774.2	178.0	S
576.578	0.0000	0.0000	79.344	0.00000	0.00000	19952.3	19774.2	178.0	S
600.578	0.0000	0.0000	79.293	0.00000	0.00000	19952.3	19774.2	178.0	S
624.578	0.0000	0.0000	79.245	0.00000	0.00000	19952.3	19774.2	178.0	S
648.578	0.0000	0.0000	79.200	0.00000	0.00000	19952.3	19774.2	178.0	S
672.578	0.0000	0.0000	79.158	0.00000	0.00000	19952.3	19774.2	178.0	S
696.578	0.0000	0.0000	79.119	0.00000	0.00000	19952.3	19774.2	178.0	S
720.578	0.0000	0.0000	79.082	0.00000	0.00000	19952.3	19774.2	178.0	S
744.578	0.0000	0.0000	79.047	0.00000	0.00000	19952.3	19774.2	178.0	S
768.578	0.0000	0.0000	79.014	0.00000	0.00000	19952.3	19774.2	178.0	S
792.578	0.0000	0.0000	78.983	----	----	19952.3	19774.2	178.0	N.A.

Detailed Results (cont,d.) *:: Scenario 15 :: FDOT 168 Hour - 168 hr - 5 yr*

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
167.733	0.0413	0.0000	81.904	0.02264	0.01872	24387.1	13869.3	2039.6	S
167.756	0.0413	0.0000	81.904	0.02263	0.01872	24390.4	13871.1	2041.1	S
167.778	0.0413	0.0000	81.904	0.02263	0.01872	24393.7	13872.9	2042.6	S
167.800	0.0413	0.0000	81.904	0.02262	0.01872	24397.0	13874.7	2044.1	S
167.822	0.0413	0.0000	81.904	0.02261	0.01871	24400.3	13876.5	2045.6	S
167.845	0.0413	0.0000	81.904	0.02261	0.01871	24403.6	13878.3	2047.1	S
167.867	0.0413	0.0000	81.904	0.02260	0.01871	24406.9	13880.1	2048.6	S
167.889	0.0413	0.0000	81.904	0.02260	0.01871	24410.2	13881.9	2050.0	S
167.911	0.0413	0.0000	81.904	0.02259	0.01871	24413.5	13883.8	2051.5	S
167.933	0.0413	0.0000	81.904	0.02258	0.01871	24416.8	13885.6	2053.0	S
167.956	0.0413	0.0000	81.904	0.02258	0.01871	24420.1	13887.4	2054.5	S
167.978	0.0413	0.0000	81.904	0.02257	0.01871	24423.4	13889.2	2056.0	S
168.000	0.0413	0.0000	81.904	0.02256	0.01871	24426.7	13891.0	2057.5	S
168.022	0.0406	0.0000	81.904	0.02254	0.01870	24430.0	13892.8	2059.0	S
168.044	0.0387	0.0000	81.904	0.02250	0.01865	24433.2	13894.6	2060.5	S
168.067	0.0346	0.0000	81.904	0.02242	0.01851	24436.1	13896.4	2062.0	S
168.089	0.0289	0.0000	81.904	0.02230	0.01823	24438.6	13898.2	2063.5	S
168.111	0.0227	0.0000	81.904	0.02217	0.01777	24440.7	13899.9	2064.9	S
168.133	0.0170	0.0000	81.904	0.02203	0.01711	24442.3	13901.7	2066.3	S
168.156	0.0121	0.0000	81.903	0.02189	0.01628	24443.5	13903.5	2067.6	S
168.178	0.0087	0.0000	81.903	0.02176	0.01528	24444.3	13905.2	2068.9	S
168.200	0.0062	0.0000	81.903	0.02164	0.01414	24444.9	13907.0	2070.1	S
168.222	0.0045	0.0000	81.902	0.02152	0.01286	24445.3	13908.7	2071.2	S
168.244	0.0032	0.0000	81.902	0.02139	0.01145	24445.6	13910.4	2072.1	S
168.267	0.0023	0.0000	81.901	0.02124	0.00986	24445.8	13912.1	2073.0	S
168.289	0.0017	0.0000	81.901	0.02104	0.00804	24446.0	13913.8	2073.7	S
168.311	0.0012	0.0000	81.900	0.02065	0.00583	24446.1	13915.5	2074.3	S
168.333	0.0008	0.0000	81.900	0.02054	0.00258	24446.2	13917.1	2074.6	S
168.356	0.0006	0.0000	81.900	0.02135	0.00000	24446.3	13918.8	2074.7	S
168.378	0.0004	0.0000	81.899	0.02197	0.00000	24446.3	13920.5	2074.7	S
168.400	0.0003	0.0000	81.899	0.02194	0.00000	24446.3	13922.3	2074.7	S
168.422	0.0002	0.0000	81.899	0.02192	0.00000	24446.3	13924.0	2074.7	S
168.444	0.0001	0.0000	81.899	0.02190	0.00000	24446.4	13925.8	2074.7	S
168.467	0.0001	0.0000	81.898	0.02187	0.00000	24446.4	13927.5	2074.7	S
168.489	0.0000	0.0000	81.898	0.02185	0.00000	24446.4	13929.3	2074.7	S
168.511	0.0000	0.0000	81.898	0.02183	0.00000	24446.4	13931.0	2074.7	S
168.533	0.0000	0.0000	81.897	0.02181	0.00000	24446.4	13932.8	2074.7	S
168.556	0.0000	0.0000	81.897	0.02178	0.00000	24446.4	13934.5	2074.7	S
168.578	0.0000	0.0000	81.897	0.02176	0.00000	24446.4	13936.3	2074.7	S
170.978	0.0000	0.0000	81.865	0.01976	0.00000	24446.4	14111.8	2074.7	S
173.378	0.0000	0.0000	81.834	0.01872	0.00000	24446.4	14277.7	2074.7	S
175.778	0.0000	0.0000	81.805	0.01785	0.00000	24446.4	14435.3	2074.7	S
178.178	0.0000	0.0000	81.776	0.01709	0.00000	24446.4	14586.1	2074.7	S
180.578	0.0000	0.0000	81.749	0.01642	0.00000	24446.4	14730.7	2074.7	S
182.978	0.0000	0.0000	81.723	0.01583	0.00000	24446.4	14869.8	2074.7	S
185.378	0.0000	0.0000	81.697	0.01529	0.00000	24446.4	15004.2	2074.7	S
187.778	0.0000	0.0000	81.672	0.01480	0.00000	24446.4	15134.0	2074.7	S
190.178	0.0000	0.0000	81.647	0.01435	0.00000	24446.4	15259.8	2074.7	S
192.578	0.0000	0.0000	81.623	0.01388	0.00000	24446.4	15381.9	2074.7	S
216.578	0.0000	0.0000	81.423	0.01045	0.00000	24446.4	16368.1	2074.7	S
240.578	0.0000	0.0000	81.247	0.00877	0.00000	24446.4	17188.5	2074.7	S
264.578	0.0000	0.0000	81.089	0.00748	0.00000	24446.4	17883.8	2074.7	S
288.578	0.0000	0.0000	80.946	0.00646	0.00000	24446.4	18481.2	2074.7	S
312.578	0.0000	0.0000	80.815	0.00564	0.00000	24446.4	19000.6	2074.7	S
336.578	0.0000	0.0000	80.696	0.00497	0.00000	24446.4	19456.5	2074.7	S
360.578	0.0000	0.0000	80.585	0.00442	0.00000	24446.4	19859.9	2074.7	S
384.578	0.0000	0.0000	80.483	0.00394	0.00000	24446.4	20219.5	2074.7	S
408.578	0.0000	0.0000	80.387	0.00354	0.00000	24446.4	20541.6	2074.7	S
432.578	0.0000	0.0000	80.298	0.00320	0.00000	24446.4	20831.6	2074.7	S
456.578	0.0000	0.0000	80.214	0.00289	0.00000	24446.4	21093.7	2074.7	S
480.578	0.0000	0.0000	80.135	0.00263	0.00000	24446.4	21331.5	2074.7	S
504.578	0.0000	0.0000	80.060	0.00240	0.00000	24446.4	21547.9	2074.7	S
528.578	0.0000	0.0000	79.990	0.00219	0.00000	24446.4	21745.5	2074.7	S
552.578	0.0000	0.0000	79.924	0.00201	0.00000	24446.4	21926.5	2074.7	S
576.578	0.0000	0.0000	79.860	0.00185	0.00000	24446.4	22092.7	2074.7	S
600.578	0.0000	0.0000	79.801	0.00161	0.00000	24446.4	22245.9	2074.7	S
624.578	0.0000	0.0000	79.741	0.00073	0.00000	24446.4	22371.7	2074.7	S
648.578	0.0000	0.0000	79.661	0.00000	0.00000	24446.4	22371.7	2074.7	S
672.578	0.0000	0.0000	79.592	0.00000	0.00000	24446.4	22371.7	2074.7	S
696.578	0.0000	0.0000	79.530	0.00000	0.00000	24446.4	22371.7	2074.7	S
720.578	0.0000	0.0000	79.473	0.00000	0.00000	24446.4	22371.7	2074.7	S
744.578	0.0000	0.0000	79.421	0.00000	0.00000	24446.4	22371.7	2074.7	S
768.578	0.0000	0.0000	79.372	0.00000	0.00000	24446.4	22371.7	2074.7	S
792.578	0.0000	0.0000	79.327	0.00000	0.00000	24446.4	22371.7	2074.7	S

Detailed Results (cont,d.) *:: Scenario 16 :: FDOT 240 Hour - 240 hr - 5 yr*

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
240.089	0.0106	0.0000	81.865	0.01545	0.00000	28384.5	16780.3	3340.0	S
240.111	0.0083	0.0000	81.865	0.01544	0.00000	28385.2	16781.5	3340.0	S
240.133	0.0062	0.0000	81.865	0.01542	0.00000	28385.8	16782.8	3340.0	S
240.156	0.0044	0.0000	81.865	0.01540	0.00000	28386.2	16784.0	3340.0	S
240.178	0.0032	0.0000	81.865	0.01538	0.00000	28386.5	16785.2	3340.0	S
240.200	0.0023	0.0000	81.865	0.01536	0.00000	28386.7	16786.5	3340.0	S
240.222	0.0016	0.0000	81.864	0.01534	0.00000	28386.9	16787.7	3340.0	S
240.244	0.0012	0.0000	81.864	0.01532	0.00000	28387.0	16788.9	3340.0	S
240.267	0.0008	0.0000	81.864	0.01529	0.00000	28387.1	16790.2	3340.0	S
240.289	0.0006	0.0000	81.864	0.01527	0.00000	28387.2	16791.4	3340.0	S
240.311	0.0004	0.0000	81.864	0.01525	0.00000	28387.2	16792.6	3340.0	S
240.333	0.0003	0.0000	81.863	0.01523	0.00000	28387.2	16793.8	3340.0	S
240.356	0.0002	0.0000	81.863	0.01522	0.00000	28387.2	16795.0	3340.0	S
240.378	0.0002	0.0000	81.863	0.01520	0.00000	28387.3	16796.2	3340.0	S
240.400	0.0001	0.0000	81.863	0.01518	0.00000	28387.3	16797.5	3340.0	S
240.422	0.0001	0.0000	81.862	0.01516	0.00000	28387.3	16798.7	3340.0	S
240.444	0.0000	0.0000	81.862	0.01515	0.00000	28387.3	16799.9	3340.0	S
240.467	0.0000	0.0000	81.862	0.01513	0.00000	28387.3	16801.1	3340.0	S
240.489	0.0000	0.0000	81.862	0.01512	0.00000	28387.3	16802.3	3340.0	S
240.511	0.0000	0.0000	81.862	0.01510	0.00000	28387.3	16803.5	3340.0	S
240.533	0.0000	0.0000	81.861	0.01509	0.00000	28387.3	16804.7	3340.0	S
240.556	0.0000	0.0000	81.861	0.01507	0.00000	28387.3	16805.9	3340.0	S
240.578	0.0000	0.0000	81.861	0.01506	0.00000	28387.3	16807.1	3340.0	S
242.978	0.0000	0.0000	81.838	0.01406	0.00000	28387.3	16930.9	3340.0	S
245.378	0.0000	0.0000	81.816	0.01357	0.00000	28387.3	17050.0	3340.0	S
247.778	0.0000	0.0000	81.794	0.01316	0.00000	28387.3	17165.4	3340.0	S
250.178	0.0000	0.0000	81.773	0.01279	0.00000	28387.3	17277.4	3340.0	S
252.578	0.0000	0.0000	81.753	0.01246	0.00000	28387.3	17386.4	3340.0	S
254.978	0.0000	0.0000	81.733	0.01215	0.00000	28387.3	17492.7	3340.0	S
257.378	0.0000	0.0000	81.713	0.01186	0.00000	28387.3	17596.3	3340.0	S
259.778	0.0000	0.0000	81.693	0.01159	0.00000	28387.3	17697.6	3340.0	S
262.178	0.0000	0.0000	81.674	0.01134	0.00000	28387.3	17796.6	3340.0	S
264.578	0.0000	0.0000	81.655	0.01107	0.00000	28387.3	17893.5	3340.0	S
288.578	0.0000	0.0000	81.490	0.00895	0.00000	28387.3	18720.8	3340.0	S
312.578	0.0000	0.0000	81.339	0.00783	0.00000	28387.3	19440.5	3340.0	S
336.578	0.0000	0.0000	81.200	0.00690	0.00000	28387.3	20073.1	3340.0	S
360.578	0.0000	0.0000	81.072	0.00612	0.00000	28387.3	20632.9	3340.0	S
384.578	0.0000	0.0000	80.952	0.00546	0.00000	28387.3	21131.0	3340.0	S
408.578	0.0000	0.0000	80.841	0.00490	0.00000	28387.3	21576.8	3340.0	S
432.578	0.0000	0.0000	80.737	0.00441	0.00000	28387.3	21977.6	3340.0	S
456.578	0.0000	0.0000	80.639	0.00399	0.00000	28387.3	22339.4	3340.0	S
480.578	0.0000	0.0000	80.548	0.00362	0.00000	28387.3	22667.3	3340.0	S
504.578	0.0000	0.0000	80.462	0.00330	0.00000	28387.3	22965.4	3340.0	S
528.578	0.0000	0.0000	80.381	0.00302	0.00000	28387.3	23237.6	3340.0	S
552.578	0.0000	0.0000	80.304	0.00276	0.00000	28387.3	23486.6	3340.0	S
576.578	0.0000	0.0000	80.231	0.00254	0.00000	28387.3	23715.3	3340.0	S
600.578	0.0000	0.0000	80.162	0.00234	0.00000	28387.3	23925.7	3340.0	S
624.578	0.0000	0.0000	80.096	0.00216	0.00000	28387.3	24119.7	3340.0	S
648.578	0.0000	0.0000	80.034	0.00200	0.00000	28387.3	24299.2	3340.0	S
672.578	0.0000	0.0000	79.974	0.00186	0.00000	28387.3	24465.4	3340.0	S
696.578	0.0000	0.0000	79.917	0.00172	0.00000	28387.3	24619.8	3340.0	S
720.578	0.0000	0.0000	79.862	0.00161	0.00000	28387.3	24763.5	3340.0	S
744.578	0.0000	0.0000	79.810	0.00150	0.00000	28387.3	24897.3	3340.0	S
768.578	0.0000	0.0000	79.760	0.00087	0.00000	28387.3	25022.3	3340.0	S
792.578	0.0000	0.0000	79.694	0.00014	0.00000	28387.3	25047.3	3340.0	S
816.578	0.0000	0.0000	79.631	0.00000	0.00000	28387.3	25047.3	3340.0	S
840.578	0.0000	0.0000	79.574	0.00000	0.00000	28387.3	25047.3	3340.0	S
864.578	0.0000	0.0000	79.521	0.00000	0.00000	28387.3	25047.3	3340.0	S
888.578	0.0000	0.0000	79.473	0.00000	0.00000	28387.3	25047.3	3340.0	S
912.578	0.0000	0.0000	79.428	0.00000	0.00000	28387.3	25047.3	3340.0	S
936.578	0.0000	0.0000	79.386	0.00000	0.00000	28387.3	25047.3	3340.0	S
960.578	0.0000	0.0000	79.346	----	----	28387.3	25047.3	3340.0	N.A.

Detailed Results :: *Scenario 17 :: FDOT 1 Hour - 1 hr - 10 yr*

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
0.000	0.0000	0.0000	77.000	0.00000	0.00000	0.0	0.0	0.0	N.A.
0.022	0.0000	0.0000	77.000	0.00000	0.00000	0.0	0.0	0.0	U
0.044	0.0000	0.0000	77.000	0.00000	0.00000	0.0	0.0	0.0	U
0.067	0.0000	0.0000	77.000	0.00000	0.00000	0.0	0.0	0.0	U
0.089	0.0000	0.0000	77.000	0.00000	0.00000	0.0	0.0	0.0	U
0.111	0.0000	0.0000	77.000	0.00000	0.00000	0.0	0.0	0.0	U
0.133	0.0000	0.0000	77.000	0.00000	0.00000	0.0	0.0	0.0	U
0.156	0.0000	0.0000	77.000	0.00017	0.00000	0.0	0.0	0.0	U
0.178	0.0007	0.0000	77.000	0.00150	0.00000	0.0	0.0	0.0	U
0.200	0.0047	0.0000	77.000	0.00804	0.00000	0.2	0.2	0.0	U
0.222	0.0222	0.0000	77.001	0.02937	0.00000	1.3	1.3	0.0	U
0.244	0.0685	0.0000	77.003	0.07991	0.00000	4.9	4.9	0.0	U
0.267	0.1606	0.0000	77.007	0.14280	0.00000	14.1	14.1	0.0	U
0.289	0.3030	0.0000	79.752	0.17142	0.00000	32.6	27.8	0.0	U/P
0.311	0.4958	0.0000	79.759	0.17239	0.00000	64.6	41.5	0.0	U/P
0.333	0.7468	0.0000	79.774	0.17408	0.00000	114.3	55.4	0.0	U/P
0.356	1.0704	0.0000	79.797	0.17665	0.00000	187.0	69.4	0.0	U/P
0.378	1.4662	0.0000	79.832	0.18024	0.00000	288.4	83.6	0.0	U/P
0.400	1.9038	0.0000	79.878	0.18490	0.00000	423.2	98.2	0.0	U/P
0.422	2.3456	0.0000	79.937	0.19059	0.00000	593.2	113.2	0.0	U/P
0.444	2.7641	0.0000	80.006	0.19739	0.00000	797.6	128.7	0.0	U/P
0.467	3.1384	0.0000	80.083	0.20509	0.00000	1033.7	144.8	0.0	U/P
0.489	3.4583	0.0000	80.167	0.21328	0.00000	1297.6	161.5	0.0	U/P
0.511	3.7223	0.0000	80.255	0.22178	0.00000	1584.8	178.9	0.0	U/P
0.533	3.9254	0.0000	80.345	0.23041	0.00000	1890.7	197.0	0.0	U/P
0.556	4.0584	0.0000	80.436	0.23900	0.00000	2210.1	215.8	0.0	U/P
0.578	4.1214	0.0000	80.526	0.24743	0.00000	2537.3	235.3	0.0	U/P
0.600	4.1361	0.0000	80.613	0.25559	0.00000	2867.6	255.4	0.0	U/P
0.622	4.1032	0.0000	80.697	0.26341	0.00000	3197.1	276.1	0.0	U/P
0.644	4.0134	0.0000	80.777	0.27081	0.00000	3521.8	297.5	0.0	U/P
0.667	3.8453	0.0000	80.852	0.27768	0.00000	3836.1	319.5	0.0	U/P
0.689	3.6218	0.0000	80.922	0.28398	0.00000	4134.8	342.0	0.0	U/P
0.711	3.3796	0.0000	80.985	0.28991	0.00000	4414.9	364.9	0.0	U/P
0.733	3.1342	0.0000	81.042	0.29551	0.00000	4675.4	388.3	0.0	U/P
0.756	2.8862	0.0000	81.093	0.30056	0.00000	4916.2	412.2	0.0	U/P
0.778	2.6480	0.0000	81.138	0.30506	0.00000	5137.6	436.4	0.0	U/P
0.800	2.4263	0.0000	81.179	0.30908	0.00000	5340.6	461.0	0.0	U/P
0.822	2.2094	0.0000	81.215	0.31262	0.00000	5526.0	485.9	0.0	U/P
0.844	1.9803	0.0000	81.247	0.31569	0.00000	5693.6	511.0	0.0	U/P
0.867	1.7173	0.0000	81.274	0.31824	0.00000	5841.5	536.4	0.0	U/P
0.889	1.4338	0.0000	81.296	0.32028	0.00000	5967.5	561.9	0.0	U/P
0.911	1.1608	0.0000	81.313	0.32181	0.00000	6071.3	587.6	0.0	U/P
0.933	0.9172	0.0000	81.325	0.32290	0.00000	6154.4	613.4	0.0	U/P
0.956	0.7124	0.0000	81.334	0.32362	0.00000	6219.6	639.3	0.0	U/P
0.978	0.5574	0.0000	81.339	0.32406	0.00000	6270.4	665.2	0.0	U/P
1.000	0.4416	0.0000	81.342	0.32428	0.00000	6310.4	691.1	0.0	U/P
1.022	0.3521	0.0000	81.343	0.32434	0.00000	6342.1	717.1	0.0	U/P
1.044	0.2797	0.0000	81.343	0.32426	0.00000	6367.4	743.0	0.0	U/P
1.067	0.2192	0.0000	81.342	0.32408	0.00000	6387.4	769.0	0.0	U/P
1.089	0.1676	0.0000	81.340	0.32380	0.00000	6402.8	794.9	0.0	U/P
1.111	0.1249	0.0000	81.336	0.32345	0.00000	6414.5	820.8	0.0	U/P
1.133	0.0908	0.0000	81.333	0.32303	0.00000	6423.2	846.6	0.0	U/P
1.156	0.0644	0.0000	81.329	0.32257	0.00000	6429.4	872.5	0.0	U/P
1.178	0.0454	0.0000	81.324	0.32208	0.00000	6433.8	898.3	0.0	U/P
1.200	0.0319	0.0000	81.319	0.32156	0.00000	6436.9	924.0	0.0	U/P
1.222	0.0224	0.0000	81.314	0.32102	0.00000	6439.0	949.7	0.0	U/P
1.244	0.0155	0.0000	81.309	0.32046	0.00000	6440.5	975.4	0.0	U/P
1.267	0.0105	0.0000	81.303	0.31990	0.00000	6441.6	1001.0	0.0	U/P
1.289	0.0069	0.0000	81.298	0.31934	0.00000	6442.3	1026.6	0.0	U/P
1.311	0.0044	0.0000	81.293	0.31876	0.00000	6442.7	1052.1	0.0	U/P
1.333	0.0028	0.0000	81.287	0.31819	0.00000	6443.0	1077.6	0.0	U/P
1.356	0.0020	0.0000	81.282	0.31761	0.00000	6443.2	1103.0	0.0	U/P
1.378	0.0013	0.0000	81.276	0.31703	0.00000	6443.4	1128.4	0.0	U/P
1.400	0.0009	0.0000	81.270	0.31645	0.00000	6443.4	1153.7	0.0	U/P
1.422	0.0006	0.0000	81.265	0.31587	0.00000	6443.5	1179.0	0.0	U/P
1.444	0.0004	0.0000	81.259	0.31529	0.00000	6443.5	1204.3	0.0	U/P
1.467	0.0002	0.0000	81.254	0.31471	0.00000	6443.6	1229.5	0.0	U/P
1.489	0.0001	0.0000	81.248	0.31413	0.00000	6443.6	1254.6	0.0	U/P
1.511	0.0000	0.0000	81.243	0.31355	0.00000	6443.6	1279.7	0.0	U/P
1.533	0.0000	0.0000	81.237	0.31297	0.00000	6443.6	1304.8	0.0	U/P
1.556	0.0000	0.0000	81.232	0.31239	0.00000	6443.6	1329.8	0.0	U/P
1.578	0.0000	0.0000	81.226	0.31183	0.00000	6443.6	1354.8	0.0	U/P
3.978	0.0000	0.0000	80.623	0.25339	0.00000	6443.6	3794.0	0.0	U/P
6.378	0.0000	0.0000	80.020	0.11223	0.00000	6443.6	5733.4	0.0	U/P

Detailed Results (cont,d.) :: *Scenario 17 :: FDOT 1 Hour - 1 hr - 10 yr*

Elapsed Time (hours)	Inflow Rate (ft^3/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft^3/s)	Overflow Discharge (ft^3/s)	Cumulative Inflow Volume (ft^3)	Cumulative Infiltration Volume (ft^3)	Cumulative Discharge Volume (ft^3)	Flow Type
8.778	0.0000	0.0000	----	----	----	6443.6	6443.6	0.0	dry
11.178	0.0000	0.0000	----	----	----	6443.6	6443.6	0.0	dry
13.578	0.0000	0.0000	----	----	----	6443.6	6443.6	0.0	dry
15.978	0.0000	0.0000	----	----	----	6443.6	6443.6	0.0	dry
18.378	0.0000	0.0000	----	----	----	6443.6	6443.6	0.0	dry
20.778	0.0000	0.0000	----	----	----	6443.6	6443.6	0.0	dry
23.178	0.0000	0.0000	----	----	----	6443.6	6443.6	0.0	dry
25.578	0.0000	0.0000	----	----	----	6443.6	6443.6	0.0	dry
49.578	0.0000	0.0000	----	----	----	6443.6	6443.6	0.0	dry
73.578	0.0000	0.0000	----	----	----	6443.6	6443.6	0.0	dry
97.578	0.0000	0.0000	----	----	----	6443.6	6443.6	0.0	dry
121.578	0.0000	0.0000	----	----	----	6443.6	6443.6	0.0	dry
145.578	0.0000	0.0000	----	----	----	6443.6	6443.6	0.0	dry
169.578	0.0000	0.0000	----	----	----	6443.6	6443.6	0.0	dry
193.578	0.0000	0.0000	----	----	----	6443.6	6443.6	0.0	dry
217.578	0.0000	0.0000	----	----	----	6443.6	6443.6	0.0	dry
241.578	0.0000	0.0000	----	----	----	6443.6	6443.6	0.0	dry
265.578	0.0000	0.0000	----	----	----	6443.6	6443.6	0.0	dry
289.578	0.0000	0.0000	----	----	----	6443.6	6443.6	0.0	dry
313.578	0.0000	0.0000	----	----	----	6443.6	6443.6	0.0	dry
337.578	0.0000	0.0000	----	----	----	6443.6	6443.6	0.0	dry
361.578	0.0000	0.0000	----	----	----	6443.6	6443.6	0.0	dry
385.578	0.0000	0.0000	----	----	----	6443.6	6443.6	0.0	dry
409.578	0.0000	0.0000	----	----	----	6443.6	6443.6	0.0	dry
433.578	0.0000	0.0000	----	----	----	6443.6	6443.6	0.0	dry
457.578	0.0000	0.0000	----	----	----	6443.6	6443.6	0.0	dry
481.578	0.0000	0.0000	----	----	----	6443.6	6443.6	0.0	dry
505.578	0.0000	0.0000	----	----	----	6443.6	6443.6	0.0	dry
529.578	0.0000	0.0000	----	----	----	6443.6	6443.6	0.0	dry
553.578	0.0000	0.0000	----	----	----	6443.6	6443.6	0.0	dry
577.578	0.0000	0.0000	----	----	----	6443.6	6443.6	0.0	dry
601.578	0.0000	0.0000	----	----	----	6443.6	6443.6	0.0	dry
625.578	0.0000	0.0000	----	----	----	6443.6	6443.6	0.0	dry
649.578	0.0000	0.0000	----	----	----	6443.6	6443.6	0.0	dry
673.578	0.0000	0.0000	----	----	----	6443.6	6443.6	0.0	dry
697.578	0.0000	0.0000	----	----	----	6443.6	6443.6	0.0	dry
721.578	0.0000	0.0000	----	----	----	6443.6	6443.6	0.0	dry

Detailed Results (cont,d.) :: *Scenario 18 :: FDOT 2 Hour - 2 hr - 10 yr*

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
1.644	0.5821	0.0000	81.477	0.33852	0.00000	7684.5	1422.2	0.0	U/P
1.667	0.5646	0.0000	81.481	0.33890	0.00000	7730.3	1449.3	0.0	U/P
1.689	0.5421	0.0000	81.484	0.33925	0.00000	7774.6	1476.5	0.0	U/P
1.711	0.5188	0.0000	81.488	0.33956	0.00000	7817.0	1503.6	0.0	U/P
1.733	0.4975	0.0000	81.490	0.33983	0.00000	7857.7	1530.8	0.0	U/P
1.756	0.4798	0.0000	81.493	0.34007	0.00000	7896.8	1558.0	0.0	U/P
1.778	0.4671	0.0000	81.495	0.34029	0.00000	7934.7	1585.2	0.0	U/P
1.800	0.4583	0.0000	81.497	0.34049	0.00000	7971.7	1612.4	0.0	U/P
1.822	0.4498	0.0000	81.499	0.34068	0.00000	8008.0	1639.7	0.0	U/P
1.844	0.4385	0.0000	81.500	0.34084	0.00000	8043.5	1666.9	0.0	U/P
1.867	0.4208	0.0000	81.502	0.34097	0.00000	8077.9	1694.2	0.0	U/P
1.889	0.3982	0.0000	81.503	0.34107	0.00000	8110.7	1721.5	0.0	U/P
1.911	0.3747	0.0000	81.504	0.34113	0.00000	8141.6	1748.8	0.0	U/P
1.933	0.3533	0.0000	81.504	0.34115	0.00000	8170.7	1776.1	0.0	U/P
1.956	0.3355	0.0000	81.504	0.34114	0.00000	8198.2	1803.4	0.0	U/P
1.978	0.3227	0.0000	81.504	0.34111	0.00000	8224.6	1830.7	0.0	U/P
2.000	0.3138	0.0000	81.504	0.34107	0.00000	8250.0	1857.9	0.0	U/P
2.022	0.3031	0.0000	81.503	0.34100	0.00000	8274.7	1885.2	0.0	U/P
2.044	0.2848	0.0000	81.502	0.34090	0.00000	8298.2	1912.5	0.0	U/P
2.067	0.2527	0.0000	81.501	0.34074	0.00000	8319.7	1939.8	0.0	U/P
2.089	0.2097	0.0000	81.499	0.34052	0.00000	8338.2	1967.0	0.0	U/P
2.111	0.1643	0.0000	81.497	0.34022	0.00000	8353.2	1994.3	0.0	U/P
2.133	0.1225	0.0000	81.494	0.33985	0.00000	8364.6	2021.5	0.0	U/P
2.156	0.0876	0.0000	81.490	0.33942	0.00000	8373.0	2048.6	0.0	U/P
2.178	0.0625	0.0000	81.485	0.33895	0.00000	8379.0	2075.8	0.0	U/P
2.200	0.0451	0.0000	81.481	0.33845	0.00000	8383.4	2102.9	0.0	U/P
2.222	0.0325	0.0000	81.476	0.33793	0.00000	8386.5	2129.9	0.0	U/P
2.244	0.0233	0.0000	81.471	0.33739	0.00000	8388.7	2156.9	0.0	U/P
2.267	0.0167	0.0000	81.465	0.33684	0.00000	8390.3	2183.9	0.0	U/P
2.289	0.0119	0.0000	81.460	0.33628	0.00000	8391.4	2210.8	0.0	U/P
2.311	0.0084	0.0000	81.455	0.33571	0.00000	8392.2	2237.7	0.0	U/P
2.333	0.0060	0.0000	81.449	0.33514	0.00000	8392.8	2264.5	0.0	U/P
2.356	0.0042	0.0000	81.444	0.33457	0.00000	8393.2	2291.3	0.0	U/P
2.378	0.0030	0.0000	81.438	0.33399	0.00000	8393.5	2318.1	0.0	U/P
2.400	0.0020	0.0000	81.433	0.33342	0.00000	8393.7	2344.8	0.0	U/P
2.422	0.0014	0.0000	81.427	0.33284	0.00000	8393.8	2371.4	0.0	U/P
2.444	0.0009	0.0000	81.422	0.33226	0.00000	8393.9	2398.0	0.0	U/P
2.467	0.0005	0.0000	81.416	0.33168	0.00000	8394.0	2424.6	0.0	U/P
2.489	0.0003	0.0000	81.411	0.33110	0.00000	8394.0	2451.1	0.0	U/P
2.511	0.0001	0.0000	81.405	0.33052	0.00000	8394.0	2477.6	0.0	U/P
2.533	0.0000	0.0000	81.400	0.32994	0.00000	8394.0	2504.0	0.0	U/P
2.556	0.0000	0.0000	81.394	0.32936	0.00000	8394.0	2530.3	0.0	U/P
2.578	0.0000	0.0000	81.388	0.32879	0.00000	8394.0	2556.7	0.0	U/P
4.978	0.0000	0.0000	80.786	0.26927	0.00000	8394.0	5136.5	0.0	U/P
7.378	0.0000	0.0000	80.183	0.17428	0.00000	8394.0	7209.7	0.0	U/P
9.778	0.0000	0.0000	79.848	0.06854	0.00000	8394.0	8148.0	0.0	U/S
12.178	0.0000	0.0000	79.740	0.01424	0.00000	8394.0	8394.0	0.0	S
14.578	0.0000	0.0000	79.619	0.00000	0.00000	8394.0	8394.0	0.0	S
16.978	0.0000	0.0000	79.528	0.00000	0.00000	8394.0	8394.0	0.0	S
19.378	0.0000	0.0000	79.455	0.00000	0.00000	8394.0	8394.0	0.0	S
21.778	0.0000	0.0000	79.392	0.00000	0.00000	8394.0	8394.0	0.0	S
24.178	0.0000	0.0000	79.337	0.00000	0.00000	8394.0	8394.0	0.0	S
26.578	0.0000	0.0000	79.289	0.00000	0.00000	8394.0	8394.0	0.0	S
50.578	0.0000	0.0000	79.014	0.00000	0.00000	8394.0	8394.0	0.0	S
74.578	0.0000	0.0000	78.832	0.00000	0.00000	8394.0	8394.0	0.0	S
98.578	0.0000	0.0000	78.700	0.00000	0.00000	8394.0	8394.0	0.0	S
122.578	0.0000	0.0000	78.599	0.00000	0.00000	8394.0	8394.0	0.0	S
146.578	0.0000	0.0000	78.517	0.00000	0.00000	8394.0	8394.0	0.0	S
170.578	0.0000	0.0000	78.449	0.00000	0.00000	8394.0	8394.0	0.0	S
194.578	0.0000	0.0000	78.392	0.00000	0.00000	8394.0	8394.0	0.0	S
218.578	0.0000	0.0000	78.342	0.00000	0.00000	8394.0	8394.0	0.0	S
242.578	0.0000	0.0000	78.298	0.00000	0.00000	8394.0	8394.0	0.0	S
266.578	0.0000	0.0000	78.259	0.00000	0.00000	8394.0	8394.0	0.0	S
290.578	0.0000	0.0000	78.224	0.00000	0.00000	8394.0	8394.0	0.0	S
314.578	0.0000	0.0000	78.192	0.00000	0.00000	8394.0	8394.0	0.0	S
338.578	0.0000	0.0000	78.163	0.00000	0.00000	8394.0	8394.0	0.0	S
362.578	0.0000	0.0000	78.136	0.00000	0.00000	8394.0	8394.0	0.0	S
386.578	0.0000	0.0000	78.111	0.00000	0.00000	8394.0	8394.0	0.0	S
410.578	0.0000	0.0000	78.089	0.00000	0.00000	8394.0	8394.0	0.0	S
434.578	0.0000	0.0000	78.067	0.00000	0.00000	8394.0	8394.0	0.0	S
458.578	0.0000	0.0000	78.047	0.00000	0.00000	8394.0	8394.0	0.0	S
482.578	0.0000	0.0000	78.029	0.00000	0.00000	8394.0	8394.0	0.0	S
506.578	0.0000	0.0000	78.011	0.00000	0.00000	8394.0	8394.0	0.0	S
530.578	0.0000	0.0000	77.995	0.00000	0.00000	8394.0	8394.0	0.0	S

Detailed Results (cont,d.) :: *Scenario 19 :: FDOT 4 Hour - 4 hr - 10 yr*

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
3.289	0.3766	0.0000	81.679	0.35946	0.00000	9785.4	2508.2	0.0	U/P
3.311	0.3694	0.0000	81.680	0.35948	0.00000	9815.2	2536.9	0.0	U/P
3.333	0.3642	0.0000	81.680	0.35949	0.00000	9844.6	2565.7	0.0	U/P
3.356	0.3605	0.0000	81.680	0.35949	0.00000	9873.6	2594.4	0.0	U/P
3.378	0.3578	0.0000	81.680	0.35949	0.00000	9902.3	2623.2	0.0	U/P
3.400	0.3558	0.0000	81.680	0.35948	0.00000	9930.8	2651.9	0.0	U/P
3.422	0.3545	0.0000	81.680	0.35948	0.00000	9959.3	2680.7	0.0	U/P
3.444	0.3535	0.0000	81.680	0.35947	0.00000	9987.6	2709.5	0.0	U/P
3.467	0.3527	0.0000	81.679	0.35946	0.00000	10015.8	2738.2	0.0	U/P
3.489	0.3522	0.0000	81.679	0.35944	0.00000	10044.0	2767.0	0.0	U/P
3.511	0.3502	0.0000	81.679	0.35943	0.00000	10072.1	2795.7	0.0	U/P
3.533	0.3436	0.0000	81.679	0.35940	0.00000	10099.9	2824.5	0.0	U/P
3.556	0.3283	0.0000	81.679	0.35934	0.00000	10126.7	2853.2	0.0	U/P
3.578	0.3032	0.0000	81.678	0.35925	0.00000	10152.0	2882.0	0.0	U/P
3.600	0.2726	0.0000	81.677	0.35911	0.00000	10175.0	2910.7	0.0	U/P
3.622	0.2421	0.0000	81.675	0.35892	0.00000	10195.6	2939.4	0.0	U/P
3.644	0.2151	0.0000	81.673	0.35869	0.00000	10213.9	2968.1	0.0	U/P
3.667	0.1939	0.0000	81.671	0.35843	0.00000	10230.3	2996.8	0.0	U/P
3.689	0.1789	0.0000	81.668	0.35814	0.00000	10245.2	3025.5	0.0	U/P
3.711	0.1683	0.0000	81.665	0.35783	0.00000	10259.1	3054.1	0.0	U/P
3.733	0.1606	0.0000	81.662	0.35751	0.00000	10272.2	3082.7	0.0	U/P
3.756	0.1550	0.0000	81.659	0.35719	0.00000	10284.8	3111.3	0.0	U/P
3.778	0.1510	0.0000	81.656	0.35685	0.00000	10297.1	3139.9	0.0	U/P
3.800	0.1481	0.0000	81.653	0.35651	0.00000	10309.0	3168.4	0.0	U/P
3.822	0.1460	0.0000	81.650	0.35617	0.00000	10320.8	3196.9	0.0	U/P
3.844	0.1445	0.0000	81.646	0.35582	0.00000	10332.4	3225.4	0.0	U/P
3.867	0.1434	0.0000	81.643	0.35548	0.00000	10343.9	3253.9	0.0	U/P
3.889	0.1426	0.0000	81.640	0.35513	0.00000	10355.4	3282.3	0.0	U/P
3.911	0.1421	0.0000	81.636	0.35478	0.00000	10366.8	3310.7	0.0	U/P
3.933	0.1417	0.0000	81.633	0.35443	0.00000	10378.1	3339.1	0.0	U/P
3.956	0.1414	0.0000	81.630	0.35408	0.00000	10389.4	3367.4	0.0	U/P
3.978	0.1411	0.0000	81.626	0.35373	0.00000	10400.7	3395.7	0.0	U/P
4.000	0.1410	0.0000	81.623	0.35338	0.00000	10412.0	3424.0	0.0	U/P
4.022	0.1388	0.0000	81.620	0.35303	0.00000	10423.2	3452.3	0.0	U/P
4.044	0.1322	0.0000	81.616	0.35266	0.00000	10434.1	3480.5	0.0	U/P
4.067	0.1183	0.0000	81.613	0.35228	0.00000	10444.1	3508.7	0.0	U/P
4.089	0.0986	0.0000	81.609	0.35186	0.00000	10452.8	3536.8	0.0	U/P
4.111	0.0775	0.0000	81.605	0.35141	0.00000	10459.8	3565.0	0.0	U/P
4.133	0.0579	0.0000	81.600	0.35092	0.00000	10465.2	3593.1	0.0	U/P
4.156	0.0414	0.0000	81.595	0.35041	0.00000	10469.2	3621.1	0.0	U/P
4.178	0.0296	0.0000	81.590	0.34988	0.00000	10472.0	3649.1	0.0	U/P
4.200	0.0213	0.0000	81.585	0.34934	0.00000	10474.1	3677.1	0.0	U/P
4.222	0.0154	0.0000	81.580	0.34878	0.00000	10475.5	3705.0	0.0	U/P
4.244	0.0110	0.0000	81.575	0.34822	0.00000	10476.6	3732.9	0.0	U/P
4.267	0.0079	0.0000	81.569	0.34765	0.00000	10477.3	3760.7	0.0	U/P
4.289	0.0057	0.0000	81.564	0.34708	0.00000	10477.9	3788.5	0.0	U/P
4.311	0.0041	0.0000	81.558	0.34651	0.00000	10478.3	3816.3	0.0	U/P
4.333	0.0029	0.0000	81.553	0.34593	0.00000	10478.6	3844.0	0.0	U/P
4.356	0.0020	0.0000	81.547	0.34536	0.00000	10478.7	3871.6	0.0	U/P
4.378	0.0014	0.0000	81.542	0.34478	0.00000	10478.9	3899.2	0.0	U/P
4.400	0.0010	0.0000	81.536	0.34420	0.00000	10479.0	3926.8	0.0	U/P
4.422	0.0007	0.0000	81.531	0.34362	0.00000	10479.1	3954.3	0.0	U/P
4.444	0.0004	0.0000	81.525	0.34304	0.00000	10479.1	3981.8	0.0	U/P
4.467	0.0003	0.0000	81.519	0.34246	0.00000	10479.1	4009.2	0.0	U/P
4.489	0.0001	0.0000	81.514	0.34188	0.00000	10479.1	4036.6	0.0	U/P
4.511	0.0000	0.0000	81.508	0.34130	0.00000	10479.1	4063.9	0.0	U/P
4.533	0.0000	0.0000	81.503	0.34072	0.00000	10479.1	4091.2	0.0	U/P
4.556	0.0000	0.0000	81.497	0.34014	0.00000	10479.1	4118.4	0.0	U/P
4.578	0.0000	0.0000	81.492	0.33956	0.00000	10479.1	4145.6	0.0	U/P
6.978	0.0000	0.0000	80.889	0.24249	0.00000	10479.1	6816.6	0.0	U/P
9.378	0.0000	0.0000	80.480	0.10855	0.00000	10479.1	8335.8	0.0	U/S
11.778	0.0000	0.0000	80.374	0.03499	0.00000	10479.1	8692.2	0.0	S
14.178	0.0000	0.0000	80.297	0.02577	0.00000	10479.1	8940.5	0.0	S
16.578	0.0000	0.0000	80.234	0.02102	0.00000	10479.1	9137.6	0.0	S
18.978	0.0000	0.0000	80.180	0.01801	0.00000	10479.1	9303.8	0.0	S
21.378	0.0000	0.0000	80.132	0.01586	0.00000	10479.1	9448.7	0.0	S
23.778	0.0000	0.0000	80.087	0.01422	0.00000	10479.1	9577.8	0.0	S
26.178	0.0000	0.0000	80.047	0.01292	0.00000	10479.1	9694.4	0.0	S
28.578	0.0000	0.0000	80.009	0.01192	0.00000	10479.1	9801.0	0.0	S
52.578	0.0000	0.0000	79.753	0.00392	0.00000	10479.1	10470.9	0.0	S
76.578	0.0000	0.0000	79.471	0.00005	0.00000	10479.1	10479.1	0.0	S
100.578	0.0000	0.0000	79.272	0.00000	0.00000	10479.1	10479.1	0.0	S
124.578	0.0000	0.0000	79.123	0.00000	0.00000	10479.1	10479.1	0.0	S
148.578	0.0000	0.0000	79.004	0.00000	0.00000	10479.1	10479.1	0.0	S

Detailed Results (cont,d.) :: *Scenario 20 :: FDOT 8 Hour - 8 hr - 10 yr*

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
8.222	0.0142	0.0000	81.397	0.32966	0.00000	13244.3	7367.3	0.0	U/P
8.244	0.0102	0.0000	81.391	0.32910	0.00000	13245.3	7393.7	0.0	U/P
8.267	0.0073	0.0000	81.386	0.32853	0.00000	13246.0	7420.0	0.0	U/P
8.289	0.0052	0.0000	81.381	0.32796	0.00000	13246.5	7446.3	0.0	U/P
8.311	0.0037	0.0000	81.375	0.32738	0.00000	13246.8	7472.5	0.0	U/P
8.333	0.0027	0.0000	81.370	0.32681	0.00000	13247.1	7498.6	0.0	U/P
8.356	0.0019	0.0000	81.364	0.32623	0.00000	13247.3	7524.8	0.0	U/P
8.378	0.0013	0.0000	81.359	0.32565	0.00000	13247.4	7550.8	0.0	U/P
8.400	0.0009	0.0000	81.353	0.32507	0.00000	13247.5	7576.9	0.0	U/P
8.422	0.0006	0.0000	81.347	0.32449	0.00000	13247.5	7602.8	0.0	U/P
8.444	0.0004	0.0000	81.342	0.32391	0.00000	13247.6	7628.8	0.0	U/P
8.467	0.0002	0.0000	81.336	0.29338	0.00000	13247.6	7654.7	0.0	U/P
8.489	0.0001	0.0000	81.332	0.13158	0.00000	13247.6	7675.7	0.0	U/S
8.511	0.0000	0.0000	81.332	0.00002	0.00000	13247.6	7675.7	0.0	S
8.533	0.0000	0.0000	81.332	0.00002	0.00000	13247.6	7675.7	0.0	S
8.556	0.0000	0.0000	81.332	0.00002	0.00000	13247.6	7675.7	0.0	S
8.578	0.0000	0.0000	81.332	0.00095	0.00000	13247.6	7675.7	0.0	S
10.978	0.0000	0.0000	81.138	0.07756	0.00000	13247.6	8549.4	0.0	S
13.378	0.0000	0.0000	81.028	0.04603	0.00000	13247.6	9015.9	0.0	S
15.778	0.0000	0.0000	80.949	0.03429	0.00000	13247.6	9344.8	0.0	S
18.178	0.0000	0.0000	80.883	0.02822	0.00000	13247.6	9608.5	0.0	S
20.578	0.0000	0.0000	80.827	0.02433	0.00000	13247.6	9832.5	0.0	S
22.978	0.0000	0.0000	80.776	0.02155	0.00000	13247.6	10029.0	0.0	S
25.378	0.0000	0.0000	80.730	0.01944	0.00000	13247.6	10204.9	0.0	S
27.778	0.0000	0.0000	80.687	0.01775	0.00000	13247.6	10364.8	0.0	S
30.178	0.0000	0.0000	80.647	0.01637	0.00000	13247.6	10511.6	0.0	S
32.578	0.0000	0.0000	80.610	0.01526	0.00000	13247.6	10647.6	0.0	S
56.578	0.0000	0.0000	80.347	0.00896	0.00000	13247.6	11547.9	0.0	S
80.578	0.0000	0.0000	80.139	0.00660	0.00000	13247.6	12196.1	0.0	S
104.578	0.0000	0.0000	79.966	0.00510	0.00000	13247.6	12688.4	0.0	S
128.578	0.0000	0.0000	79.818	0.00324	0.00000	13247.6	13077.2	0.0	S
152.578	0.0000	0.0000	79.662	0.00099	0.00000	13247.6	13247.6	0.0	S
176.578	0.0000	0.0000	79.506	0.00000	0.00000	13247.6	13247.6	0.0	S
200.578	0.0000	0.0000	79.380	0.00000	0.00000	13247.6	13247.6	0.0	S
224.578	0.0000	0.0000	79.274	0.00000	0.00000	13247.6	13247.6	0.0	S
248.578	0.0000	0.0000	79.183	0.00000	0.00000	13247.6	13247.6	0.0	S
272.578	0.0000	0.0000	79.104	0.00000	0.00000	13247.6	13247.6	0.0	S
296.578	0.0000	0.0000	79.034	0.00000	0.00000	13247.6	13247.6	0.0	S
320.578	0.0000	0.0000	78.971	0.00000	0.00000	13247.6	13247.6	0.0	S
344.578	0.0000	0.0000	78.914	0.00000	0.00000	13247.6	13247.6	0.0	S
368.578	0.0000	0.0000	78.863	0.00000	0.00000	13247.6	13247.6	0.0	S
392.578	0.0000	0.0000	78.816	0.00000	0.00000	13247.6	13247.6	0.0	S
416.578	0.0000	0.0000	78.772	0.00000	0.00000	13247.6	13247.6	0.0	S
440.578	0.0000	0.0000	78.732	0.00000	0.00000	13247.6	13247.6	0.0	S
464.578	0.0000	0.0000	78.695	0.00000	0.00000	13247.6	13247.6	0.0	S
488.578	0.0000	0.0000	78.661	0.00000	0.00000	13247.6	13247.6	0.0	S
512.578	0.0000	0.0000	78.628	0.00000	0.00000	13247.6	13247.6	0.0	S
536.578	0.0000	0.0000	78.598	0.00000	0.00000	13247.6	13247.6	0.0	S
560.578	0.0000	0.0000	78.569	0.00000	0.00000	13247.6	13247.6	0.0	S
584.578	0.0000	0.0000	78.542	0.00000	0.00000	13247.6	13247.6	0.0	S
608.578	0.0000	0.0000	78.517	0.00000	0.00000	13247.6	13247.6	0.0	S
632.578	0.0000	0.0000	78.493	0.00000	0.00000	13247.6	13247.6	0.0	S
656.578	0.0000	0.0000	78.470	0.00000	0.00000	13247.6	13247.6	0.0	S
680.578	0.0000	0.0000	78.449	0.00000	0.00000	13247.6	13247.6	0.0	S
704.578	0.0000	0.0000	78.428	0.00000	0.00000	13247.6	13247.6	0.0	S
728.578	0.0000	0.0000	78.409	----	----	13247.6	13247.6	0.0	N.A.

Detailed Results (cont,d.) *:: Scenario 21 :: FDOT 24 Hour - 24 hr - 10 yr*

Elapsed Time (hours)	Inflow Rate (ft^3/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft^3/s)	Overflow Discharge (ft^3/s)	Cumulative Inflow Volume (ft^3)	Cumulative Infiltration Volume (ft^3)	Cumulative Discharge Volume (ft^3)	Flow Type
34.178	0.0000	0.0000	81.642	0.03092	0.00000	18719.0	10770.3	865.6	S
36.578	0.0000	0.0000	81.592	0.02804	0.00000	18719.0	11023.7	865.6	S
38.978	0.0000	0.0000	81.545	0.02571	0.00000	18719.0	11254.9	865.6	S
41.378	0.0000	0.0000	81.502	0.02377	0.00000	18719.0	11467.9	865.6	S
43.778	0.0000	0.0000	81.462	0.02214	0.00000	18719.0	11665.7	865.6	S
46.178	0.0000	0.0000	81.423	0.02075	0.00000	18719.0	11850.6	865.6	S
48.578	0.0000	0.0000	81.387	0.01956	0.00000	18719.0	12024.3	865.6	S
72.578	0.0000	0.0000	81.118	0.01230	0.00000	18719.0	13238.9	865.6	S
96.578	0.0000	0.0000	80.899	0.00942	0.00000	18719.0	14150.2	865.6	S
120.578	0.0000	0.0000	80.715	0.00752	0.00000	18719.0	14866.8	865.6	S
144.578	0.0000	0.0000	80.555	0.00618	0.00000	18719.0	15449.7	865.6	S
168.578	0.0000	0.0000	80.414	0.00520	0.00000	18719.0	15935.4	865.6	S
192.578	0.0000	0.0000	80.287	0.00444	0.00000	18719.0	16347.6	865.6	S
216.578	0.0000	0.0000	80.172	0.00384	0.00000	18719.0	16702.4	865.6	S
240.578	0.0000	0.0000	80.067	0.00335	0.00000	18719.0	17011.0	865.6	S
264.578	0.0000	0.0000	79.970	0.00295	0.00000	18719.0	17281.9	865.6	S
288.578	0.0000	0.0000	79.881	0.00262	0.00000	18719.0	17521.4	865.6	S
312.578	0.0000	0.0000	79.798	0.00192	0.00000	18719.0	17734.7	865.6	S
336.578	0.0000	0.0000	79.706	0.00069	0.00000	18719.0	17853.4	865.6	S
360.578	0.0000	0.0000	79.604	0.00000	0.00000	18719.0	17853.4	865.6	S
384.578	0.0000	0.0000	79.516	0.00000	0.00000	18719.0	17853.4	865.6	S
408.578	0.0000	0.0000	79.439	0.00000	0.00000	18719.0	17853.4	865.6	S
432.578	0.0000	0.0000	79.369	0.00000	0.00000	18719.0	17853.4	865.6	S
456.578	0.0000	0.0000	79.307	0.00000	0.00000	18719.0	17853.4	865.6	S
480.578	0.0000	0.0000	79.249	0.00000	0.00000	18719.0	17853.4	865.6	S
504.578	0.0000	0.0000	79.197	0.00000	0.00000	18719.0	17853.4	865.6	S
528.578	0.0000	0.0000	79.148	0.00000	0.00000	18719.0	17853.4	865.6	S
552.578	0.0000	0.0000	79.103	0.00000	0.00000	18719.0	17853.4	865.6	S
576.578	0.0000	0.0000	79.061	0.00000	0.00000	18719.0	17853.4	865.6	S
600.578	0.0000	0.0000	79.022	0.00000	0.00000	18719.0	17853.4	865.6	S
624.578	0.0000	0.0000	78.985	0.00000	0.00000	18719.0	17853.4	865.6	S
648.578	0.0000	0.0000	78.950	0.00000	0.00000	18719.0	17853.4	865.6	S
672.578	0.0000	0.0000	78.917	0.00000	0.00000	18719.0	17853.4	865.6	S
696.578	0.0000	0.0000	78.886	0.00000	0.00000	18719.0	17853.4	865.6	S
720.578	0.0000	0.0000	78.857	0.00000	0.00000	18719.0	17853.4	865.6	S
744.578	0.0000	0.0000	78.829	----	----	18719.0	17853.4	865.6	N.A.

Detailed Results (cont,d.) :: *Scenario 22 :: FDOT 72 Hour - 72 hr - 10 yr*

Elapsed Time (hours)	Inflow Rate (ft^3/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft^3/s)	Overflow Discharge (ft^3/s)	Cumulative Inflow Volume (ft^3)	Cumulative Infiltration Volume (ft^3)	Cumulative Discharge Volume (ft^3)	Flow Type
72.356	0.0005	0.0000	81.896	0.02654	0.00000	22478.5	11675.5	2368.4	S
72.378	0.0003	0.0000	81.896	0.02651	0.00000	22478.6	11677.6	2368.4	S
72.400	0.0002	0.0000	81.896	0.02648	0.00000	22478.6	11679.7	2368.4	S
72.422	0.0001	0.0000	81.895	0.02645	0.00000	22478.6	11681.8	2368.4	S
72.444	0.0001	0.0000	81.895	0.02642	0.00000	22478.6	11683.9	2368.4	S
72.467	0.0001	0.0000	81.895	0.02640	0.00000	22478.6	11686.1	2368.4	S
72.489	0.0000	0.0000	81.894	0.02637	0.00000	22478.6	11688.2	2368.4	S
72.511	0.0000	0.0000	81.894	0.02635	0.00000	22478.6	11690.3	2368.4	S
72.533	0.0000	0.0000	81.893	0.02632	0.00000	22478.6	11692.4	2368.4	S
72.556	0.0000	0.0000	81.893	0.02630	0.00000	22478.6	11694.5	2368.4	S
72.578	0.0000	0.0000	81.893	0.02628	0.00000	22478.6	11696.6	2368.4	S
74.978	0.0000	0.0000	81.853	0.02429	0.00000	22478.6	11911.1	2368.4	S
77.378	0.0000	0.0000	81.815	0.02328	0.00000	22478.6	12116.3	2368.4	S
79.778	0.0000	0.0000	81.778	0.02236	0.00000	22478.6	12313.4	2368.4	S
82.178	0.0000	0.0000	81.743	0.02147	0.00000	22478.6	12502.7	2368.4	S
84.578	0.0000	0.0000	81.708	0.02060	0.00000	22478.6	12684.4	2368.4	S
86.978	0.0000	0.0000	81.674	0.01976	0.00000	22478.6	12858.7	2368.4	S
89.378	0.0000	0.0000	81.642	0.01896	0.00000	22478.6	13025.9	2368.4	S
91.778	0.0000	0.0000	81.610	0.01820	0.00000	22478.6	13186.3	2368.4	S
94.178	0.0000	0.0000	81.580	0.01749	0.00000	22478.6	13340.4	2368.4	S
96.578	0.0000	0.0000	81.550	0.01676	0.00000	22478.6	13488.4	2368.4	S
120.578	0.0000	0.0000	81.315	0.01168	0.00000	22478.6	14613.9	2368.4	S
144.578	0.0000	0.0000	81.116	0.00939	0.00000	22478.6	15507.0	2368.4	S
168.578	0.0000	0.0000	80.942	0.00775	0.00000	22478.6	16236.8	2368.4	S
192.578	0.0000	0.0000	80.787	0.00653	0.00000	22478.6	16846.8	2368.4	S
216.578	0.0000	0.0000	80.649	0.00559	0.00000	22478.6	17365.8	2368.4	S
240.578	0.0000	0.0000	80.524	0.00485	0.00000	22478.6	17813.5	2368.4	S
264.578	0.0000	0.0000	80.410	0.00425	0.00000	22478.6	18204.0	2368.4	S
288.578	0.0000	0.0000	80.305	0.00375	0.00000	22478.6	18547.7	2368.4	S
312.578	0.0000	0.0000	80.207	0.00334	0.00000	22478.6	18852.5	2368.4	S
336.578	0.0000	0.0000	80.116	0.00299	0.00000	22478.6	19124.5	2368.4	S
360.578	0.0000	0.0000	80.032	0.00268	0.00000	22478.6	19368.4	2368.4	S
384.578	0.0000	0.0000	79.952	0.00242	0.00000	22478.6	19588.0	2368.4	S
408.578	0.0000	0.0000	79.878	0.00219	0.00000	22478.6	19786.6	2368.4	S
432.578	0.0000	0.0000	79.808	0.00187	0.00000	22478.6	19966.7	2368.4	S
456.578	0.0000	0.0000	79.738	0.00083	0.00000	22478.6	20110.2	2368.4	S
480.578	0.0000	0.0000	79.647	0.00000	0.00000	22478.6	20110.2	2368.4	S
504.578	0.0000	0.0000	79.569	0.00000	0.00000	22478.6	20110.2	2368.4	S
528.578	0.0000	0.0000	79.499	0.00000	0.00000	22478.6	20110.2	2368.4	S
552.578	0.0000	0.0000	79.436	0.00000	0.00000	22478.6	20110.2	2368.4	S
576.578	0.0000	0.0000	79.378	0.00000	0.00000	22478.6	20110.2	2368.4	S
600.578	0.0000	0.0000	79.325	0.00000	0.00000	22478.6	20110.2	2368.4	S
624.578	0.0000	0.0000	79.276	0.00000	0.00000	22478.6	20110.2	2368.4	S
648.578	0.0000	0.0000	79.231	0.00000	0.00000	22478.6	20110.2	2368.4	S
672.578	0.0000	0.0000	79.188	0.00000	0.00000	22478.6	20110.2	2368.4	S
696.578	0.0000	0.0000	79.148	0.00000	0.00000	22478.6	20110.2	2368.4	S
720.578	0.0000	0.0000	79.110	0.00000	0.00000	22478.6	20110.2	2368.4	S
744.578	0.0000	0.0000	79.075	0.00000	0.00000	22478.6	20110.2	2368.4	S
768.578	0.0000	0.0000	79.041	0.00000	0.00000	22478.6	20110.2	2368.4	S
792.578	0.0000	0.0000	79.009	----	----	22478.6	20110.2	2368.4	N.A.

Detailed Results (cont,d.) *:: Scenario 23 :: FDOT 168 Hour - 168 hr - 10 yr*

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
167.733	0.0474	0.0000	81.920	0.01846	0.04003	28319.3	14798.9	4955.4	S
167.756	0.0474	0.0000	81.920	0.01846	0.03987	28323.0	14800.4	4958.6	S
167.778	0.0474	0.0000	81.920	0.01846	0.03971	28326.8	14801.9	4961.8	S
167.800	0.0474	0.0000	81.920	0.01846	0.03956	28330.6	14803.4	4964.9	S
167.822	0.0474	0.0000	81.919	0.01846	0.03940	28334.4	14804.8	4968.1	S
167.845	0.0474	0.0000	81.919	0.01847	0.03925	28338.2	14806.3	4971.2	S
167.867	0.0474	0.0000	81.919	0.01847	0.03910	28342.0	14807.8	4974.4	S
167.889	0.0474	0.0000	81.919	0.01847	0.03895	28345.8	14809.3	4977.5	S
167.911	0.0474	0.0000	81.919	0.01848	0.03880	28349.6	14810.7	4980.6	S
167.933	0.0474	0.0000	81.919	0.01848	0.03866	28353.4	14812.2	4983.7	S
167.956	0.0474	0.0000	81.919	0.01848	0.03851	28357.1	14813.7	4986.8	S
167.978	0.0474	0.0000	81.918	0.01848	0.03837	28360.9	14815.2	4989.9	S
168.000	0.0474	0.0000	81.918	0.01848	0.03822	28364.7	14816.7	4992.9	S
168.022	0.0467	0.0000	81.918	0.01848	0.03808	28368.5	14818.1	4996.0	S
168.044	0.0444	0.0000	81.918	0.01846	0.03791	28372.1	14819.6	4999.0	S
168.067	0.0398	0.0000	81.918	0.01841	0.03770	28375.5	14821.1	5002.0	S
168.089	0.0332	0.0000	81.918	0.01834	0.03740	28378.4	14822.6	5005.1	S
168.111	0.0261	0.0000	81.917	0.01825	0.03700	28380.8	14824.0	5008.0	S
168.133	0.0195	0.0000	81.917	0.01815	0.03649	28382.6	14825.5	5011.0	S
168.156	0.0139	0.0000	81.916	0.01805	0.03589	28383.9	14826.9	5013.9	S
168.178	0.0099	0.0000	81.916	0.01796	0.03522	28384.9	14828.4	5016.7	S
168.200	0.0072	0.0000	81.915	0.01788	0.03448	28385.6	14829.8	5019.5	S
168.222	0.0052	0.0000	81.914	0.01780	0.03371	28386.1	14831.2	5022.2	S
168.244	0.0037	0.0000	81.914	0.01773	0.03290	28386.4	14832.7	5024.9	S
168.267	0.0027	0.0000	81.913	0.01767	0.03207	28386.7	14834.1	5027.5	S
168.289	0.0019	0.0000	81.912	0.01761	0.03121	28386.9	14835.5	5030.0	S
168.311	0.0014	0.0000	81.912	0.01755	0.03033	28387.0	14836.9	5032.5	S
168.333	0.0010	0.0000	81.911	0.01750	0.02944	28387.1	14838.3	5034.9	S
168.356	0.0007	0.0000	81.910	0.01746	0.02853	28387.2	14839.7	5037.2	S
168.378	0.0005	0.0000	81.910	0.01741	0.02760	28387.2	14841.1	5039.4	S
168.400	0.0003	0.0000	81.909	0.01737	0.02666	28387.2	14842.5	5041.6	S
168.422	0.0002	0.0000	81.908	0.01733	0.02571	28387.3	14843.9	5043.7	S
168.444	0.0001	0.0000	81.908	0.01729	0.02474	28387.3	14845.2	5045.7	S
168.467	0.0001	0.0000	81.907	0.01726	0.02375	28387.3	14846.6	5047.7	S
168.489	0.0000	0.0000	81.906	0.01722	0.02274	28387.3	14848.0	5049.5	S
168.511	0.0000	0.0000	81.906	0.01719	0.02172	28387.3	14849.4	5051.3	S
168.533	0.0000	0.0000	81.905	0.01716	0.02067	28387.3	14850.8	5053.0	S
168.556	0.0000	0.0000	81.905	0.01713	0.01960	28387.3	14852.1	5054.6	S
168.578	0.0000	0.0000	81.904	0.01703	0.01850	28387.3	14853.5	5056.1	S
170.978	0.0000	0.0000	81.877	0.01243	0.00000	28387.3	14923.4	5136.0	S
173.378	0.0000	0.0000	81.850	0.01649	0.00000	28387.3	15068.4	5136.0	S
175.778	0.0000	0.0000	81.824	0.01595	0.00000	28387.3	15208.4	5136.0	S
178.178	0.0000	0.0000	81.799	0.01544	0.00000	28387.3	15343.9	5136.0	S
180.578	0.0000	0.0000	81.775	0.01498	0.00000	28387.3	15475.3	5136.0	S
182.978	0.0000	0.0000	81.750	0.01455	0.00000	28387.3	15602.8	5136.0	S
185.378	0.0000	0.0000	81.727	0.01415	0.00000	28387.3	15726.7	5136.0	S
187.778	0.0000	0.0000	81.704	0.01377	0.00000	28387.3	15847.3	5136.0	S
190.178	0.0000	0.0000	81.681	0.01342	0.00000	28387.3	15964.7	5136.0	S
192.578	0.0000	0.0000	81.659	0.01304	0.00000	28387.3	16079.2	5136.0	S
216.578	0.0000	0.0000	81.470	0.01009	0.00000	28387.3	17024.7	5136.0	S
240.578	0.0000	0.0000	81.301	0.00858	0.00000	28387.3	17823.2	5136.0	S
264.578	0.0000	0.0000	81.148	0.00740	0.00000	28387.3	18508.0	5136.0	S
288.578	0.0000	0.0000	81.009	0.00645	0.00000	28387.3	19101.8	5136.0	S
312.578	0.0000	0.0000	80.881	0.00567	0.00000	28387.3	19621.8	5136.0	S
336.578	0.0000	0.0000	80.763	0.00502	0.00000	28387.3	20081.1	5136.0	S
360.578	0.0000	0.0000	80.654	0.00448	0.00000	28387.3	20489.6	5136.0	S
384.578	0.0000	0.0000	80.553	0.00402	0.00000	28387.3	20855.1	5136.0	S
408.578	0.0000	0.0000	80.458	0.00362	0.00000	28387.3	21183.8	5136.0	S
432.578	0.0000	0.0000	80.369	0.00327	0.00000	28387.3	21480.6	5136.0	S
456.578	0.0000	0.0000	80.286	0.00297	0.00000	28387.3	21749.7	5136.0	S
480.578	0.0000	0.0000	80.207	0.00271	0.00000	28387.3	21994.5	5136.0	S
504.578	0.0000	0.0000	80.133	0.00248	0.00000	28387.3	22217.9	5136.0	S
528.578	0.0000	0.0000	80.062	0.00227	0.00000	28387.3	22422.5	5136.0	S
552.578	0.0000	0.0000	79.995	0.00209	0.00000	28387.3	22610.2	5136.0	S
576.578	0.0000	0.0000	79.932	0.00192	0.00000	28387.3	22783.1	5136.0	S
600.578	0.0000	0.0000	79.872	0.00178	0.00000	28387.3	22942.6	5136.0	S
624.578	0.0000	0.0000	79.815	0.00165	0.00000	28387.3	23090.2	5136.0	S
648.578	0.0000	0.0000	79.760	0.00093	0.00000	28387.3	23227.0	5136.0	S
672.578	0.0000	0.0000	79.687	0.00014	0.00000	28387.3	23251.3	5136.0	S
696.578	0.0000	0.0000	79.619	0.00000	0.00000	28387.3	23251.3	5136.0	S
720.578	0.0000	0.0000	79.558	0.00000	0.00000	28387.3	23251.3	5136.0	S
744.578	0.0000	0.0000	79.502	0.00000	0.00000	28387.3	23251.3	5136.0	S
768.578	0.0000	0.0000	79.450	0.00000	0.00000	28387.3	23251.3	5136.0	S
792.578	0.0000	0.0000	79.402	0.00000	0.00000	28387.3	23251.3	5136.0	S

Detailed Results (cont,d.) :: *Scenario 24 :: FDOT 240 Hour - 240 hr - 10 yr*

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
240.089	0.0120	0.0000	81.900	0.01489	0.00009	32612.4	17532.9	6625.5	S
240.111	0.0094	0.0000	81.900	0.01518	0.00000	32613.2	17534.1	6625.5	S
240.133	0.0070	0.0000	81.900	0.01519	0.00000	32613.9	17535.3	6625.5	S
240.156	0.0050	0.0000	81.900	0.01517	0.00000	32614.4	17536.5	6625.5	S
240.178	0.0036	0.0000	81.900	0.01515	0.00000	32614.7	17537.7	6625.5	S
240.200	0.0026	0.0000	81.899	0.01512	0.00000	32614.9	17538.9	6625.5	S
240.222	0.0019	0.0000	81.899	0.01510	0.00000	32615.1	17540.1	6625.5	S
240.244	0.0013	0.0000	81.899	0.01508	0.00000	32615.3	17541.4	6625.5	S
240.267	0.0010	0.0000	81.899	0.01506	0.00000	32615.3	17542.6	6625.5	S
240.289	0.0007	0.0000	81.899	0.01504	0.00000	32615.4	17543.8	6625.5	S
240.311	0.0005	0.0000	81.898	0.01502	0.00000	32615.5	17545.0	6625.5	S
240.333	0.0004	0.0000	81.898	0.01500	0.00000	32615.5	17546.2	6625.5	S
240.356	0.0002	0.0000	81.898	0.01498	0.00000	32615.5	17547.4	6625.5	S
240.378	0.0002	0.0000	81.898	0.01496	0.00000	32615.5	17548.6	6625.5	S
240.400	0.0001	0.0000	81.898	0.01494	0.00000	32615.5	17549.8	6625.5	S
240.422	0.0001	0.0000	81.897	0.01492	0.00000	32615.6	17551.0	6625.5	S
240.444	0.0001	0.0000	81.897	0.01491	0.00000	32615.6	17552.1	6625.5	S
240.467	0.0000	0.0000	81.897	0.01489	0.00000	32615.6	17553.3	6625.5	S
240.489	0.0000	0.0000	81.897	0.01487	0.00000	32615.6	17554.5	6625.5	S
240.511	0.0000	0.0000	81.896	0.01486	0.00000	32615.6	17555.7	6625.5	S
240.533	0.0000	0.0000	81.896	0.01485	0.00000	32615.6	17556.9	6625.5	S
240.556	0.0000	0.0000	81.896	0.01483	0.00000	32615.6	17558.1	6625.5	S
240.578	0.0000	0.0000	81.896	0.01482	0.00000	32615.6	17559.3	6625.5	S
242.978	0.0000	0.0000	81.874	0.01379	0.00000	32615.6	17680.8	6625.5	S
245.378	0.0000	0.0000	81.852	0.01329	0.00000	32615.6	17797.6	6625.5	S
247.778	0.0000	0.0000	81.831	0.01286	0.00000	32615.6	17910.5	6625.5	S
250.178	0.0000	0.0000	81.811	0.01249	0.00000	32615.6	18019.9	6625.5	S
252.578	0.0000	0.0000	81.791	0.01215	0.00000	32615.6	18126.3	6625.5	S
254.978	0.0000	0.0000	81.772	0.01185	0.00000	32615.6	18230.0	6625.5	S
257.378	0.0000	0.0000	81.752	0.01157	0.00000	32615.6	18331.1	6625.5	S
259.778	0.0000	0.0000	81.734	0.01131	0.00000	32615.6	18429.9	6625.5	S
262.178	0.0000	0.0000	81.715	0.01108	0.00000	32615.6	18526.6	6625.5	S
264.578	0.0000	0.0000	81.697	0.01082	0.00000	32615.6	18621.3	6625.5	S
288.578	0.0000	0.0000	81.537	0.00882	0.00000	32615.6	19433.9	6625.5	S
312.578	0.0000	0.0000	81.390	0.00775	0.00000	32615.6	20144.6	6625.5	S
336.578	0.0000	0.0000	81.254	0.00687	0.00000	32615.6	20772.9	6625.5	S
360.578	0.0000	0.0000	81.128	0.00613	0.00000	32615.6	21331.9	6625.5	S
384.578	0.0000	0.0000	81.011	0.00549	0.00000	32615.6	21831.8	6625.5	S
408.578	0.0000	0.0000	80.901	0.00494	0.00000	32615.6	22280.8	6625.5	S
432.578	0.0000	0.0000	80.798	0.00447	0.00000	32615.6	22685.9	6625.5	S
456.578	0.0000	0.0000	80.702	0.00405	0.00000	32615.6	23052.7	6625.5	S
480.578	0.0000	0.0000	80.611	0.00369	0.00000	32615.6	23386.1	6625.5	S
504.578	0.0000	0.0000	80.525	0.00337	0.00000	32615.6	23690.2	6625.5	S
528.578	0.0000	0.0000	80.444	0.00309	0.00000	32615.6	23968.3	6625.5	S
552.578	0.0000	0.0000	80.368	0.00284	0.00000	32615.6	24223.5	6625.5	S
576.578	0.0000	0.0000	80.295	0.00261	0.00000	32615.6	24458.3	6625.5	S
600.578	0.0000	0.0000	80.226	0.00241	0.00000	32615.6	24674.8	6625.5	S
624.578	0.0000	0.0000	80.160	0.00223	0.00000	32615.6	24875.0	6625.5	S
648.578	0.0000	0.0000	80.097	0.00207	0.00000	32615.6	25060.4	6625.5	S
672.578	0.0000	0.0000	80.037	0.00192	0.00000	32615.6	25232.5	6625.5	S
696.578	0.0000	0.0000	79.980	0.00179	0.00000	32615.6	25392.5	6625.5	S
720.578	0.0000	0.0000	79.925	0.00167	0.00000	32615.6	25541.6	6625.5	S
744.578	0.0000	0.0000	79.872	0.00156	0.00000	32615.6	25680.7	6625.5	S
768.578	0.0000	0.0000	79.822	0.00146	0.00000	32615.6	25810.8	6625.5	S
792.578	0.0000	0.0000	79.773	0.00104	0.00000	32615.6	25932.5	6625.5	S
816.578	0.0000	0.0000	79.716	0.00033	0.00000	32615.6	25990.1	6625.5	S
840.578	0.0000	0.0000	79.652	0.00000	0.00000	32615.6	25990.1	6625.5	S
864.578	0.0000	0.0000	79.596	0.00000	0.00000	32615.6	25990.1	6625.5	S
888.578	0.0000	0.0000	79.544	0.00000	0.00000	32615.6	25990.1	6625.5	S
912.578	0.0000	0.0000	79.496	0.00000	0.00000	32615.6	25990.1	6625.5	S
936.578	0.0000	0.0000	79.452	0.00000	0.00000	32615.6	25990.1	6625.5	S
960.578	0.0000	0.0000	79.410	----	----	32615.6	25990.1	6625.5	N.A.

Detailed Results :: *Scenario 25 :: FDOT 1 Hour - 1 hr - 25 yr*

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
0.000	0.0000	0.0000	77.000	0.00000	0.00000	0.0	0.0	0.0	N.A.
0.022	0.0000	0.0000	77.000	0.00000	0.00000	0.0	0.0	0.0	U
0.044	0.0000	0.0000	77.000	0.00000	0.00000	0.0	0.0	0.0	U
0.067	0.0000	0.0000	77.000	0.00000	0.00000	0.0	0.0	0.0	U
0.089	0.0000	0.0000	77.000	0.00000	0.00000	0.0	0.0	0.0	U
0.111	0.0000	0.0000	77.000	0.00000	0.00000	0.0	0.0	0.0	U
0.133	0.0000	0.0000	77.000	0.00004	0.00000	0.0	0.0	0.0	U
0.156	0.0001	0.0000	77.000	0.00071	0.00000	0.0	0.0	0.0	U
0.178	0.0025	0.0000	77.000	0.00425	0.00000	0.1	0.1	0.0	U
0.200	0.0118	0.0000	77.000	0.01721	0.00000	0.7	0.7	0.0	U
0.222	0.0427	0.0000	77.002	0.05266	0.00000	2.9	2.9	0.0	U
0.244	0.1134	0.0000	77.005	0.12449	0.00000	9.1	9.1	0.0	U
0.267	0.2425	0.0000	79.750	0.17117	0.00000	23.3	22.8	0.0	U/P
0.289	0.4316	0.0000	79.756	0.17196	0.00000	50.3	36.5	0.0	U/P
0.311	0.6772	0.0000	79.768	0.17345	0.00000	94.7	50.3	0.0	U/P
0.333	0.9864	0.0000	79.789	0.17580	0.00000	161.2	64.2	0.0	U/P
0.356	1.3763	0.0000	79.821	0.17919	0.00000	255.7	78.4	0.0	U/P
0.378	1.8458	0.0000	79.865	0.18375	0.00000	384.6	92.9	0.0	U/P
0.400	2.3587	0.0000	79.923	0.18950	0.00000	552.8	107.8	0.0	U/P
0.422	2.8705	0.0000	79.995	0.19659	0.00000	761.9	123.2	0.0	U/P
0.444	3.3495	0.0000	80.077	0.20486	0.00000	1010.7	139.3	0.0	U/P
0.467	3.7722	0.0000	80.168	0.21383	0.00000	1295.6	156.0	0.0	U/P
0.489	4.1282	0.0000	80.265	0.22326	0.00000	1611.6	173.5	0.0	U/P
0.511	4.4178	0.0000	80.366	0.23296	0.00000	1953.5	191.7	0.0	U/P
0.533	4.6366	0.0000	80.469	0.24271	0.00000	2315.6	210.8	0.0	U/P
0.556	4.7744	0.0000	80.571	0.25236	0.00000	2692.1	230.6	0.0	U/P
0.578	4.8322	0.0000	80.671	0.26177	0.00000	3076.3	251.1	0.0	U/P
0.600	4.8353	0.0000	80.769	0.27083	0.00000	3463.0	272.5	0.0	U/P
0.622	4.7850	0.0000	80.862	0.27949	0.00000	3847.9	294.5	0.0	U/P
0.644	4.6706	0.0000	80.950	0.28782	0.00000	4226.1	317.2	0.0	U/P
0.667	4.4673	0.0000	81.033	0.29591	0.00000	4591.6	340.5	0.0	U/P
0.689	4.2016	0.0000	81.109	0.30346	0.00000	4938.3	364.5	0.0	U/P
0.711	3.9157	0.0000	81.178	0.31030	0.00000	5263.0	389.1	0.0	U/P
0.733	3.6273	0.0000	81.240	0.31644	0.00000	5564.8	414.2	0.0	U/P
0.756	3.3371	0.0000	81.295	0.32194	0.00000	5843.3	439.7	0.0	U/P
0.778	3.0591	0.0000	81.345	0.32685	0.00000	6099.2	465.7	0.0	U/P
0.800	2.8010	0.0000	81.389	0.33121	0.00000	6333.6	492.0	0.0	U/P
0.822	2.5489	0.0000	81.429	0.33507	0.00000	6547.6	518.7	0.0	U/P
0.844	2.2835	0.0000	81.463	0.33841	0.00000	6740.9	545.6	0.0	U/P
0.867	1.9793	0.0000	81.493	0.34121	0.00000	6911.4	572.8	0.0	U/P
0.889	1.6521	0.0000	81.517	0.34344	0.00000	7056.6	600.2	0.0	U/P
0.911	1.3372	0.0000	81.535	0.34513	0.00000	7176.2	627.8	0.0	U/P
0.933	1.0563	0.0000	81.549	0.34634	0.00000	7272.0	655.4	0.0	U/P
0.956	0.8203	0.0000	81.559	0.34716	0.00000	7347.0	683.2	0.0	U/P
0.978	0.6418	0.0000	81.565	0.34767	0.00000	7405.5	711.0	0.0	U/P
1.000	0.5084	0.0000	81.568	0.34795	0.00000	7451.5	738.8	0.0	U/P
1.022	0.4053	0.0000	81.570	0.34805	0.00000	7488.1	766.7	0.0	U/P
1.044	0.3219	0.0000	81.570	0.34801	0.00000	7517.2	794.5	0.0	U/P
1.067	0.2523	0.0000	81.569	0.34786	0.00000	7540.1	822.3	0.0	U/P
1.089	0.1929	0.0000	81.567	0.34760	0.00000	7557.9	850.2	0.0	U/P
1.111	0.1437	0.0000	81.564	0.34727	0.00000	7571.4	877.9	0.0	U/P
1.133	0.1044	0.0000	81.561	0.34687	0.00000	7581.3	905.7	0.0	U/P
1.156	0.0741	0.0000	81.557	0.34641	0.00000	7588.5	933.4	0.0	U/P
1.178	0.0522	0.0000	81.552	0.34592	0.00000	7593.5	961.1	0.0	U/P
1.200	0.0367	0.0000	81.547	0.34540	0.00000	7597.1	988.8	0.0	U/P
1.222	0.0257	0.0000	81.542	0.34487	0.00000	7599.6	1016.4	0.0	U/P
1.244	0.0178	0.0000	81.537	0.34432	0.00000	7601.3	1044.0	0.0	U/P
1.267	0.0121	0.0000	81.532	0.34376	0.00000	7602.5	1071.5	0.0	U/P
1.289	0.0080	0.0000	81.526	0.34319	0.00000	7603.3	1099.0	0.0	U/P
1.311	0.0051	0.0000	81.521	0.34262	0.00000	7603.8	1126.4	0.0	U/P
1.333	0.0033	0.0000	81.515	0.34205	0.00000	7604.2	1153.8	0.0	U/P
1.356	0.0023	0.0000	81.510	0.34147	0.00000	7604.4	1181.1	0.0	U/P
1.378	0.0015	0.0000	81.504	0.34089	0.00000	7604.5	1208.4	0.0	U/P
1.400	0.0010	0.0000	81.499	0.34031	0.00000	7604.6	1235.7	0.0	U/P
1.422	0.0007	0.0000	81.493	0.33973	0.00000	7604.7	1262.9	0.0	U/P
1.444	0.0004	0.0000	81.488	0.33915	0.00000	7604.8	1290.0	0.0	U/P
1.467	0.0002	0.0000	81.482	0.33857	0.00000	7604.8	1317.1	0.0	U/P
1.489	0.0001	0.0000	81.477	0.33799	0.00000	7604.8	1344.2	0.0	U/P
1.511	0.0000	0.0000	81.471	0.33741	0.00000	7604.8	1371.2	0.0	U/P
1.533	0.0000	0.0000	81.466	0.33683	0.00000	7604.8	1398.2	0.0	U/P
1.556	0.0000	0.0000	81.460	0.33625	0.00000	7604.8	1425.1	0.0	U/P
1.578	0.0000	0.0000	81.454	0.33568	0.00000	7604.8	1452.0	0.0	U/P
3.978	0.0000	0.0000	80.852	0.27578	0.00000	7604.8	4089.9	0.0	U/P
6.378	0.0000	0.0000	80.249	0.12313	0.00000	7604.8	6217.5	0.0	U/P

Detailed Results (cont,d.) :: *Scenario 25 :: FDOT 1 Hour - 1 hr - 25 yr*

Elapsed Time (hours)	Inflow Rate (ft^3/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft^3/s)	Overflow Discharge (ft^3/s)	Cumulative Inflow Volume (ft^3)	Cumulative Infiltration Volume (ft^3)	Cumulative Discharge Volume (ft^3)	Flow Type
8.778	0.0000	0.0000	----	----	----	7604.8	7604.8	0.0	dry
11.178	0.0000	0.0000	----	----	----	7604.8	7604.8	0.0	dry
13.578	0.0000	0.0000	----	----	----	7604.8	7604.8	0.0	dry
15.978	0.0000	0.0000	----	----	----	7604.8	7604.8	0.0	dry
18.378	0.0000	0.0000	----	----	----	7604.8	7604.8	0.0	dry
20.778	0.0000	0.0000	----	----	----	7604.8	7604.8	0.0	dry
23.178	0.0000	0.0000	----	----	----	7604.8	7604.8	0.0	dry
25.578	0.0000	0.0000	----	----	----	7604.8	7604.8	0.0	dry
49.578	0.0000	0.0000	----	----	----	7604.8	7604.8	0.0	dry
73.578	0.0000	0.0000	----	----	----	7604.8	7604.8	0.0	dry
97.578	0.0000	0.0000	----	----	----	7604.8	7604.8	0.0	dry
121.578	0.0000	0.0000	----	----	----	7604.8	7604.8	0.0	dry
145.578	0.0000	0.0000	----	----	----	7604.8	7604.8	0.0	dry
169.578	0.0000	0.0000	----	----	----	7604.8	7604.8	0.0	dry
193.578	0.0000	0.0000	----	----	----	7604.8	7604.8	0.0	dry
217.578	0.0000	0.0000	----	----	----	7604.8	7604.8	0.0	dry
241.578	0.0000	0.0000	----	----	----	7604.8	7604.8	0.0	dry
265.578	0.0000	0.0000	----	----	----	7604.8	7604.8	0.0	dry
289.578	0.0000	0.0000	----	----	----	7604.8	7604.8	0.0	dry
313.578	0.0000	0.0000	----	----	----	7604.8	7604.8	0.0	dry
337.578	0.0000	0.0000	----	----	----	7604.8	7604.8	0.0	dry
361.578	0.0000	0.0000	----	----	----	7604.8	7604.8	0.0	dry
385.578	0.0000	0.0000	----	----	----	7604.8	7604.8	0.0	dry
409.578	0.0000	0.0000	----	----	----	7604.8	7604.8	0.0	dry
433.578	0.0000	0.0000	----	----	----	7604.8	7604.8	0.0	dry
457.578	0.0000	0.0000	----	----	----	7604.8	7604.8	0.0	dry
481.578	0.0000	0.0000	----	----	----	7604.8	7604.8	0.0	dry
505.578	0.0000	0.0000	----	----	----	7604.8	7604.8	0.0	dry
529.578	0.0000	0.0000	----	----	----	7604.8	7604.8	0.0	dry
553.578	0.0000	0.0000	----	----	----	7604.8	7604.8	0.0	dry
577.578	0.0000	0.0000	----	----	----	7604.8	7604.8	0.0	dry
601.578	0.0000	0.0000	----	----	----	7604.8	7604.8	0.0	dry
625.578	0.0000	0.0000	----	----	----	7604.8	7604.8	0.0	dry
649.578	0.0000	0.0000	----	----	----	7604.8	7604.8	0.0	dry
673.578	0.0000	0.0000	----	----	----	7604.8	7604.8	0.0	dry
697.578	0.0000	0.0000	----	----	----	7604.8	7604.8	0.0	dry
721.578	0.0000	0.0000	----	----	----	7604.8	7604.8	0.0	dry

Detailed Results (cont,d.) :: *Scenario 26 :: FDOT 2 Hour - 2 hr - 25 yr*

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
1.644	0.6633	0.0000	81.710	0.36291	0.00000	8955.4	1517.8	0.0	U/P
1.667	0.6433	0.0000	81.715	0.36336	0.00000	9007.7	1546.8	0.0	U/P
1.689	0.6176	0.0000	81.719	0.36376	0.00000	9058.1	1575.9	0.0	U/P
1.711	0.5910	0.0000	81.722	0.36413	0.00000	9106.4	1605.0	0.0	U/P
1.733	0.5667	0.0000	81.726	0.36445	0.00000	9152.8	1634.2	0.0	U/P
1.756	0.5465	0.0000	81.729	0.36474	0.00000	9197.3	1663.4	0.0	U/P
1.778	0.5321	0.0000	81.731	0.36501	0.00000	9240.4	1692.5	0.0	U/P
1.800	0.5220	0.0000	81.734	0.36526	0.00000	9282.6	1721.8	0.0	U/P
1.822	0.5123	0.0000	81.736	0.36549	0.00000	9324.0	1751.0	0.0	U/P
1.844	0.4993	0.0000	81.738	0.36570	0.00000	9364.4	1780.2	0.0	U/P
1.867	0.4792	0.0000	81.740	0.36588	0.00000	9403.6	1809.5	0.0	U/P
1.889	0.4534	0.0000	81.742	0.36602	0.00000	9440.9	1838.8	0.0	U/P
1.911	0.4267	0.0000	81.743	0.36612	0.00000	9476.1	1868.1	0.0	U/P
1.933	0.4023	0.0000	81.744	0.36617	0.00000	9509.2	1897.4	0.0	U/P
1.956	0.3820	0.0000	81.744	0.36620	0.00000	9540.6	1926.6	0.0	U/P
1.978	0.3674	0.0000	81.744	0.36621	0.00000	9570.6	1955.9	0.0	U/P
2.000	0.3573	0.0000	81.744	0.36619	0.00000	9599.6	1985.2	0.0	U/P
2.022	0.3450	0.0000	81.744	0.36615	0.00000	9627.7	2014.5	0.0	U/P
2.044	0.3243	0.0000	81.743	0.36608	0.00000	9654.4	2043.8	0.0	U/P
2.067	0.2877	0.0000	81.742	0.36595	0.00000	9678.9	2073.1	0.0	U/P
2.089	0.2387	0.0000	81.741	0.36575	0.00000	9700.0	2102.4	0.0	U/P
2.111	0.1870	0.0000	81.738	0.36547	0.00000	9717.0	2131.6	0.0	U/P
2.133	0.1394	0.0000	81.735	0.36511	0.00000	9730.0	2160.9	0.0	U/P
2.156	0.0997	0.0000	81.732	0.36469	0.00000	9739.6	2190.0	0.0	U/P
2.178	0.0712	0.0000	81.727	0.36423	0.00000	9746.4	2219.2	0.0	U/P
2.200	0.0513	0.0000	81.723	0.36373	0.00000	9751.3	2248.3	0.0	U/P
2.222	0.0370	0.0000	81.718	0.36321	0.00000	9754.9	2277.4	0.0	U/P
2.244	0.0265	0.0000	81.713	0.36268	0.00000	9757.4	2306.4	0.0	U/P
2.267	0.0190	0.0000	81.708	0.36213	0.00000	9759.2	2335.4	0.0	U/P
2.289	0.0135	0.0000	81.702	0.36157	0.00000	9760.5	2364.4	0.0	U/P
2.311	0.0096	0.0000	81.697	0.36100	0.00000	9761.5	2393.3	0.0	U/P
2.333	0.0068	0.0000	81.691	0.36043	0.00000	9762.1	2422.1	0.0	U/P
2.356	0.0048	0.0000	81.686	0.35986	0.00000	9762.6	2451.0	0.0	U/P
2.378	0.0034	0.0000	81.681	0.35929	0.00000	9762.9	2479.7	0.0	U/P
2.400	0.0023	0.0000	81.675	0.35871	0.00000	9763.1	2508.4	0.0	U/P
2.422	0.0016	0.0000	81.669	0.35813	0.00000	9763.3	2537.1	0.0	U/P
2.444	0.0010	0.0000	81.664	0.35755	0.00000	9763.4	2565.7	0.0	U/P
2.467	0.0006	0.0000	81.658	0.35697	0.00000	9763.5	2594.3	0.0	U/P
2.489	0.0003	0.0000	81.653	0.35639	0.00000	9763.5	2622.9	0.0	U/P
2.511	0.0001	0.0000	81.647	0.35581	0.00000	9763.5	2651.3	0.0	U/P
2.533	0.0000	0.0000	81.642	0.35523	0.00000	9763.5	2679.8	0.0	U/P
2.556	0.0000	0.0000	81.636	0.35465	0.00000	9763.5	2708.2	0.0	U/P
2.578	0.0000	0.0000	81.631	0.35408	0.00000	9763.5	2736.5	0.0	U/P
4.978	0.0000	0.0000	81.028	0.32049	0.00000	9763.5	5532.3	0.0	U/P
7.378	0.0000	0.0000	80.282	0.17753	0.00000	9763.5	8274.6	0.0	U/S
9.778	0.0000	0.0000	80.176	0.03190	0.00000	9763.5	8600.0	0.0	S
12.178	0.0000	0.0000	80.100	0.02339	0.00000	9763.5	8825.8	0.0	S
14.578	0.0000	0.0000	80.038	0.01901	0.00000	9763.5	9004.2	0.0	S
16.978	0.0000	0.0000	79.984	0.01622	0.00000	9763.5	9154.2	0.0	S
19.378	0.0000	0.0000	79.936	0.01424	0.00000	9763.5	9284.5	0.0	S
21.778	0.0000	0.0000	79.893	0.01274	0.00000	9763.5	9400.3	0.0	S
24.178	0.0000	0.0000	79.853	0.01155	0.00000	9763.5	9504.6	0.0	S
26.578	0.0000	0.0000	79.816	0.01019	0.00000	9763.5	9599.8	0.0	S
50.578	0.0000	0.0000	79.488	0.00095	0.00000	9763.5	9763.5	0.0	S
74.578	0.0000	0.0000	79.247	0.00000	0.00000	9763.5	9763.5	0.0	S
98.578	0.0000	0.0000	79.075	0.00000	0.00000	9763.5	9763.5	0.0	S
122.578	0.0000	0.0000	78.943	0.00000	0.00000	9763.5	9763.5	0.0	S
146.578	0.0000	0.0000	78.838	0.00000	0.00000	9763.5	9763.5	0.0	S
170.578	0.0000	0.0000	78.752	0.00000	0.00000	9763.5	9763.5	0.0	S
194.578	0.0000	0.0000	78.678	0.00000	0.00000	9763.5	9763.5	0.0	S
218.578	0.0000	0.0000	78.615	0.00000	0.00000	9763.5	9763.5	0.0	S
242.578	0.0000	0.0000	78.559	0.00000	0.00000	9763.5	9763.5	0.0	S
266.578	0.0000	0.0000	78.510	0.00000	0.00000	9763.5	9763.5	0.0	S
290.578	0.0000	0.0000	78.466	0.00000	0.00000	9763.5	9763.5	0.0	S
314.578	0.0000	0.0000	78.426	0.00000	0.00000	9763.5	9763.5	0.0	S
338.578	0.0000	0.0000	78.389	0.00000	0.00000	9763.5	9763.5	0.0	S
362.578	0.0000	0.0000	78.356	0.00000	0.00000	9763.5	9763.5	0.0	S
386.578	0.0000	0.0000	78.325	0.00000	0.00000	9763.5	9763.5	0.0	S
410.578	0.0000	0.0000	78.297	0.00000	0.00000	9763.5	9763.5	0.0	S
434.578	0.0000	0.0000	78.270	0.00000	0.00000	9763.5	9763.5	0.0	S
458.578	0.0000	0.0000	78.245	0.00000	0.00000	9763.5	9763.5	0.0	S
482.578	0.0000	0.0000	78.222	0.00000	0.00000	9763.5	9763.5	0.0	S
506.578	0.0000	0.0000	78.201	0.00000	0.00000	9763.5	9763.5	0.0	S
530.578	0.0000	0.0000	78.180	0.00000	0.00000	9763.5	9763.5	0.0	S

Detailed Results (cont,d.) :: *Scenario 27 :: FDOT 4 Hour - 4 hr - 25 yr*

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
3.289	0.4283	0.0000	81.932	0.38587	0.05025	11349.0	2683.7	36.2	U/P
3.311	0.4200	0.0000	81.932	0.38585	0.05011	11382.9	2714.6	40.2	U/P
3.333	0.4141	0.0000	81.931	0.38582	0.04990	11416.3	2745.4	44.2	U/P
3.356	0.4099	0.0000	81.931	0.38578	0.04963	11449.2	2776.3	48.2	U/P
3.378	0.4068	0.0000	81.931	0.38574	0.04931	11481.9	2807.2	52.2	U/P
3.400	0.4046	0.0000	81.930	0.38569	0.04897	11514.4	2838.0	56.1	U/P
3.422	0.4030	0.0000	81.930	0.38564	0.04861	11546.7	2868.9	60.0	U/P
3.444	0.4019	0.0000	81.929	0.38560	0.04824	11578.9	2899.7	63.9	U/P
3.467	0.4010	0.0000	81.929	0.38555	0.04785	11611.0	2930.6	67.7	U/P
3.489	0.4004	0.0000	81.928	0.38550	0.04746	11643.0	2961.4	71.5	U/P
3.511	0.3982	0.0000	81.928	0.38544	0.04705	11675.0	2992.3	75.3	U/P
3.533	0.3906	0.0000	81.927	0.38537	0.04659	11706.5	3023.1	79.1	U/P
3.556	0.3732	0.0000	81.927	0.38528	0.04597	11737.1	3053.9	82.8	U/P
3.578	0.3446	0.0000	81.925	0.38515	0.04506	11765.8	3084.7	86.4	U/P
3.600	0.3099	0.0000	81.924	0.38497	0.04373	11792.0	3115.5	90.0	U/P
3.622	0.2752	0.0000	81.922	0.38474	0.04192	11815.4	3146.3	93.4	U/P
3.644	0.2445	0.0000	81.920	0.38447	0.03958	11836.2	3177.1	96.6	U/P
3.667	0.2204	0.0000	81.917	0.38416	0.03670	11854.8	3207.8	99.7	U/P
3.689	0.2033	0.0000	81.914	0.38384	0.03329	11871.7	3238.6	102.5	U/P
3.711	0.1913	0.0000	81.911	0.38350	0.02927	11887.5	3269.3	105.0	U/P
3.733	0.1825	0.0000	81.908	0.38315	0.02446	11902.4	3299.9	107.1	U/P
3.756	0.1762	0.0000	81.904	0.38280	0.01838	11916.8	3330.6	108.9	U/P
3.778	0.1716	0.0000	81.901	0.38245	0.00897	11930.7	3361.2	110.0	U/P
3.800	0.1683	0.0000	81.898	0.38211	0.00000	11944.3	3391.8	110.3	U/P
3.822	0.1659	0.0000	81.895	0.38178	0.00000	11957.7	3422.3	110.3	U/P
3.844	0.1642	0.0000	81.892	0.38145	0.00000	11970.9	3452.8	110.3	U/P
3.867	0.1630	0.0000	81.888	0.38112	0.00000	11984.0	3483.3	110.3	U/P
3.889	0.1621	0.0000	81.885	0.38079	0.00000	11997.0	3513.8	110.3	U/P
3.911	0.1615	0.0000	81.882	0.38045	0.00000	12009.9	3544.3	110.3	U/P
3.933	0.1610	0.0000	81.879	0.38012	0.00000	12022.8	3574.7	110.3	U/P
3.956	0.1607	0.0000	81.876	0.37978	0.00000	12035.7	3605.1	110.3	U/P
3.978	0.1604	0.0000	81.872	0.37945	0.00000	12048.5	3635.5	110.3	U/P
4.000	0.1602	0.0000	81.869	0.37911	0.00000	12061.3	3665.8	110.3	U/P
4.022	0.1577	0.0000	81.866	0.37877	0.00000	12074.1	3696.1	110.3	U/P
4.044	0.1502	0.0000	81.863	0.37842	0.00000	12086.4	3726.4	110.3	U/P
4.067	0.1344	0.0000	81.859	0.37804	0.00000	12097.8	3756.7	110.3	U/P
4.089	0.1121	0.0000	81.855	0.37763	0.00000	12107.6	3786.9	110.3	U/P
4.111	0.0881	0.0000	81.851	0.37718	0.00000	12115.6	3817.1	110.3	U/P
4.133	0.0658	0.0000	81.847	0.37671	0.00000	12121.8	3847.2	110.3	U/P
4.156	0.0470	0.0000	81.842	0.37620	0.00000	12126.3	3877.4	110.3	U/P
4.178	0.0336	0.0000	81.837	0.37567	0.00000	12129.5	3907.4	110.3	U/P
4.200	0.0242	0.0000	81.832	0.37513	0.00000	12131.8	3937.5	110.3	U/P
4.222	0.0175	0.0000	81.827	0.37458	0.00000	12133.5	3967.5	110.3	U/P
4.244	0.0125	0.0000	81.822	0.37402	0.00000	12134.7	3997.4	110.3	U/P
4.267	0.0090	0.0000	81.816	0.37345	0.00000	12135.6	4027.3	110.3	U/P
4.289	0.0065	0.0000	81.811	0.37288	0.00000	12136.2	4057.1	110.3	U/P
4.311	0.0046	0.0000	81.805	0.37231	0.00000	12136.6	4087.0	110.3	U/P
4.333	0.0033	0.0000	81.800	0.37173	0.00000	12136.9	4116.7	110.3	U/P
4.356	0.0023	0.0000	81.794	0.37116	0.00000	12137.2	4146.4	110.3	U/P
4.378	0.0016	0.0000	81.789	0.37058	0.00000	12137.3	4176.1	110.3	U/P
4.400	0.0011	0.0000	81.783	0.37000	0.00000	12137.4	4205.7	110.3	U/P
4.422	0.0008	0.0000	81.778	0.36942	0.00000	12137.5	4235.3	110.3	U/P
4.444	0.0005	0.0000	81.772	0.36884	0.00000	12137.6	4264.8	110.3	U/P
4.467	0.0003	0.0000	81.766	0.36826	0.00000	12137.6	4294.3	110.3	U/P
4.489	0.0001	0.0000	81.761	0.36768	0.00000	12137.6	4323.8	110.3	U/P
4.511	0.0000	0.0000	81.755	0.36710	0.00000	12137.6	4353.1	110.3	U/P
4.533	0.0000	0.0000	81.750	0.36652	0.00000	12137.6	4382.5	110.3	U/P
4.556	0.0000	0.0000	81.744	0.36594	0.00000	12137.6	4411.8	110.3	U/P
4.578	0.0000	0.0000	81.739	0.36536	0.00000	12137.6	4441.0	110.3	U/P
6.978	0.0000	0.0000	81.136	0.23258	0.00000	12137.6	7334.4	110.3	U/P
9.378	0.0000	0.0000	80.865	0.08943	0.00000	12137.6	8460.1	110.3	U/S
11.778	0.0000	0.0000	80.757	0.04132	0.00000	12137.6	8879.7	110.3	S
14.178	0.0000	0.0000	80.679	0.03065	0.00000	12137.6	9174.2	110.3	S
16.578	0.0000	0.0000	80.615	0.02514	0.00000	12137.6	9409.4	110.3	S
18.978	0.0000	0.0000	80.559	0.02163	0.00000	12137.6	9608.6	110.3	S
21.378	0.0000	0.0000	80.509	0.01912	0.00000	12137.6	9783.1	110.3	S
23.778	0.0000	0.0000	80.464	0.01721	0.00000	12137.6	9939.0	110.3	S
26.178	0.0000	0.0000	80.422	0.01569	0.00000	12137.6	10080.4	110.3	S
28.578	0.0000	0.0000	80.383	0.01452	0.00000	12137.6	10210.1	110.3	S
52.578	0.0000	0.0000	80.117	0.00817	0.00000	12137.6	11039.4	110.3	S
76.578	0.0000	0.0000	79.909	0.00572	0.00000	12137.6	11621.9	110.3	S
100.578	0.0000	0.0000	79.732	0.00235	0.00000	12137.6	12027.3	110.3	S
124.578	0.0000	0.0000	79.526	0.00000	0.00000	12137.6	12027.3	110.3	S
148.578	0.0000	0.0000	79.369	0.00000	0.00000	12137.6	12027.3	110.3	S

Detailed Results (cont,d.) :: *Scenario 28 :: FDOT 8 Hour - 8 hr - 25 yr*

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
8.222	0.0159	0.0000	81.654	0.00002	0.00000	15025.5	7682.6	195.8	S
8.244	0.0114	0.0000	81.654	0.00002	0.00000	15026.5	7682.6	195.8	S
8.267	0.0082	0.0000	81.654	0.00003	0.00000	15027.3	7682.6	195.8	S
8.289	0.0058	0.0000	81.655	0.00003	0.00000	15027.9	7682.6	195.8	S
8.311	0.0042	0.0000	81.655	0.00003	0.00000	15028.3	7682.6	195.8	S
8.333	0.0030	0.0000	81.655	0.00003	0.00000	15028.6	7682.6	195.8	S
8.356	0.0021	0.0000	81.655	0.00004	0.00000	15028.8	7682.6	195.8	S
8.378	0.0015	0.0000	81.655	0.00004	0.00000	15028.9	7682.6	195.8	S
8.400	0.0010	0.0000	81.655	0.00004	0.00000	15029.0	7682.6	195.8	S
8.422	0.0007	0.0000	81.655	0.00004	0.00000	15029.1	7682.6	195.8	S
8.444	0.0005	0.0000	81.655	0.00005	0.00000	15029.1	7682.6	195.8	S
8.467	0.0003	0.0000	81.655	0.00005	0.00000	15029.2	7682.6	195.8	S
8.489	0.0001	0.0000	81.655	0.00006	0.00000	15029.2	7682.6	195.8	S
8.511	0.0000	0.0000	81.655	0.00006	0.00000	15029.2	7682.6	195.8	S
8.533	0.0000	0.0000	81.655	0.00006	0.00000	15029.2	7682.6	195.8	S
8.556	0.0000	0.0000	81.655	0.00007	0.00000	15029.2	7682.6	195.8	S
8.578	0.0000	0.0000	81.655	0.00112	0.00000	15029.2	7682.6	195.8	S
10.978	0.0000	0.0000	81.457	0.08738	0.00000	15029.2	8668.8	195.8	S
13.378	0.0000	0.0000	81.347	0.05178	0.00000	15029.2	9192.6	195.8	S
15.778	0.0000	0.0000	81.266	0.03875	0.00000	15029.2	9563.7	195.8	S
18.178	0.0000	0.0000	81.200	0.03200	0.00000	15029.2	9862.2	195.8	S
20.578	0.0000	0.0000	81.142	0.02766	0.00000	15029.2	10116.6	195.8	S
22.978	0.0000	0.0000	81.090	0.02456	0.00000	15029.2	10340.3	195.8	S
25.378	0.0000	0.0000	81.043	0.02219	0.00000	15029.2	10541.0	195.8	S
27.778	0.0000	0.0000	80.999	0.02031	0.00000	15029.2	10723.7	195.8	S
30.178	0.0000	0.0000	80.958	0.01876	0.00000	15029.2	10891.9	195.8	S
32.578	0.0000	0.0000	80.920	0.01751	0.00000	15029.2	11047.9	195.8	S
56.578	0.0000	0.0000	80.649	0.01042	0.00000	15029.2	12090.6	195.8	S
80.578	0.0000	0.0000	80.433	0.00776	0.00000	15029.2	12848.9	195.8	S
104.578	0.0000	0.0000	80.254	0.00605	0.00000	15029.2	13430.9	195.8	S
128.578	0.0000	0.0000	80.100	0.00488	0.00000	15029.2	13894.8	195.8	S
152.578	0.0000	0.0000	79.966	0.00404	0.00000	15029.2	14274.7	195.8	S
176.578	0.0000	0.0000	79.846	0.00323	0.00000	15029.2	14592.4	195.8	S
200.578	0.0000	0.0000	79.733	0.00139	0.00000	15029.2	14833.4	195.8	S
224.578	0.0000	0.0000	79.592	0.00000	0.00000	15029.2	14833.4	195.8	S
248.578	0.0000	0.0000	79.476	0.00000	0.00000	15029.2	14833.4	195.8	S
272.578	0.0000	0.0000	79.377	0.00000	0.00000	15029.2	14833.4	195.8	S
296.578	0.0000	0.0000	79.291	0.00000	0.00000	15029.2	14833.4	195.8	S
320.578	0.0000	0.0000	79.215	0.00000	0.00000	15029.2	14833.4	195.8	S
344.578	0.0000	0.0000	79.147	0.00000	0.00000	15029.2	14833.4	195.8	S
368.578	0.0000	0.0000	79.086	0.00000	0.00000	15029.2	14833.4	195.8	S
392.578	0.0000	0.0000	79.030	0.00000	0.00000	15029.2	14833.4	195.8	S
416.578	0.0000	0.0000	78.979	0.00000	0.00000	15029.2	14833.4	195.8	S
440.578	0.0000	0.0000	78.932	0.00000	0.00000	15029.2	14833.4	195.8	S
464.578	0.0000	0.0000	78.889	0.00000	0.00000	15029.2	14833.4	195.8	S
488.578	0.0000	0.0000	78.848	0.00000	0.00000	15029.2	14833.4	195.8	S
512.578	0.0000	0.0000	78.811	0.00000	0.00000	15029.2	14833.4	195.8	S
536.578	0.0000	0.0000	78.776	0.00000	0.00000	15029.2	14833.4	195.8	S
560.578	0.0000	0.0000	78.743	0.00000	0.00000	15029.2	14833.4	195.8	S
584.578	0.0000	0.0000	78.712	0.00000	0.00000	15029.2	14833.4	195.8	S
608.578	0.0000	0.0000	78.683	0.00000	0.00000	15029.2	14833.4	195.8	S
632.578	0.0000	0.0000	78.655	0.00000	0.00000	15029.2	14833.4	195.8	S
656.578	0.0000	0.0000	78.629	0.00000	0.00000	15029.2	14833.4	195.8	S
680.578	0.0000	0.0000	78.605	0.00000	0.00000	15029.2	14833.4	195.8	S
704.578	0.0000	0.0000	78.581	0.00000	0.00000	15029.2	14833.4	195.8	S
728.578	0.0000	0.0000	78.559	----	----	15029.2	14833.4	195.8	N.A.

Detailed Results (cont,d.) :: *Scenario 29 :: FDOT 24 Hour - 24 hr - 25 yr*

Elapsed Time (hours)	Inflow Rate (ft^3/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft^3/s)	Overflow Discharge (ft^3/s)	Cumulative Inflow Volume (ft^3)	Cumulative Infiltration Volume (ft^3)	Cumulative Discharge Volume (ft^3)	Flow Type
34.178	0.0000	0.0000	81.716	0.03090	0.00000	21411.4	10644.4	3299.1	S
36.578	0.0000	0.0000	81.667	0.02822	0.00000	21411.4	10898.8	3299.1	S
38.978	0.0000	0.0000	81.621	0.02597	0.00000	21411.4	11132.0	3299.1	S
41.378	0.0000	0.0000	81.579	0.02409	0.00000	21411.4	11347.6	3299.1	S
43.778	0.0000	0.0000	81.538	0.02249	0.00000	21411.4	11548.3	3299.1	S
46.178	0.0000	0.0000	81.500	0.02111	0.00000	21411.4	11736.2	3299.1	S
48.578	0.0000	0.0000	81.464	0.01992	0.00000	21411.4	11913.1	3299.1	S
72.578	0.0000	0.0000	81.196	0.01263	0.00000	21411.4	13157.8	3299.1	S
96.578	0.0000	0.0000	80.976	0.00971	0.00000	21411.4	14095.9	3299.1	S
120.578	0.0000	0.0000	80.791	0.00778	0.00000	21411.4	14836.0	3299.1	S
144.578	0.0000	0.0000	80.630	0.00641	0.00000	21411.4	15439.6	3299.1	S
168.578	0.0000	0.0000	80.487	0.00540	0.00000	21411.4	15943.7	3299.1	S
192.578	0.0000	0.0000	80.360	0.00462	0.00000	21411.4	16372.5	3299.1	S
216.578	0.0000	0.0000	80.244	0.00400	0.00000	21411.4	16742.2	3299.1	S
240.578	0.0000	0.0000	80.137	0.00350	0.00000	21411.4	17064.5	3299.1	S
264.578	0.0000	0.0000	80.040	0.00309	0.00000	21411.4	17347.7	3299.1	S
288.578	0.0000	0.0000	79.949	0.00275	0.00000	21411.4	17598.6	3299.1	S
312.578	0.0000	0.0000	79.865	0.00245	0.00000	21411.4	17822.2	3299.1	S
336.578	0.0000	0.0000	79.786	0.00168	0.00000	21411.4	18022.5	3299.1	S
360.578	0.0000	0.0000	79.695	0.00052	0.00000	21411.4	18112.3	3299.1	S
384.578	0.0000	0.0000	79.599	0.00000	0.00000	21411.4	18112.3	3299.1	S
408.578	0.0000	0.0000	79.515	0.00000	0.00000	21411.4	18112.3	3299.1	S
432.578	0.0000	0.0000	79.441	0.00000	0.00000	21411.4	18112.3	3299.1	S
456.578	0.0000	0.0000	79.375	0.00000	0.00000	21411.4	18112.3	3299.1	S
480.578	0.0000	0.0000	79.315	0.00000	0.00000	21411.4	18112.3	3299.1	S
504.578	0.0000	0.0000	79.259	0.00000	0.00000	21411.4	18112.3	3299.1	S
528.578	0.0000	0.0000	79.208	0.00000	0.00000	21411.4	18112.3	3299.1	S
552.578	0.0000	0.0000	79.161	0.00000	0.00000	21411.4	18112.3	3299.1	S
576.578	0.0000	0.0000	79.117	0.00000	0.00000	21411.4	18112.3	3299.1	S
600.578	0.0000	0.0000	79.076	0.00000	0.00000	21411.4	18112.3	3299.1	S
624.578	0.0000	0.0000	79.038	0.00000	0.00000	21411.4	18112.3	3299.1	S
648.578	0.0000	0.0000	79.002	0.00000	0.00000	21411.4	18112.3	3299.1	S
672.578	0.0000	0.0000	78.968	0.00000	0.00000	21411.4	18112.3	3299.1	S
696.578	0.0000	0.0000	78.935	0.00000	0.00000	21411.4	18112.3	3299.1	S
720.578	0.0000	0.0000	78.905	0.00000	0.00000	21411.4	18112.3	3299.1	S
744.578	0.0000	0.0000	78.876	----	----	21411.4	18112.3	3299.1	N.A.

Detailed Results (cont,d.) :: *Scenario 30 :: FDOT 72 Hour - 72 hr - 25 yr*

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
72.356	0.0006	0.0000	81.920	0.02051	0.04001	28387.2	12355.1	7467.2	S
72.378	0.0004	0.0000	81.919	0.02047	0.03914	28387.2	12356.8	7470.4	S
72.400	0.0003	0.0000	81.918	0.02045	0.03826	28387.2	12358.4	7473.5	S
72.422	0.0002	0.0000	81.918	0.02042	0.03738	28387.3	12360.0	7476.5	S
72.444	0.0001	0.0000	81.917	0.02039	0.03648	28387.3	12361.7	7479.5	S
72.467	0.0001	0.0000	81.916	0.02037	0.03558	28387.3	12363.3	7482.4	S
72.489	0.0000	0.0000	81.915	0.02035	0.03467	28387.3	12364.9	7485.2	S
72.511	0.0000	0.0000	81.914	0.02033	0.03375	28387.3	12366.6	7487.9	S
72.533	0.0000	0.0000	81.914	0.02031	0.03282	28387.3	12368.2	7490.6	S
72.556	0.0000	0.0000	81.913	0.02030	0.03188	28387.3	12369.8	7493.2	S
72.578	0.0000	0.0000	81.912	0.02016	0.03093	28387.3	12371.4	7495.7	S
74.978	0.0000	0.0000	81.879	0.01329	0.00000	28387.3	12420.8	7629.3	S
77.378	0.0000	0.0000	81.846	0.02065	0.00000	28387.3	12601.1	7629.3	S
79.778	0.0000	0.0000	81.813	0.02016	0.00000	28387.3	12777.6	7629.3	S
82.178	0.0000	0.0000	81.781	0.01961	0.00000	28387.3	12949.5	7629.3	S
84.578	0.0000	0.0000	81.749	0.01902	0.00000	28387.3	13116.4	7629.3	S
86.978	0.0000	0.0000	81.718	0.01841	0.00000	28387.3	13278.1	7629.3	S
89.378	0.0000	0.0000	81.688	0.01780	0.00000	28387.3	13434.5	7629.3	S
91.778	0.0000	0.0000	81.659	0.01720	0.00000	28387.3	13585.7	7629.3	S
94.178	0.0000	0.0000	81.630	0.01661	0.00000	28387.3	13731.7	7629.3	S
96.578	0.0000	0.0000	81.603	0.01599	0.00000	28387.3	13872.7	7629.3	S
120.578	0.0000	0.0000	81.378	0.01149	0.00000	28387.3	14971.4	7629.3	S
144.578	0.0000	0.0000	81.184	0.00937	0.00000	28387.3	15857.9	7629.3	S
168.578	0.0000	0.0000	81.013	0.00782	0.00000	28387.3	16590.8	7629.3	S
192.578	0.0000	0.0000	80.861	0.00663	0.00000	28387.3	17208.4	7629.3	S
216.578	0.0000	0.0000	80.724	0.00571	0.00000	28387.3	17737.0	7629.3	S
240.578	0.0000	0.0000	80.599	0.00497	0.00000	28387.3	18195.3	7629.3	S
264.578	0.0000	0.0000	80.485	0.00437	0.00000	28387.3	18596.6	7629.3	S
288.578	0.0000	0.0000	80.380	0.00388	0.00000	28387.3	18951.2	7629.3	S
312.578	0.0000	0.0000	80.282	0.00346	0.00000	28387.3	19266.5	7629.3	S
336.578	0.0000	0.0000	80.191	0.00310	0.00000	28387.3	19548.7	7629.3	S
360.578	0.0000	0.0000	80.106	0.00279	0.00000	28387.3	19802.3	7629.3	S
384.578	0.0000	0.0000	80.026	0.00252	0.00000	28387.3	20031.1	7629.3	S
408.578	0.0000	0.0000	79.951	0.00229	0.00000	28387.3	20238.4	7629.3	S
432.578	0.0000	0.0000	79.881	0.00208	0.00000	28387.3	20426.7	7629.3	S
456.578	0.0000	0.0000	79.814	0.00190	0.00000	28387.3	20598.5	7629.3	S
480.578	0.0000	0.0000	79.751	0.00092	0.00000	28387.3	20755.7	7629.3	S
504.578	0.0000	0.0000	79.664	0.00001	0.00000	28387.3	20758.0	7629.3	S
528.578	0.0000	0.0000	79.587	0.00000	0.00000	28387.3	20758.0	7629.3	S
552.578	0.0000	0.0000	79.520	0.00000	0.00000	28387.3	20758.0	7629.3	S
576.578	0.0000	0.0000	79.458	0.00000	0.00000	28387.3	20758.0	7629.3	S
600.578	0.0000	0.0000	79.402	0.00000	0.00000	28387.3	20758.0	7629.3	S
624.578	0.0000	0.0000	79.350	0.00000	0.00000	28387.3	20758.0	7629.3	S
648.578	0.0000	0.0000	79.302	0.00000	0.00000	28387.3	20758.0	7629.3	S
672.578	0.0000	0.0000	79.257	0.00000	0.00000	28387.3	20758.0	7629.3	S
696.578	0.0000	0.0000	79.215	0.00000	0.00000	28387.3	20758.0	7629.3	S
720.578	0.0000	0.0000	79.176	0.00000	0.00000	28387.3	20758.0	7629.3	S
744.578	0.0000	0.0000	79.138	0.00000	0.00000	28387.3	20758.0	7629.3	S
768.578	0.0000	0.0000	79.103	0.00000	0.00000	28387.3	20758.0	7629.3	S
792.578	0.0000	0.0000	79.070	----	----	28387.3	20758.0	7629.3	N.A.

Detailed Results (cont,d.) *:: Scenario 31 :: FDOT 168 Hour - 168 hr - 25 yr*

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
159.511	0.2992	0.0000	82.426	0.03056	0.20469	31739.8	15500.4	4676.4	S
159.533	0.2992	0.0000	82.427	0.03055	0.20484	31763.7	15502.8	4692.7	S
159.556	0.2992	0.0000	82.427	0.03055	0.20500	31787.6	15505.2	4709.1	S
159.578	0.2992	0.0000	82.428	0.03054	0.20516	31811.6	15507.7	4725.5	S
159.600	0.2992	0.0000	82.429	0.03053	0.20531	31835.5	15510.1	4742.0	S
159.622	0.2992	0.0000	82.430	0.03053	0.20547	31859.5	15512.6	4758.4	S
159.644	0.2992	0.0000	82.431	0.03053	0.20562	31883.4	15515.0	4774.8	S
159.667	0.2992	0.0000	82.431	0.03052	0.20578	31907.3	15517.5	4791.3	S
159.689	0.2993	0.0000	82.432	0.03052	0.20593	31931.3	15519.9	4807.8	S
159.711	0.2993	0.0000	82.433	0.03051	0.20608	31955.2	15522.3	4824.2	S
159.733	0.2993	0.0000	82.434	0.03051	0.20623	31979.2	15524.8	4840.7	S
159.756	0.2993	0.0000	82.435	0.03050	0.20639	32003.1	15527.2	4857.2	S
159.778	0.2993	0.0000	82.435	0.03050	0.20654	32027.0	15529.7	4873.7	S
159.800	0.2993	0.0000	82.436	0.03049	0.20669	32051.0	15532.1	4890.3	S
159.822	0.2993	0.0000	82.437	0.03049	0.20684	32074.9	15534.5	4906.8	S
159.845	0.2993	0.0000	82.438	0.03048	0.20699	32098.9	15537.0	4923.4	S
159.867	0.2993	0.0000	82.438	0.03048	0.20714	32122.8	15539.4	4939.9	S
159.889	0.2993	0.0000	82.439	0.03047	0.20729	32146.8	15541.9	4956.5	S
159.911	0.2993	0.0000	82.440	0.03047	0.20744	32170.7	15544.3	4973.1	S
159.933	0.2993	0.0000	82.441	0.03046	0.20758	32194.6	15546.7	4989.7	S
159.956	0.2993	0.0000	82.442	0.03046	0.20773	32218.6	15549.2	5006.3	S
159.978	0.2993	0.0000	82.442	0.03045	0.20788	32242.5	15551.6	5022.9	S
160.000	0.2993	0.0000	82.443	0.03045	0.20803	32266.5	15554.0	5039.6	S
160.022	0.2962	0.0000	82.444	0.03043	0.20817	32290.3	15556.5	5056.2	S
160.044	0.2860	0.0000	82.444	0.03038	0.20829	32313.6	15558.9	5072.9	S
160.067	0.2637	0.0000	82.445	0.03028	0.20838	32335.6	15561.3	5089.5	S
160.089	0.2312	0.0000	82.445	0.03011	0.20840	32355.3	15563.8	5106.2	S
160.111	0.1951	0.0000	82.445	0.02989	0.20834	32372.4	15566.2	5122.9	S
160.133	0.1608	0.0000	82.444	0.02962	0.20820	32386.6	15568.5	5139.5	S
160.156	0.1316	0.0000	82.443	0.02931	0.20798	32398.3	15570.9	5156.2	S
160.178	0.1100	0.0000	82.441	0.02899	0.20770	32408.0	15573.2	5172.8	S
160.200	0.0949	0.0000	82.440	0.02867	0.20737	32416.2	15575.5	5189.4	S
160.222	0.0841	0.0000	82.438	0.02835	0.20702	32423.4	15577.8	5206.0	S
160.244	0.0762	0.0000	82.436	0.02804	0.20664	32429.8	15580.1	5222.5	S
160.267	0.0705	0.0000	82.434	0.02774	0.20625	32435.6	15582.3	5239.1	S
160.289	0.0664	0.0000	82.432	0.02745	0.20585	32441.1	15584.5	5255.5	S
160.311	0.0634	0.0000	82.430	0.02717	0.20544	32446.3	15586.7	5272.0	S
160.333	0.0613	0.0000	82.428	0.02691	0.20502	32451.3	15588.9	5288.4	S
160.356	0.0598	0.0000	82.425	0.02666	0.20460	32456.1	15591.0	5304.8	S
160.378	0.0587	0.0000	82.423	0.02641	0.20418	32460.9	15593.1	5321.2	S
160.400	0.0579	0.0000	82.421	0.02618	0.20376	32465.5	15595.2	5337.5	S
160.422	0.0573	0.0000	82.419	0.02596	0.20333	32470.2	15597.3	5353.8	S
160.444	0.0569	0.0000	82.417	0.02575	0.20290	32474.7	15599.4	5370.0	S
160.467	0.0565	0.0000	82.414	0.02555	0.20248	32479.3	15601.4	5386.2	S
160.489	0.0563	0.0000	82.412	0.02535	0.20205	32483.8	15603.5	5402.4	S
160.511	0.0561	0.0000	82.410	0.02517	0.20162	32488.3	15605.5	5418.5	S
160.533	0.0560	0.0000	82.408	0.02499	0.20119	32492.7	15607.5	5434.7	S
160.556	0.0560	0.0000	82.406	0.02481	0.20077	32497.2	15609.5	5450.7	S
160.578	0.0560	0.0000	82.404	0.02464	0.20034	32501.7	15611.5	5466.8	S
160.600	0.0560	0.0000	82.402	0.02448	0.19991	32506.2	15613.4	5482.8	S
160.622	0.0560	0.0000	82.399	0.02433	0.19949	32510.7	15615.4	5498.8	S
160.644	0.0560	0.0000	82.397	0.02417	0.19906	32515.2	15617.3	5514.7	S
160.667	0.0560	0.0000	82.395	0.02403	0.19864	32519.6	15619.2	5530.6	S
160.689	0.0560	0.0000	82.393	0.02388	0.19821	32524.1	15621.2	5546.5	S
160.711	0.0560	0.0000	82.391	0.02374	0.19779	32528.6	15623.1	5562.3	S
160.733	0.0560	0.0000	82.389	0.02361	0.19736	32533.1	15625.0	5578.1	S
160.756	0.0560	0.0000	82.387	0.02348	0.19694	32537.6	15626.8	5593.9	S
160.778	0.0560	0.0000	82.385	0.02335	0.19651	32542.1	15628.7	5609.6	S
160.800	0.0560	0.0000	82.383	0.02322	0.19609	32546.5	15630.6	5625.4	S
160.822	0.0560	0.0000	82.380	0.02310	0.19567	32551.0	15632.4	5641.0	S
160.845	0.0560	0.0000	82.378	0.02298	0.19524	32555.5	15634.3	5656.7	S
160.867	0.0560	0.0000	82.376	0.02286	0.19482	32560.0	15636.1	5672.3	S
160.889	0.0560	0.0000	82.374	0.02275	0.19440	32564.5	15637.9	5687.8	S
160.911	0.0560	0.0000	82.372	0.02263	0.19398	32569.0	15639.7	5703.4	S
160.933	0.0560	0.0000	82.370	0.02252	0.19355	32573.4	15641.6	5718.9	S
160.956	0.0560	0.0000	82.368	0.02241	0.19313	32577.9	15643.4	5734.3	S
160.978	0.0560	0.0000	82.366	0.02231	0.19271	32582.4	15645.1	5749.8	S
161.000	0.0560	0.0000	82.364	0.02220	0.19229	32586.9	15646.9	5765.2	S
161.022	0.0560	0.0000	82.362	0.02210	0.19187	32591.4	15648.7	5780.5	S
161.044	0.0560	0.0000	82.360	0.02200	0.19145	32595.9	15650.5	5795.9	S
161.067	0.0560	0.0000	82.358	0.02190	0.19103	32600.3	15652.2	5811.2	S
161.089	0.0560	0.0000	82.356	0.02180	0.19061	32604.8	15654.0	5826.4	S
161.111	0.0560	0.0000	82.354	0.02171	0.19019	32609.3	15655.7	5841.7	S
161.133	0.0560	0.0000	82.352	0.02161	0.18977	32613.8	15657.4	5856.9	S

Detailed Results (cont,d.) *:: Scenario 31 :: FDOT 168 Hour - 168 hr - 25 yr*

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
167.733	0.0561	0.0000	81.983	0.01409	0.08128	33945.5	16035.7	8994.6	S
167.756	0.0561	0.0000	81.982	0.01409	0.08101	33950.0	16036.8	9001.1	S
167.778	0.0561	0.0000	81.982	0.01409	0.08074	33954.5	16037.9	9007.6	S
167.800	0.0561	0.0000	81.981	0.01409	0.08047	33958.9	16039.1	9014.0	S
167.822	0.0561	0.0000	81.981	0.01409	0.08019	33963.4	16040.2	9020.4	S
167.845	0.0561	0.0000	81.980	0.01410	0.07992	33967.9	16041.3	9026.8	S
167.867	0.0561	0.0000	81.980	0.01410	0.07966	33972.4	16042.5	9033.2	S
167.889	0.0561	0.0000	81.979	0.01410	0.07939	33976.9	16043.6	9039.6	S
167.911	0.0561	0.0000	81.979	0.01410	0.07912	33981.4	16044.7	9045.9	S
167.933	0.0561	0.0000	81.978	0.01410	0.07885	33985.8	16045.8	9052.3	S
167.956	0.0561	0.0000	81.978	0.01411	0.07859	33990.3	16047.0	9058.5	S
167.978	0.0561	0.0000	81.977	0.01411	0.07833	33994.8	16048.1	9064.8	S
168.000	0.0561	0.0000	81.976	0.01411	0.07806	33999.3	16049.2	9071.1	S
168.022	0.0552	0.0000	81.976	0.01411	0.07780	34003.8	16050.3	9077.3	S
168.044	0.0526	0.0000	81.975	0.01409	0.07752	34008.1	16051.5	9083.5	S
168.067	0.0471	0.0000	81.975	0.01406	0.07721	34012.1	16052.6	9089.7	S
168.089	0.0392	0.0000	81.974	0.01400	0.07686	34015.5	16053.7	9095.9	S
168.111	0.0308	0.0000	81.973	0.01393	0.07645	34018.3	16054.8	9102.0	S
168.133	0.0230	0.0000	81.972	0.01385	0.07598	34020.5	16056.0	9108.1	S
168.156	0.0165	0.0000	81.971	0.01376	0.07545	34022.0	16057.1	9114.2	S
168.178	0.0118	0.0000	81.970	0.01367	0.07489	34023.2	16058.2	9120.2	S
168.200	0.0085	0.0000	81.969	0.01359	0.07429	34024.0	16059.2	9126.1	S
168.222	0.0061	0.0000	81.968	0.01351	0.07368	34024.6	16060.3	9132.1	S
168.244	0.0044	0.0000	81.967	0.01344	0.07304	34025.0	16061.4	9137.9	S
168.267	0.0032	0.0000	81.966	0.01337	0.07240	34025.3	16062.5	9143.8	S
168.289	0.0023	0.0000	81.965	0.01330	0.07175	34025.5	16063.5	9149.5	S
168.311	0.0016	0.0000	81.963	0.01324	0.07109	34025.7	16064.6	9155.2	S
168.333	0.0011	0.0000	81.962	0.01318	0.07042	34025.8	16065.7	9160.9	S
168.356	0.0008	0.0000	81.961	0.01312	0.06975	34025.8	16066.7	9166.5	S
168.378	0.0006	0.0000	81.960	0.01307	0.06908	34025.9	16067.8	9172.1	S
168.400	0.0004	0.0000	81.959	0.01302	0.06841	34025.9	16068.8	9177.6	S
168.422	0.0003	0.0000	81.958	0.01298	0.06773	34026.0	16069.9	9183.0	S
168.444	0.0002	0.0000	81.956	0.01293	0.06705	34026.0	16070.9	9188.4	S
168.467	0.0001	0.0000	81.955	0.01289	0.06637	34026.0	16071.9	9193.7	S
168.489	0.0001	0.0000	81.954	0.01286	0.06569	34026.0	16073.0	9199.0	S
168.511	0.0000	0.0000	81.953	0.01282	0.06501	34026.0	16074.0	9204.2	S
168.533	0.0000	0.0000	81.952	0.01279	0.06432	34026.0	16075.0	9209.4	S
168.556	0.0000	0.0000	81.951	0.01275	0.06364	34026.0	16076.0	9214.5	S
168.578	0.0000	0.0000	81.950	0.01253	0.06295	34026.0	16077.0	9219.6	S
170.978	0.0000	0.0000	81.903	-0.00201	0.01627	34026.0	15991.9	9561.8	S
173.378	0.0000	0.0000	81.881	0.00988	0.00000	34026.0	16042.3	9632.1	S
175.778	0.0000	0.0000	81.859	0.01383	0.00000	34026.0	16162.5	9632.1	S
178.178	0.0000	0.0000	81.837	0.01362	0.00000	34026.0	16281.2	9632.1	S
180.578	0.0000	0.0000	81.816	0.01337	0.00000	34026.0	16397.8	9632.1	S
182.978	0.0000	0.0000	81.794	0.01312	0.00000	34026.0	16512.3	9632.1	S
185.378	0.0000	0.0000	81.773	0.01286	0.00000	34026.0	16624.5	9632.1	S
187.778	0.0000	0.0000	81.753	0.01261	0.00000	34026.0	16734.5	9632.1	S
190.178	0.0000	0.0000	81.732	0.01235	0.00000	34026.0	16842.3	9632.1	S
192.578	0.0000	0.0000	81.712	0.01205	0.00000	34026.0	16947.9	9632.1	S
216.578	0.0000	0.0000	81.536	0.00964	0.00000	34026.0	17842.4	9632.1	S
240.578	0.0000	0.0000	81.376	0.00835	0.00000	34026.0	18613.3	9632.1	S
264.578	0.0000	0.0000	81.230	0.00730	0.00000	34026.0	19285.2	9632.1	S
288.578	0.0000	0.0000	81.096	0.00644	0.00000	34026.0	19875.4	9632.1	S
312.578	0.0000	0.0000	80.972	0.00571	0.00000	34026.0	20397.4	9632.1	S
336.578	0.0000	0.0000	80.856	0.00510	0.00000	34026.0	20861.9	9632.1	S
360.578	0.0000	0.0000	80.749	0.00457	0.00000	34026.0	21277.9	9632.1	S
384.578	0.0000	0.0000	80.649	0.00412	0.00000	34026.0	21652.0	9632.1	S
408.578	0.0000	0.0000	80.555	0.00373	0.00000	34026.0	21990.0	9632.1	S
432.578	0.0000	0.0000	80.467	0.00339	0.00000	34026.0	22296.5	9632.1	S
456.578	0.0000	0.0000	80.384	0.00309	0.00000	34026.0	22575.5	9632.1	S
480.578	0.0000	0.0000	80.305	0.00283	0.00000	34026.0	22830.3	9632.1	S
504.578	0.0000	0.0000	80.231	0.00259	0.00000	34026.0	23063.7	9632.1	S
528.578	0.0000	0.0000	80.160	0.00238	0.00000	34026.0	23278.1	9632.1	S
552.578	0.0000	0.0000	80.093	0.00220	0.00000	34026.0	23475.5	9632.1	S
576.578	0.0000	0.0000	80.030	0.00203	0.00000	34026.0	23657.7	9632.1	S
600.578	0.0000	0.0000	79.969	0.00188	0.00000	34026.0	23826.2	9632.1	S
624.578	0.0000	0.0000	79.911	0.00174	0.00000	34026.0	23982.5	9632.1	S
648.578	0.0000	0.0000	79.856	0.00162	0.00000	34026.0	24127.8	9632.1	S
672.578	0.0000	0.0000	79.803	0.00151	0.00000	34026.0	24263.0	9632.1	S
696.578	0.0000	0.0000	79.752	0.00076	0.00000	34026.0	24389.0	9632.1	S
720.578	0.0000	0.0000	79.681	0.00003	0.00000	34026.0	24393.9	9632.1	S
744.578	0.0000	0.0000	79.618	0.00000	0.00000	34026.0	24393.9	9632.1	S
768.578	0.0000	0.0000	79.561	0.00000	0.00000	34026.0	24393.9	9632.1	S
792.578	0.0000	0.0000	79.509	0.00000	0.00000	34026.0	24393.9	9632.1	S

Detailed Results (cont,d.) :: *Scenario 32 :: FDOT 240 Hour - 240 hr - 25 yr*

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
184.178	0.1552	0.0000	82.571	0.03943	0.23124	31535.3	15349.7	3685.4	S
184.200	0.1330	0.0000	82.569	0.03901	0.23097	31546.8	15352.8	3703.9	S
184.222	0.1173	0.0000	82.568	0.03859	0.23067	31556.8	15355.9	3722.4	S
184.244	0.1058	0.0000	82.566	0.03817	0.23034	31565.7	15359.0	3740.8	S
184.267	0.0975	0.0000	82.564	0.03777	0.22998	31573.9	15362.0	3759.2	S
184.289	0.0916	0.0000	82.562	0.03738	0.22962	31581.4	15365.0	3777.6	S
184.311	0.0872	0.0000	82.559	0.03701	0.22924	31588.6	15368.0	3796.0	S
184.333	0.0841	0.0000	82.557	0.03666	0.22886	31595.4	15371.0	3814.3	S
184.356	0.0819	0.0000	82.555	0.03632	0.22847	31602.1	15373.9	3832.6	S
184.378	0.0803	0.0000	82.553	0.03599	0.22808	31608.6	15376.8	3850.8	S
184.400	0.0791	0.0000	82.551	0.03568	0.22769	31614.9	15379.6	3869.1	S
184.422	0.0783	0.0000	82.548	0.03538	0.22729	31621.2	15382.5	3887.3	S
184.444	0.0777	0.0000	82.546	0.03510	0.22690	31627.5	15385.3	3905.4	S
184.467	0.0772	0.0000	82.544	0.03482	0.22650	31633.7	15388.1	3923.6	S
184.489	0.0768	0.0000	82.542	0.03456	0.22610	31639.8	15390.9	3941.7	S
184.511	0.0766	0.0000	82.539	0.03430	0.22570	31646.0	15393.6	3959.7	S
184.533	0.0765	0.0000	82.537	0.03406	0.22531	31652.1	15396.4	3977.8	S
184.556	0.0764	0.0000	82.535	0.03383	0.22491	31658.2	15399.1	3995.8	S
184.578	0.0764	0.0000	82.533	0.03360	0.22451	31664.3	15401.8	4013.8	S
184.600	0.0764	0.0000	82.530	0.03338	0.22412	31670.4	15404.5	4031.7	S
184.622	0.0764	0.0000	82.528	0.03317	0.22372	31676.6	15407.1	4049.6	S
184.644	0.0764	0.0000	82.526	0.03296	0.22333	31682.7	15409.8	4067.5	S
184.667	0.0764	0.0000	82.524	0.03276	0.22293	31688.8	15412.4	4085.4	S
184.689	0.0764	0.0000	82.521	0.03257	0.22254	31694.9	15415.0	4103.2	S
184.711	0.0764	0.0000	82.519	0.03238	0.22214	31701.0	15417.6	4121.0	S
184.733	0.0764	0.0000	82.517	0.03219	0.22175	31707.1	15420.2	4138.7	S
184.756	0.0764	0.0000	82.515	0.03201	0.22136	31713.2	15422.8	4156.4	S
184.778	0.0764	0.0000	82.513	0.03184	0.22096	31719.4	15425.3	4174.1	S
184.800	0.0764	0.0000	82.511	0.03166	0.22057	31725.5	15427.8	4191.8	S
184.822	0.0764	0.0000	82.508	0.03150	0.22018	31731.6	15430.4	4209.4	S
184.845	0.0764	0.0000	82.506	0.03133	0.21979	31737.7	15432.9	4227.0	S
184.867	0.0764	0.0000	82.504	0.03117	0.21939	31743.8	15435.4	4244.6	S
184.889	0.0764	0.0000	82.502	0.03101	0.21900	31749.9	15437.9	4262.1	S
184.911	0.0764	0.0000	82.500	0.03086	0.21861	31756.1	15440.4	4279.6	S
184.933	0.0764	0.0000	82.498	0.03071	0.21822	31762.2	15442.8	4297.1	S
184.956	0.0764	0.0000	82.495	0.03056	0.21783	31768.3	15445.3	4314.5	S
184.978	0.0764	0.0000	82.493	0.03041	0.21744	31774.4	15447.7	4332.0	S
185.000	0.0764	0.0000	82.491	0.03027	0.21705	31780.5	15450.1	4349.3	S
185.022	0.0764	0.0000	82.489	0.03013	0.21666	31786.6	15452.5	4366.7	S
185.044	0.0764	0.0000	82.487	0.02999	0.21627	31792.8	15455.0	4384.0	S
185.067	0.0764	0.0000	82.485	0.02985	0.21588	31798.9	15457.3	4401.3	S
185.089	0.0764	0.0000	82.483	0.02972	0.21549	31805.0	15459.7	4418.5	S
185.111	0.0764	0.0000	82.481	0.02959	0.21511	31811.1	15462.1	4435.8	S
185.133	0.0764	0.0000	82.479	0.02946	0.21472	31817.2	15464.5	4453.0	S
185.156	0.0764	0.0000	82.476	0.02933	0.21433	31823.3	15466.8	4470.1	S
185.178	0.0764	0.0000	82.474	0.02921	0.21394	31829.5	15469.2	4487.3	S
185.200	0.0764	0.0000	82.472	0.02908	0.21356	31835.6	15471.5	4504.4	S
185.222	0.0764	0.0000	82.470	0.02896	0.21317	31841.7	15473.8	4521.4	S
185.244	0.0764	0.0000	82.468	0.02884	0.21278	31847.8	15476.1	4538.5	S
185.267	0.0764	0.0000	82.466	0.02872	0.21240	31853.9	15478.4	4555.5	S
185.289	0.0764	0.0000	82.464	0.02860	0.21201	31860.0	15480.7	4572.4	S
185.311	0.0764	0.0000	82.462	0.02849	0.21163	31866.1	15483.0	4589.4	S
185.333	0.0764	0.0000	82.460	0.02837	0.21124	31872.3	15485.3	4606.3	S
185.356	0.0764	0.0000	82.458	0.02826	0.21085	31878.4	15487.5	4623.2	S
185.378	0.0764	0.0000	82.456	0.02815	0.21047	31884.5	15489.8	4640.0	S
185.400	0.0764	0.0000	82.454	0.02804	0.21009	31890.6	15492.0	4656.9	S
185.422	0.0764	0.0000	82.452	0.02793	0.20970	31896.7	15494.3	4673.7	S
185.444	0.0764	0.0000	82.450	0.02782	0.20932	31902.8	15496.5	4690.4	S
185.467	0.0765	0.0000	82.448	0.02772	0.20893	31909.0	15498.7	4707.1	S
185.489	0.0765	0.0000	82.446	0.02761	0.20855	31915.1	15500.9	4723.8	S
185.511	0.0765	0.0000	82.444	0.02751	0.20817	31921.2	15503.2	4740.5	S
185.533	0.0765	0.0000	82.442	0.02741	0.20778	31927.3	15505.3	4757.2	S
185.556	0.0765	0.0000	82.440	0.02731	0.20740	31933.4	15507.5	4773.8	S
185.578	0.0765	0.0000	82.438	0.02721	0.20702	31939.5	15509.7	4790.3	S
185.600	0.0765	0.0000	82.436	0.02711	0.20664	31945.7	15511.9	4806.9	S
185.622	0.0765	0.0000	82.434	0.02701	0.20625	31951.8	15514.1	4823.4	S
185.644	0.0765	0.0000	82.432	0.02692	0.20587	31957.9	15516.2	4839.9	S
185.667	0.0765	0.0000	82.430	0.02682	0.20549	31964.0	15518.4	4856.3	S
185.689	0.0765	0.0000	82.428	0.02673	0.20511	31970.1	15520.5	4872.8	S
185.711	0.0765	0.0000	82.426	0.02663	0.20473	31976.2	15522.6	4889.2	S
185.733	0.0765	0.0000	82.424	0.02654	0.20435	31982.4	15524.8	4905.5	S
185.756	0.0765	0.0000	82.422	0.02645	0.20397	31988.5	15526.9	4921.9	S
185.778	0.0765	0.0000	82.420	0.02636	0.20359	31994.6	15529.0	4938.2	S
185.800	0.0765	0.0000	82.418	0.02627	0.20321	32000.7	15531.1	4954.4	S

Detailed Results (cont,d.) *:: Scenario 32 :: FDOT 240 Hour - 240 hr - 25 yr*

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
240.089	0.0144	0.0000	81.900	0.01368	0.00599	39667.7	18630.0	12581.2	S
240.111	0.0113	0.0000	81.900	0.01349	0.00534	39668.7	18631.1	12581.6	S
240.133	0.0085	0.0000	81.900	0.01325	0.00434	39669.5	18632.1	12582.0	S
240.156	0.0061	0.0000	81.900	0.01281	0.00281	39670.1	18633.2	12582.3	S
240.178	0.0043	0.0000	81.900	0.01319	0.00000	39670.5	18634.2	12582.4	S
240.200	0.0031	0.0000	81.900	0.01388	0.00000	39670.8	18635.3	12582.4	S
240.222	0.0023	0.0000	81.900	0.01386	0.00000	39671.0	18636.4	12582.4	S
240.244	0.0016	0.0000	81.899	0.01384	0.00000	39671.2	18637.5	12582.4	S
240.267	0.0012	0.0000	81.899	0.01382	0.00000	39671.3	18638.6	12582.4	S
240.289	0.0008	0.0000	81.899	0.01380	0.00000	39671.4	18639.7	12582.4	S
240.311	0.0006	0.0000	81.899	0.01378	0.00000	39671.4	18640.8	12582.4	S
240.333	0.0004	0.0000	81.899	0.01377	0.00000	39671.5	18641.9	12582.4	S
240.356	0.0003	0.0000	81.899	0.01375	0.00000	39671.5	18643.0	12582.4	S
240.378	0.0002	0.0000	81.898	0.01373	0.00000	39671.5	18644.1	12582.4	S
240.400	0.0001	0.0000	81.898	0.01372	0.00000	39671.5	18645.2	12582.4	S
240.422	0.0001	0.0000	81.898	0.01370	0.00000	39671.5	18646.3	12582.4	S
240.444	0.0001	0.0000	81.898	0.01369	0.00000	39671.5	18647.4	12582.4	S
240.467	0.0000	0.0000	81.898	0.01367	0.00000	39671.5	18648.5	12582.4	S
240.489	0.0000	0.0000	81.897	0.01366	0.00000	39671.5	18649.6	12582.4	S
240.511	0.0000	0.0000	81.897	0.01365	0.00000	39671.5	18650.7	12582.4	S
240.533	0.0000	0.0000	81.897	0.01363	0.00000	39671.5	18651.8	12582.4	S
240.556	0.0000	0.0000	81.897	0.01362	0.00000	39671.5	18652.9	12582.4	S
240.578	0.0000	0.0000	81.897	0.01361	0.00000	39671.5	18654.0	12582.4	S
242.978	0.0000	0.0000	81.876	0.01275	0.00000	39671.5	18766.1	12582.4	S
245.378	0.0000	0.0000	81.856	0.01234	0.00000	39671.5	18874.3	12582.4	S
247.778	0.0000	0.0000	81.837	0.01199	0.00000	39671.5	18979.4	12582.4	S
250.178	0.0000	0.0000	81.818	0.01168	0.00000	39671.5	19081.6	12582.4	S
252.578	0.0000	0.0000	81.799	0.01140	0.00000	39671.5	19181.2	12582.4	S
254.978	0.0000	0.0000	81.781	0.01115	0.00000	39671.5	19278.6	12582.4	S
257.378	0.0000	0.0000	81.763	0.01091	0.00000	39671.5	19373.8	12582.4	S
259.778	0.0000	0.0000	81.745	0.01068	0.00000	39671.5	19467.1	12582.4	S
262.178	0.0000	0.0000	81.728	0.01047	0.00000	39671.5	19558.5	12582.4	S
264.578	0.0000	0.0000	81.711	0.01025	0.00000	39671.5	19648.1	12582.4	S
288.578	0.0000	0.0000	81.559	0.00845	0.00000	39671.5	20424.2	12582.4	S
312.578	0.0000	0.0000	81.419	0.00748	0.00000	39671.5	21107.9	12582.4	S
336.578	0.0000	0.0000	81.289	0.00667	0.00000	39671.5	21716.3	12582.4	S
360.578	0.0000	0.0000	81.167	0.00599	0.00000	39671.5	22261.3	12582.4	S
384.578	0.0000	0.0000	81.054	0.00540	0.00000	39671.5	22751.6	12582.4	S
408.578	0.0000	0.0000	80.947	0.00488	0.00000	39671.5	23194.3	12582.4	S
432.578	0.0000	0.0000	80.847	0.00443	0.00000	39671.5	23595.5	12582.4	S
456.578	0.0000	0.0000	80.752	0.00404	0.00000	39671.5	23960.4	12582.4	S
480.578	0.0000	0.0000	80.663	0.00369	0.00000	39671.5	24293.3	12582.4	S
504.578	0.0000	0.0000	80.579	0.00338	0.00000	39671.5	24598.2	12582.4	S
528.578	0.0000	0.0000	80.500	0.00311	0.00000	39671.5	24878.0	12582.4	S
552.578	0.0000	0.0000	80.424	0.00287	0.00000	39671.5	25135.5	12582.4	S
576.578	0.0000	0.0000	80.352	0.00265	0.00000	39671.5	25373.2	12582.4	S
600.578	0.0000	0.0000	80.284	0.00245	0.00000	39671.5	25593.0	12582.4	S
624.578	0.0000	0.0000	80.218	0.00227	0.00000	39671.5	25796.8	12582.4	S
648.578	0.0000	0.0000	80.156	0.00211	0.00000	39671.5	25986.0	12582.4	S
672.578	0.0000	0.0000	80.096	0.00197	0.00000	39671.5	26162.0	12582.4	S
696.578	0.0000	0.0000	80.039	0.00183	0.00000	39671.5	26326.0	12582.4	S
720.578	0.0000	0.0000	79.984	0.00171	0.00000	39671.5	26479.0	12582.4	S
744.578	0.0000	0.0000	79.932	0.00160	0.00000	39671.5	26622.1	12582.4	S
768.578	0.0000	0.0000	79.881	0.00150	0.00000	39671.5	26756.1	12582.4	S
792.578	0.0000	0.0000	79.833	0.00141	0.00000	39671.5	26881.7	12582.4	S
816.578	0.0000	0.0000	79.786	0.00120	0.00000	39671.5	26999.6	12582.4	S
840.578	0.0000	0.0000	79.737	0.00052	0.00000	39671.5	27089.1	12582.4	S
864.578	0.0000	0.0000	79.674	0.00000	0.00000	39671.5	27089.1	12582.4	S
888.578	0.0000	0.0000	79.619	0.00000	0.00000	39671.5	27089.1	12582.4	S
912.578	0.0000	0.0000	79.568	0.00000	0.00000	39671.5	27089.1	12582.4	S
936.578	0.0000	0.0000	79.521	0.00000	0.00000	39671.5	27089.1	12582.4	S
960.578	0.0000	0.0000	79.477	----	----	39671.5	27089.1	12582.4	N.A.

Detailed Results (cont,d.) :: *Scenario 33 :: FDOT 1 Hour - 1 hr - 50 yr*

Elapsed Time (hours)	Inflow Rate (ft^3/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft^3/s)	Overflow Discharge (ft^3/s)	Cumulative Inflow Volume (ft^3)	Cumulative Infiltration Volume (ft^3)	Cumulative Discharge Volume (ft^3)	Flow Type
8.778	0.0000	0.0000	79.950	0.10434	0.00000	8694.5	8177.0	0.0	U/S
11.178	0.0000	0.0000	79.846	0.02692	0.00000	8694.5	8452.5	0.0	S
13.578	0.0000	0.0000	79.771	0.01400	0.00000	8694.5	8642.2	0.0	S
15.978	0.0000	0.0000	79.685	0.00303	0.00000	8694.5	8694.5	0.0	S
18.378	0.0000	0.0000	79.603	0.00000	0.00000	8694.5	8694.5	0.0	S
20.778	0.0000	0.0000	79.534	0.00000	0.00000	8694.5	8694.5	0.0	S
23.178	0.0000	0.0000	79.474	0.00000	0.00000	8694.5	8694.5	0.0	S
25.578	0.0000	0.0000	79.421	0.00000	0.00000	8694.5	8694.5	0.0	S
49.578	0.0000	0.0000	79.123	0.00000	0.00000	8694.5	8694.5	0.0	S
73.578	0.0000	0.0000	78.928	0.00000	0.00000	8694.5	8694.5	0.0	S
97.578	0.0000	0.0000	78.787	0.00000	0.00000	8694.5	8694.5	0.0	S
121.578	0.0000	0.0000	78.678	0.00000	0.00000	8694.5	8694.5	0.0	S
145.578	0.0000	0.0000	78.591	0.00000	0.00000	8694.5	8694.5	0.0	S
169.578	0.0000	0.0000	78.519	0.00000	0.00000	8694.5	8694.5	0.0	S
193.578	0.0000	0.0000	78.458	0.00000	0.00000	8694.5	8694.5	0.0	S
217.578	0.0000	0.0000	78.405	0.00000	0.00000	8694.5	8694.5	0.0	S
241.578	0.0000	0.0000	78.358	0.00000	0.00000	8694.5	8694.5	0.0	S
265.578	0.0000	0.0000	78.317	0.00000	0.00000	8694.5	8694.5	0.0	S
289.578	0.0000	0.0000	78.279	0.00000	0.00000	8694.5	8694.5	0.0	S
313.578	0.0000	0.0000	78.246	0.00000	0.00000	8694.5	8694.5	0.0	S
337.578	0.0000	0.0000	78.215	0.00000	0.00000	8694.5	8694.5	0.0	S
361.578	0.0000	0.0000	78.187	0.00000	0.00000	8694.5	8694.5	0.0	S
385.578	0.0000	0.0000	78.161	0.00000	0.00000	8694.5	8694.5	0.0	S
409.578	0.0000	0.0000	78.137	0.00000	0.00000	8694.5	8694.5	0.0	S
433.578	0.0000	0.0000	78.114	0.00000	0.00000	8694.5	8694.5	0.0	S
457.578	0.0000	0.0000	78.093	0.00000	0.00000	8694.5	8694.5	0.0	S
481.578	0.0000	0.0000	78.073	0.00000	0.00000	8694.5	8694.5	0.0	S
505.578	0.0000	0.0000	78.055	0.00000	0.00000	8694.5	8694.5	0.0	S
529.578	0.0000	0.0000	78.037	0.00000	0.00000	8694.5	8694.5	0.0	S
553.578	0.0000	0.0000	78.021	0.00000	0.00000	8694.5	8694.5	0.0	S
577.578	0.0000	0.0000	78.005	0.00000	0.00000	8694.5	8694.5	0.0	S
601.578	0.0000	0.0000	77.990	0.00000	0.00000	8694.5	8694.5	0.0	S
625.578	0.0000	0.0000	77.976	0.00000	0.00000	8694.5	8694.5	0.0	S
649.578	0.0000	0.0000	77.963	0.00000	0.00000	8694.5	8694.5	0.0	S
673.578	0.0000	0.0000	77.950	0.00000	0.00000	8694.5	8694.5	0.0	S
697.578	0.0000	0.0000	77.938	0.00000	0.00000	8694.5	8694.5	0.0	S
721.578	0.0000	0.0000	77.926	----	----	8694.5	8694.5	0.0	N.A.

Detailed Results (cont,d.) :: *Scenario 34 :: FDOT 2 Hour - 2 hr - 50 yr*

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
1.644	0.7538	0.0000	81.951	0.38817	0.06396	10389.2	1618.9	32.0	U/P
1.667	0.7310	0.0000	81.955	0.38859	0.06649	10448.6	1650.0	37.2	U/P
1.689	0.7018	0.0000	81.959	0.38895	0.06869	10505.9	1681.1	42.6	U/P
1.711	0.6716	0.0000	81.962	0.38927	0.07055	10560.9	1712.2	48.1	U/P
1.733	0.6439	0.0000	81.965	0.38955	0.07213	10613.5	1743.4	53.9	U/P
1.756	0.6210	0.0000	81.968	0.38979	0.07345	10664.1	1774.6	59.7	U/P
1.778	0.6045	0.0000	81.970	0.39000	0.07460	10713.1	1805.8	65.6	U/P
1.800	0.5930	0.0000	81.972	0.39019	0.07561	10761.0	1837.0	71.6	U/P
1.822	0.5820	0.0000	81.973	0.39036	0.07651	10808.0	1868.2	77.7	U/P
1.844	0.5673	0.0000	81.975	0.39051	0.07731	10854.0	1899.4	83.8	U/P
1.867	0.5444	0.0000	81.976	0.39062	0.07795	10898.4	1930.7	90.1	U/P
1.889	0.5151	0.0000	81.977	0.39069	0.07839	10940.8	1961.9	96.3	U/P
1.911	0.4847	0.0000	81.978	0.39072	0.07861	10980.8	1993.2	102.6	U/P
1.933	0.4569	0.0000	81.978	0.39070	0.07862	11018.5	2024.4	108.9	U/P
1.956	0.4339	0.0000	81.977	0.39065	0.07845	11054.1	2055.7	115.2	U/P
1.978	0.4173	0.0000	81.977	0.39057	0.07814	11088.1	2086.9	121.4	U/P
2.000	0.4058	0.0000	81.976	0.39048	0.07772	11121.1	2118.2	127.7	U/P
2.022	0.3919	0.0000	81.975	0.39036	0.07721	11153.0	2149.4	133.9	U/P
2.044	0.3683	0.0000	81.974	0.39021	0.07657	11183.4	2180.6	140.0	U/P
2.067	0.3267	0.0000	81.972	0.38999	0.07568	11211.2	2211.8	146.1	U/P
2.089	0.2711	0.0000	81.970	0.38970	0.07443	11235.1	2243.0	152.1	U/P
2.111	0.2124	0.0000	81.966	0.38933	0.07272	11254.4	2274.2	158.0	U/P
2.133	0.1584	0.0000	81.962	0.38888	0.07053	11269.3	2305.3	163.7	U/P
2.156	0.1132	0.0000	81.958	0.38837	0.06787	11280.1	2336.4	169.3	U/P
2.178	0.0809	0.0000	81.953	0.38781	0.06480	11287.9	2367.5	174.6	U/P
2.200	0.0583	0.0000	81.947	0.38722	0.06134	11293.5	2398.5	179.6	U/P
2.222	0.0421	0.0000	81.942	0.38662	0.05752	11297.5	2429.4	184.4	U/P
2.244	0.0301	0.0000	81.936	0.38600	0.05333	11300.4	2460.3	188.8	U/P
2.267	0.0215	0.0000	81.930	0.38537	0.04870	11302.4	2491.2	192.9	U/P
2.289	0.0153	0.0000	81.924	0.38474	0.04355	11303.9	2522.0	196.6	U/P
2.311	0.0109	0.0000	81.918	0.38411	0.03771	11304.9	2552.7	199.8	U/P
2.333	0.0077	0.0000	81.912	0.38348	0.03082	11305.7	2583.4	202.6	U/P
2.356	0.0055	0.0000	81.906	0.38286	0.02200	11306.2	2614.1	204.7	U/P
2.378	0.0038	0.0000	81.900	0.38226	0.00550	11306.6	2644.7	205.8	U/P
2.400	0.0026	0.0000	81.895	0.38167	0.00000	11306.8	2675.3	206.0	U/P
2.422	0.0018	0.0000	81.889	0.38109	0.00000	11307.0	2705.8	206.0	U/P
2.444	0.0012	0.0000	81.884	0.38051	0.00000	11307.1	2736.2	206.0	U/P
2.467	0.0007	0.0000	81.878	0.37993	0.00000	11307.2	2766.7	206.0	U/P
2.489	0.0003	0.0000	81.873	0.37935	0.00000	11307.3	2797.0	206.0	U/P
2.511	0.0001	0.0000	81.867	0.37877	0.00000	11307.3	2827.3	206.0	U/P
2.533	0.0000	0.0000	81.862	0.37819	0.00000	11307.3	2857.6	206.0	U/P
2.556	0.0000	0.0000	81.856	0.37761	0.00000	11307.3	2887.9	206.0	U/P
2.578	0.0000	0.0000	81.850	0.37704	0.00000	11307.3	2918.0	206.0	U/P
4.978	0.0000	0.0000	81.248	0.31649	0.00000	11307.3	5912.3	206.0	U/P
7.378	0.0000	0.0000	80.641	0.16534	0.00000	11307.3	8386.9	206.0	U/S
9.778	0.0000	0.0000	80.534	0.03758	0.00000	11307.3	8769.3	206.0	S
12.178	0.0000	0.0000	80.457	0.02776	0.00000	11307.3	9036.3	206.0	S
14.578	0.0000	0.0000	80.394	0.02271	0.00000	11307.3	9249.0	206.0	S
16.978	0.0000	0.0000	80.339	0.01949	0.00000	11307.3	9428.7	206.0	S
19.378	0.0000	0.0000	80.290	0.01720	0.00000	11307.3	9585.8	206.0	S
21.778	0.0000	0.0000	80.245	0.01545	0.00000	11307.3	9725.9	206.0	S
24.178	0.0000	0.0000	80.204	0.01406	0.00000	11307.3	9852.7	206.0	S
26.578	0.0000	0.0000	80.166	0.01299	0.00000	11307.3	9968.8	206.0	S
50.578	0.0000	0.0000	79.906	0.00655	0.00000	11307.3	10704.0	206.0	S
74.578	0.0000	0.0000	79.681	0.00230	0.00000	11307.3	11101.3	206.0	S
98.578	0.0000	0.0000	79.452	0.00000	0.00000	11307.3	11101.3	206.0	S
122.578	0.0000	0.0000	79.282	0.00000	0.00000	11307.3	11101.3	206.0	S
146.578	0.0000	0.0000	79.150	0.00000	0.00000	11307.3	11101.3	206.0	S
170.578	0.0000	0.0000	79.041	0.00000	0.00000	11307.3	11101.3	206.0	S
194.578	0.0000	0.0000	78.950	0.00000	0.00000	11307.3	11101.3	206.0	S
218.578	0.0000	0.0000	78.872	0.00000	0.00000	11307.3	11101.3	206.0	S
242.578	0.0000	0.0000	78.804	0.00000	0.00000	11307.3	11101.3	206.0	S
266.578	0.0000	0.0000	78.744	0.00000	0.00000	11307.3	11101.3	206.0	S
290.578	0.0000	0.0000	78.690	0.00000	0.00000	11307.3	11101.3	206.0	S
314.578	0.0000	0.0000	78.642	0.00000	0.00000	11307.3	11101.3	206.0	S
338.578	0.0000	0.0000	78.598	0.00000	0.00000	11307.3	11101.3	206.0	S
362.578	0.0000	0.0000	78.558	0.00000	0.00000	11307.3	11101.3	206.0	S
386.578	0.0000	0.0000	78.521	0.00000	0.00000	11307.3	11101.3	206.0	S
410.578	0.0000	0.0000	78.487	0.00000	0.00000	11307.3	11101.3	206.0	S
434.578	0.0000	0.0000	78.455	0.00000	0.00000	11307.3	11101.3	206.0	S
458.578	0.0000	0.0000	78.426	0.00000	0.00000	11307.3	11101.3	206.0	S
482.578	0.0000	0.0000	78.398	0.00000	0.00000	11307.3	11101.3	206.0	S
506.578	0.0000	0.0000	78.372	0.00000	0.00000	11307.3	11101.3	206.0	S
530.578	0.0000	0.0000	78.348	0.00000	0.00000	11307.3	11101.3	206.0	S

Detailed Results (cont,d.) :: *Scenario 35 :: FDOT 4 Hour - 4 hr - 50 yr*

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
3.289	0.4797	0.0000	82.140	0.40878	0.13827	12920.4	2846.7	256.0	U/P
3.311	0.4705	0.0000	82.139	0.40867	0.13799	12958.4	2879.4	267.0	U/P
3.333	0.4639	0.0000	82.138	0.40854	0.13767	12995.8	2912.1	278.0	U/P
3.356	0.4591	0.0000	82.137	0.40841	0.13734	13032.7	2944.8	289.0	U/P
3.378	0.4557	0.0000	82.136	0.40827	0.13699	13069.3	2977.5	300.0	U/P
3.400	0.4532	0.0000	82.134	0.40813	0.13663	13105.7	3010.1	310.9	U/P
3.422	0.4514	0.0000	82.133	0.40799	0.13626	13141.9	3042.8	321.9	U/P
3.444	0.4501	0.0000	82.132	0.40785	0.13589	13177.9	3075.4	332.8	U/P
3.467	0.4491	0.0000	82.130	0.40770	0.13552	13213.9	3108.0	343.6	U/P
3.489	0.4484	0.0000	82.129	0.40756	0.13514	13249.8	3140.6	354.4	U/P
3.511	0.4459	0.0000	82.128	0.40741	0.13475	13285.6	3173.2	365.2	U/P
3.533	0.4375	0.0000	82.127	0.40724	0.13435	13320.9	3205.8	376.0	U/P
3.556	0.4180	0.0000	82.125	0.40705	0.13389	13355.1	3238.4	386.7	U/P
3.578	0.3860	0.0000	82.123	0.40681	0.13332	13387.3	3270.9	397.4	U/P
3.600	0.3471	0.0000	82.121	0.40652	0.13261	13416.6	3303.5	408.0	U/P
3.622	0.3082	0.0000	82.118	0.40617	0.13174	13442.8	3336.0	418.6	U/P
3.644	0.2738	0.0000	82.114	0.40577	0.13072	13466.1	3368.5	429.1	U/P
3.667	0.2468	0.0000	82.111	0.40532	0.12956	13486.9	3400.9	439.5	U/P
3.689	0.2277	0.0000	82.107	0.40486	0.12830	13505.9	3433.3	449.8	U/P
3.711	0.2142	0.0000	82.102	0.40437	0.12696	13523.6	3465.7	460.1	U/P
3.733	0.2044	0.0000	82.098	0.40387	0.12557	13540.3	3498.0	470.2	U/P
3.756	0.1973	0.0000	82.093	0.40336	0.12413	13556.4	3530.3	480.1	U/P
3.778	0.1922	0.0000	82.089	0.40284	0.12265	13572.0	3562.5	490.0	U/P
3.800	0.1885	0.0000	82.084	0.40232	0.12114	13587.2	3594.8	499.8	U/P
3.822	0.1858	0.0000	82.080	0.40180	0.11960	13602.2	3626.9	509.4	U/P
3.844	0.1839	0.0000	82.075	0.40128	0.11805	13617.0	3659.0	518.9	U/P
3.867	0.1825	0.0000	82.070	0.40076	0.11647	13631.6	3691.1	528.3	U/P
3.889	0.1815	0.0000	82.066	0.40023	0.11487	13646.2	3723.2	537.5	U/P
3.911	0.1808	0.0000	82.061	0.39972	0.11325	13660.7	3755.2	546.7	U/P
3.933	0.1803	0.0000	82.056	0.39920	0.11162	13675.1	3787.1	555.7	U/P
3.956	0.1799	0.0000	82.052	0.39868	0.10996	13689.5	3819.0	564.5	U/P
3.978	0.1796	0.0000	82.047	0.39817	0.10829	13703.9	3850.9	573.3	U/P
4.000	0.1794	0.0000	82.043	0.39766	0.10660	13718.3	3882.7	581.8	U/P
4.022	0.1766	0.0000	82.038	0.39714	0.10489	13732.5	3914.5	590.3	U/P
4.044	0.1682	0.0000	82.033	0.39662	0.10312	13746.3	3946.3	598.6	U/P
4.067	0.1505	0.0000	82.029	0.39607	0.10127	13759.1	3978.0	606.8	U/P
4.089	0.1255	0.0000	82.024	0.39548	0.09927	13770.1	4009.7	614.8	U/P
4.111	0.0986	0.0000	82.018	0.39486	0.09709	13779.1	4041.3	622.7	U/P
4.133	0.0736	0.0000	82.013	0.39421	0.09472	13786.0	4072.8	630.3	U/P
4.156	0.0527	0.0000	82.007	0.39352	0.09217	13791.0	4104.3	637.8	U/P
4.178	0.0376	0.0000	82.000	0.39281	0.08944	13794.6	4135.8	645.1	U/P
4.200	0.0271	0.0000	81.994	0.39212	0.08657	13797.2	4167.2	652.1	U/P
4.222	0.0196	0.0000	81.988	0.39143	0.08356	13799.1	4198.5	658.9	U/P
4.244	0.0141	0.0000	81.981	0.39075	0.08040	13800.4	4229.8	665.5	U/P
4.267	0.0101	0.0000	81.975	0.39007	0.07711	13801.4	4261.1	671.8	U/P
4.289	0.0072	0.0000	81.968	0.38938	0.07367	13802.1	4292.2	677.8	U/P
4.311	0.0052	0.0000	81.962	0.38870	0.07006	13802.6	4323.4	683.6	U/P
4.333	0.0037	0.0000	81.955	0.38802	0.06628	13802.9	4354.4	689.0	U/P
4.356	0.0026	0.0000	81.949	0.38734	0.06229	13803.2	4385.4	694.2	U/P
4.378	0.0018	0.0000	81.942	0.38667	0.05806	13803.4	4416.4	699.0	U/P
4.400	0.0013	0.0000	81.936	0.38601	0.05353	13803.5	4447.3	703.4	U/P
4.422	0.0009	0.0000	81.930	0.38535	0.04863	13803.6	4478.2	707.5	U/P
4.444	0.0006	0.0000	81.923	0.38469	0.04324	13803.6	4509.0	711.2	U/P
4.467	0.0003	0.0000	81.917	0.38405	0.03716	13803.7	4539.7	714.4	U/P
4.489	0.0002	0.0000	81.911	0.38341	0.02999	13803.7	4570.4	717.1	U/P
4.511	0.0001	0.0000	81.905	0.38279	0.02066	13803.7	4601.1	719.1	U/P
4.533	0.0000	0.0000	81.900	0.38218	0.00000	13803.7	4631.7	720.0	U/P
4.556	0.0000	0.0000	81.894	0.38159	0.00000	13803.7	4662.2	720.0	U/P
4.578	0.0000	0.0000	81.889	0.38189	0.00000	13803.7	4692.7	720.0	U/P
6.978	0.0000	0.0000	81.102	0.24928	0.00000	13803.7	8540.0	720.0	U/S
9.378	0.0000	0.0000	80.993	0.04540	0.00000	13803.7	9000.2	720.0	S
11.778	0.0000	0.0000	80.913	0.03380	0.00000	13803.7	9324.4	720.0	S
14.178	0.0000	0.0000	80.848	0.02781	0.00000	13803.7	9584.3	720.0	S
16.578	0.0000	0.0000	80.791	0.02397	0.00000	13803.7	9805.0	720.0	S
18.978	0.0000	0.0000	80.741	0.02123	0.00000	13803.7	9998.5	720.0	S
21.378	0.0000	0.0000	80.695	0.01914	0.00000	13803.7	10171.8	720.0	S
23.778	0.0000	0.0000	80.652	0.01747	0.00000	13803.7	10329.2	720.0	S
26.178	0.0000	0.0000	80.613	0.01611	0.00000	13803.7	10473.7	720.0	S
28.578	0.0000	0.0000	80.575	0.01501	0.00000	13803.7	10607.6	720.0	S
52.578	0.0000	0.0000	80.314	0.00880	0.00000	13803.7	11492.5	720.0	S
76.578	0.0000	0.0000	80.106	0.00648	0.00000	13803.7	12128.8	720.0	S
100.578	0.0000	0.0000	79.934	0.00500	0.00000	13803.7	12611.5	720.0	S
124.578	0.0000	0.0000	79.787	0.00273	0.00000	13803.7	12992.3	720.0	S
148.578	0.0000	0.0000	79.617	0.00053	0.00000	13803.7	13083.7	720.0	S

Detailed Results (cont,d.) :: *Scenario 36 :: FDOT 8 Hour - 8 hr - 50 yr*

Elapsed Time (hours)	Inflow Rate (ft^3/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft^3/s)	Overflow Discharge (ft^3/s)	Cumulative Inflow Volume (ft^3)	Cumulative Infiltration Volume (ft^3)	Cumulative Discharge Volume (ft^3)	Flow Type
8.222	0.0178	0.0000	81.866	0.00008	0.00000	17147.4	7675.7	1203.0	S
8.244	0.0128	0.0000	81.866	0.00008	0.00000	17148.6	7675.7	1203.0	S
8.267	0.0092	0.0000	81.867	0.00009	0.00000	17149.5	7675.7	1203.0	S
8.289	0.0066	0.0000	81.867	0.00010	0.00000	17150.1	7675.7	1203.0	S
8.311	0.0047	0.0000	81.867	0.00011	0.00000	17150.5	7675.7	1203.0	S
8.333	0.0033	0.0000	81.867	0.00011	0.00000	17150.9	7675.7	1203.0	S
8.356	0.0024	0.0000	81.867	0.00012	0.00000	17151.1	7675.7	1203.0	S
8.378	0.0017	0.0000	81.867	0.00013	0.00000	17151.3	7675.8	1203.0	S
8.400	0.0011	0.0000	81.867	0.00015	0.00000	17151.4	7675.8	1203.0	S
8.422	0.0008	0.0000	81.867	0.00016	0.00000	17151.4	7675.8	1203.0	S
8.444	0.0005	0.0000	81.867	0.00017	0.00000	17151.5	7675.8	1203.0	S
8.467	0.0003	0.0000	81.867	0.00019	0.00000	17151.5	7675.8	1203.0	S
8.489	0.0001	0.0000	81.867	0.00021	0.00000	17151.5	7675.8	1203.0	S
8.511	0.0000	0.0000	81.867	0.00023	0.00000	17151.6	7675.8	1203.0	S
8.533	0.0000	0.0000	81.867	0.00025	0.00000	17151.6	7675.9	1203.0	S
8.556	0.0000	0.0000	81.867	0.00027	0.00000	17151.6	7675.9	1203.0	S
8.578	0.0000	0.0000	81.867	0.00141	0.00000	17151.6	7675.9	1203.0	S
10.978	0.0000	0.0000	81.666	0.09403	0.00000	17151.6	8738.3	1203.0	S
13.378	0.0000	0.0000	81.555	0.05567	0.00000	17151.6	9300.7	1203.0	S
15.778	0.0000	0.0000	81.474	0.04177	0.00000	17151.6	9700.3	1203.0	S
18.178	0.0000	0.0000	81.407	0.03456	0.00000	17151.6	10022.5	1203.0	S
20.578	0.0000	0.0000	81.349	0.02993	0.00000	17151.6	10297.5	1203.0	S
22.978	0.0000	0.0000	81.296	0.02661	0.00000	17151.6	10539.7	1203.0	S
25.378	0.0000	0.0000	81.249	0.02408	0.00000	17151.6	10757.4	1203.0	S
27.778	0.0000	0.0000	81.204	0.02206	0.00000	17151.6	10955.8	1203.0	S
30.178	0.0000	0.0000	81.163	0.02041	0.00000	17151.6	11138.6	1203.0	S
32.578	0.0000	0.0000	81.124	0.01907	0.00000	17151.6	11308.4	1203.0	S
56.578	0.0000	0.0000	80.848	0.01143	0.00000	17151.6	12449.5	1203.0	S
80.578	0.0000	0.0000	80.628	0.00856	0.00000	17151.6	13284.1	1203.0	S
104.578	0.0000	0.0000	80.444	0.00672	0.00000	17151.6	13928.2	1203.0	S
128.578	0.0000	0.0000	80.286	0.00545	0.00000	17151.6	14444.5	1203.0	S
152.578	0.0000	0.0000	80.148	0.00453	0.00000	17151.6	14869.7	1203.0	S
176.578	0.0000	0.0000	80.024	0.00383	0.00000	17151.6	15226.8	1203.0	S
200.578	0.0000	0.0000	79.913	0.00328	0.00000	17151.6	15531.4	1203.0	S
224.578	0.0000	0.0000	79.812	0.00241	0.00000	17151.6	15794.4	1203.0	S
248.578	0.0000	0.0000	79.704	0.00089	0.00000	17151.6	15948.5	1203.0	S
272.578	0.0000	0.0000	79.584	0.00000	0.00000	17151.6	15948.5	1203.0	S
296.578	0.0000	0.0000	79.482	0.00000	0.00000	17151.6	15948.5	1203.0	S
320.578	0.0000	0.0000	79.394	0.00000	0.00000	17151.6	15948.5	1203.0	S
344.578	0.0000	0.0000	79.316	0.00000	0.00000	17151.6	15948.5	1203.0	S
368.578	0.0000	0.0000	79.246	0.00000	0.00000	17151.6	15948.5	1203.0	S
392.578	0.0000	0.0000	79.183	0.00000	0.00000	17151.6	15948.5	1203.0	S
416.578	0.0000	0.0000	79.126	0.00000	0.00000	17151.6	15948.5	1203.0	S
440.578	0.0000	0.0000	79.073	0.00000	0.00000	17151.6	15948.5	1203.0	S
464.578	0.0000	0.0000	79.025	0.00000	0.00000	17151.6	15948.5	1203.0	S
488.578	0.0000	0.0000	78.980	0.00000	0.00000	17151.6	15948.5	1203.0	S
512.578	0.0000	0.0000	78.939	0.00000	0.00000	17151.6	15948.5	1203.0	S
536.578	0.0000	0.0000	78.900	0.00000	0.00000	17151.6	15948.5	1203.0	S
560.578	0.0000	0.0000	78.864	0.00000	0.00000	17151.6	15948.5	1203.0	S
584.578	0.0000	0.0000	78.830	0.00000	0.00000	17151.6	15948.5	1203.0	S
608.578	0.0000	0.0000	78.798	0.00000	0.00000	17151.6	15948.5	1203.0	S
632.578	0.0000	0.0000	78.768	0.00000	0.00000	17151.6	15948.5	1203.0	S
656.578	0.0000	0.0000	78.739	0.00000	0.00000	17151.6	15948.5	1203.0	S
680.578	0.0000	0.0000	78.712	0.00000	0.00000	17151.6	15948.5	1203.0	S
704.578	0.0000	0.0000	78.687	0.00000	0.00000	17151.6	15948.5	1203.0	S
728.578	0.0000	0.0000	78.662	----	----	17151.6	15948.5	1203.0	N.A.

Detailed Results (cont,d.) :: *Scenario 37 :: FDOT 24 Hour - 24 hr - 50 yr*

Elapsed Time (hours)	Inflow Rate (ft^3/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft^3/s)	Overflow Discharge (ft^3/s)	Cumulative Inflow Volume (ft^3)	Cumulative Infiltration Volume (ft^3)	Cumulative Discharge Volume (ft^3)	Flow Type
34.178	0.0000	0.0000	81.760	0.03065	0.00000	24446.4	10571.0	6175.7	S
36.578	0.0000	0.0000	81.712	0.02813	0.00000	24446.4	10824.1	6175.7	S
38.978	0.0000	0.0000	81.667	0.02598	0.00000	24446.4	11057.1	6175.7	S
41.378	0.0000	0.0000	81.625	0.02416	0.00000	24446.4	11273.0	6175.7	S
43.778	0.0000	0.0000	81.585	0.02260	0.00000	24446.4	11474.5	6175.7	S
46.178	0.0000	0.0000	81.547	0.02125	0.00000	24446.4	11663.5	6175.7	S
48.578	0.0000	0.0000	81.511	0.02007	0.00000	24446.4	11841.6	6175.7	S
72.578	0.0000	0.0000	81.244	0.01281	0.00000	24446.4	13101.6	6175.7	S
96.578	0.0000	0.0000	81.025	0.00988	0.00000	24446.4	14054.8	6175.7	S
120.578	0.0000	0.0000	80.839	0.00793	0.00000	24446.4	14808.7	6175.7	S
144.578	0.0000	0.0000	80.677	0.00655	0.00000	24446.4	15424.7	6175.7	S
168.578	0.0000	0.0000	80.534	0.00552	0.00000	24446.4	15940.2	6175.7	S
192.578	0.0000	0.0000	80.406	0.00473	0.00000	24446.4	16379.2	6175.7	S
216.578	0.0000	0.0000	80.289	0.00411	0.00000	24446.4	16758.3	6175.7	S
240.578	0.0000	0.0000	80.182	0.00360	0.00000	24446.4	17089.0	6175.7	S
264.578	0.0000	0.0000	80.084	0.00318	0.00000	24446.4	17380.1	6175.7	S
288.578	0.0000	0.0000	79.992	0.00283	0.00000	24446.4	17638.2	6175.7	S
312.578	0.0000	0.0000	79.907	0.00253	0.00000	24446.4	17868.3	6175.7	S
336.578	0.0000	0.0000	79.828	0.00227	0.00000	24446.4	18074.7	6175.7	S
360.578	0.0000	0.0000	79.754	0.00113	0.00000	24446.4	18260.6	6175.7	S
384.578	0.0000	0.0000	79.654	0.00006	0.00000	24446.4	18270.6	6175.7	S
408.578	0.0000	0.0000	79.566	0.00000	0.00000	24446.4	18270.6	6175.7	S
432.578	0.0000	0.0000	79.489	0.00000	0.00000	24446.4	18270.6	6175.7	S
456.578	0.0000	0.0000	79.420	0.00000	0.00000	24446.4	18270.6	6175.7	S
480.578	0.0000	0.0000	79.357	0.00000	0.00000	24446.4	18270.6	6175.7	S
504.578	0.0000	0.0000	79.300	0.00000	0.00000	24446.4	18270.6	6175.7	S
528.578	0.0000	0.0000	79.248	0.00000	0.00000	24446.4	18270.6	6175.7	S
552.578	0.0000	0.0000	79.199	0.00000	0.00000	24446.4	18270.6	6175.7	S
576.578	0.0000	0.0000	79.154	0.00000	0.00000	24446.4	18270.6	6175.7	S
600.578	0.0000	0.0000	79.112	0.00000	0.00000	24446.4	18270.6	6175.7	S
624.578	0.0000	0.0000	79.072	0.00000	0.00000	24446.4	18270.6	6175.7	S
648.578	0.0000	0.0000	79.035	0.00000	0.00000	24446.4	18270.6	6175.7	S
672.578	0.0000	0.0000	79.000	0.00000	0.00000	24446.4	18270.6	6175.7	S
696.578	0.0000	0.0000	78.967	0.00000	0.00000	24446.4	18270.6	6175.7	S
720.578	0.0000	0.0000	78.936	0.00000	0.00000	24446.4	18270.6	6175.7	S
744.578	0.0000	0.0000	78.906	----	----	24446.4	18270.6	6175.7	N.A.

Detailed Results (cont,d.) *:: Scenario 38 :: FDOT 72 Hour - 72 hr - 50 yr*

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
64.133	0.1652	0.0000	82.498	0.03715	0.21833	29226.7	11981.1	5219.6	S
64.156	0.1450	0.0000	82.497	0.03692	0.21810	29239.1	11984.1	5237.0	S
64.178	0.1305	0.0000	82.495	0.03669	0.21783	29250.1	11987.1	5254.5	S
64.200	0.1203	0.0000	82.494	0.03645	0.21754	29260.2	11990.0	5271.9	S
64.222	0.1130	0.0000	82.492	0.03622	0.21723	29269.5	11992.9	5289.3	S
64.244	0.1076	0.0000	82.490	0.03599	0.21690	29278.3	11995.8	5306.7	S
64.267	0.1038	0.0000	82.489	0.03577	0.21657	29286.8	11998.6	5324.0	S
64.289	0.1010	0.0000	82.487	0.03556	0.21622	29295.0	12001.5	5341.3	S
64.311	0.0990	0.0000	82.485	0.03536	0.21588	29303.0	12004.3	5358.6	S
64.333	0.0976	0.0000	82.483	0.03516	0.21553	29310.8	12007.2	5375.8	S
64.356	0.0965	0.0000	82.481	0.03497	0.21517	29318.6	12010.0	5393.1	S
64.378	0.0958	0.0000	82.479	0.03479	0.21482	29326.3	12012.7	5410.3	S
64.400	0.0952	0.0000	82.477	0.03462	0.21446	29333.9	12015.5	5427.4	S
64.422	0.0949	0.0000	82.475	0.03446	0.21411	29341.5	12018.3	5444.6	S
64.444	0.0946	0.0000	82.473	0.03430	0.21375	29349.1	12021.0	5461.7	S
64.467	0.0943	0.0000	82.471	0.03415	0.21339	29356.7	12023.8	5478.8	S
64.489	0.0942	0.0000	82.470	0.03400	0.21304	29364.2	12026.5	5495.8	S
64.511	0.0941	0.0000	82.468	0.03386	0.21268	29371.7	12029.2	5512.9	S
64.533	0.0940	0.0000	82.466	0.03372	0.21232	29379.3	12031.9	5529.9	S
64.556	0.0940	0.0000	82.464	0.03359	0.21197	29386.8	12034.6	5546.8	S
64.578	0.0940	0.0000	82.462	0.03346	0.21161	29394.3	12037.3	5563.8	S
64.600	0.0940	0.0000	82.460	0.03334	0.21125	29401.8	12040.0	5580.7	S
64.622	0.0940	0.0000	82.458	0.03321	0.21090	29409.3	12042.6	5597.6	S
64.644	0.0940	0.0000	82.456	0.03310	0.21054	29416.9	12045.3	5614.4	S
64.667	0.0940	0.0000	82.454	0.03298	0.21019	29424.4	12047.9	5631.3	S
64.689	0.0940	0.0000	82.453	0.03287	0.20983	29431.9	12050.6	5648.1	S
64.711	0.0940	0.0000	82.451	0.03276	0.20948	29439.4	12053.2	5664.8	S
64.733	0.0940	0.0000	82.449	0.03265	0.20913	29447.0	12055.8	5681.6	S
64.756	0.0940	0.0000	82.447	0.03255	0.20877	29454.5	12058.4	5698.3	S
64.778	0.0940	0.0000	82.445	0.03245	0.20842	29462.0	12061.0	5715.0	S
64.800	0.0940	0.0000	82.443	0.03235	0.20807	29469.5	12063.6	5731.7	S
64.822	0.0940	0.0000	82.441	0.03225	0.20772	29477.0	12066.2	5748.3	S
64.844	0.0940	0.0000	82.440	0.03215	0.20736	29484.6	12068.8	5764.9	S
64.867	0.0940	0.0000	82.438	0.03206	0.20701	29492.1	12071.3	5781.5	S
64.889	0.0940	0.0000	82.436	0.03197	0.20666	29499.6	12073.9	5798.0	S
64.911	0.0940	0.0000	82.434	0.03188	0.20631	29507.1	12076.4	5814.5	S
64.933	0.0940	0.0000	82.432	0.03179	0.20596	29514.7	12079.0	5831.0	S
64.956	0.0940	0.0000	82.431	0.03170	0.20561	29522.2	12081.5	5847.5	S
64.978	0.0940	0.0000	82.429	0.03161	0.20526	29529.7	12084.1	5863.9	S
65.000	0.0940	0.0000	82.427	0.03153	0.20491	29537.2	12086.6	5880.3	S
65.022	0.0940	0.0000	82.425	0.03144	0.20456	29544.8	12089.1	5896.7	S
65.044	0.0940	0.0000	82.423	0.03136	0.20421	29552.3	12091.6	5913.1	S
65.067	0.0940	0.0000	82.422	0.03128	0.20386	29559.8	12094.1	5929.4	S
65.089	0.0940	0.0000	82.420	0.03119	0.20351	29567.3	12096.6	5945.7	S
65.111	0.0940	0.0000	82.418	0.03111	0.20317	29574.8	12099.1	5961.9	S
65.133	0.0940	0.0000	82.416	0.03104	0.20282	29582.4	12101.6	5978.2	S
65.156	0.0940	0.0000	82.414	0.03096	0.20247	29589.9	12104.1	5994.4	S
65.178	0.0940	0.0000	82.413	0.03088	0.20212	29597.4	12106.6	6010.6	S
65.200	0.0940	0.0000	82.411	0.03080	0.20178	29604.9	12109.0	6026.7	S
65.222	0.0940	0.0000	82.409	0.03073	0.20143	29612.5	12111.5	6042.9	S
65.244	0.0940	0.0000	82.407	0.03065	0.20108	29620.0	12113.9	6059.0	S
65.267	0.0940	0.0000	82.406	0.03058	0.20074	29627.5	12116.4	6075.0	S
65.289	0.0940	0.0000	82.404	0.03051	0.20039	29635.0	12118.8	6091.1	S
65.311	0.0940	0.0000	82.402	0.03043	0.20005	29642.5	12121.3	6107.1	S
65.333	0.0940	0.0000	82.400	0.03036	0.19970	29650.1	12123.7	6123.1	S
65.356	0.0940	0.0000	82.399	0.03029	0.19936	29657.6	12126.1	6139.0	S
65.378	0.0940	0.0000	82.397	0.03022	0.19901	29665.1	12128.5	6155.0	S
65.400	0.0940	0.0000	82.395	0.03015	0.19867	29672.6	12131.0	6170.9	S
65.422	0.0940	0.0000	82.394	0.03008	0.19832	29680.2	12133.4	6186.8	S
65.444	0.0940	0.0000	82.392	0.03001	0.19798	29687.7	12135.8	6202.6	S
65.467	0.0940	0.0000	82.390	0.02995	0.19764	29695.2	12138.2	6218.4	S
65.489	0.0940	0.0000	82.388	0.02988	0.19729	29702.7	12140.6	6234.2	S
65.511	0.0940	0.0000	82.387	0.02981	0.19695	29710.2	12143.0	6250.0	S
65.533	0.0940	0.0000	82.385	0.02975	0.19661	29717.8	12145.3	6265.8	S
65.556	0.0940	0.0000	82.383	0.02968	0.19627	29725.3	12147.7	6281.5	S
65.578	0.0940	0.0000	82.382	0.02962	0.19593	29732.8	12150.1	6297.2	S
65.600	0.0940	0.0000	82.380	0.02955	0.19558	29740.3	12152.5	6312.8	S
65.622	0.0940	0.0000	82.378	0.02949	0.19524	29747.9	12154.8	6328.5	S
65.644	0.0940	0.0000	82.377	0.02942	0.19490	29755.4	12157.2	6344.1	S
65.667	0.0940	0.0000	82.375	0.02936	0.19456	29762.9	12159.5	6359.6	S
65.689	0.0940	0.0000	82.373	0.02930	0.19422	29770.4	12161.9	6375.2	S
65.711	0.0940	0.0000	82.372	0.02923	0.19388	29778.0	12164.2	6390.7	S
65.733	0.0940	0.0000	82.370	0.02917	0.19354	29785.5	12166.5	6406.2	S
65.756	0.0940	0.0000	82.368	0.02911	0.19320	29793.0	12168.9	6421.7	S

Detailed Results (cont,d.) *:: Scenario 38 :: FDOT 72 Hour - 72 hr - 50 yr*

Elapsed Time (hours)	Inflow Rate (ft^3/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft^3/s)	Overflow Discharge (ft^3/s)	Cumulative Inflow Volume (ft^3)	Cumulative Infiltration Volume (ft^3)	Cumulative Discharge Volume (ft^3)	Flow Type
72.356	0.0006	0.0000	81.965	0.01857	0.07188	31205.5	12705.4	9686.3	S
72.378	0.0004	0.0000	81.964	0.01852	0.07116	31205.5	12706.9	9692.0	S
72.400	0.0003	0.0000	81.962	0.01848	0.07045	31205.5	12708.4	9697.7	S
72.422	0.0002	0.0000	81.961	0.01844	0.06973	31205.6	12709.9	9703.3	S
72.444	0.0001	0.0000	81.960	0.01841	0.06901	31205.6	12711.3	9708.8	S
72.467	0.0001	0.0000	81.959	0.01837	0.06829	31205.6	12712.8	9714.3	S
72.489	0.0000	0.0000	81.957	0.01833	0.06756	31205.6	12714.3	9719.8	S
72.511	0.0000	0.0000	81.956	0.01830	0.06684	31205.6	12715.7	9725.1	S
72.533	0.0000	0.0000	81.955	0.01827	0.06611	31205.6	12717.2	9730.5	S
72.556	0.0000	0.0000	81.954	0.01824	0.06538	31205.6	12718.7	9735.7	S
72.578	0.0000	0.0000	81.952	0.01800	0.06465	31205.6	12720.1	9740.9	S
74.978	0.0000	0.0000	81.902	0.00305	0.01319	31205.6	12662.4	10077.2	S
77.378	0.0000	0.0000	81.872	0.01603	0.00000	31205.6	12772.9	10134.2	S
79.778	0.0000	0.0000	81.841	0.01911	0.00000	31205.6	12939.3	10134.2	S
82.178	0.0000	0.0000	81.811	0.01877	0.00000	31205.6	13103.1	10134.2	S
84.578	0.0000	0.0000	81.781	0.01833	0.00000	31205.6	13263.5	10134.2	S
86.978	0.0000	0.0000	81.751	0.01784	0.00000	31205.6	13419.9	10134.2	S
89.378	0.0000	0.0000	81.722	0.01733	0.00000	31205.6	13571.9	10134.2	S
91.778	0.0000	0.0000	81.694	0.01680	0.00000	31205.6	13719.3	10134.2	S
94.178	0.0000	0.0000	81.666	0.01627	0.00000	31205.6	13862.1	10134.2	S
96.578	0.0000	0.0000	81.639	0.01571	0.00000	31205.6	14000.5	10134.2	S
120.578	0.0000	0.0000	81.418	0.01145	0.00000	31205.6	15092.1	10134.2	S
144.578	0.0000	0.0000	81.227	0.00941	0.00000	31205.6	15979.8	10134.2	S
168.578	0.0000	0.0000	81.057	0.00788	0.00000	31205.6	16717.8	10134.2	S
192.578	0.0000	0.0000	80.906	0.00671	0.00000	31205.6	17342.0	10134.2	S
216.578	0.0000	0.0000	80.769	0.00580	0.00000	31205.6	17877.9	10134.2	S
240.578	0.0000	0.0000	80.645	0.00506	0.00000	31205.6	18343.6	10134.2	S
264.578	0.0000	0.0000	80.531	0.00446	0.00000	31205.6	18752.3	10134.2	S
288.578	0.0000	0.0000	80.425	0.00396	0.00000	31205.6	19113.9	10134.2	S
312.578	0.0000	0.0000	80.327	0.00354	0.00000	31205.6	19436.2	10134.2	S
336.578	0.0000	0.0000	80.236	0.00317	0.00000	31205.6	19724.8	10134.2	S
360.578	0.0000	0.0000	80.151	0.00286	0.00000	31205.6	19984.6	10134.2	S
384.578	0.0000	0.0000	80.070	0.00259	0.00000	31205.6	20219.2	10134.2	S
408.578	0.0000	0.0000	79.995	0.00235	0.00000	31205.6	20432.0	10134.2	S
432.578	0.0000	0.0000	79.924	0.00214	0.00000	31205.6	20625.5	10134.2	S
456.578	0.0000	0.0000	79.857	0.00196	0.00000	31205.6	20802.2	10134.2	S
480.578	0.0000	0.0000	79.793	0.00156	0.00000	31205.6	20964.0	10134.2	S
504.578	0.0000	0.0000	79.725	0.00062	0.00000	31205.6	21071.4	10134.2	S
528.578	0.0000	0.0000	79.643	0.00000	0.00000	31205.6	21071.4	10134.2	S
552.578	0.0000	0.0000	79.571	0.00000	0.00000	31205.6	21071.4	10134.2	S
576.578	0.0000	0.0000	79.507	0.00000	0.00000	31205.6	21071.4	10134.2	S
600.578	0.0000	0.0000	79.449	0.00000	0.00000	31205.6	21071.4	10134.2	S
624.578	0.0000	0.0000	79.395	0.00000	0.00000	31205.6	21071.4	10134.2	S
648.578	0.0000	0.0000	79.345	0.00000	0.00000	31205.6	21071.4	10134.2	S
672.578	0.0000	0.0000	79.299	0.00000	0.00000	31205.6	21071.4	10134.2	S
696.578	0.0000	0.0000	79.255	0.00000	0.00000	31205.6	21071.4	10134.2	S
720.578	0.0000	0.0000	79.215	0.00000	0.00000	31205.6	21071.4	10134.2	S
744.578	0.0000	0.0000	79.176	0.00000	0.00000	31205.6	21071.4	10134.2	S
768.578	0.0000	0.0000	79.140	0.00000	0.00000	31205.6	21071.4	10134.2	S
792.578	0.0000	0.0000	79.106	----	----	31205.6	21071.4	10134.2	N.A.

Detailed Results (cont,d.) *:: Scenario 39 :: FDOT 168 Hour - 168 hr - 50 yr*

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
159.511	0.3224	0.0000	82.494	0.03063	0.21765	34384.6	16068.7	6313.9	S
159.533	0.3225	0.0000	82.495	0.03062	0.21782	34410.4	16071.1	6331.3	S
159.556	0.3225	0.0000	82.496	0.03062	0.21798	34436.2	16073.6	6348.7	S
159.578	0.3225	0.0000	82.497	0.03062	0.21815	34462.0	16076.0	6366.2	S
159.600	0.3225	0.0000	82.498	0.03062	0.21832	34487.8	16078.5	6383.6	S
159.622	0.3225	0.0000	82.499	0.03061	0.21848	34513.6	16080.9	6401.1	S
159.644	0.3225	0.0000	82.500	0.03061	0.21865	34539.4	16083.4	6418.6	S
159.667	0.3225	0.0000	82.501	0.03061	0.21881	34565.2	16085.8	6436.1	S
159.689	0.3225	0.0000	82.502	0.03061	0.21898	34591.0	16088.3	6453.6	S
159.711	0.3225	0.0000	82.503	0.03060	0.21914	34616.8	16090.7	6471.1	S
159.733	0.3225	0.0000	82.504	0.03060	0.21931	34642.6	16093.2	6488.6	S
159.756	0.3225	0.0000	82.504	0.03060	0.21947	34668.4	16095.6	6506.2	S
159.778	0.3225	0.0000	82.505	0.03059	0.21963	34694.2	16098.1	6523.8	S
159.800	0.3225	0.0000	82.506	0.03059	0.21979	34720.0	16100.5	6541.3	S
159.822	0.3225	0.0000	82.507	0.03059	0.21996	34745.8	16103.0	6558.9	S
159.845	0.3225	0.0000	82.508	0.03059	0.22012	34771.6	16105.4	6576.5	S
159.867	0.3225	0.0000	82.509	0.03058	0.22028	34797.4	16107.9	6594.1	S
159.889	0.3225	0.0000	82.510	0.03058	0.22044	34823.2	16110.3	6611.8	S
159.911	0.3225	0.0000	82.511	0.03058	0.22060	34849.0	16112.8	6629.4	S
159.933	0.3225	0.0000	82.512	0.03057	0.22075	34874.8	16115.2	6647.1	S
159.956	0.3225	0.0000	82.512	0.03057	0.22091	34900.5	16117.7	6664.7	S
159.978	0.3225	0.0000	82.513	0.03057	0.22107	34926.3	16120.1	6682.4	S
160.000	0.3225	0.0000	82.514	0.03056	0.22123	34952.1	16122.5	6700.1	S
160.022	0.3191	0.0000	82.515	0.03054	0.22138	34977.8	16125.0	6717.8	S
160.044	0.3081	0.0000	82.516	0.03050	0.22152	35002.9	16127.4	6735.5	S
160.067	0.2842	0.0000	82.516	0.03039	0.22162	35026.6	16129.9	6753.3	S
160.089	0.2492	0.0000	82.517	0.03022	0.22165	35047.9	16132.3	6771.0	S
160.111	0.2103	0.0000	82.516	0.02998	0.22160	35066.3	16134.7	6788.7	S
160.133	0.1733	0.0000	82.516	0.02970	0.22147	35081.6	16137.1	6806.4	S
160.156	0.1418	0.0000	82.514	0.02937	0.22126	35094.3	16139.5	6824.1	S
160.178	0.1186	0.0000	82.513	0.02904	0.22099	35104.7	16141.8	6841.8	S
160.200	0.1023	0.0000	82.511	0.02869	0.22068	35113.5	16144.1	6859.5	S
160.222	0.0906	0.0000	82.509	0.02836	0.22034	35121.2	16146.4	6877.1	S
160.244	0.0821	0.0000	82.507	0.02803	0.21998	35128.1	16148.6	6894.8	S
160.267	0.0760	0.0000	82.505	0.02771	0.21960	35134.4	16150.9	6912.3	S
160.289	0.0716	0.0000	82.503	0.02740	0.21921	35140.3	16153.1	6929.9	S
160.311	0.0684	0.0000	82.501	0.02711	0.21881	35145.9	16155.3	6947.4	S
160.333	0.0661	0.0000	82.499	0.02683	0.21841	35151.3	16157.4	6964.9	S
160.356	0.0644	0.0000	82.496	0.02656	0.21800	35156.5	16159.5	6982.4	S
160.378	0.0632	0.0000	82.494	0.02631	0.21760	35161.6	16161.7	6999.8	S
160.400	0.0623	0.0000	82.492	0.02606	0.21718	35166.7	16163.8	7017.2	S
160.422	0.0617	0.0000	82.490	0.02583	0.21677	35171.6	16165.8	7034.5	S
160.444	0.0613	0.0000	82.487	0.02560	0.21636	35176.6	16167.9	7051.9	S
160.467	0.0609	0.0000	82.485	0.02539	0.21594	35181.4	16169.9	7069.2	S
160.489	0.0607	0.0000	82.483	0.02518	0.21553	35186.3	16171.9	7086.4	S
160.511	0.0605	0.0000	82.481	0.02498	0.21512	35191.1	16174.0	7103.6	S
160.533	0.0604	0.0000	82.478	0.02479	0.21470	35196.0	16175.9	7120.8	S
160.556	0.0604	0.0000	82.476	0.02461	0.21429	35200.8	16177.9	7138.0	S
160.578	0.0604	0.0000	82.474	0.02443	0.21387	35205.6	16179.9	7155.1	S
160.600	0.0604	0.0000	82.472	0.02426	0.21346	35210.5	16181.8	7172.2	S
160.622	0.0604	0.0000	82.470	0.02410	0.21305	35215.3	16183.8	7189.3	S
160.644	0.0604	0.0000	82.467	0.02393	0.21263	35220.1	16185.7	7206.3	S
160.667	0.0604	0.0000	82.465	0.02378	0.21222	35225.0	16187.6	7223.3	S
160.689	0.0604	0.0000	82.463	0.02363	0.21181	35229.8	16189.5	7240.3	S
160.711	0.0604	0.0000	82.461	0.02348	0.21140	35234.6	16191.4	7257.2	S
160.733	0.0604	0.0000	82.459	0.02334	0.21099	35239.5	16193.2	7274.1	S
160.756	0.0604	0.0000	82.456	0.02320	0.21058	35244.3	16195.1	7290.9	S
160.778	0.0604	0.0000	82.454	0.02306	0.21017	35249.1	16197.0	7307.8	S
160.800	0.0604	0.0000	82.452	0.02293	0.20975	35254.0	16198.8	7324.6	S
160.822	0.0604	0.0000	82.450	0.02280	0.20934	35258.8	16200.6	7341.3	S
160.845	0.0604	0.0000	82.448	0.02267	0.20893	35263.6	16202.4	7358.1	S
160.867	0.0604	0.0000	82.446	0.02254	0.20852	35268.4	16204.3	7374.8	S
160.889	0.0604	0.0000	82.444	0.02242	0.20811	35273.3	16206.1	7391.4	S
160.911	0.0604	0.0000	82.441	0.02231	0.20771	35278.1	16207.8	7408.1	S
160.933	0.0604	0.0000	82.439	0.02219	0.20730	35282.9	16209.6	7424.7	S
160.956	0.0604	0.0000	82.437	0.02207	0.20689	35287.8	16211.4	7441.2	S
160.978	0.0604	0.0000	82.435	0.02196	0.20648	35292.6	16213.2	7457.8	S
161.000	0.0604	0.0000	82.433	0.02185	0.20607	35297.4	16214.9	7474.3	S
161.022	0.0604	0.0000	82.431	0.02174	0.20566	35302.3	16216.6	7490.7	S
161.044	0.0604	0.0000	82.429	0.02164	0.20525	35307.1	16218.4	7507.2	S
161.067	0.0604	0.0000	82.427	0.02153	0.20485	35311.9	16220.1	7523.6	S
161.089	0.0604	0.0000	82.425	0.02143	0.20444	35316.7	16221.8	7539.9	S
161.111	0.0604	0.0000	82.422	0.02133	0.20403	35321.6	16223.5	7556.3	S
161.133	0.0604	0.0000	82.420	0.02123	0.20362	35326.4	16225.2	7572.6	S

Detailed Results (cont,d.) :: *Scenario 39 :: FDOT 168 Hour - 168 hr - 50 yr*

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
167.733	0.0604	0.0000	82.017	0.01289	0.09659	36761.3	16582.8	11070.3	S
167.756	0.0604	0.0000	82.016	0.01289	0.09631	36766.2	16583.8	11078.0	S
167.778	0.0604	0.0000	82.016	0.01289	0.09602	36771.0	16584.9	11085.7	S
167.800	0.0604	0.0000	82.015	0.01289	0.09574	36775.8	16585.9	11093.3	S
167.822	0.0604	0.0000	82.014	0.01289	0.09546	36780.7	16586.9	11101.0	S
167.845	0.0604	0.0000	82.014	0.01289	0.09517	36785.5	16588.0	11108.6	S
167.867	0.0604	0.0000	82.013	0.01290	0.09489	36790.3	16589.0	11116.2	S
167.889	0.0604	0.0000	82.012	0.01290	0.09461	36795.2	16590.0	11123.8	S
167.911	0.0604	0.0000	82.012	0.01290	0.09433	36800.0	16591.1	11131.4	S
167.933	0.0604	0.0000	82.011	0.01290	0.09406	36804.8	16592.1	11138.9	S
167.956	0.0604	0.0000	82.010	0.01290	0.09378	36809.7	16593.1	11146.4	S
167.978	0.0604	0.0000	82.010	0.01290	0.09350	36814.5	16594.2	11153.9	S
168.000	0.0604	0.0000	82.009	0.01290	0.09322	36819.3	16595.2	11161.4	S
168.022	0.0595	0.0000	82.008	0.01290	0.09295	36824.1	16596.2	11168.8	S
168.044	0.0567	0.0000	82.008	0.01288	0.09266	36828.8	16597.2	11176.2	S
168.067	0.0507	0.0000	82.007	0.01284	0.09234	36833.1	16598.3	11183.6	S
168.089	0.0423	0.0000	82.006	0.01279	0.09198	36836.8	16599.3	11191.0	S
168.111	0.0332	0.0000	82.005	0.01271	0.09158	36839.8	16600.3	11198.4	S
168.133	0.0248	0.0000	82.004	0.01263	0.09111	36842.1	16601.3	11205.7	S
168.156	0.0177	0.0000	82.003	0.01254	0.09060	36843.8	16602.3	11212.9	S
168.178	0.0127	0.0000	82.002	0.01245	0.09006	36845.0	16603.3	11220.2	S
168.200	0.0091	0.0000	82.000	0.01236	0.08948	36845.9	16604.3	11227.3	S
168.222	0.0066	0.0000	81.999	0.01227	0.08889	36846.5	16605.3	11234.5	S
168.244	0.0047	0.0000	81.998	0.01219	0.08828	36847.0	16606.3	11241.6	S
168.267	0.0034	0.0000	81.996	0.01211	0.08767	36847.3	16607.3	11248.6	S
168.289	0.0024	0.0000	81.995	0.01204	0.08704	36847.6	16608.2	11255.6	S
168.311	0.0017	0.0000	81.994	0.01197	0.08641	36847.7	16609.2	11262.5	S
168.333	0.0012	0.0000	81.992	0.01191	0.08578	36847.8	16610.1	11269.4	S
168.356	0.0009	0.0000	81.991	0.01185	0.08514	36847.9	16611.1	11276.2	S
168.378	0.0006	0.0000	81.990	0.01179	0.08450	36848.0	16612.0	11283.0	S
168.400	0.0004	0.0000	81.988	0.01174	0.08386	36848.0	16613.0	11289.8	S
168.422	0.0003	0.0000	81.987	0.01168	0.08322	36848.1	16613.9	11296.5	S
168.444	0.0002	0.0000	81.986	0.01164	0.08258	36848.1	16614.9	11303.1	S
168.467	0.0001	0.0000	81.984	0.01159	0.08193	36848.1	16615.8	11309.7	S
168.489	0.0001	0.0000	81.983	0.01155	0.08128	36848.1	16616.7	11316.2	S
168.511	0.0000	0.0000	81.982	0.01150	0.08064	36848.1	16617.6	11322.7	S
168.533	0.0000	0.0000	81.980	0.01147	0.07999	36848.1	16618.6	11329.1	S
168.556	0.0000	0.0000	81.979	0.01143	0.07934	36848.1	16619.5	11335.5	S
168.578	0.0000	0.0000	81.978	0.01120	0.07869	36848.1	16620.4	11341.8	S
170.978	0.0000	0.0000	81.912	-0.00728	0.03062	36848.1	16515.3	11814.0	S
173.378	0.0000	0.0000	81.891	0.00533	0.00000	36848.1	16494.5	11946.3	S
175.778	0.0000	0.0000	81.871	0.01303	0.00000	36848.1	16607.4	11946.3	S
178.178	0.0000	0.0000	81.850	0.01291	0.00000	36848.1	16719.6	11946.3	S
180.578	0.0000	0.0000	81.830	0.01273	0.00000	36848.1	16830.5	11946.3	S
182.978	0.0000	0.0000	81.809	0.01253	0.00000	36848.1	16939.7	11946.3	S
185.378	0.0000	0.0000	81.789	0.01231	0.00000	36848.1	17047.0	11946.3	S
187.778	0.0000	0.0000	81.770	0.01209	0.00000	36848.1	17152.4	11946.3	S
190.178	0.0000	0.0000	81.750	0.01186	0.00000	36848.1	17255.9	11946.3	S
192.578	0.0000	0.0000	81.731	0.01160	0.00000	36848.1	17357.4	11946.3	S
216.578	0.0000	0.0000	81.560	0.00943	0.00000	36848.1	18228.4	11946.3	S
240.578	0.0000	0.0000	81.405	0.00823	0.00000	36848.1	18986.3	11946.3	S
264.578	0.0000	0.0000	81.262	0.00725	0.00000	36848.1	19651.4	11946.3	S
288.578	0.0000	0.0000	81.130	0.00642	0.00000	36848.1	20238.7	11946.3	S
312.578	0.0000	0.0000	81.007	0.00571	0.00000	36848.1	20760.2	11946.3	S
336.578	0.0000	0.0000	80.893	0.00511	0.00000	36848.1	21225.9	11946.3	S
360.578	0.0000	0.0000	80.786	0.00460	0.00000	36848.1	21643.8	11946.3	S
384.578	0.0000	0.0000	80.687	0.00415	0.00000	36848.1	22020.5	11946.3	S
408.578	0.0000	0.0000	80.593	0.00377	0.00000	36848.1	22361.6	11946.3	S
432.578	0.0000	0.0000	80.505	0.00343	0.00000	36848.1	22671.4	11946.3	S
456.578	0.0000	0.0000	80.422	0.00313	0.00000	36848.1	22953.9	11946.3	S
480.578	0.0000	0.0000	80.344	0.00287	0.00000	36848.1	23212.3	11946.3	S
504.578	0.0000	0.0000	80.270	0.00263	0.00000	36848.1	23449.4	11946.3	S
528.578	0.0000	0.0000	80.200	0.00243	0.00000	36848.1	23667.4	11946.3	S
552.578	0.0000	0.0000	80.133	0.00224	0.00000	36848.1	23868.5	11946.3	S
576.578	0.0000	0.0000	80.069	0.00207	0.00000	36848.1	24054.3	11946.3	S
600.578	0.0000	0.0000	80.008	0.00192	0.00000	36848.1	24226.4	11946.3	S
624.578	0.0000	0.0000	79.950	0.00178	0.00000	36848.1	24386.1	11946.3	S
648.578	0.0000	0.0000	79.894	0.00166	0.00000	36848.1	24534.7	11946.3	S
672.578	0.0000	0.0000	79.841	0.00155	0.00000	36848.1	24673.1	11946.3	S
696.578	0.0000	0.0000	79.790	0.00132	0.00000	36848.1	24802.2	11946.3	S
720.578	0.0000	0.0000	79.737	0.00058	0.00000	36848.1	24901.8	11946.3	S
744.578	0.0000	0.0000	79.669	0.00000	0.00000	36848.1	24901.8	11946.3	S
768.578	0.0000	0.0000	79.609	0.00000	0.00000	36848.1	24901.8	11946.3	S
792.578	0.0000	0.0000	79.554	0.00000	0.00000	36848.1	24901.8	11946.3	S

Detailed Results (cont,d.) :: *Scenario 40 :: FDOT 240 Hour - 240 hr - 50 yr*

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
185.822	0.0816	0.0000	82.531	0.02582	0.22418	34316.9	16115.8	5964.4	S
185.845	0.0816	0.0000	82.529	0.02572	0.22381	34323.4	16117.9	5982.3	S
185.867	0.0816	0.0000	82.527	0.02563	0.22345	34329.9	16119.9	6000.2	S
185.889	0.0816	0.0000	82.525	0.02554	0.22308	34336.4	16122.0	6018.1	S
185.911	0.0816	0.0000	82.522	0.02545	0.22271	34343.0	16124.0	6035.9	S
185.933	0.0816	0.0000	82.520	0.02536	0.22234	34349.5	16126.0	6053.7	S
185.956	0.0816	0.0000	82.518	0.02528	0.22198	34356.0	16128.1	6071.5	S
185.978	0.0816	0.0000	82.516	0.02519	0.22161	34362.6	16130.1	6089.2	S
186.000	0.0816	0.0000	82.514	0.02510	0.22124	34369.1	16132.1	6107.0	S
186.022	0.0816	0.0000	82.512	0.02502	0.22088	34375.6	16134.1	6124.6	S
186.044	0.0816	0.0000	82.510	0.02493	0.22051	34382.1	16136.1	6142.3	S
186.067	0.0816	0.0000	82.508	0.02485	0.22015	34388.7	16138.1	6159.9	S
186.089	0.0816	0.0000	82.506	0.02477	0.21978	34395.2	16140.1	6177.5	S
186.111	0.0816	0.0000	82.504	0.02468	0.21941	34401.7	16142.0	6195.1	S
186.133	0.0816	0.0000	82.502	0.02460	0.21905	34408.2	16144.0	6212.6	S
186.156	0.0816	0.0000	82.500	0.02452	0.21868	34414.8	16146.0	6230.1	S
186.178	0.0816	0.0000	82.498	0.02444	0.21832	34421.3	16147.9	6247.6	S
186.200	0.0816	0.0000	82.496	0.02436	0.21796	34427.8	16149.9	6265.1	S
186.222	0.0816	0.0000	82.494	0.02428	0.21759	34434.4	16151.8	6282.5	S
186.244	0.0816	0.0000	82.492	0.02421	0.21723	34440.9	16153.8	6299.9	S
186.267	0.0816	0.0000	82.490	0.02413	0.21686	34447.4	16155.7	6317.2	S
186.289	0.0816	0.0000	82.488	0.02405	0.21650	34453.9	16157.6	6334.6	S
186.311	0.0816	0.0000	82.486	0.02398	0.21614	34460.5	16159.6	6351.9	S
186.333	0.0816	0.0000	82.484	0.02390	0.21577	34467.0	16161.5	6369.2	S
186.356	0.0816	0.0000	82.482	0.02383	0.21541	34473.5	16163.4	6386.4	S
186.378	0.0816	0.0000	82.480	0.02375	0.21505	34480.1	16165.3	6403.6	S
186.400	0.0816	0.0000	82.478	0.02368	0.21468	34486.6	16167.2	6420.8	S
186.422	0.0816	0.0000	82.476	0.02361	0.21432	34493.1	16169.1	6438.0	S
186.444	0.0816	0.0000	82.474	0.02354	0.21396	34499.6	16171.0	6455.1	S
186.467	0.0816	0.0000	82.473	0.02346	0.21360	34506.2	16172.8	6472.2	S
186.489	0.0816	0.0000	82.471	0.02339	0.21324	34512.7	16174.7	6489.3	S
186.511	0.0816	0.0000	82.469	0.02332	0.21287	34519.2	16176.6	6506.3	S
186.533	0.0816	0.0000	82.467	0.02325	0.21251	34525.7	16178.4	6523.3	S
186.556	0.0816	0.0000	82.465	0.02318	0.21215	34532.3	16180.3	6540.3	S
186.578	0.0816	0.0000	82.463	0.02312	0.21179	34538.8	16182.2	6557.3	S
186.600	0.0816	0.0000	82.461	0.02305	0.21143	34545.3	16184.0	6574.2	S
186.622	0.0816	0.0000	82.459	0.02298	0.21107	34551.9	16185.8	6591.1	S
186.644	0.0816	0.0000	82.457	0.02291	0.21071	34558.4	16187.7	6608.0	S
186.667	0.0816	0.0000	82.455	0.02285	0.21035	34564.9	16189.5	6624.8	S
186.689	0.0816	0.0000	82.453	0.02278	0.20999	34571.4	16191.3	6641.6	S
186.711	0.0816	0.0000	82.451	0.02272	0.20963	34578.0	16193.2	6658.4	S
186.733	0.0816	0.0000	82.450	0.02265	0.20927	34584.5	16195.0	6675.2	S
186.756	0.0816	0.0000	82.448	0.02259	0.20891	34591.0	16196.8	6691.9	S
186.778	0.0816	0.0000	82.446	0.02252	0.20855	34597.5	16198.6	6708.6	S
186.800	0.0816	0.0000	82.444	0.02246	0.20819	34604.1	16200.4	6725.3	S
186.822	0.0816	0.0000	82.442	0.02240	0.20783	34610.6	16202.2	6741.9	S
186.845	0.0816	0.0000	82.440	0.02233	0.20748	34617.1	16204.0	6758.5	S
186.867	0.0816	0.0000	82.438	0.02227	0.20712	34623.7	16205.7	6775.1	S
186.889	0.0816	0.0000	82.436	0.02221	0.20676	34630.2	16207.5	6791.7	S
186.911	0.0816	0.0000	82.435	0.02215	0.20640	34636.7	16209.3	6808.2	S
186.933	0.0816	0.0000	82.433	0.02209	0.20605	34643.2	16211.1	6824.7	S
186.956	0.0816	0.0000	82.431	0.02203	0.20569	34649.8	16212.8	6841.2	S
186.978	0.0816	0.0000	82.429	0.02197	0.20533	34656.3	16214.6	6857.6	S
187.000	0.0816	0.0000	82.427	0.02191	0.20498	34662.8	16216.3	6874.0	S
187.022	0.0816	0.0000	82.425	0.02185	0.20462	34669.4	16218.1	6890.4	S
187.044	0.0816	0.0000	82.424	0.02180	0.20426	34675.9	16219.8	6906.8	S
187.067	0.0816	0.0000	82.422	0.02174	0.20391	34682.4	16221.6	6923.1	S
187.089	0.0816	0.0000	82.420	0.02168	0.20355	34688.9	16223.3	6939.4	S
187.111	0.0816	0.0000	82.418	0.02162	0.20320	34695.5	16225.1	6955.7	S
187.133	0.0816	0.0000	82.416	0.02157	0.20284	34702.0	16226.8	6971.9	S
187.156	0.0816	0.0000	82.415	0.02151	0.20249	34708.5	16228.5	6988.1	S
187.178	0.0816	0.0000	82.413	0.02146	0.20213	34715.0	16230.2	7004.3	S
187.200	0.0816	0.0000	82.411	0.02140	0.20178	34721.6	16231.9	7020.4	S
187.222	0.0816	0.0000	82.409	0.02135	0.20142	34728.1	16233.7	7036.6	S
187.244	0.0816	0.0000	82.407	0.02129	0.20107	34734.6	16235.4	7052.7	S
187.267	0.0816	0.0000	82.406	0.02124	0.20071	34741.2	16237.1	7068.7	S
187.289	0.0816	0.0000	82.404	0.02118	0.20036	34747.7	16238.8	7084.8	S
187.311	0.0816	0.0000	82.402	0.02113	0.20001	34754.2	16240.4	7100.8	S
187.333	0.0816	0.0000	82.400	0.02108	0.19966	34760.7	16242.1	7116.8	S
187.356	0.0816	0.0000	82.398	0.02103	0.19930	34767.3	16243.8	7132.7	S
187.378	0.0816	0.0000	82.397	0.02097	0.19895	34773.8	16245.5	7148.7	S
187.400	0.0816	0.0000	82.395	0.02092	0.19860	34780.3	16247.2	7164.6	S
187.422	0.0816	0.0000	82.393	0.02087	0.19825	34786.9	16248.8	7180.5	S
187.444	0.0816	0.0000	82.391	0.02082	0.19789	34793.4	16250.5	7196.3	S

Detailed Results (cont,d.) :: *Scenario 40 :: FDOT 240 Hour - 240 hr - 50 yr*

Elapsed Time (hours)	Inflow Rate (ft^3/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft^3/s)	Overflow Discharge (ft^3/s)	Cumulative Inflow Volume (ft^3)	Cumulative Infiltration Volume (ft^3)	Cumulative Discharge Volume (ft^3)	Flow Type
240.089	0.0154	0.0000	81.901	0.01334	0.00785	42492.0	19038.5	14995.2	S
240.111	0.0121	0.0000	81.901	0.01319	0.00731	42493.1	19039.5	14995.8	S
240.133	0.0090	0.0000	81.901	0.01303	0.00651	42493.9	19040.6	14996.4	S
240.156	0.0065	0.0000	81.900	0.01284	0.00543	42494.5	19041.6	14996.9	S
240.178	0.0046	0.0000	81.900	0.01250	0.00396	42495.0	19042.6	14997.2	S
240.200	0.0033	0.0000	81.900	0.01246	0.00167	42495.3	19043.6	14997.5	S
240.222	0.0024	0.0000	81.900	0.01304	0.00000	42495.5	19044.6	14997.5	S
240.244	0.0017	0.0000	81.900	0.01344	0.00000	42495.7	19045.7	14997.5	S
240.267	0.0012	0.0000	81.900	0.01342	0.00000	42495.8	19046.8	14997.5	S
240.289	0.0009	0.0000	81.899	0.01341	0.00000	42495.9	19047.9	14997.5	S
240.311	0.0006	0.0000	81.899	0.01339	0.00000	42496.0	19048.9	14997.5	S
240.333	0.0005	0.0000	81.899	0.01337	0.00000	42496.0	19050.0	14997.5	S
240.356	0.0003	0.0000	81.899	0.01336	0.00000	42496.0	19051.1	14997.5	S
240.378	0.0002	0.0000	81.899	0.01334	0.00000	42496.1	19052.1	14997.5	S
240.400	0.0002	0.0000	81.898	0.01333	0.00000	42496.1	19053.2	14997.5	S
240.422	0.0001	0.0000	81.898	0.01331	0.00000	42496.1	19054.3	14997.5	S
240.444	0.0001	0.0000	81.898	0.01330	0.00000	42496.1	19055.3	14997.5	S
240.467	0.0000	0.0000	81.898	0.01329	0.00000	42496.1	19056.4	14997.5	S
240.489	0.0000	0.0000	81.898	0.01327	0.00000	42496.1	19057.5	14997.5	S
240.511	0.0000	0.0000	81.897	0.01326	0.00000	42496.1	19058.5	14997.5	S
240.533	0.0000	0.0000	81.897	0.01325	0.00000	42496.1	19059.6	14997.5	S
240.556	0.0000	0.0000	81.897	0.01324	0.00000	42496.1	19060.6	14997.5	S
240.578	0.0000	0.0000	81.897	0.01323	0.00000	42496.1	19061.7	14997.5	S
242.978	0.0000	0.0000	81.877	0.01240	0.00000	42496.1	19170.8	14997.5	S
245.378	0.0000	0.0000	81.858	0.01201	0.00000	42496.1	19276.0	14997.5	S
247.778	0.0000	0.0000	81.839	0.01167	0.00000	42496.1	19378.2	14997.5	S
250.178	0.0000	0.0000	81.820	0.01138	0.00000	42496.1	19477.7	14997.5	S
252.578	0.0000	0.0000	81.802	0.01111	0.00000	42496.1	19574.8	14997.5	S
254.978	0.0000	0.0000	81.785	0.01087	0.00000	42496.1	19669.7	14997.5	S
257.378	0.0000	0.0000	81.767	0.01064	0.00000	42496.1	19762.6	14997.5	S
259.778	0.0000	0.0000	81.750	0.01043	0.00000	42496.1	19853.6	14997.5	S
262.178	0.0000	0.0000	81.733	0.01023	0.00000	42496.1	19942.8	14997.5	S
264.578	0.0000	0.0000	81.716	0.01002	0.00000	42496.1	20030.4	14997.5	S
288.578	0.0000	0.0000	81.567	0.00831	0.00000	42496.1	20792.2	14997.5	S
312.578	0.0000	0.0000	81.430	0.00738	0.00000	42496.1	21465.7	14997.5	S
336.578	0.0000	0.0000	81.301	0.00660	0.00000	42496.1	22067.0	14997.5	S
360.578	0.0000	0.0000	81.182	0.00594	0.00000	42496.1	22607.0	14997.5	S
384.578	0.0000	0.0000	81.069	0.00537	0.00000	42496.1	23093.9	14997.5	S
408.578	0.0000	0.0000	80.964	0.00486	0.00000	42496.1	23534.4	14997.5	S
432.578	0.0000	0.0000	80.864	0.00442	0.00000	42496.1	23934.3	14997.5	S
456.578	0.0000	0.0000	80.771	0.00403	0.00000	42496.1	24298.6	14997.5	S
480.578	0.0000	0.0000	80.683	0.00369	0.00000	42496.1	24631.6	14997.5	S
504.578	0.0000	0.0000	80.599	0.00339	0.00000	42496.1	24936.7	14997.5	S
528.578	0.0000	0.0000	80.520	0.00312	0.00000	42496.1	25217.1	14997.5	S
552.578	0.0000	0.0000	80.445	0.00288	0.00000	42496.1	25475.5	14997.5	S
576.578	0.0000	0.0000	80.373	0.00266	0.00000	42496.1	25714.2	14997.5	S
600.578	0.0000	0.0000	80.305	0.00247	0.00000	42496.1	25935.2	14997.5	S
624.578	0.0000	0.0000	80.240	0.00229	0.00000	42496.1	26140.2	14997.5	S
648.578	0.0000	0.0000	80.178	0.00213	0.00000	42496.1	26330.7	14997.5	S
672.578	0.0000	0.0000	80.118	0.00198	0.00000	42496.1	26508.1	14997.5	S
696.578	0.0000	0.0000	80.061	0.00185	0.00000	42496.1	26673.5	14997.5	S
720.578	0.0000	0.0000	80.006	0.00173	0.00000	42496.1	26828.0	14997.5	S
744.578	0.0000	0.0000	79.954	0.00162	0.00000	42496.1	26972.5	14997.5	S
768.578	0.0000	0.0000	79.903	0.00152	0.00000	42496.1	27107.9	14997.5	S
792.578	0.0000	0.0000	79.855	0.00143	0.00000	42496.1	27234.9	14997.5	S
816.578	0.0000	0.0000	79.808	0.00134	0.00000	42496.1	27354.2	14997.5	S
840.578	0.0000	0.0000	79.763	0.00084	0.00000	42496.1	27466.4	14997.5	S
864.578	0.0000	0.0000	79.705	0.00019	0.00000	42496.1	27498.6	14997.5	S
888.578	0.0000	0.0000	79.647	0.00000	0.00000	42496.1	27498.6	14997.5	S
912.578	0.0000	0.0000	79.595	0.00000	0.00000	42496.1	27498.6	14997.5	S
936.578	0.0000	0.0000	79.547	0.00000	0.00000	42496.1	27498.6	14997.5	S
960.578	0.0000	0.0000	79.502	----	----	42496.1	27498.6	14997.5	N.A.

Detailed Results (cont,d.) :: *Scenario 41 :: FDOT 1 Hour - 1 hr - 100 yr*

Elapsed Time (hours)	Inflow Rate (ft^3/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft^3/s)	Overflow Discharge (ft^3/s)	Cumulative Inflow Volume (ft^3)	Cumulative Infiltration Volume (ft^3)	Cumulative Discharge Volume (ft^3)	Flow Type
8.778	0.0000	0.0000	80.267	0.08984	0.00000	9791.0	8270.2	77.9	U/S
11.178	0.0000	0.0000	80.162	0.03167	0.00000	9791.0	8593.4	77.9	S
13.578	0.0000	0.0000	80.085	0.02322	0.00000	9791.0	8817.4	77.9	S
15.978	0.0000	0.0000	80.023	0.01886	0.00000	9791.0	8994.6	77.9	S
18.378	0.0000	0.0000	79.970	0.01609	0.00000	9791.0	9143.3	77.9	S
20.778	0.0000	0.0000	79.922	0.01412	0.00000	9791.0	9272.6	77.9	S
23.178	0.0000	0.0000	79.878	0.01263	0.00000	9791.0	9387.4	77.9	S
25.578	0.0000	0.0000	79.839	0.01112	0.00000	9791.0	9490.9	77.9	S
49.578	0.0000	0.0000	79.504	0.00129	0.00000	9791.0	9713.1	77.9	S
73.578	0.0000	0.0000	79.253	0.00000	0.00000	9791.0	9713.1	77.9	S
97.578	0.0000	0.0000	79.077	0.00000	0.00000	9791.0	9713.1	77.9	S
121.578	0.0000	0.0000	78.943	0.00000	0.00000	9791.0	9713.1	77.9	S
145.578	0.0000	0.0000	78.836	0.00000	0.00000	9791.0	9713.1	77.9	S
169.578	0.0000	0.0000	78.749	0.00000	0.00000	9791.0	9713.1	77.9	S
193.578	0.0000	0.0000	78.675	0.00000	0.00000	9791.0	9713.1	77.9	S
217.578	0.0000	0.0000	78.611	0.00000	0.00000	9791.0	9713.1	77.9	S
241.578	0.0000	0.0000	78.555	0.00000	0.00000	9791.0	9713.1	77.9	S
265.578	0.0000	0.0000	78.506	0.00000	0.00000	9791.0	9713.1	77.9	S
289.578	0.0000	0.0000	78.461	0.00000	0.00000	9791.0	9713.1	77.9	S
313.578	0.0000	0.0000	78.421	0.00000	0.00000	9791.0	9713.1	77.9	S
337.578	0.0000	0.0000	78.385	0.00000	0.00000	9791.0	9713.1	77.9	S
361.578	0.0000	0.0000	78.351	0.00000	0.00000	9791.0	9713.1	77.9	S
385.578	0.0000	0.0000	78.320	0.00000	0.00000	9791.0	9713.1	77.9	S
409.578	0.0000	0.0000	78.292	0.00000	0.00000	9791.0	9713.1	77.9	S
433.578	0.0000	0.0000	78.266	0.00000	0.00000	9791.0	9713.1	77.9	S
457.578	0.0000	0.0000	78.241	0.00000	0.00000	9791.0	9713.1	77.9	S
481.578	0.0000	0.0000	78.218	0.00000	0.00000	9791.0	9713.1	77.9	S
505.578	0.0000	0.0000	78.196	0.00000	0.00000	9791.0	9713.1	77.9	S
529.578	0.0000	0.0000	78.176	0.00000	0.00000	9791.0	9713.1	77.9	S
553.578	0.0000	0.0000	78.156	0.00000	0.00000	9791.0	9713.1	77.9	S
577.578	0.0000	0.0000	78.138	0.00000	0.00000	9791.0	9713.1	77.9	S
601.578	0.0000	0.0000	78.121	0.00000	0.00000	9791.0	9713.1	77.9	S
625.578	0.0000	0.0000	78.104	0.00000	0.00000	9791.0	9713.1	77.9	S
649.578	0.0000	0.0000	78.089	0.00000	0.00000	9791.0	9713.1	77.9	S
673.578	0.0000	0.0000	78.074	0.00000	0.00000	9791.0	9713.1	77.9	S
697.578	0.0000	0.0000	78.059	0.00000	0.00000	9791.0	9713.1	77.9	S
721.578	0.0000	0.0000	78.046	----	----	9791.0	9713.1	77.9	N.A.

Detailed Results (cont,d.) :: *Scenario 42 :: FDOT 2 Hour - 2 hr - 100 yr*

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
1.644	0.8343	0.0000	82.132	0.40811	0.13583	11676.3	1702.1	205.7	U/P
1.667	0.8091	0.0000	82.135	0.40851	0.13693	11742.1	1734.8	216.6	U/P
1.689	0.7768	0.0000	82.139	0.40886	0.13791	11805.5	1767.5	227.6	U/P
1.711	0.7433	0.0000	82.142	0.40916	0.13874	11866.3	1800.2	238.7	U/P
1.733	0.7126	0.0000	82.144	0.40941	0.13944	11924.5	1832.9	249.8	U/P
1.756	0.6872	0.0000	82.146	0.40962	0.14002	11980.5	1865.7	261.0	U/P
1.778	0.6690	0.0000	82.148	0.40980	0.14052	12034.8	1898.5	272.2	U/P
1.800	0.6563	0.0000	82.149	0.40997	0.14095	12087.8	1931.3	283.5	U/P
1.822	0.6441	0.0000	82.151	0.41011	0.14133	12139.8	1964.1	294.8	U/P
1.844	0.6277	0.0000	82.152	0.41022	0.14165	12190.7	1996.9	306.1	U/P
1.867	0.6024	0.0000	82.153	0.41029	0.14189	12239.9	2029.7	317.4	U/P
1.889	0.5699	0.0000	82.153	0.41032	0.14202	12286.8	2062.5	328.8	U/P
1.911	0.5363	0.0000	82.153	0.41029	0.14202	12331.0	2095.4	340.2	U/P
1.933	0.5056	0.0000	82.153	0.41023	0.14190	12372.7	2128.2	351.5	U/P
1.956	0.4801	0.0000	82.152	0.41012	0.14168	12412.1	2161.0	362.9	U/P
1.978	0.4617	0.0000	82.151	0.40998	0.14137	12449.8	2193.8	374.2	U/P
2.000	0.4490	0.0000	82.150	0.40983	0.14100	12486.2	2226.6	385.5	U/P
2.022	0.4336	0.0000	82.148	0.40965	0.14058	12521.5	2259.4	396.7	U/P
2.044	0.4075	0.0000	82.146	0.40942	0.14008	12555.2	2292.1	408.0	U/P
2.067	0.3615	0.0000	82.144	0.40913	0.13945	12585.9	2324.9	419.1	U/P
2.089	0.2999	0.0000	82.141	0.40875	0.13860	12612.4	2357.6	430.3	U/P
2.111	0.2350	0.0000	82.137	0.40828	0.13750	12633.8	2390.3	441.3	U/P
2.133	0.1752	0.0000	82.133	0.40772	0.13615	12650.2	2422.9	452.3	U/P
2.156	0.1253	0.0000	82.127	0.40709	0.13458	12662.2	2455.5	463.1	U/P
2.178	0.0895	0.0000	82.121	0.40640	0.13281	12670.8	2488.0	473.8	U/P
2.200	0.0645	0.0000	82.115	0.40568	0.13090	12677.0	2520.5	484.3	U/P
2.222	0.0465	0.0000	82.108	0.40493	0.12888	12681.4	2553.0	494.7	U/P
2.244	0.0333	0.0000	82.102	0.40416	0.12677	12684.6	2585.3	504.9	U/P
2.267	0.0238	0.0000	82.095	0.40338	0.12459	12686.9	2617.6	515.0	U/P
2.289	0.0170	0.0000	82.088	0.40259	0.12233	12688.5	2649.9	524.9	U/P
2.311	0.0120	0.0000	82.081	0.40180	0.12002	12689.7	2682.0	534.6	U/P
2.333	0.0085	0.0000	82.074	0.40101	0.11765	12690.5	2714.1	544.1	U/P
2.356	0.0060	0.0000	82.067	0.40021	0.11523	12691.1	2746.2	553.4	U/P
2.378	0.0042	0.0000	82.060	0.39942	0.11275	12691.5	2778.2	562.5	U/P
2.400	0.0029	0.0000	82.052	0.39863	0.11023	12691.8	2810.1	571.4	U/P
2.422	0.0020	0.0000	82.045	0.39784	0.10765	12692.0	2842.0	580.1	U/P
2.444	0.0013	0.0000	82.038	0.39705	0.10502	12692.1	2873.8	588.7	U/P
2.467	0.0008	0.0000	82.031	0.39626	0.10233	12692.2	2905.5	596.9	U/P
2.489	0.0004	0.0000	82.024	0.39548	0.09958	12692.2	2937.2	605.0	U/P
2.511	0.0001	0.0000	82.017	0.39471	0.09676	12692.2	2968.8	612.9	U/P
2.533	0.0000	0.0000	82.011	0.39393	0.09388	12692.2	3000.3	620.5	U/P
2.556	0.0000	0.0000	82.004	0.39316	0.09091	12692.2	3031.8	627.9	U/P
2.578	0.0000	0.0000	81.997	0.39242	0.08787	12692.2	3063.2	635.0	U/P
4.978	0.0000	0.0000	81.318	0.31053	0.00000	12692.2	6123.4	1060.5	U/P
7.378	0.0000	0.0000	80.772	0.15681	0.00000	12692.2	8429.2	1060.5	U/S
9.778	0.0000	0.0000	80.664	0.03974	0.00000	12692.2	8833.1	1060.5	S
12.178	0.0000	0.0000	80.586	0.02942	0.00000	12692.2	9115.9	1060.5	S
14.578	0.0000	0.0000	80.523	0.02410	0.00000	12692.2	9341.5	1060.5	S
16.978	0.0000	0.0000	80.467	0.02072	0.00000	12692.2	9532.4	1060.5	S
19.378	0.0000	0.0000	80.418	0.01830	0.00000	12692.2	9699.5	1060.5	S
21.778	0.0000	0.0000	80.373	0.01646	0.00000	12692.2	9848.7	1060.5	S
24.178	0.0000	0.0000	80.331	0.01500	0.00000	12692.2	9983.9	1060.5	S
26.578	0.0000	0.0000	80.293	0.01387	0.00000	12692.2	10107.8	1060.5	S
50.578	0.0000	0.0000	80.029	0.00777	0.00000	12692.2	10897.3	1060.5	S
74.578	0.0000	0.0000	79.823	0.00425	0.00000	12692.2	11449.9	1060.5	S
98.578	0.0000	0.0000	79.611	0.00105	0.00000	12692.2	11631.8	1060.5	S
122.578	0.0000	0.0000	79.421	0.00000	0.00000	12692.2	11631.8	1060.5	S
146.578	0.0000	0.0000	79.275	0.00000	0.00000	12692.2	11631.8	1060.5	S
170.578	0.0000	0.0000	79.157	0.00000	0.00000	12692.2	11631.8	1060.5	S
194.578	0.0000	0.0000	79.058	0.00000	0.00000	12692.2	11631.8	1060.5	S
218.578	0.0000	0.0000	78.973	0.00000	0.00000	12692.2	11631.8	1060.5	S
242.578	0.0000	0.0000	78.900	0.00000	0.00000	12692.2	11631.8	1060.5	S
266.578	0.0000	0.0000	78.835	0.00000	0.00000	12692.2	11631.8	1060.5	S
290.578	0.0000	0.0000	78.777	0.00000	0.00000	12692.2	11631.8	1060.5	S
314.578	0.0000	0.0000	78.725	0.00000	0.00000	12692.2	11631.8	1060.5	S
338.578	0.0000	0.0000	78.678	0.00000	0.00000	12692.2	11631.8	1060.5	S
362.578	0.0000	0.0000	78.636	0.00000	0.00000	12692.2	11631.8	1060.5	S
386.578	0.0000	0.0000	78.596	0.00000	0.00000	12692.2	11631.8	1060.5	S
410.578	0.0000	0.0000	78.560	0.00000	0.00000	12692.2	11631.8	1060.5	S
434.578	0.0000	0.0000	78.526	0.00000	0.00000	12692.2	11631.8	1060.5	S
458.578	0.0000	0.0000	78.495	0.00000	0.00000	12692.2	11631.8	1060.5	S
482.578	0.0000	0.0000	78.465	0.00000	0.00000	12692.2	11631.8	1060.5	S
506.578	0.0000	0.0000	78.438	0.00000	0.00000	12692.2	11631.8	1060.5	S
530.578	0.0000	0.0000	78.412	0.00000	0.00000	12692.2	11631.8	1060.5	S

Detailed Results (cont,d.) :: *Scenario 43 :: FDOT 4 Hour - 4 hr - 100 yr*

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
3.289	0.5414	0.0000	82.380	0.43571	0.19562	14813.8	3032.9	504.8	U/P
3.311	0.5309	0.0000	82.379	0.43556	0.19537	14856.7	3067.7	520.5	U/P
3.333	0.5235	0.0000	82.378	0.43541	0.19510	14898.9	3102.5	536.1	U/P
3.356	0.5181	0.0000	82.376	0.43525	0.19482	14940.6	3137.4	551.7	U/P
3.378	0.5142	0.0000	82.375	0.43509	0.19452	14981.9	3172.2	567.3	U/P
3.400	0.5113	0.0000	82.373	0.43492	0.19421	15022.9	3207.0	582.8	U/P
3.422	0.5093	0.0000	82.372	0.43475	0.19390	15063.7	3241.8	598.3	U/P
3.444	0.5079	0.0000	82.370	0.43457	0.19359	15104.4	3276.5	613.8	U/P
3.467	0.5067	0.0000	82.369	0.43440	0.19327	15145.0	3311.3	629.3	U/P
3.489	0.5059	0.0000	82.367	0.43423	0.19295	15185.5	3346.0	644.8	U/P
3.511	0.5031	0.0000	82.366	0.43405	0.19262	15225.8	3380.8	660.2	U/P
3.533	0.4936	0.0000	82.364	0.43385	0.19228	15265.7	3415.5	675.6	U/P
3.556	0.4716	0.0000	82.362	0.43362	0.19190	15304.3	3450.2	690.9	U/P
3.578	0.4354	0.0000	82.360	0.43335	0.19144	15340.6	3484.9	706.3	U/P
3.600	0.3916	0.0000	82.357	0.43301	0.19088	15373.7	3519.5	721.6	U/P
3.622	0.3478	0.0000	82.354	0.43262	0.19020	15403.3	3554.2	736.8	U/P
3.644	0.3089	0.0000	82.350	0.43217	0.18940	15429.5	3588.7	752.0	U/P
3.667	0.2785	0.0000	82.346	0.43168	0.18852	15453.0	3623.3	767.1	U/P
3.689	0.2569	0.0000	82.341	0.43116	0.18755	15474.4	3657.8	782.2	U/P
3.711	0.2417	0.0000	82.337	0.43062	0.18654	15494.4	3692.3	797.1	U/P
3.733	0.2306	0.0000	82.332	0.43006	0.18549	15513.3	3726.7	812.0	U/P
3.756	0.2226	0.0000	82.327	0.42950	0.18441	15531.4	3761.1	826.8	U/P
3.778	0.2168	0.0000	82.322	0.42892	0.18330	15549.0	3795.4	841.5	U/P
3.800	0.2126	0.0000	82.316	0.42834	0.18218	15566.1	3829.7	856.1	U/P
3.822	0.2096	0.0000	82.311	0.42776	0.18104	15583.0	3864.0	870.7	U/P
3.844	0.2075	0.0000	82.306	0.42718	0.17989	15599.7	3898.2	885.1	U/P
3.867	0.2059	0.0000	82.301	0.42659	0.17874	15616.3	3932.3	899.4	U/P
3.889	0.2048	0.0000	82.296	0.42601	0.17757	15632.7	3966.4	913.7	U/P
3.911	0.2040	0.0000	82.290	0.42543	0.17640	15649.0	4000.5	927.9	U/P
3.933	0.2034	0.0000	82.285	0.42485	0.17522	15665.3	4034.5	941.9	U/P
3.956	0.2029	0.0000	82.280	0.42427	0.17404	15681.6	4068.5	955.9	U/P
3.978	0.2026	0.0000	82.275	0.42369	0.17285	15697.8	4102.4	969.8	U/P
4.000	0.2024	0.0000	82.270	0.42311	0.17165	15714.0	4136.2	983.5	U/P
4.022	0.1992	0.0000	82.265	0.42252	0.17045	15730.1	4170.1	997.2	U/P
4.044	0.1898	0.0000	82.259	0.42193	0.16922	15745.6	4203.9	1010.8	U/P
4.067	0.1698	0.0000	82.254	0.42130	0.16794	15760.0	4237.6	1024.3	U/P
4.089	0.1416	0.0000	82.248	0.42064	0.16658	15772.5	4271.3	1037.7	U/P
4.111	0.1113	0.0000	82.242	0.41994	0.16512	15782.6	4304.9	1050.9	U/P
4.133	0.0831	0.0000	82.236	0.41920	0.16355	15790.4	4338.4	1064.1	U/P
4.156	0.0594	0.0000	82.229	0.41842	0.16189	15796.1	4372.0	1077.1	U/P
4.178	0.0424	0.0000	82.222	0.41763	0.16015	15800.1	4405.4	1090.0	U/P
4.200	0.0306	0.0000	82.215	0.41681	0.15835	15803.1	4438.8	1102.7	U/P
4.222	0.0221	0.0000	82.207	0.41599	0.15650	15805.2	4472.1	1115.3	U/P
4.244	0.0159	0.0000	82.200	0.41516	0.15461	15806.7	4505.3	1127.8	U/P
4.267	0.0114	0.0000	82.193	0.41432	0.15268	15807.8	4538.5	1140.1	U/P
4.289	0.0081	0.0000	82.185	0.41348	0.15072	15808.6	4571.6	1152.2	U/P
4.311	0.0058	0.0000	82.178	0.41264	0.14873	15809.1	4604.7	1164.2	U/P
4.333	0.0041	0.0000	82.170	0.41180	0.14671	15809.5	4637.6	1176.0	U/P
4.356	0.0029	0.0000	82.163	0.41097	0.14466	15809.8	4670.6	1187.7	U/P
4.378	0.0021	0.0000	82.155	0.41013	0.14259	15810.0	4703.4	1199.1	U/P
4.400	0.0014	0.0000	82.148	0.40929	0.14049	15810.1	4736.2	1210.5	U/P
4.422	0.0010	0.0000	82.140	0.40846	0.13836	15810.2	4768.9	1221.6	U/P
4.444	0.0006	0.0000	82.133	0.40763	0.13621	15810.3	4801.5	1232.6	U/P
4.467	0.0004	0.0000	82.125	0.40680	0.13403	15810.3	4834.1	1243.4	U/P
4.489	0.0002	0.0000	82.118	0.40597	0.13182	15810.4	4866.6	1254.0	U/P
4.511	0.0001	0.0000	82.111	0.40515	0.12958	15810.4	4899.1	1264.5	U/P
4.533	0.0000	0.0000	82.103	0.40433	0.12730	15810.4	4931.4	1274.8	U/P
4.556	0.0000	0.0000	82.096	0.40351	0.12500	15810.4	4963.8	1284.9	U/P
4.578	0.0000	0.0000	82.089	0.40423	0.12266	15810.4	4996.0	1294.8	U/P
6.978	0.0000	0.0000	81.200	0.29069	0.00000	15810.4	9541.5	1294.8	U/S
9.378	0.0000	0.0000	81.091	0.04714	0.00000	15810.4	10019.1	1294.8	S
11.778	0.0000	0.0000	81.011	0.03515	0.00000	15810.4	10356.1	1294.8	S
14.178	0.0000	0.0000	80.945	0.02895	0.00000	15810.4	10626.5	1294.8	S
16.578	0.0000	0.0000	80.888	0.02497	0.00000	15810.4	10856.3	1294.8	S
18.978	0.0000	0.0000	80.837	0.02213	0.00000	15810.4	11058.0	1294.8	S
21.378	0.0000	0.0000	80.791	0.01996	0.00000	15810.4	11238.7	1294.8	S
23.778	0.0000	0.0000	80.748	0.01824	0.00000	15810.4	11402.9	1294.8	S
26.178	0.0000	0.0000	80.708	0.01683	0.00000	15810.4	11553.9	1294.8	S
28.578	0.0000	0.0000	80.671	0.01569	0.00000	15810.4	11693.7	1294.8	S
52.578	0.0000	0.0000	80.406	0.00924	0.00000	15810.4	12621.3	1294.8	S
76.578	0.0000	0.0000	80.196	0.00682	0.00000	15810.4	13290.6	1294.8	S
100.578	0.0000	0.0000	80.022	0.00528	0.00000	15810.4	13800.0	1294.8	S
124.578	0.0000	0.0000	79.873	0.00414	0.00000	15810.4	14203.0	1294.8	S
148.578	0.0000	0.0000	79.741	0.00181	0.00000	15810.4	14515.6	1294.8	S

Detailed Results (cont,d.) :: *Scenario 44 :: FDOT 8 Hour - 8 hr - 100 yr*

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
8.222	0.0191	0.0000	81.972	-0.00002	0.07577	18490.4	7681.8	1954.5	S
8.244	0.0137	0.0000	81.971	-0.00002	0.07532	18491.7	7681.8	1960.6	S
8.267	0.0098	0.0000	81.970	-0.00002	0.07484	18492.7	7681.8	1966.6	S
8.289	0.0070	0.0000	81.969	-0.00001	0.07434	18493.4	7681.8	1972.5	S
8.311	0.0050	0.0000	81.968	0.00001	0.07382	18493.8	7681.8	1978.5	S
8.333	0.0036	0.0000	81.967	0.00002	0.07328	18494.2	7681.8	1984.4	S
8.356	0.0025	0.0000	81.966	0.00005	0.07273	18494.4	7681.8	1990.2	S
8.378	0.0018	0.0000	81.965	0.00007	0.07218	18494.6	7681.8	1996.0	S
8.400	0.0012	0.0000	81.964	0.00010	0.07162	18494.7	7681.8	2001.7	S
8.422	0.0008	0.0000	81.963	0.00013	0.07106	18494.8	7681.8	2007.5	S
8.444	0.0005	0.0000	81.962	0.00017	0.07050	18494.9	7681.8	2013.1	S
8.467	0.0003	0.0000	81.961	0.00021	0.06993	18494.9	7681.8	2018.7	S
8.489	0.0002	0.0000	81.960	0.00025	0.06936	18494.9	7681.8	2024.3	S
8.511	0.0001	0.0000	81.959	0.00030	0.06879	18494.9	7681.9	2029.8	S
8.533	0.0000	0.0000	81.958	0.00035	0.06821	18494.9	7681.9	2035.3	S
8.556	0.0000	0.0000	81.957	0.00041	0.06764	18494.9	7681.9	2040.7	S
8.578	0.0000	0.0000	81.956	0.00129	0.06706	18494.9	7682.0	2046.1	S
10.978	0.0000	0.0000	81.755	0.08011	0.00000	18494.9	8487.2	2335.9	S
13.378	0.0000	0.0000	81.644	0.05734	0.00000	18494.9	9066.2	2335.9	S
15.778	0.0000	0.0000	81.562	0.04307	0.00000	18494.9	9478.0	2335.9	S
18.178	0.0000	0.0000	81.495	0.03566	0.00000	18494.9	9810.4	2335.9	S
20.578	0.0000	0.0000	81.436	0.03090	0.00000	18494.9	10094.2	2335.9	S
22.978	0.0000	0.0000	81.384	0.02749	0.00000	18494.9	10344.3	2335.9	S
25.378	0.0000	0.0000	81.336	0.02489	0.00000	18494.9	10569.3	2335.9	S
27.778	0.0000	0.0000	81.291	0.02282	0.00000	18494.9	10774.4	2335.9	S
30.178	0.0000	0.0000	81.250	0.02111	0.00000	18494.9	10963.5	2335.9	S
32.578	0.0000	0.0000	81.211	0.01974	0.00000	18494.9	11139.3	2335.9	S
56.578	0.0000	0.0000	80.932	0.01187	0.00000	18494.9	12323.3	2335.9	S
80.578	0.0000	0.0000	80.710	0.00891	0.00000	18494.9	13191.0	2335.9	S
104.578	0.0000	0.0000	80.524	0.00700	0.00000	18494.9	13862.2	2335.9	S
128.578	0.0000	0.0000	80.365	0.00569	0.00000	18494.9	14401.3	2335.9	S
152.578	0.0000	0.0000	80.225	0.00474	0.00000	18494.9	14846.2	2335.9	S
176.578	0.0000	0.0000	80.100	0.00402	0.00000	18494.9	15220.7	2335.9	S
200.578	0.0000	0.0000	79.987	0.00345	0.00000	18494.9	15540.7	2335.9	S
224.578	0.0000	0.0000	79.885	0.00300	0.00000	18494.9	15817.3	2335.9	S
248.578	0.0000	0.0000	79.790	0.00198	0.00000	18494.9	16058.9	2335.9	S
272.578	0.0000	0.0000	79.682	0.00058	0.00000	18494.9	16159.1	2335.9	S
296.578	0.0000	0.0000	79.570	0.00000	0.00000	18494.9	16159.1	2335.9	S
320.578	0.0000	0.0000	79.475	0.00000	0.00000	18494.9	16159.1	2335.9	S
344.578	0.0000	0.0000	79.392	0.00000	0.00000	18494.9	16159.1	2335.9	S
368.578	0.0000	0.0000	79.318	0.00000	0.00000	18494.9	16159.1	2335.9	S
392.578	0.0000	0.0000	79.251	0.00000	0.00000	18494.9	16159.1	2335.9	S
416.578	0.0000	0.0000	79.191	0.00000	0.00000	18494.9	16159.1	2335.9	S
440.578	0.0000	0.0000	79.136	0.00000	0.00000	18494.9	16159.1	2335.9	S
464.578	0.0000	0.0000	79.085	0.00000	0.00000	18494.9	16159.1	2335.9	S
488.578	0.0000	0.0000	79.038	0.00000	0.00000	18494.9	16159.1	2335.9	S
512.578	0.0000	0.0000	78.995	0.00000	0.00000	18494.9	16159.1	2335.9	S
536.578	0.0000	0.0000	78.955	0.00000	0.00000	18494.9	16159.1	2335.9	S
560.578	0.0000	0.0000	78.917	0.00000	0.00000	18494.9	16159.1	2335.9	S
584.578	0.0000	0.0000	78.882	0.00000	0.00000	18494.9	16159.1	2335.9	S
608.578	0.0000	0.0000	78.848	0.00000	0.00000	18494.9	16159.1	2335.9	S
632.578	0.0000	0.0000	78.817	0.00000	0.00000	18494.9	16159.1	2335.9	S
656.578	0.0000	0.0000	78.787	0.00000	0.00000	18494.9	16159.1	2335.9	S
680.578	0.0000	0.0000	78.759	0.00000	0.00000	18494.9	16159.1	2335.9	S
704.578	0.0000	0.0000	78.732	0.00000	0.00000	18494.9	16159.1	2335.9	S
728.578	0.0000	0.0000	78.707	----	----	18494.9	16159.1	2335.9	N.A.

Detailed Results (cont,d.) :: *Scenario 45 :: FDOT 24 Hour - 24 hr - 100 yr*

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
21.378	0.1743	0.0000	82.501	0.06624	0.21888	27544.4	9609.3	5889.3	S
21.400	0.1740	0.0000	82.500	0.06606	0.21863	27558.3	9614.5	5906.8	S
21.422	0.1738	0.0000	82.499	0.06586	0.21838	27572.2	9619.8	5924.3	S
21.444	0.1736	0.0000	82.497	0.06566	0.21813	27586.1	9625.1	5941.8	S
21.467	0.1735	0.0000	82.496	0.06543	0.21789	27600.0	9630.3	5959.2	S
21.489	0.1735	0.0000	82.494	0.06520	0.21764	27613.9	9635.6	5976.6	S
21.511	0.1734	0.0000	82.493	0.06495	0.21739	27627.7	9640.8	5994.0	S
21.533	0.1734	0.0000	82.492	0.06469	0.21714	27641.6	9645.9	6011.4	S
21.556	0.1734	0.0000	82.490	0.06442	0.21689	27655.5	9651.1	6028.8	S
21.578	0.1734	0.0000	82.489	0.06414	0.21665	27669.4	9656.3	6046.1	S
21.600	0.1734	0.0000	82.488	0.06385	0.21640	27683.2	9661.4	6063.4	S
21.622	0.1734	0.0000	82.486	0.06355	0.21616	27697.1	9666.5	6080.7	S
21.644	0.1734	0.0000	82.485	0.06324	0.21592	27711.0	9671.5	6098.0	S
21.667	0.1734	0.0000	82.484	0.06292	0.21567	27724.8	9676.6	6115.3	S
21.689	0.1734	0.0000	82.482	0.06260	0.21543	27738.7	9681.6	6132.5	S
21.711	0.1734	0.0000	82.481	0.06226	0.21519	27752.6	9686.6	6149.8	S
21.733	0.1734	0.0000	82.480	0.06192	0.21495	27766.5	9691.6	6167.0	S
21.756	0.1734	0.0000	82.479	0.06157	0.21471	27780.3	9696.5	6184.2	S
21.778	0.1734	0.0000	82.477	0.06121	0.21448	27794.2	9701.4	6201.3	S
21.800	0.1734	0.0000	82.476	0.06085	0.21424	27808.1	9706.3	6218.5	S
21.822	0.1734	0.0000	82.475	0.06048	0.21400	27821.9	9711.2	6235.6	S
21.844	0.1734	0.0000	82.473	0.06011	0.21377	27835.8	9716.0	6252.7	S
21.867	0.1734	0.0000	82.472	0.05973	0.21354	27849.7	9720.8	6269.8	S
21.889	0.1734	0.0000	82.471	0.05934	0.21331	27863.5	9725.5	6286.9	S
21.911	0.1734	0.0000	82.470	0.05896	0.21307	27877.4	9730.3	6303.9	S
21.933	0.1734	0.0000	82.469	0.05856	0.21284	27891.3	9735.0	6321.0	S
21.956	0.1734	0.0000	82.467	0.05817	0.21262	27905.2	9739.6	6338.0	S
21.978	0.1734	0.0000	82.466	0.05776	0.21239	27919.0	9744.3	6355.0	S
22.000	0.1734	0.0000	82.465	0.05736	0.21216	27932.9	9748.9	6372.0	S
22.022	0.1721	0.0000	82.464	0.05695	0.21194	27946.7	9753.5	6388.9	S
22.044	0.1682	0.0000	82.462	0.05652	0.21170	27960.3	9758.0	6405.9	S
22.067	0.1598	0.0000	82.461	0.05607	0.21146	27973.5	9762.5	6422.8	S
22.089	0.1479	0.0000	82.460	0.05560	0.21119	27985.8	9767.0	6439.7	S
22.111	0.1350	0.0000	82.458	0.05511	0.21090	27997.1	9771.4	6456.6	S
22.133	0.1229	0.0000	82.456	0.05459	0.21057	28007.4	9775.8	6473.5	S
22.156	0.1127	0.0000	82.455	0.05407	0.21022	28016.8	9780.1	6490.3	S
22.178	0.1053	0.0000	82.453	0.05353	0.20986	28025.5	9784.4	6507.1	S
22.200	0.1002	0.0000	82.451	0.05300	0.20947	28033.8	9788.7	6523.9	S
22.222	0.0965	0.0000	82.449	0.05247	0.20908	28041.6	9792.9	6540.6	S
22.244	0.0938	0.0000	82.447	0.05194	0.20869	28049.2	9797.1	6557.3	S
22.267	0.0919	0.0000	82.444	0.05141	0.20828	28056.7	9801.2	6574.0	S
22.289	0.0904	0.0000	82.442	0.05088	0.20788	28064.0	9805.3	6590.6	S
22.311	0.0894	0.0000	82.440	0.05036	0.20747	28071.2	9809.4	6607.3	S
22.333	0.0887	0.0000	82.438	0.04985	0.20707	28078.3	9813.4	6623.8	S
22.356	0.0882	0.0000	82.436	0.04934	0.20666	28085.4	9817.3	6640.4	S
22.378	0.0878	0.0000	82.434	0.04883	0.20625	28092.4	9821.3	6656.9	S
22.400	0.0875	0.0000	82.432	0.04833	0.20584	28099.4	9825.2	6673.4	S
22.422	0.0873	0.0000	82.430	0.04783	0.20544	28106.4	9829.0	6689.8	S
22.444	0.0872	0.0000	82.428	0.04734	0.20503	28113.4	9832.8	6706.3	S
22.467	0.0871	0.0000	82.425	0.04685	0.20463	28120.4	9836.6	6722.6	S
22.489	0.0870	0.0000	82.423	0.04637	0.20422	28127.3	9840.3	6739.0	S
22.511	0.0869	0.0000	82.421	0.04589	0.20382	28134.3	9844.0	6755.3	S
22.533	0.0869	0.0000	82.419	0.04541	0.20342	28141.2	9847.6	6771.6	S
22.556	0.0869	0.0000	82.417	0.04494	0.20302	28148.2	9851.3	6787.9	S
22.578	0.0869	0.0000	82.415	0.04448	0.20262	28155.1	9854.8	6804.1	S
22.600	0.0869	0.0000	82.413	0.04402	0.20222	28162.1	9858.4	6820.3	S
22.622	0.0869	0.0000	82.411	0.04356	0.20182	28169.0	9861.9	6836.4	S
22.644	0.0869	0.0000	82.409	0.04311	0.20143	28176.0	9865.3	6852.6	S
22.667	0.0869	0.0000	82.407	0.04266	0.20103	28183.0	9868.8	6868.7	S
22.689	0.0869	0.0000	82.405	0.04222	0.20064	28189.9	9872.2	6884.7	S
22.711	0.0869	0.0000	82.403	0.04178	0.20025	28196.9	9875.5	6900.8	S
22.733	0.0869	0.0000	82.401	0.04135	0.19986	28203.8	9878.9	6916.8	S
22.756	0.0869	0.0000	82.399	0.04092	0.19947	28210.8	9882.1	6932.8	S
22.778	0.0869	0.0000	82.397	0.04050	0.19908	28217.7	9885.4	6948.7	S
22.800	0.0869	0.0000	82.395	0.04009	0.19869	28224.7	9888.6	6964.6	S
22.822	0.0869	0.0000	82.393	0.03968	0.19830	28231.6	9891.8	6980.5	S
22.844	0.0869	0.0000	82.392	0.03928	0.19792	28238.6	9895.0	6996.3	S
22.867	0.0869	0.0000	82.390	0.03888	0.19753	28245.5	9898.1	7012.2	S
22.889	0.0869	0.0000	82.388	0.03849	0.19715	28252.5	9901.2	7027.9	S
22.911	0.0869	0.0000	82.386	0.03810	0.19676	28259.4	9904.3	7043.7	S
22.933	0.0869	0.0000	82.384	0.03772	0.19638	28266.4	9907.3	7059.4	S
22.956	0.0869	0.0000	82.382	0.03735	0.19600	28273.3	9910.3	7075.1	S
22.978	0.0869	0.0000	82.380	0.03699	0.19562	28280.3	9913.3	7090.8	S
23.000	0.0869	0.0000	82.378	0.03663	0.19524	28287.3	9916.2	7106.4	S

Detailed Results (cont,d.) :: *Scenario 45 :: FDOT 24 Hour - 24 hr - 100 yr*

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
34.178	0.0000	0.0000	81.817	0.02984	0.00000	28640.8	10567.2	10071.9	S
36.578	0.0000	0.0000	81.770	0.02761	0.00000	28640.8	10814.7	10071.9	S
38.978	0.0000	0.0000	81.727	0.02566	0.00000	28640.8	11044.3	10071.9	S
41.378	0.0000	0.0000	81.686	0.02398	0.00000	28640.8	11258.2	10071.9	S
43.778	0.0000	0.0000	81.647	0.02252	0.00000	28640.8	11458.7	10071.9	S
46.178	0.0000	0.0000	81.610	0.02124	0.00000	28640.8	11647.4	10071.9	S
48.578	0.0000	0.0000	81.574	0.02011	0.00000	28640.8	11825.8	10071.9	S
72.578	0.0000	0.0000	81.310	0.01299	0.00000	28640.8	13100.1	10071.9	S
96.578	0.0000	0.0000	81.091	0.01008	0.00000	28640.8	14070.6	10071.9	S
120.578	0.0000	0.0000	80.905	0.00812	0.00000	28640.8	14841.7	10071.9	S
144.578	0.0000	0.0000	80.743	0.00673	0.00000	28640.8	15474.0	10071.9	S
168.578	0.0000	0.0000	80.600	0.00569	0.00000	28640.8	16004.5	10071.9	S
192.578	0.0000	0.0000	80.471	0.00489	0.00000	28640.8	16457.5	10071.9	S
216.578	0.0000	0.0000	80.353	0.00425	0.00000	28640.8	16849.5	10071.9	S
240.578	0.0000	0.0000	80.246	0.00373	0.00000	28640.8	17192.1	10071.9	S
264.578	0.0000	0.0000	80.147	0.00330	0.00000	28640.8	17494.2	10071.9	S
288.578	0.0000	0.0000	80.054	0.00294	0.00000	28640.8	17762.4	10071.9	S
312.578	0.0000	0.0000	79.969	0.00263	0.00000	28640.8	18001.9	10071.9	S
336.578	0.0000	0.0000	79.888	0.00237	0.00000	28640.8	18216.9	10071.9	S
360.578	0.0000	0.0000	79.813	0.00204	0.00000	28640.8	18410.7	10071.9	S
384.578	0.0000	0.0000	79.740	0.00092	0.00000	28640.8	18569.0	10071.9	S
408.578	0.0000	0.0000	79.643	0.00000	0.00000	28640.8	18569.0	10071.9	S
432.578	0.0000	0.0000	79.560	0.00000	0.00000	28640.8	18569.0	10071.9	S
456.578	0.0000	0.0000	79.487	0.00000	0.00000	28640.8	18569.0	10071.9	S
480.578	0.0000	0.0000	79.421	0.00000	0.00000	28640.8	18569.0	10071.9	S
504.578	0.0000	0.0000	79.361	0.00000	0.00000	28640.8	18569.0	10071.9	S
528.578	0.0000	0.0000	79.306	0.00000	0.00000	28640.8	18569.0	10071.9	S
552.578	0.0000	0.0000	79.255	0.00000	0.00000	28640.8	18569.0	10071.9	S
576.578	0.0000	0.0000	79.208	0.00000	0.00000	28640.8	18569.0	10071.9	S
600.578	0.0000	0.0000	79.164	0.00000	0.00000	28640.8	18569.0	10071.9	S
624.578	0.0000	0.0000	79.123	0.00000	0.00000	28640.8	18569.0	10071.9	S
648.578	0.0000	0.0000	79.085	0.00000	0.00000	28640.8	18569.0	10071.9	S
672.578	0.0000	0.0000	79.048	0.00000	0.00000	28640.8	18569.0	10071.9	S
696.578	0.0000	0.0000	79.014	0.00000	0.00000	28640.8	18569.0	10071.9	S
720.578	0.0000	0.0000	78.981	0.00000	0.00000	28640.8	18569.0	10071.9	S
744.578	0.0000	0.0000	78.950	----	----	28640.8	18569.0	10071.9	N.A.

Detailed Results (cont,d.) :: *Scenario 46 :: FDOT 72 Hour - 72 hr - 100 yr*

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
67.422	0.1083	0.0000	82.442	0.02480	0.20781	35306.9	13011.6	10629.6	S
67.444	0.1083	0.0000	82.440	0.02475	0.20751	35315.6	13013.6	10646.2	S
67.467	0.1083	0.0000	82.439	0.02471	0.20721	35324.2	13015.6	10662.8	S
67.489	0.1083	0.0000	82.437	0.02467	0.20691	35332.9	13017.5	10679.3	S
67.511	0.1083	0.0000	82.436	0.02463	0.20661	35341.6	13019.5	10695.9	S
67.533	0.1083	0.0000	82.434	0.02459	0.20632	35350.2	13021.5	10712.4	S
67.556	0.1083	0.0000	82.433	0.02455	0.20602	35358.9	13023.4	10728.9	S
67.578	0.1083	0.0000	82.431	0.02451	0.20572	35367.5	13025.4	10745.4	S
67.600	0.1083	0.0000	82.430	0.02447	0.20542	35376.2	13027.4	10761.8	S
67.622	0.1083	0.0000	82.428	0.02443	0.20512	35384.9	13029.3	10778.2	S
67.644	0.1083	0.0000	82.427	0.02439	0.20483	35393.5	13031.3	10794.6	S
67.667	0.1083	0.0000	82.425	0.02435	0.20453	35402.2	13033.2	10811.0	S
67.689	0.1083	0.0000	82.423	0.02431	0.20423	35410.9	13035.2	10827.4	S
67.711	0.1083	0.0000	82.422	0.02427	0.20394	35419.5	13037.1	10843.7	S
67.733	0.1083	0.0000	82.420	0.02423	0.20364	35428.2	13039.0	10860.0	S
67.756	0.1083	0.0000	82.419	0.02419	0.20335	35436.8	13041.0	10876.3	S
67.778	0.1083	0.0000	82.417	0.02415	0.20305	35445.5	13042.9	10892.5	S
67.800	0.1083	0.0000	82.416	0.02411	0.20276	35454.2	13044.8	10908.7	S
67.822	0.1083	0.0000	82.414	0.02407	0.20246	35462.8	13046.8	10925.0	S
67.844	0.1083	0.0000	82.413	0.02404	0.20217	35471.5	13048.7	10941.1	S
67.867	0.1083	0.0000	82.411	0.02400	0.20187	35480.2	13050.6	10957.3	S
67.889	0.1083	0.0000	82.410	0.02396	0.20158	35488.8	13052.5	10973.4	S
67.911	0.1083	0.0000	82.408	0.02392	0.20128	35497.5	13054.5	10989.6	S
67.933	0.1083	0.0000	82.407	0.02389	0.20099	35506.1	13056.4	11005.6	S
67.956	0.1083	0.0000	82.405	0.02385	0.20070	35514.8	13058.3	11021.7	S
67.978	0.1083	0.0000	82.404	0.02381	0.20041	35523.5	13060.2	11037.8	S
68.000	0.1082	0.0000	82.403	0.02377	0.20011	35532.1	13062.1	11053.8	S
68.022	0.1070	0.0000	82.401	0.02373	0.19982	35540.7	13064.0	11069.8	S
68.044	0.1038	0.0000	82.400	0.02368	0.19952	35549.2	13065.9	11085.8	S
68.067	0.0975	0.0000	82.398	0.02361	0.19921	35557.2	13067.8	11101.7	S
68.089	0.0891	0.0000	82.396	0.02353	0.19888	35564.7	13069.7	11117.6	S
68.111	0.0803	0.0000	82.395	0.02344	0.19853	35571.5	13071.5	11133.5	S
68.133	0.0721	0.0000	82.393	0.02333	0.19816	35577.6	13073.4	11149.4	S
68.156	0.0654	0.0000	82.391	0.02322	0.19777	35583.1	13075.3	11165.2	S
68.178	0.0607	0.0000	82.389	0.02311	0.19736	35588.1	13077.1	11181.0	S
68.200	0.0573	0.0000	82.387	0.02299	0.19694	35592.8	13079.0	11196.8	S
68.222	0.0549	0.0000	82.385	0.02288	0.19652	35597.3	13080.8	11212.5	S
68.244	0.0531	0.0000	82.383	0.02277	0.19609	35601.6	13082.6	11228.2	S
68.267	0.0519	0.0000	82.380	0.02266	0.19566	35605.8	13084.4	11243.9	S
68.289	0.0509	0.0000	82.378	0.02256	0.19523	35610.0	13086.3	11259.6	S
68.311	0.0503	0.0000	82.376	0.02246	0.19479	35614.0	13088.1	11275.2	S
68.333	0.0498	0.0000	82.374	0.02236	0.19436	35618.0	13089.8	11290.7	S
68.356	0.0495	0.0000	82.372	0.02226	0.19392	35622.0	13091.6	11306.2	S
68.378	0.0492	0.0000	82.370	0.02217	0.19348	35625.9	13093.4	11321.7	S
68.400	0.0490	0.0000	82.368	0.02208	0.19304	35629.9	13095.2	11337.2	S
68.422	0.0489	0.0000	82.366	0.02200	0.19260	35633.8	13096.9	11352.6	S
68.444	0.0488	0.0000	82.363	0.02191	0.19216	35637.7	13098.7	11368.0	S
68.467	0.0487	0.0000	82.361	0.02183	0.19172	35641.6	13100.5	11383.4	S
68.489	0.0487	0.0000	82.359	0.02175	0.19128	35645.5	13102.2	11398.7	S
68.511	0.0487	0.0000	82.357	0.02167	0.19084	35649.4	13103.9	11414.0	S
68.533	0.0486	0.0000	82.355	0.02160	0.19040	35653.3	13105.7	11429.2	S
68.556	0.0486	0.0000	82.353	0.02152	0.18996	35657.2	13107.4	11444.4	S
68.578	0.0486	0.0000	82.351	0.02145	0.18952	35661.1	13109.1	11459.6	S
68.600	0.0486	0.0000	82.349	0.02138	0.18908	35664.9	13110.8	11474.8	S
68.622	0.0486	0.0000	82.347	0.02131	0.18864	35668.8	13112.5	11489.9	S
68.644	0.0486	0.0000	82.345	0.02125	0.18821	35672.7	13114.2	11505.0	S
68.667	0.0486	0.0000	82.342	0.02118	0.18777	35676.6	13115.9	11520.0	S
68.689	0.0486	0.0000	82.340	0.02111	0.18733	35680.5	13117.6	11535.0	S
68.711	0.0486	0.0000	82.338	0.02105	0.18689	35684.4	13119.3	11550.0	S
68.733	0.0486	0.0000	82.336	0.02099	0.18645	35688.3	13121.0	11564.9	S
68.756	0.0486	0.0000	82.334	0.02092	0.18601	35692.2	13122.7	11579.8	S
68.778	0.0486	0.0000	82.332	0.02086	0.18557	35696.1	13124.3	11594.7	S
68.800	0.0486	0.0000	82.330	0.02080	0.18514	35700.0	13126.0	11609.5	S
68.822	0.0486	0.0000	82.328	0.02074	0.18470	35703.9	13127.7	11624.3	S
68.844	0.0486	0.0000	82.326	0.02069	0.18426	35707.8	13129.3	11639.0	S
68.867	0.0486	0.0000	82.324	0.02063	0.18382	35711.6	13131.0	11653.8	S
68.889	0.0486	0.0000	82.322	0.02057	0.18338	35715.5	13132.6	11668.5	S
68.911	0.0486	0.0000	82.320	0.02052	0.18295	35719.4	13134.3	11683.1	S
68.933	0.0486	0.0000	82.318	0.02046	0.18251	35723.3	13135.9	11697.7	S
68.956	0.0486	0.0000	82.316	0.02041	0.18207	35727.2	13137.5	11712.3	S
68.978	0.0486	0.0000	82.314	0.02036	0.18164	35731.1	13139.2	11726.9	S
69.000	0.0486	0.0000	82.312	0.02030	0.18120	35735.0	13140.8	11741.4	S
69.022	0.0486	0.0000	82.310	0.02025	0.18076	35738.9	13142.4	11755.8	S
69.044	0.0486	0.0000	82.308	0.02020	0.18032	35742.8	13144.0	11770.3	S

Detailed Results (cont,d.) *:: Scenario 46 :: FDOT 72 Hour - 72 hr - 100 yr*

Elapsed Time (hours)	Inflow Rate (ft^3/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft^3/s)	Overflow Discharge (ft^3/s)	Cumulative Inflow Volume (ft^3)	Cumulative Infiltration Volume (ft^3)	Cumulative Discharge Volume (ft^3)	Flow Type
72.356	0.0007	0.0000	82.066	0.01587	0.11503	36283.4	13357.3	13538.0	S
72.378	0.0005	0.0000	82.064	0.01582	0.11440	36283.5	13358.5	13547.2	S
72.400	0.0003	0.0000	82.062	0.01576	0.11377	36283.5	13359.8	13556.3	S
72.422	0.0002	0.0000	82.061	0.01571	0.11314	36283.5	13361.1	13565.4	S
72.444	0.0002	0.0000	82.059	0.01566	0.11251	36283.5	13362.3	13574.4	S
72.467	0.0001	0.0000	82.057	0.01562	0.11188	36283.6	13363.6	13583.4	S
72.489	0.0000	0.0000	82.055	0.01557	0.11124	36283.6	13364.8	13592.3	S
72.511	0.0000	0.0000	82.054	0.01553	0.11061	36283.6	13366.1	13601.2	S
72.533	0.0000	0.0000	82.052	0.01549	0.10998	36283.6	13367.3	13610.0	S
72.556	0.0000	0.0000	82.050	0.01545	0.10934	36283.6	13368.5	13618.8	S
72.578	0.0000	0.0000	82.048	0.01519	0.10871	36283.6	13369.8	13627.5	S
74.978	0.0000	0.0000	81.939	-0.00918	0.05590	36283.6	13274.1	14338.6	S
77.378	0.0000	0.0000	81.901	0.00302	0.00788	36283.6	13211.1	14614.1	S
79.778	0.0000	0.0000	81.874	0.01533	0.00000	36283.6	13326.4	14648.2	S
82.178	0.0000	0.0000	81.846	0.01724	0.00000	36283.6	13476.0	14648.2	S
84.578	0.0000	0.0000	81.819	0.01701	0.00000	36283.6	13624.2	14648.2	S
86.978	0.0000	0.0000	81.791	0.01669	0.00000	36283.6	13769.9	14648.2	S
89.378	0.0000	0.0000	81.765	0.01631	0.00000	36283.6	13912.5	14648.2	S
91.778	0.0000	0.0000	81.738	0.01591	0.00000	36283.6	14051.8	14648.2	S
94.178	0.0000	0.0000	81.712	0.01550	0.00000	36283.6	14187.5	14648.2	S
96.578	0.0000	0.0000	81.687	0.01503	0.00000	36283.6	14319.6	14648.2	S
120.578	0.0000	0.0000	81.474	0.01127	0.00000	36283.6	15386.6	14648.2	S
144.578	0.0000	0.0000	81.288	0.00937	0.00000	36283.6	16267.1	14648.2	S
168.578	0.0000	0.0000	81.122	0.00793	0.00000	36283.6	17006.5	14648.2	S
192.578	0.0000	0.0000	80.972	0.00679	0.00000	36283.6	17636.6	14648.2	S
216.578	0.0000	0.0000	80.837	0.00590	0.00000	36283.6	18180.6	14648.2	S
240.578	0.0000	0.0000	80.713	0.00517	0.00000	36283.6	18655.3	14648.2	S
264.578	0.0000	0.0000	80.599	0.00457	0.00000	36283.6	19073.6	14648.2	S
288.578	0.0000	0.0000	80.494	0.00407	0.00000	36283.6	19444.8	14648.2	S
312.578	0.0000	0.0000	80.396	0.00364	0.00000	36283.6	19776.4	14648.2	S
336.578	0.0000	0.0000	80.304	0.00328	0.00000	36283.6	20074.2	14648.2	S
360.578	0.0000	0.0000	80.219	0.00296	0.00000	36283.6	20342.7	14648.2	S
384.578	0.0000	0.0000	80.138	0.00268	0.00000	36283.6	20585.8	14648.2	S
408.578	0.0000	0.0000	80.062	0.00244	0.00000	36283.6	20806.5	14648.2	S
432.578	0.0000	0.0000	79.991	0.00223	0.00000	36283.6	21007.7	14648.2	S
456.578	0.0000	0.0000	79.923	0.00204	0.00000	36283.6	21191.7	14648.2	S
480.578	0.0000	0.0000	79.859	0.00187	0.00000	36283.6	21360.5	14648.2	S
504.578	0.0000	0.0000	79.798	0.00159	0.00000	36283.6	21515.7	14648.2	S
528.578	0.0000	0.0000	79.736	0.00069	0.00000	36283.6	21635.4	14648.2	S
552.578	0.0000	0.0000	79.656	0.00000	0.00000	36283.6	21635.4	14648.2	S
576.578	0.0000	0.0000	79.586	0.00000	0.00000	36283.6	21635.4	14648.2	S
600.578	0.0000	0.0000	79.524	0.00000	0.00000	36283.6	21635.4	14648.2	S
624.578	0.0000	0.0000	79.466	0.00000	0.00000	36283.6	21635.4	14648.2	S
648.578	0.0000	0.0000	79.414	0.00000	0.00000	36283.6	21635.4	14648.2	S
672.578	0.0000	0.0000	79.365	0.00000	0.00000	36283.6	21635.4	14648.2	S
696.578	0.0000	0.0000	79.319	0.00000	0.00000	36283.6	21635.4	14648.2	S
720.578	0.0000	0.0000	79.277	0.00000	0.00000	36283.6	21635.4	14648.2	S
744.578	0.0000	0.0000	79.237	0.00000	0.00000	36283.6	21635.4	14648.2	S
768.578	0.0000	0.0000	79.199	0.00000	0.00000	36283.6	21635.4	14648.2	S
792.578	0.0000	0.0000	79.163	----	----	36283.6	21635.4	14648.2	N.A.

Detailed Results (cont,d.) *:: Scenario 47 :: FDOT 168 Hour - 168 hr - 100 yr*

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
161.156	0.0691	0.0000	82.552	0.02151	0.22796	40761.1	17088.5	11296.3	S
161.178	0.0691	0.0000	82.550	0.02140	0.22757	40766.6	17090.2	11314.5	S
161.200	0.0691	0.0000	82.548	0.02130	0.22719	40772.2	17091.9	11332.7	S
161.222	0.0691	0.0000	82.546	0.02119	0.22680	40777.7	17093.6	11350.9	S
161.244	0.0691	0.0000	82.543	0.02109	0.22642	40783.2	17095.3	11369.0	S
161.267	0.0691	0.0000	82.541	0.02099	0.22603	40788.8	17097.0	11387.1	S
161.289	0.0691	0.0000	82.539	0.02089	0.22565	40794.3	17098.6	11405.1	S
161.311	0.0691	0.0000	82.537	0.02079	0.22526	40799.8	17100.3	11423.2	S
161.333	0.0691	0.0000	82.535	0.02069	0.22488	40805.3	17102.0	11441.2	S
161.356	0.0691	0.0000	82.532	0.02060	0.22449	40810.9	17103.6	11459.2	S
161.378	0.0691	0.0000	82.530	0.02050	0.22411	40816.4	17105.3	11477.1	S
161.400	0.0691	0.0000	82.528	0.02041	0.22373	40821.9	17106.9	11495.0	S
161.422	0.0691	0.0000	82.526	0.02031	0.22334	40827.4	17108.5	11512.9	S
161.444	0.0691	0.0000	82.524	0.02022	0.22296	40833.0	17110.2	11530.8	S
161.467	0.0691	0.0000	82.522	0.02013	0.22258	40838.5	17111.8	11548.6	S
161.489	0.0691	0.0000	82.520	0.02004	0.22219	40844.0	17113.4	11566.4	S
161.511	0.0691	0.0000	82.517	0.01996	0.22181	40849.5	17115.0	11584.1	S
161.533	0.0691	0.0000	82.515	0.01987	0.22143	40855.1	17116.6	11601.9	S
161.556	0.0691	0.0000	82.513	0.01978	0.22104	40860.6	17118.2	11619.6	S
161.578	0.0691	0.0000	82.511	0.01970	0.22066	40866.1	17119.7	11637.2	S
161.600	0.0691	0.0000	82.509	0.01961	0.22028	40871.6	17121.3	11654.9	S
161.622	0.0691	0.0000	82.507	0.01953	0.21990	40877.2	17122.9	11672.5	S
161.644	0.0691	0.0000	82.505	0.01945	0.21952	40882.7	17124.4	11690.0	S
161.667	0.0691	0.0000	82.503	0.01937	0.21914	40888.2	17126.0	11707.6	S
161.689	0.0691	0.0000	82.501	0.01929	0.21875	40893.7	17127.5	11725.1	S
161.711	0.0691	0.0000	82.498	0.01921	0.21837	40899.3	17129.1	11742.6	S
161.733	0.0691	0.0000	82.496	0.01913	0.21799	40904.8	17130.6	11760.0	S
161.756	0.0691	0.0000	82.494	0.01905	0.21761	40910.3	17132.1	11777.5	S
161.778	0.0691	0.0000	82.492	0.01898	0.21723	40915.8	17133.7	11794.9	S
161.800	0.0691	0.0000	82.490	0.01890	0.21685	40921.4	17135.2	11812.2	S
161.822	0.0691	0.0000	82.488	0.01883	0.21647	40926.9	17136.7	11829.6	S
161.845	0.0691	0.0000	82.486	0.01875	0.21609	40932.4	17138.2	11846.9	S
161.867	0.0691	0.0000	82.484	0.01868	0.21571	40937.9	17139.7	11864.1	S
161.889	0.0691	0.0000	82.482	0.01860	0.21533	40943.5	17141.2	11881.4	S
161.911	0.0691	0.0000	82.480	0.01853	0.21495	40949.0	17142.7	11898.6	S
161.933	0.0691	0.0000	82.478	0.01846	0.21457	40954.5	17144.1	11915.8	S
161.956	0.0691	0.0000	82.476	0.01839	0.21419	40960.0	17145.6	11932.9	S
161.978	0.0691	0.0000	82.474	0.01832	0.21381	40965.6	17147.1	11950.0	S
162.000	0.0691	0.0000	82.472	0.01825	0.21343	40971.1	17148.5	11967.1	S
162.022	0.0691	0.0000	82.470	0.01818	0.21305	40976.6	17150.0	11984.2	S
162.044	0.0691	0.0000	82.468	0.01812	0.21267	40982.1	17151.4	12001.2	S
162.067	0.0691	0.0000	82.466	0.01805	0.21230	40987.7	17152.9	12018.2	S
162.089	0.0691	0.0000	82.464	0.01798	0.21192	40993.2	17154.3	12035.2	S
162.111	0.0691	0.0000	82.462	0.01792	0.21154	40998.7	17155.8	12052.1	S
162.133	0.0691	0.0000	82.460	0.01785	0.21116	41004.3	17157.2	12069.0	S
162.156	0.0691	0.0000	82.458	0.01779	0.21078	41009.8	17158.6	12085.9	S
162.178	0.0691	0.0000	82.456	0.01772	0.21041	41015.3	17160.0	12102.8	S
162.200	0.0691	0.0000	82.454	0.01766	0.21003	41020.8	17161.5	12119.6	S
162.222	0.0691	0.0000	82.452	0.01760	0.20965	41026.4	17162.9	12136.4	S
162.244	0.0691	0.0000	82.450	0.01753	0.20927	41031.9	17164.3	12153.1	S
162.267	0.0691	0.0000	82.448	0.01747	0.20890	41037.4	17165.7	12169.8	S
162.289	0.0691	0.0000	82.446	0.01741	0.20852	41042.9	17167.1	12186.5	S
162.311	0.0691	0.0000	82.444	0.01735	0.20814	41048.5	17168.5	12203.2	S
162.333	0.0691	0.0000	82.442	0.01729	0.20777	41054.0	17169.8	12219.8	S
162.356	0.0691	0.0000	82.440	0.01723	0.20739	41059.5	17171.2	12236.5	S
162.378	0.0691	0.0000	82.438	0.01717	0.20702	41065.0	17172.6	12253.0	S
162.400	0.0691	0.0000	82.436	0.01712	0.20664	41070.6	17174.0	12269.6	S
162.422	0.0691	0.0000	82.434	0.01706	0.20626	41076.1	17175.3	12286.1	S
162.444	0.0691	0.0000	82.432	0.01700	0.20589	41081.6	17176.7	12302.6	S
162.467	0.0691	0.0000	82.430	0.01694	0.20551	41087.1	17178.1	12319.0	S
162.489	0.0691	0.0000	82.428	0.01689	0.20514	41092.7	17179.4	12335.5	S
162.511	0.0691	0.0000	82.426	0.01683	0.20476	41098.2	17180.8	12351.9	S
162.533	0.0691	0.0000	82.424	0.01678	0.20439	41103.7	17182.1	12368.2	S
162.556	0.0691	0.0000	82.422	0.01672	0.20402	41109.2	17183.5	12384.6	S
162.578	0.0691	0.0000	82.420	0.01667	0.20364	41114.8	17184.8	12400.9	S
162.600	0.0691	0.0000	82.419	0.01661	0.20327	41120.3	17186.1	12417.1	S
162.622	0.0691	0.0000	82.417	0.01656	0.20289	41125.8	17187.4	12433.4	S
162.644	0.0691	0.0000	82.415	0.01651	0.20252	41131.3	17188.8	12449.6	S
162.667	0.0691	0.0000	82.413	0.01646	0.20215	41136.9	17190.1	12465.8	S
162.689	0.0691	0.0000	82.411	0.01640	0.20177	41142.4	17191.4	12481.9	S
162.711	0.0691	0.0000	82.409	0.01635	0.20140	41147.9	17192.7	12498.1	S
162.733	0.0691	0.0000	82.407	0.01630	0.20103	41153.5	17194.0	12514.2	S
162.756	0.0691	0.0000	82.405	0.01625	0.20065	41159.0	17195.3	12530.2	S
162.778	0.0691	0.0000	82.403	0.01620	0.20028	41164.5	17196.6	12546.3	S

Detailed Results (cont,d.) *:: Scenario 47 :: FDOT 168 Hour - 168 hr - 100 yr*

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
167.733	0.0691	0.0000	82.094	0.01140	0.12442	42396.9	17428.3	15417.2	S
167.756	0.0691	0.0000	82.093	0.01140	0.12413	42402.4	17429.3	15427.1	S
167.778	0.0691	0.0000	82.092	0.01139	0.12383	42407.9	17430.2	15437.1	S
167.800	0.0691	0.0000	82.092	0.01139	0.12354	42413.4	17431.1	15447.0	S
167.822	0.0691	0.0000	82.091	0.01139	0.12325	42419.0	17432.0	15456.8	S
167.845	0.0691	0.0000	82.090	0.01138	0.12296	42424.5	17432.9	15466.7	S
167.867	0.0691	0.0000	82.089	0.01138	0.12266	42430.0	17433.8	15476.5	S
167.889	0.0691	0.0000	82.088	0.01138	0.12237	42435.5	17434.7	15486.3	S
167.911	0.0691	0.0000	82.087	0.01137	0.12208	42441.1	17435.6	15496.1	S
167.933	0.0691	0.0000	82.086	0.01137	0.12179	42446.6	17436.5	15505.8	S
167.956	0.0691	0.0000	82.085	0.01137	0.12150	42452.1	17437.5	15515.6	S
167.978	0.0691	0.0000	82.084	0.01137	0.12121	42457.7	17438.4	15525.3	S
168.000	0.0691	0.0000	82.084	0.01136	0.12093	42463.2	17439.3	15535.0	S
168.022	0.0680	0.0000	82.083	0.01135	0.12064	42468.7	17440.2	15544.6	S
168.044	0.0648	0.0000	82.082	0.01133	0.12034	42474.0	17441.1	15554.3	S
168.067	0.0580	0.0000	82.081	0.01129	0.12002	42478.9	17442.0	15563.9	S
168.089	0.0484	0.0000	82.080	0.01123	0.11966	42483.2	17442.9	15573.5	S
168.111	0.0380	0.0000	82.078	0.01115	0.11925	42486.6	17443.8	15583.0	S
168.133	0.0284	0.0000	82.077	0.01105	0.11880	42489.3	17444.7	15592.5	S
168.156	0.0203	0.0000	82.076	0.01095	0.11831	42491.2	17445.6	15602.0	S
168.178	0.0145	0.0000	82.074	0.01085	0.11779	42492.6	17446.4	15611.5	S
168.200	0.0105	0.0000	82.073	0.01075	0.11725	42493.6	17447.3	15620.9	S
168.222	0.0076	0.0000	82.071	0.01065	0.11669	42494.3	17448.1	15630.2	S
168.244	0.0054	0.0000	82.069	0.01055	0.11611	42494.8	17449.0	15639.5	S
168.267	0.0039	0.0000	82.068	0.01046	0.11553	42495.2	17449.8	15648.8	S
168.289	0.0028	0.0000	82.066	0.01038	0.11494	42495.5	17450.7	15658.0	S
168.311	0.0020	0.0000	82.064	0.01030	0.11435	42495.7	17451.5	15667.2	S
168.333	0.0014	0.0000	82.062	0.01022	0.11376	42495.8	17452.3	15676.3	S
168.356	0.0010	0.0000	82.061	0.01014	0.11316	42495.9	17453.1	15685.4	S
168.378	0.0007	0.0000	82.059	0.01007	0.11256	42496.0	17453.9	15694.4	S
168.400	0.0005	0.0000	82.057	0.01001	0.11196	42496.0	17454.7	15703.4	S
168.422	0.0003	0.0000	82.056	0.00994	0.11135	42496.1	17455.5	15712.3	S
168.444	0.0002	0.0000	82.054	0.00988	0.11075	42496.1	17456.3	15721.2	S
168.467	0.0001	0.0000	82.052	0.00982	0.11015	42496.1	17457.1	15730.1	S
168.489	0.0001	0.0000	82.051	0.00977	0.10954	42496.1	17457.9	15738.9	S
168.511	0.0000	0.0000	82.049	0.00972	0.10893	42496.1	17458.7	15747.6	S
168.533	0.0000	0.0000	82.047	0.00967	0.10833	42496.1	17459.5	15756.3	S
168.556	0.0000	0.0000	82.046	0.00962	0.10772	42496.1	17460.2	15764.9	S
168.578	0.0000	0.0000	82.044	0.00937	0.10711	42496.1	17461.0	15773.5	S
170.978	0.0000	0.0000	81.942	-0.01319	0.05750	42496.1	17327.9	16484.7	S
173.378	0.0000	0.0000	81.903	-0.00333	0.01431	42496.1	17233.1	16794.9	S
175.778	0.0000	0.0000	81.885	0.00802	0.00000	42496.1	17270.4	16856.7	S
178.178	0.0000	0.0000	81.866	0.01172	0.00000	42496.1	17371.6	16856.7	S
180.578	0.0000	0.0000	81.847	0.01169	0.00000	42496.1	17472.9	16856.7	S
182.978	0.0000	0.0000	81.829	0.01160	0.00000	42496.1	17573.6	16856.7	S
185.378	0.0000	0.0000	81.810	0.01146	0.00000	42496.1	17673.3	16856.7	S
187.778	0.0000	0.0000	81.792	0.01131	0.00000	42496.1	17771.7	16856.7	S
190.178	0.0000	0.0000	81.774	0.01116	0.00000	42496.1	17868.8	16856.7	S
192.578	0.0000	0.0000	81.755	0.01095	0.00000	42496.1	17964.5	16856.7	S
216.578	0.0000	0.0000	81.593	0.00912	0.00000	42496.1	18802.3	16856.7	S
240.578	0.0000	0.0000	81.443	0.00805	0.00000	42496.1	19540.4	16856.7	S
264.578	0.0000	0.0000	81.304	0.00715	0.00000	42496.1	20194.0	16856.7	S
288.578	0.0000	0.0000	81.175	0.00637	0.00000	42496.1	20775.4	16856.7	S
312.578	0.0000	0.0000	81.055	0.00570	0.00000	42496.1	21294.7	16856.7	S
336.578	0.0000	0.0000	80.943	0.00512	0.00000	42496.1	21760.5	16856.7	S
360.578	0.0000	0.0000	80.838	0.00463	0.00000	42496.1	22180.2	16856.7	S
384.578	0.0000	0.0000	80.739	0.00419	0.00000	42496.1	22559.8	16856.7	S
408.578	0.0000	0.0000	80.647	0.00381	0.00000	42496.1	22904.5	16856.7	S
432.578	0.0000	0.0000	80.560	0.00348	0.00000	42496.1	23218.5	16856.7	S
456.578	0.0000	0.0000	80.477	0.00318	0.00000	42496.1	23505.5	16856.7	S
480.578	0.0000	0.0000	80.399	0.00292	0.00000	42496.1	23768.6	16856.7	S
504.578	0.0000	0.0000	80.325	0.00269	0.00000	42496.1	24010.5	16856.7	S
528.578	0.0000	0.0000	80.255	0.00248	0.00000	42496.1	24233.5	16856.7	S
552.578	0.0000	0.0000	80.188	0.00230	0.00000	42496.1	24439.5	16856.7	S
576.578	0.0000	0.0000	80.124	0.00213	0.00000	42496.1	24630.2	16856.7	S
600.578	0.0000	0.0000	80.063	0.00198	0.00000	42496.1	24807.2	16856.7	S
624.578	0.0000	0.0000	80.005	0.00184	0.00000	42496.1	24971.6	16856.7	S
648.578	0.0000	0.0000	79.949	0.00171	0.00000	42496.1	25124.8	16856.7	S
672.578	0.0000	0.0000	79.896	0.00160	0.00000	42496.1	25267.6	16856.7	S
696.578	0.0000	0.0000	79.845	0.00150	0.00000	42496.1	25401.1	16856.7	S
720.578	0.0000	0.0000	79.796	0.00138	0.00000	42496.1	25526.0	16856.7	S
744.578	0.0000	0.0000	79.748	0.00066	0.00000	42496.1	25639.4	16856.7	S
768.578	0.0000	0.0000	79.681	0.00000	0.00000	42496.1	25639.4	16856.7	S
792.578	0.0000	0.0000	79.622	0.00000	0.00000	42496.1	25639.4	16856.7	S

Detailed Results (cont,d.) :: *Scenario 48 :: FDOT 240 Hour - 240 hr - 100 yr*

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
189.111	0.0919	0.0000	82.469	0.01672	0.21297	40027.8	17474.3	10714.1	S
189.133	0.0919	0.0000	82.467	0.01668	0.21264	40035.2	17475.6	10731.1	S
189.156	0.0919	0.0000	82.466	0.01665	0.21232	40042.5	17476.9	10748.1	S
189.178	0.0919	0.0000	82.464	0.01661	0.21200	40049.9	17478.3	10765.1	S
189.200	0.0919	0.0000	82.462	0.01658	0.21168	40057.2	17479.6	10782.0	S
189.222	0.0919	0.0000	82.461	0.01654	0.21136	40064.6	17480.9	10799.0	S
189.244	0.0919	0.0000	82.459	0.01651	0.21104	40071.9	17482.3	10815.9	S
189.267	0.0919	0.0000	82.457	0.01648	0.21072	40079.3	17483.6	10832.7	S
189.289	0.0919	0.0000	82.456	0.01644	0.21040	40086.6	17484.9	10849.6	S
189.311	0.0919	0.0000	82.454	0.01641	0.21008	40094.0	17486.2	10866.4	S
189.333	0.0919	0.0000	82.452	0.01637	0.20976	40101.3	17487.5	10883.2	S
189.356	0.0919	0.0000	82.450	0.01634	0.20944	40108.7	17488.8	10900.0	S
189.378	0.0919	0.0000	82.449	0.01631	0.20912	40116.0	17490.1	10916.7	S
189.400	0.0919	0.0000	82.447	0.01627	0.20880	40123.4	17491.4	10933.4	S
189.422	0.0919	0.0000	82.445	0.01624	0.20848	40130.7	17492.7	10950.1	S
189.444	0.0919	0.0000	82.444	0.01621	0.20816	40138.1	17494.0	10966.8	S
189.467	0.0919	0.0000	82.442	0.01618	0.20784	40145.4	17495.3	10983.4	S
189.489	0.0919	0.0000	82.440	0.01614	0.20752	40152.8	17496.6	11000.0	S
189.511	0.0919	0.0000	82.439	0.01611	0.20720	40160.1	17497.9	11016.6	S
189.533	0.0919	0.0000	82.437	0.01608	0.20689	40167.5	17499.2	11033.2	S
189.556	0.0919	0.0000	82.435	0.01605	0.20657	40174.8	17500.5	11049.7	S
189.578	0.0919	0.0000	82.434	0.01602	0.20625	40182.2	17501.8	11066.2	S
189.600	0.0919	0.0000	82.432	0.01599	0.20593	40189.5	17503.0	11082.7	S
189.622	0.0919	0.0000	82.431	0.01596	0.20562	40196.9	17504.3	11099.2	S
189.644	0.0919	0.0000	82.429	0.01592	0.20530	40204.2	17505.6	11115.6	S
189.667	0.0919	0.0000	82.427	0.01589	0.20498	40211.6	17506.9	11132.0	S
189.689	0.0919	0.0000	82.426	0.01586	0.20467	40218.9	17508.1	11148.4	S
189.711	0.0919	0.0000	82.424	0.01583	0.20435	40226.3	17509.4	11164.8	S
189.733	0.0919	0.0000	82.422	0.01580	0.20403	40233.6	17510.7	11181.1	S
189.756	0.0919	0.0000	82.421	0.01577	0.20372	40241.0	17511.9	11197.4	S
189.778	0.0919	0.0000	82.419	0.01574	0.20340	40248.3	17513.2	11213.7	S
189.800	0.0919	0.0000	82.418	0.01571	0.20309	40255.7	17514.5	11230.0	S
189.822	0.0919	0.0000	82.416	0.01569	0.20277	40263.0	17515.7	11246.2	S
189.845	0.0919	0.0000	82.414	0.01566	0.20246	40270.4	17517.0	11262.4	S
189.867	0.0919	0.0000	82.413	0.01563	0.20214	40277.7	17518.2	11278.6	S
189.889	0.0919	0.0000	82.411	0.01560	0.20183	40285.1	17519.5	11294.8	S
189.911	0.0919	0.0000	82.410	0.01557	0.20152	40292.4	17520.7	11310.9	S
189.933	0.0919	0.0000	82.408	0.01554	0.20120	40299.8	17522.0	11327.0	S
189.956	0.0919	0.0000	82.406	0.01551	0.20089	40307.1	17523.2	11343.1	S
189.978	0.0919	0.0000	82.405	0.01549	0.20058	40314.5	17524.4	11359.1	S
190.000	0.0919	0.0000	82.403	0.01546	0.20026	40321.8	17525.7	11375.2	S
190.022	0.0919	0.0000	82.402	0.01543	0.19995	40329.2	17526.9	11391.2	S
190.044	0.0919	0.0000	82.400	0.01540	0.19964	40336.5	17528.1	11407.2	S
190.067	0.0919	0.0000	82.399	0.01538	0.19932	40343.9	17529.4	11423.1	S
190.089	0.0919	0.0000	82.397	0.01535	0.19901	40351.3	17530.6	11439.1	S
190.111	0.0919	0.0000	82.395	0.01532	0.19870	40358.6	17531.8	11455.0	S
190.133	0.0919	0.0000	82.394	0.01530	0.19839	40366.0	17533.1	11470.9	S
190.156	0.0919	0.0000	82.392	0.01527	0.19808	40373.3	17534.3	11486.7	S
190.178	0.0919	0.0000	82.391	0.01524	0.19777	40380.7	17535.5	11502.5	S
190.200	0.0919	0.0000	82.389	0.01522	0.19746	40388.0	17536.7	11518.4	S
190.222	0.0919	0.0000	82.388	0.01519	0.19715	40395.4	17537.9	11534.1	S
190.244	0.0919	0.0000	82.386	0.01516	0.19684	40402.7	17539.1	11549.9	S
190.267	0.0919	0.0000	82.385	0.01514	0.19653	40410.1	17540.4	11565.6	S
190.289	0.0919	0.0000	82.383	0.01511	0.19622	40417.4	17541.6	11581.3	S
190.311	0.0919	0.0000	82.382	0.01509	0.19591	40424.8	17542.8	11597.0	S
190.333	0.0919	0.0000	82.380	0.01506	0.19560	40432.1	17544.0	11612.7	S
190.356	0.0919	0.0000	82.379	0.01504	0.19529	40439.5	17545.2	11628.3	S
190.378	0.0919	0.0000	82.377	0.01501	0.19498	40446.8	17546.4	11643.9	S
190.400	0.0919	0.0000	82.376	0.01499	0.19467	40454.2	17547.6	11659.5	S
190.422	0.0919	0.0000	82.374	0.01496	0.19436	40461.5	17548.8	11675.1	S
190.444	0.0919	0.0000	82.373	0.01494	0.19405	40468.9	17550.0	11690.6	S
190.467	0.0919	0.0000	82.371	0.01492	0.19375	40476.2	17551.2	11706.1	S
190.489	0.0919	0.0000	82.370	0.01489	0.19344	40483.6	17552.4	11721.6	S
190.511	0.0919	0.0000	82.368	0.01487	0.19313	40490.9	17553.6	11737.1	S
190.533	0.0919	0.0000	82.367	0.01484	0.19283	40498.3	17554.8	11752.5	S
190.556	0.0919	0.0000	82.365	0.01482	0.19252	40505.6	17555.9	11767.9	S
190.578	0.0919	0.0000	82.364	0.01480	0.19221	40513.0	17557.1	11783.3	S
190.600	0.0919	0.0000	82.362	0.01477	0.19191	40520.3	17558.3	11798.7	S
190.622	0.0919	0.0000	82.361	0.01475	0.19160	40527.7	17559.5	11814.0	S
190.644	0.0919	0.0000	82.359	0.01473	0.19130	40535.0	17560.7	11829.3	S
190.667	0.0919	0.0000	82.358	0.01470	0.19099	40542.4	17561.8	11844.6	S
190.689	0.0919	0.0000	82.356	0.01468	0.19069	40549.7	17563.0	11859.9	S
190.711	0.0919	0.0000	82.355	0.01466	0.19038	40557.1	17564.2	11875.1	S
190.733	0.0919	0.0000	82.353	0.01464	0.19008	40564.4	17565.4	11890.4	S

Detailed Results (cont,d.) :: *Scenario 48 :: FDOT 240 Hour - 240 hr - 100 yr*

Elapsed Time (hours)	Inflow Rate (ft³/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft³/s)	Overflow Discharge (ft³/s)	Cumulative Inflow Volume (ft³)	Cumulative Infiltration Volume (ft³)	Cumulative Discharge Volume (ft³)	Flow Type
240.089	0.0173	0.0000	81.902	0.01255	0.01155	48143.2	19833.3	19846.6	S
240.111	0.0136	0.0000	81.902	0.01244	0.01113	48144.4	19834.3	19847.5	S
240.133	0.0102	0.0000	81.901	0.01232	0.01052	48145.4	19835.3	19848.4	S
240.156	0.0073	0.0000	81.901	0.01220	0.00974	48146.1	19836.3	19849.2	S
240.178	0.0052	0.0000	81.901	0.01209	0.00879	48146.6	19837.3	19849.9	S
240.200	0.0037	0.0000	81.901	0.01198	0.00768	48146.9	19838.3	19850.6	S
240.222	0.0027	0.0000	81.901	0.01184	0.00638	48147.2	19839.2	19851.2	S
240.244	0.0019	0.0000	81.900	0.01160	0.00483	48147.4	19840.1	19851.6	S
240.267	0.0014	0.0000	81.900	0.01131	0.00275	48147.5	19841.1	19851.9	S
240.289	0.0010	0.0000	81.900	0.01185	0.00000	48147.6	19842.0	19852.0	S
240.311	0.0007	0.0000	81.900	0.01253	0.00000	48147.7	19843.0	19852.0	S
240.333	0.0005	0.0000	81.900	0.01252	0.00000	48147.7	19844.0	19852.0	S
240.356	0.0004	0.0000	81.899	0.01250	0.00000	48147.8	19845.0	19852.0	S
240.378	0.0003	0.0000	81.899	0.01249	0.00000	48147.8	19846.0	19852.0	S
240.400	0.0002	0.0000	81.899	0.01248	0.00000	48147.8	19847.0	19852.0	S
240.422	0.0001	0.0000	81.899	0.01247	0.00000	48147.8	19848.0	19852.0	S
240.444	0.0001	0.0000	81.899	0.01246	0.00000	48147.8	19849.0	19852.0	S
240.467	0.0000	0.0000	81.899	0.01244	0.00000	48147.8	19850.0	19852.0	S
240.489	0.0000	0.0000	81.898	0.01243	0.00000	48147.8	19850.9	19852.0	S
240.511	0.0000	0.0000	81.898	0.01242	0.00000	48147.8	19851.9	19852.0	S
240.533	0.0000	0.0000	81.898	0.01241	0.00000	48147.8	19852.9	19852.0	S
240.556	0.0000	0.0000	81.898	0.01240	0.00000	48147.8	19853.9	19852.0	S
240.578	0.0000	0.0000	81.898	0.01239	0.00000	48147.8	19854.9	19852.0	S
242.978	0.0000	0.0000	81.879	0.01168	0.00000	48147.8	19957.4	19852.0	S
245.378	0.0000	0.0000	81.861	0.01133	0.00000	48147.8	20056.7	19852.0	S
247.778	0.0000	0.0000	81.843	0.01105	0.00000	48147.8	20153.3	19852.0	S
250.178	0.0000	0.0000	81.825	0.01080	0.00000	48147.8	20247.6	19852.0	S
252.578	0.0000	0.0000	81.808	0.01057	0.00000	48147.8	20339.8	19852.0	S
254.978	0.0000	0.0000	81.791	0.01036	0.00000	48147.8	20430.2	19852.0	S
257.378	0.0000	0.0000	81.775	0.01016	0.00000	48147.8	20518.8	19852.0	S
259.778	0.0000	0.0000	81.758	0.00998	0.00000	48147.8	20605.8	19852.0	S
262.178	0.0000	0.0000	81.742	0.00981	0.00000	48147.8	20691.3	19852.0	S
264.578	0.0000	0.0000	81.726	0.00961	0.00000	48147.8	20775.3	19852.0	S
288.578	0.0000	0.0000	81.582	0.00806	0.00000	48147.8	21512.2	19852.0	S
312.578	0.0000	0.0000	81.449	0.00720	0.00000	48147.8	22167.8	19852.0	S
336.578	0.0000	0.0000	81.325	0.00648	0.00000	48147.8	22756.1	19852.0	S
360.578	0.0000	0.0000	81.208	0.00585	0.00000	48147.8	23286.7	19852.0	S
384.578	0.0000	0.0000	81.098	0.00530	0.00000	48147.8	23767.1	19852.0	S
408.578	0.0000	0.0000	80.995	0.00482	0.00000	48147.8	24203.1	19852.0	S
432.578	0.0000	0.0000	80.897	0.00440	0.00000	48147.8	24600.2	19852.0	S
456.578	0.0000	0.0000	80.805	0.00402	0.00000	48147.8	24963.0	19852.0	S
480.578	0.0000	0.0000	80.718	0.00369	0.00000	48147.8	25295.5	19852.0	S
504.578	0.0000	0.0000	80.636	0.00340	0.00000	48147.8	25600.9	19852.0	S
528.578	0.0000	0.0000	80.558	0.00313	0.00000	48147.8	25882.3	19852.0	S
552.578	0.0000	0.0000	80.483	0.00290	0.00000	48147.8	26142.2	19852.0	S
576.578	0.0000	0.0000	80.412	0.00268	0.00000	48147.8	26382.8	19852.0	S
600.578	0.0000	0.0000	80.344	0.00249	0.00000	48147.8	26605.9	19852.0	S
624.578	0.0000	0.0000	80.280	0.00232	0.00000	48147.8	26813.3	19852.0	S
648.578	0.0000	0.0000	80.218	0.00216	0.00000	48147.8	27006.3	19852.0	S
672.578	0.0000	0.0000	80.158	0.00201	0.00000	48147.8	27186.2	19852.0	S
696.578	0.0000	0.0000	80.101	0.00188	0.00000	48147.8	27354.3	19852.0	S
720.578	0.0000	0.0000	80.047	0.00176	0.00000	48147.8	27511.5	19852.0	S
744.578	0.0000	0.0000	79.994	0.00165	0.00000	48147.8	27658.7	19852.0	S
768.578	0.0000	0.0000	79.944	0.00155	0.00000	48147.8	27796.7	19852.0	S
792.578	0.0000	0.0000	79.895	0.00146	0.00000	48147.8	27926.3	19852.0	S
816.578	0.0000	0.0000	79.848	0.00137	0.00000	48147.8	28048.2	19852.0	S
840.578	0.0000	0.0000	79.803	0.00129	0.00000	48147.8	28162.9	19852.0	S
864.578	0.0000	0.0000	79.760	0.00077	0.00000	48147.8	28271.1	19852.0	S
888.578	0.0000	0.0000	79.703	0.00014	0.00000	48147.8	28295.8	19852.0	S
912.578	0.0000	0.0000	79.647	0.00000	0.00000	48147.8	28295.8	19852.0	S
936.578	0.0000	0.0000	79.596	0.00000	0.00000	48147.8	28295.8	19852.0	S
960.578	0.0000	0.0000	79.550	----	----	48147.8	28295.8	19852.0	N.A.

Detailed Results :: *Scenario 49 :: WQTV*

Elapsed Time (hours)	Inflow Rate (ft^3/s)	Outside Recharge (ft/day)	Stage Elevation (ft datum)	Infiltration Rate (ft^3/s)	Overflow Discharge (ft^3/s)	Cumulative Inflow Volume (ft^3)	Cumulative Infiltration Volume (ft^3)	Cumulative Discharge Volume (ft^3)	Flow Type
0.000	828.5000	0.0000	77.000	0.00000	0.00000	0.0	0.0	0.0	N.A.
0.002	828.5000	0.0000	81.199	0.30874	0.00000	4971.0	1.9	0.0	U/P
2.400	0.0000	0.0000	80.597	0.16789	0.00000	4971.0	2417.1	0.0	U/P
6.000	0.0000	0.0000	----	----	----	4971.0	4971.0	0.0	dry
12.000	0.0000	0.0000	----	----	----	4971.0	4971.0	0.0	dry
24.000	0.0000	0.0000	----	----	----	4971.0	4971.0	0.0	dry
36.000	0.0000	0.0000	----	----	----	4971.0	4971.0	0.0	dry
48.000	0.0000	0.0000	----	----	----	4971.0	4971.0	0.0	dry
60.000	0.0000	0.0000	----	----	----	4971.0	4971.0	0.0	dry
72.000	0.0000	0.0000	----	----	----	4971.0	4971.0	0.0	dry
84.000	0.0000	0.0000	----	----	----	4971.0	4971.0	0.0	dry
96.000	0.0000	0.0000	----	----	----	4971.0	4971.0	0.0	dry

Appendix B

Operation and Maintenance Requirements and
Erosion and Sedimentation Control Requirement

Proposed operation and maintenance and soil erosion and sediment control practices are outlined in the following paragraphs.

Surface Water Management Facilities

The man-made surface water facility shall be maintained free of sediments and debris. Areas shall be inspected on a routine basis and nuisance plants shall be removed a minimum of twice annually. Grassed areas shall be mowed a minimum of 6 times per year. The natural systems shall be least disturbed as possible. Minimal maintenance is required for the natural and undisturbed areas. All ponds shall be inspected monthly. Monthly documentation shall be noted based upon the inspection findings.

Erosion Control

All erosion damage at spillways, outfall structures, and along pond side slopes shall be repaired (grading and grassing) as conditions occur. All side slopes and other areas disturbed by construction shall be stabilized by sodding, hydro-mulching or other appropriate vegetative or non-vegetative erosion control measures.

Swale/Ditch

All swales, if any, shall be maintained free of debris and sediment. Sediments shall be removed when the depth has been reduced by 20 percent. Sediments removed from swales/ditches should be evenly spread over grassed areas away from the stormwater management facilities.

Culverts, Pipes and Structures

All pipes, if any, shall be inspected bi-annually. Culverts and pipes shall be maintained free of debris and sediment. Sediments removed from culverts and pipes should be evenly spread over grassed areas away from the stormwater management facilities.
The structures and paved flow lines, if any, shall be maintained clear of debris. Remove any debris and silt collected in inlets and pipes as routine inspections dictates.

Inspection Reporting

Annual inspection reports, prepared by a properly licensed professional engineer, should be submitted to the water management district as appropriate. The engineer shall inspect the site and report on the status and function of the system. Noted deficiencies and/or maintenance requirements shall be reported to the owner with recommendations for repairs. Repairs shall be executed.

Limerock/Sinkhole

If continuous limerock is encountered during excavation of the swales/pond or if a sinkhole forms in the area of a drainage swale/pond the engineer of record shall be notified by either the contractor or the established operation and maintenance entity. The engineer of record shall inspect the repaired area upon completion of the repair.

Where continuous limerock is encountered during excavation of the swales/ponds, the limerock shall be over excavated by 2 feet and replaced with clayey soils that extend 2 feet beyond the perimeter of the limerock outcropping. The clayey soil shall have at least 20% passing the no. 200 sieve, compacted to 95% of standard proctor, and compacted in a wet condition with moisture 2% - 4% above optimum.

All swales/ponds shall be inspected monthly for sinkhole occurrence. Should a sinkhole occur, the area shall be repaired as soon as possible. Repair shall include filling (limerock such as road base material, clay/sand mixture, or concrete if necessary). A 2-foot deep cap that extends 2 feet beyond the perimeter of the sinkhole shall be constructed with clayey soils. The clayey soil shall have at least 20% passing the no. 200 sieve, compacted to 95% of standard proctor, and compacted in a wet condition with moisture 2% - 4% above optimum. The clay soil cap shall be re-graded to prevent concentration of waters (ponding) and re-vegetated.

Outfall Structures

All outfall and drawdown orifices are to be inspected bi-annually for sediment or debris in the flow line of weirs or orifices. All sediment and debris should be removed and disposed of in an approved manner.

Discharge to Conservation Management Areas Maintenance and Repair

The stormwater management facilities shall be inspected after rainfall events greater than three inches for any indications of erosion. If any indications are noticed, then these should be repaired as soon as possible so as to prevent any blow outs from future rainfall events. The conditions of the facilities should be repaired to those conditions depicted on the approved Final Development Plans.

Operation & Maintenance Entity:

D.E. Foods, LLC
170 Olde Forge Road
Hanover, MA 02339

Appendix C

Geotechnical Report





ECS Florida, LLC

Geotechnical Engineering Report

QSR Alachua

Tax Parcel 03053-002-001
Alachua, Alachua County, Florida

ECS Project Number 35:31000

December 4, 2020

December 4, 2020

Mr. Dan Whitney
DE Foods, LLC
101 Accord Park Drive
Norwell, MA 02061

ECS Project No. 35:31000
Client ID: PH49

Reference: Geotechnical Engineering Report
QSR Alachua
Tax Parcel 03053-002-001
Alachua, Alachua County, Florida

Dear Mr. Whitney:

ECS Florida, LLC (ECS) has completed the subsurface exploration, laboratory testing, and geotechnical engineering analyses for the above-referenced project. Our services were performed in general accordance with our Proposal No. 35:17217, dated October 28, 2020. This report presents our understanding of the geotechnical aspects of the project, the results of the field exploration and laboratory testing conducted, and our design and construction recommendations.

It has been our pleasure to be of service to DE Foods, LLC during the design phase of this project. We would appreciate the opportunity to remain involved during the continuation of the design phase, and we would like to provide our services during construction phase operations as well to verify the assumptions of subsurface conditions made for this report. Should you have any questions concerning the information contained in this report, or if we can be of further assistance to you, please contact us.

Respectfully submitted,

ECS Florida, LLC.

David Spangler, P.E.
Principal Engineer
Registered Florida No. 58770
dspangler@ecslimited.com

Corey A. Dunlap, P.E.
Senior Engineer
Registered Florida No. 77678
cdunlap@ecslimited.com

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Addressee 1 pdf
Daniel Young, P.E. – CHW Professional Consultants, Inc. 1 pdf

TABLE OF CONTENTS

EXECUTIVE SUMMARY 1
1.0 INTRODUCTION 2
1.1 General 2
1.2 Scope of Services 2
1.3 Authorization 3
2.0 PROJECT INFORMATION 4
2.1 Project Location 4
2.2 Site Conditions 4
2.3 Project Description 4
3.0 FIELD EXPLORATION 6
3.1 Field Exploration Program 6
3.1.1 SPT and Auger Borings 6
3.2 Regional/Site Geology 6
3.3 Soil Survey Mapping 7
3.4 Subsurface Characterization 8
3.5 Groundwater Level 9
4.0 LABORATORY TESTING 10
5.0 DESIGN RECOMMENDATIONS 11
5.1 General 11
5.2 Foundation Design Recommendations 11
5.2.1 Foundations 11
5.2.2 Floor Slabs 12
5.3 Site Design Considerations 12
5.3.1 Pavement Sections 12
5.3.2 Stormwater Management Facilities 14
5.3.3 Borrow Suitability 14
5.4 General Design Considerations 15
6.0 SITE CONSTRUCTION RECOMMENDATIONS 16
6.1 Subgrade Preparation 16
6.1.1 Stripping and Grubbing 16
6.1.2 Proofrolling 16
6.1.3 Temporary Groundwater Control 17
6.1.4 Subgrade Compaction 17
6.1.5 Structural Backfill and Fill Soils 17
6.1.6 Foundation Areas 18
6.1.7 Pavement Areas 18
6.2 Utility Installations 18
6.3 General Construction Considerations 19
7.0 QUALITY CONTROL TESTING 21
8.0 CLOSING 22

FIGURES

Figure 1 Site Location Plan
Figure 2 Boring Location Diagram
Generalized Subsurface Soil Profile Section A-A'
Generalized Subsurface Soil Profile Section B-B'

APPENDICES

Appendix A – Field Operations
Soil Boring Logs
Field Exploration Procedures
Key to Soil Classification

Appendix B – Laboratory Testing
Laboratory Data

EXECUTIVE SUMMARY

The following summarizes the main findings of the exploration, particularly those that may have a cost impact on the planned development. Further, our principal foundation recommendations are summarized. Information gleaned from the executive summary should not be utilized in lieu of reading the entire geotechnical report.

The geotechnical exploration performed for the planned development included 6 soil test borings drilled to depths between 5 and 20 feet. Bulk samples for laboratory testing of permeability and Limerock Bearing Ratio (LBR) were collected from the site.

The borings generally encountered approximately 4 to 5 feet of silty soils underlain by a stratum of clayey soils. The clayey soils were found to have low swell potential.

The planned single-story restaurant building can be supported by conventional shallow foundations consisting of column or strip footings bearing and natural soils. We recommend the shallow foundations be designed for a net soil bearing pressure of 2,500 psf. Details of the assumed foundation subgrade elevations and loads are contained in the body of the report.

A stormwater basin will be located on the east of the building boundary. Our recommended soil parameters for stormwater management designs are included in the body of the report. Based upon the soil types identified in this area, undercutting and replacement of clayey soils should be expected. The undercutting is not intended to improve basin performance, but rather ensure soil fines do not create a thin confining layer at the basin bottom.

The results of the borings indicate conventional pavements are suitable for this project.

We recommend ECS be retained to provide quality assurance services throughout these critical phases of construction to provide general consultation as issues arise. ECS is not responsible for the conclusions, opinions, or recommendations of others based on the data in this report.

1.0 INTRODUCTION

1.1 GENERAL

The purpose of this study was to provide geotechnical information to assist building foundation, pavement, and stormwater management designs. This report contains the results of our subsurface explorations and laboratory testing programs, site characterization, engineering analyses, and project recommendations.

The recommendations developed for this report are based on project information supplied by CHW Professional Consultants, Inc. (CHW), the project civil engineer.

1.2 SCOPE OF SERVICES

To obtain the necessary geotechnical information required for design:

- Two (2) Standard Penetration Test (SPT) borings were performed to depths of 20 feet beneath grade in the area of the planned structure.
- Two (2) auger borings were performed to depths of 15 feet beneath grade in the area of the planned for stormwater management.
- Two (2) auger borings were performed to depths of 5 feet beneath grade in the area of the planned pavements.

A laboratory-testing program was also implemented to characterize the physical and engineering properties of the subsurface soils.

This report discusses our exploratory and testing procedures, presents our findings and evaluations and includes the following.

- A brief review and description of our field and laboratory test procedures and the results of testing conducted.
- A review of surface topographical features and site conditions.
- A review of area and site geologic conditions.
- A review of subsurface soil stratigraphy with pertinent available physical properties.
- Final copies of our test boring logs.
- Recommended foundation type(s).
- General recommendations for pavement design, including a recommended design LBR value.
- Recommendations for design and construction of stormwater management facilities.
- Recommendations for site preparation and construction of compacted fills, including an evaluation of on-site soils for use as compacted fills and delineation of potentially unsuitable soils and/or soils exhibiting excessive moisture at the time of sampling.

1.3 AUTHORIZATION

Our services were provided in accordance with our Proposal No. 35:17217, dated October 28, 2020, as authorized by Daniel Whitney with DE Foods, LLC on October 29, 2020, and includes the Terms and Conditions of Service outlined with our Proposal.

2.0 PROJECT INFORMATION

2.1 PROJECT LOCATION

The project site is located at the Tax Parcel 03053-002-001 southeast of the US 441 and NW 167th Boulevard intersection in Alachua, Alachua County, Florida. The site is generally surrounded by commercial and residential developments. The general site location is shown on Figure 1.

2.2 SITE CONDITIONS

At the time of our exploration, the site was undeveloped, with surface cover consisting of large trees and heavy underbrush. Surface water was not observed near planned construction areas at the time of our exploration.

A site survey was not available to our office at the time of this report preparation. However, based on publicly available information, we understand that the site generally slopes downward to the east. Review of readily available published data indicates the existing ground surface elevations generally range from El. +82 to El. +86 feet[1] (NAVD88).

2.3 PROJECT DESCRIPTION

Project information was provided by CHW via a Request for Proposal and several discussions. We were provided with a copy of a site plan for the subject site. This plan indicated the boundary limits for the property, the existing roadways adjacent to the site, and the layout of the proposed construction.

We understand the proposed building includes a single-story structure approximately ±3,700 square feet. The following information explains our understanding of the structure and its loads:

Table 2.3.1 Design Values

SUBJECT	DESIGN INFORMATION / EXPECTATIONS
Building Footprint	Approximately 3,700 square feet in plan view
# of Stories	1 story above grade maximum
Usage	Commercial
Framing	We anticipate that the building will be principally load bearing wood framed or masonry walls with interior columns
Column Loads	50 kips maximum
Wall Loads	2 kips per linear foot (klf) maximum
Fill/Cuts	Less than 2 feet of fill (and only nominal cuts) will be required to achieve final grades in structural areas.

[1] Alachua County Growth Management Website, https://mapgenius.alachuacounty.us/

A parking lot will be located west and south of the building. A drive-thru aisle will be located on the east side of the building. A stormwater basin will be located near the east property boundary.

If actual building loads or fill/cut heights vary from these conditions, then the recommendations in this report may need to be re-evaluated. We should be contacted if any of the above project information is incorrect so that we may reevaluate our recommendations.

3.0 FIELD EXPLORATION

3.1 FIELD EXPLORATION PROGRAM

We performed a field exploration on November 19, 2020. The approximate boring locations are indicated on the attached Boring Location Diagram (Figure 2). Our personnel determined the boring locations using readily available aerial photographs and handheld GPS device. The boring locations on the referenced Boring Location Diagram should be considered accurate only to the degree implied by the method of measurement used.

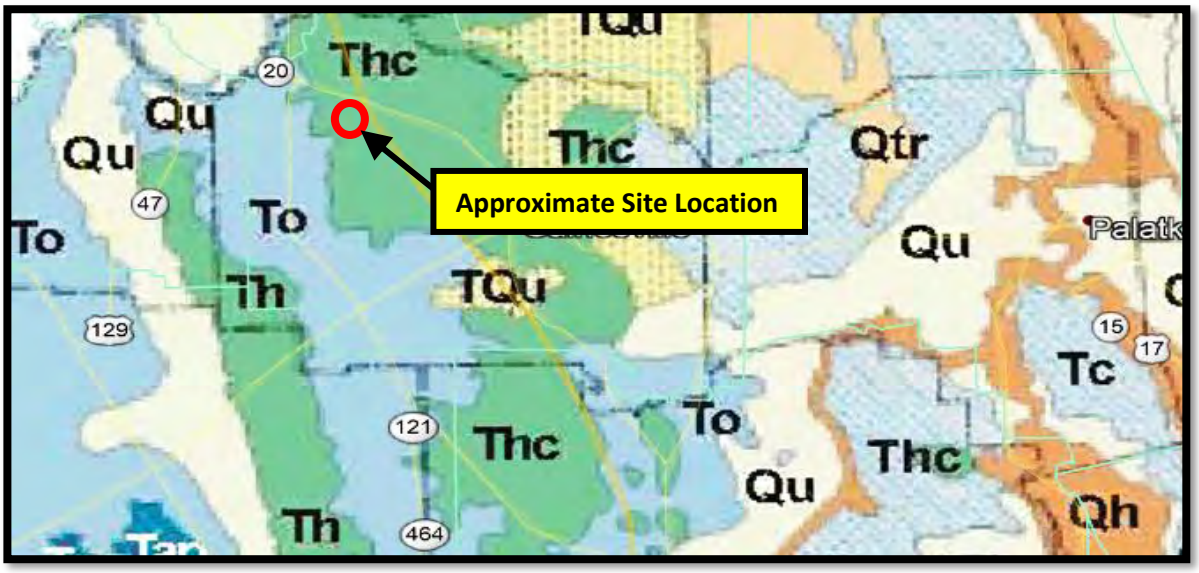
3.1.1 SPT and Auger Borings

The SPT borings were performed in general accordance with the methodology outlined in ASTM D 1586. Split-spoon soil samples recovered during performance of the borings were visually classified in the field and representative portions of the samples were transported to our laboratory for further evaluation.

The auger borings were performed in general accordance with the methodology outlined in ASTM D 1452. Representative soil samples also were recovered from the auger borings and returned to our laboratory for further evaluation. A summary of the field procedures is included in Appendix A.

3.2 REGIONAL/SITE GEOLOGY

The site is located near the central portion of Alachua County. This area of Alachua County maps as the Coosawhatchie Formation geological region[2]. The following description is from the Geological Survey



[2] Open-File Report 80, Thomas M. Scott, P.G. No. 99, Text to Accompany the Geological Map of Florida, Florida Geological Survey, 2001.

Thc – Coosawhatchie Formation – The Coosawhatchie Formation is sediments of the Miocene Series that is exposed or lies beneath a thin overburden on the eastern flank of the Ocala Platform from southern Columbia County to southern Marion County. Within the outcrop region, the Coosawhatchie Formation varies from a light gray to olive gray, poorly consolidated, variable clayey and phosphatic sand with few fossils, to an olive gray, poorly to moderately consolidated, slightly sandy, silty clay with few to no fossils. Occasionally, the sands will contain a dolomite component and, rarely, the dominant lithology will be dolostone or limestone. Silicified nodules are often present in the Coosawhatchie Formation sediments in the outcrop region. The sediment may contain 20 percent or more phosphate (Scott, 1988). Permeability of the Coosawhatchie Formation is generally low, forming part of the intermediate confining unit/ aquifer system.

The Miocene sediments consist of siliciclastics, carbonates and mixed siliciclastics-carbonate lithologies with numerous lateral and vertical facies changes. The importance of the Miocene sediments in Florida is twofold - first, these sediments contain valuable mineral resources, primarily phosphate and absorptive clays; and second, the Miocene sediments comprise the intermediate confining unit and aquifer system. Whereas the principal geological hazard associated with Paleogene carbonates is karst development, the hazards associated with the Miocene sediments are radon gas and swelling clays.

3.3 SOIL SURVEY MAPPING

Based on the Web Soil Survey for Alachua County, Florida, as prepared by the U.S. Department of Agriculture Natural Resource Conservation Service[3], the predominant soil types existing within the site area are described in the following table. The site area is illustrated superimposed on the USDA-NRCS Soil Survey Map included as Figure 3.3.1:

Table 3.3.1 Soil Survey Data

Soil ID	Soil Type	Typical Profile	Hydrology	Hydrologic Soil Classification	Estimated Seasonal High Groundwater Level(1)
4	Arredondo-Urban land complex 0 to 5% slopes	0 to 49 inches: fine sand 49 to 54 inches: loamy sand 54 to 86 inches: sandy clay loam	Well drained	A	80+
30	Kendrick Sand 2 to 5% slopes	0 to 26 inches: sand 26 to 31 inches: fine sandy loam 31 to 90 inches: sandy clay loam	Well drained	B	80+

(1) Inches below ground surface at time of survey.

[3] Soil Survey of Alachua County, Florida. Soil Conservation Service, U.S. Department of Agriculture.

Figure 3.3.1 Soil Survey Map



3.4 SUBSURFACE CHARACTERIZATION

The subsurface conditions encountered were generally consistent with published soil survey information and geological mapping. A graphical presentation of the generalized subsurface conditions is presented in the Figures. Detailed boring records are included in Appendix A. It should be understood that the soil conditions will vary between the boring locations. The following table description summarizes the soil conditions encountered.

Table 3.4.1 Subsurface Stratigraphy

Approximate Depth Range (ft)	Stratum	Description	Ranges of SPT(1) N-values (bpf)
0 to 4-5	I	SILTY SAND (SM), brown to dark brown, moist, loose	4 to 8
0 to 20	II	CLAYEY SAND (SC) and VERY CLAYEY SAND (SC/CL), brown, orange, light gray to gray, light tan to tan, moist to wet, loose to medium dense	9 to 28

Notes: (1) Standard Penetration Test

3.5 GROUNDWATER LEVEL

Measured Groundwater: Groundwater was not encountered in the borings at the time of drilling within the explored depths. We note that groundwater levels will fluctuate due to seasonal climatic variations, surface water runoff patterns, construction operations, and other interrelated factors.

Estimated Seasonal High Groundwater: The normal seasonal high groundwater level is affected by a number of factors. The drainage characteristics of the soils, land surface elevation, relief points such as drainage ditches, lakes, rivers, swamp areas, etc., and distance to relief points are some of the more important factors influencing the seasonal high groundwater level.

Based on our interpretation of the site conditions, including the boring logs and Soil Survey information, we estimate the seasonal high groundwater table will be perched on top of the clayey sands at depths of approximately 4 to 5 feet beneath existing ground surface. The perched seasonal high groundwater table is only expected to develop after periods of intense or sustained seasonal rainfall. It is possible that groundwater levels may exceed the estimated normal seasonal high groundwater level as a result of significant or prolonged rains.

4.0 LABORATORY TESTING

The laboratory testing performed by ECS for this project consisted of selected tests performed on samples obtained during our field exploration operations. The following paragraphs briefly discuss the results of the completed laboratory testing program.

An experienced geotechnical engineer visually classified each soil sample from the test borings on the basis of texture and plasticity in accordance with the Unified Soil Classification System (USCS) in general accordance with ASTM D 2488. Additionally, the following laboratory tests were performed to aid in classifying the soils and to further evaluate their engineering properties:

- Five (5) percent soil fines passing the No. 200 sieve determinations (ASTM D 2487)
- Five (5) natural moisture content determinations (ASTM D 2216)
- One (1) Atterberg Limits tests (ASTM D 4318)
- One (1) constant head hydraulic conductivity tests (ASTM D 2434)
- One (1) Limerock Bearing Ratio (LBR) test (FM 5-515)

The laboratory tests indicate the tested soils consist of silty sand and very clayey sand.

- The tested silty sand (SM) contains approximately 18.2 to 30.4 percent soil fines, natural moisture contents of about 11.6 to 12.3 percent, and a hydraulic conductivity value of approximately 18 feet per day.
- The tested very clayey sand (SC/CL) contains approximately 34.4 to 46.7 percent soil fines, natural moisture contents of 14.8 to 26.4 percent, and is non-plastic. The Atterberg Limits results indicate the tested clayey sand has low swell potential[4].
- The LBR test indicates the surficial silty sand (SM) has a maximum dry density of 123 pcf, an optimum moisture content of 11.6 percent, and an LBR value of 70.

The results of the laboratory testing are shown in the Summary of Laboratory Test Data included in Appendix B.

[4] Foundations in Expansive Soils, Department of the Army, TM 5-818-7, September 1993.

5.0 DESIGN RECOMMENDATIONS

5.1 GENERAL

Our geotechnical engineering evaluation of the site and subsurface conditions at the property, with respect to the planned construction and our recommendations for site preparation and foundation support, are based on (1) our site observations, (2) the field and laboratory test data obtained, (3) our understanding of the project information and structural conditions as presented in this report, and (4) our experience with similar soil and loading conditions.

If the stated structural or grading conditions are incorrect, or should the location of the structure or pavement areas be changed, please contact us so that we can review our recommendations. Also, the discovery of any site or subsurface conditions during construction that deviate from the data obtained during this geotechnical exploration should also be reported to us for our evaluation.

The recommendations in the subsequent sections of this report present design and construction techniques that are appropriate for the planned construction. We recommend that ECS be provided the opportunity to review the foundation plans and earthwork specifications to verify that our recommendations have been properly interpreted and implemented.

5.2 FOUNDATION DESIGN RECOMMENDATIONS

Based on the results of our exploration, we consider the subsurface conditions at the site adaptable for support of the proposed structure on a properly designed conventional shallow foundation system. Provided the site preparation and earthwork construction recommendations outlined in Section 6.0 of this report are performed, the following parameters may be used for foundation design.

5.2.1 Foundations

Provided subgrades and structural fills are prepared as discussed herein, the proposed structure can be supported by conventional shallow foundations. The design of the foundation shall utilize the following parameters:

Table 5.2.1.1 Foundation Design

Design Parameter	Column Footing	Wall Footing
Minimum Width	24 inches	18 inches
Minimum Footing Embedment Depth (below slab or finished grade)	24 inches	18 inches
Estimated Maximum Total Settlement	1 inch	1 inch
Estimated Maximum Differential Settlement	Less than 0.5 inches between columns	Less than 0.5 inches over 50 feet
Net Allowable Bearing Pressure[1]	2,500 psf	
Acceptable Bearing Soil Material	Stratum I or Compacted Fill	

1. Net allowable bearing pressure is the applied pressure in excess of the surrounding overburden soils above the base of the foundation.

Depending on the final floor elevations of the buildings, we anticipate that most of the soils at the foundation bearing elevation are anticipated to be suitable for support of the proposed structure, after prepared in accordance with Section 6.0 of this report. The bearing level soils, after compaction, should exhibit densities equivalent to 95 percent of the Modified Proctor maximum dry density (ASTM D 1557) to a depth of at least one foot below foundation bearing levels.

5.2.2 Floor Slabs

The floor slabs can be constructed as a slab-on-ground, provided the site is prepared as outlined in Section 6.0. A minimum clearance of 2 feet is recommended between the estimated seasonal high groundwater table and the bottom of the floor slab. It is recommended the floor slab bearing soils be covered with an impervious membrane to reduce moisture entry and floor dampness. A 6-mil thick plastic membrane is commonly used for this purpose. Care should be exercised not to tear large sections of the membrane during placement of reinforcing steel and concrete.

Subgrade Modulus: Provided the placement of Structural Fill per the recommendations discussed herein, the slab may be designed assuming a modulus of subgrade reaction, k_1 of 100 pci (lbs/cu. inch). The modulus of subgrade reaction value is based on empirical data.

Slab Isolation: Ground-supported slabs should be isolated from the foundations and foundation-supported elements of the structure so that differential movement between the foundations and slab will not induce excessive shear and bending stresses in the floor slab. Where the structural configuration prevents the use of a free-floating slab, the slab should be designed with suitable reinforcement and load transfer devices to preclude overstressing of the slab.

5.3 SITE DESIGN CONSIDERATIONS

5.3.1 Pavement Sections

Typical Asphalt Section: Based on the results of our exploration, we consider the subsurface conditions at the site favorable for support of a flexible pavement section when constructed on properly prepared subgrade soils as outlined in Section 6.0 of this report. Typical pavement sections used in northeast Florida are shown on the following table. If requested, we can prepare a project-specific pavement design if specific traffic data is provided. We recommend an LBR of 70 to be considered in the pavement design.

Table 5.3.1.1: Typical Flexible Pavement Section

Pavement Layer	Auto Parking & Traffic Lanes	Truck Areas
Asphaltic Concrete Wearing Surface	1.5"	2.0"
Limerock Base	6.0"	8.0"
Stabilized Subgrade	12.0"	12.0"

Wearing Surface: The wearing surface should consist of Florida Department of Transportation (FDOT) Type S asphaltic concrete having a minimum Marshall Stability of 1,500 lbs. Specific requirements for Type S asphaltic concrete wearing surface are outlined in the 2000 edition of the *Florida Department of Transportation, Standard Specifications for Road and Bridge Construction*. As an alternative, the wearing surface may consist of a Type SP-9.5 asphaltic concrete as outlined in the current edition of the *Standard Specifications.*

Base and Subgrade: The limerock base course should have a minimum Limerock Bearing Ratio (LBR) of 100 and should be compacted to 100 percent of the Modified Proctor maximum dry density (ASTM D 1557) value.

The subgrade material should have a minimum LBR of 40 and be compacted to 98 percent of the modified Proctor maximum dry density (ASTM D 1557) value.

Rigid Concrete Pavements: For Typical concrete pavement sections used in north central Florida are shown on the tables below. If requested, we can prepare a project specific pavement design if specific traffic data is provided. Our recommendations for slab thickness for standard duty and heavy duty concrete pavements are based on a) subgrade soils densified to 98 percent of the modified Proctor maximum dry density (ASTM D 1557) b) modulus of subgrade reaction (k) equal to 200 pounds per cubic inch, c) a 20 year design life.

Table 5.3.1.2: Typical Standard Duty Concrete Pavement Section

Minimum Pavement Thickness	Maximum Control Joint Spacing	Recommended Sawcut Depth
5 Inches	10 Feet x 10 Feet	1-1/4 Inches

Table 5.3.1.3: Typical Heavy Duty Concrete Pavement Section

Minimum Pavement Thickness	Maximum Control Joint Spacing	Recommended Sawcut Depth
6 Inches	12 Feet x 12 Feet	1-1/2 Inches

We recommend using concrete with a minimum 28-day compressive strength of 4,000 psi and a minimum 28-day flexural strength (modulus of rupture) of at least 600 pounds per square inch, based on 3rd point loading of concrete beam test samples. Layout of the sawcut control joints should form square panels, and the depth of sawcut joint should be at least ¼ of the concrete slab thickness. The joints should be sawed within six hours of concrete placement or as soon as the concrete has developed sufficient strength to support workers and equipment. We recommend allowing ECS Florida, LLC. to review and comment on the final concrete pavement design, including section and joint details (type of joints, joint spacing, etc.), prior to the start of construction.

For further details on concrete pavement construction, please reference the “Guide to Jointing Unreinforced Pavements”, published by the America Concrete Pavement Association, and “Building Quality Concrete Parking Areas”, published by the Portland Cement Association.

5.3.2 Stormwater Management Facilities

5.3.2.1 Soil Parameters for Design

Considering the findings of the test borings, laboratory tests, review of readily published information, and our experience with these types of soils, we recommend the following soil parameters be used for stormwater design.

- Base elevation of effective or mobilized aquifer (average depth of confining layer) equal to 5 feet beneath current grade.
- Unsaturated vertical infiltration rate of 12 feet per day.
- Horizontal hydraulic conductivity equal to 18 feet per day.
- Specific yield (fillable porosity) of 25 percent.
- Average seasonal high groundwater table depth equal to 5 feet beneath current grade.

The measured permeability/vertical infiltration rates should not be construed to represent the actual pond exfiltration rate. For pond design calculations, we recommend a factor of safety be applied to the above permeability/vertical infiltration rate value(s).

Undercutting: Considering the results of the test borings, it is possible silty or clayey soils (i.e. soils with greater than 15% soil fines) will be present at the basin bottom. Should this condition be apparent during construction, we recommend these soils be undercut a minimum of 2 feet and backfilled with sands and sands with silt (SP, SP-SM) having a maximum of 10% soil fines and a minimum vertical infiltration rate equal to that used for design. The intent of this undercutting and replacement is (1) to provide a uniform sand layer at the basin bottom that allows for lateral migration of water to the deeper deposits of sand; (2) and reduce the potential for soil fines leaching out of the soils when standing water is present in the basin which can result in a thin layer of confining material on the basin bottom reducing the effectiveness of the basin. The undercutting is not intended to improve basin performance, but rather ensure the soil fines do not create a thin confining layer on the basin bottom.

5.3.3 Borrow Suitability

Based on the boring results and classification of the soil samples, soils ranging from sands to clays are expected to be excavated from the site, in particular the stormwater management areas. A description of each soil type that could be excavated from the site with respect to fill suitability is presented below:

- Any sands (SP), sands with silt (SP-SM), and sands with clay (SP-SC) are considered suitable for use as fill soil. The soils containing surficial organic material will require removal and are unsuitable as structural fill. The organic soils could be used in landscape berms.

- Any silty and clayey sands (SM, SC) having a maximum of 30% soil fines may also be used as structural fill; however, we note that these soils will be more difficult to compact due to their tendency to retain soil moisture and will require drying. Depending on the anticipated time for completing the site work portion of the project and the drying time required to reduce the potential for pumping and yielding of these soils during placement and compaction operations, these soils may not be feasible for use as fill material. Soils

containing surficial organic material (topsoil) will require removal, and are considered unsuitable for use as embankment fill. The organic soils may be used in landscape berms.

Silty and clayey sands with greater than 30% soil fines are especially moisture sensitive and typically at least marginally plastic. We do not recommend using silty and clayey soils with greater than 30% soil fines as structural fill unless these materials are thoroughly mixed with sandy soils. We can provide soil mixing recommendations to engineer more suitable fill materials at your request.

- We do not recommend reuse of sandy clays to clays (CL or CH) as structural fill material because of their high plasticity and high affinity for moisture.

It should be anticipated that the soils in the proposed pond areas below the groundwater level will have moisture contents in excess of the Modified Proctor optimum moisture content. This will require stockpiling or spreading to drain the excess moisture. Generally, the wet soils should be dried to bring the soil moisture content within ±2 percent of the soil's optimum moisture content to facilitate placement and compaction.

5.4 GENERAL DESIGN CONSIDERATIONS

Gutters: Roof gutters should be considered to divert discharge away from the structure. We recommend, if at all possible, the gutter downspouts discharge a minimum of 10 feet from the structure to reduce the amount of water collecting around the foundations. Roof gutters and proper surface soil grading away from the structures is imperative in reducing the risks for foundation movement associated with expansive soils.

Landscaping: Existing and planted trees and large “tree-like” shrubbery with potential for developing large root systems should be located a minimum distance of half their final height away from the structure. The purpose of this is to reduce the potential for foundation or slab movements from the growth of root systems as the landscaping matures. Consideration should also be given to using landscaping that has a low water demand, so that excessive irrigation is not conducted around the structures.

6.0 SITE CONSTRUCTION RECOMMENDATIONS

6.1 SUBGRADE PREPARATION

Site preparation as outlined in this section should be performed to provide more uniform foundation bearing conditions, to reduce the potential for post-construction settlements of the planned structure and to maintain the integrity of a flexible pavement section.

6.1.1 Stripping and Grubbing

Prior to construction, the location of existing underground utilities within the construction area should be established. Provisions should then be made to relocate interfering utilities to appropriate locations. Underground pipes that are not properly removed or plugged may serve as conduits for subsurface erosion, which may subsequently lead to excessive settlement of overlying structures.

The "footprint" of the proposed building plus a minimum additional margin of 5 feet, and of the hardscape areas (parking/driveway) plus a minimum additional margin of 3 feet, should be stripped of all surface vegetation, stumps, debris, organic topsoil, or other deleterious materials. During grubbing operations, roots with a diameter greater than 0.5-inch, stumps, or small roots in a concentrated state, should be grubbed and completely removed.

It should be anticipated that 6 inches to 12 inches of topsoil and soils containing significant amounts of organic materials may be encountered across the site. The actual depths of unsuitable soils and materials should be determined by ECS using visual observation and judgment during earthwork operations. Any topsoil removed from the building and parking/drive areas can be stockpiled and used subsequently in non-structural areas.

6.1.2 Proofrolling

After removing all unsuitable surface materials, cutting to the proposed grade, and prior to the placement of any structural fill or other construction materials, the exposed subgrade should be examined by the Geotechnical Engineer or authorized representative. The exposed subgrade should be thoroughly proofrolled with previously approved construction equipment having a minimum axle load of 10 tons (e.g. fully loaded tandem-axle dump truck). The areas subject to proofrolling should be traversed by the equipment in two perpendicular (orthogonal) directions with overlapping passes of the vehicle under the observation of the Geotechnical Engineer or authorized representative. This procedure is intended to assist in identifying any localized yielding materials. In the event that unstable or “pumping” subgrade is identified by the proofrolling, those areas should be marked for repair prior to the placement of any subsequent structural fill or other construction materials. Methods of repair of unstable subgrade, such as undercutting or moisture conditioning or chemical stabilization, should be discussed with the Geotechnical Engineer to determine the appropriate procedure with regard to the existing conditions causing the instability. A test pit(s) may be excavated to explore the shallow subsurface materials in the area of the instability to help in determined the cause of the observed unstable materials and to assist in the evaluation of the appropriate remedial action to stabilize the subgrade.

6.1.3 Temporary Groundwater Control

Because of the need for densification of the soils within the upper 2 feet below the stripped surface, temporary groundwater control measures may be required if the groundwater level is within 2 feet below the stripped and grubbed surface at the time of construction. Should groundwater control measures become necessary, dewatering methods should be determined by the contractor. We recommend the groundwater control measures, if necessary; remain in place until compaction of the existing soils is completed. The dewatering method should be maintained until backfilling has reached a height of 2 feet above the groundwater level at the time of construction. The site should be graded to direct surface water runoff from the construction area.

Note that discharge of produced groundwater to surface waters of the state from dewatering operations or other site activities is regulated and requires a permit from the State of Florida Department of Environmental Protection (FDEP). This permit is termed a Generic Permit for the Discharge of Produced Groundwater From Any Non-Contaminated Site Activity. If discharge of produced groundwater is anticipated, we recommend sampling and testing of the groundwater early in the site design phase to prevent project delays during construction. ECS can provide the sampling, testing, and professional consulting required to evaluate compliance with the regulations.

6.1.4 Subgrade Compaction

Subgrade Compaction: After completing the clearing and stripping operations and installing the temporary groundwater control measures (if required), the exposed surface should be compacted with a vibratory drum roller having a minimum static, at-drum weight, on the order of 4 tons to 6 tons. Typically, the material should exhibit moisture contents within ±2 percentage points of the modified Proctor optimum moisture content (ASTM D 1557) during the compaction operations. Compaction should continue until densities of at least 95 percent of the modified Proctor maximum dry density (ASTM D 1557) have been achieved within the upper 1 foot of the compacted natural soils at the site.

Should the bearing level soils experience pumping and soil strength loss during the compaction operations, compaction work should be immediately terminated, and (1) the disturbed soils should be removed and backfilled with compacted structural fill, or (2) the excess moisture content within the disturbed soils should be allowed to dissipate before recompacting.

Care should be exercised to avoid damaging any nearby structures while the compaction operation is underway. Prior to commencing compaction, occupants of adjacent structures should be notified, and the existing conditions of the structures should be documented with photographs and survey (if deemed necessary). Compaction should cease if deemed detrimental to adjacent structures, and ECS should be contacted immediately. We recommend the vibratory roller remain a minimum of 50 feet from existing structures. Within this zone, use of a track-mounted bulldozer, or a vibratory roller operating in the static mode, is recommended.

6.1.5 Structural Backfill and Fill Soils

Structural backfill or fill required for site development should be placed in loose lifts not exceeding 12 inches in thickness when compacted by the use of the vibratory drum roller described in Section 6.1.4. The lift thickness should be reduced to 8 inches if the roller operates in the static mode or if

track-mounted compaction equipment is used. If hand-held compaction equipment is used, the lift thickness should be further reduced to 6 inches.

Structural fill is defined as a non-plastic, inorganic, granular soil having less than 10 percent material passing the No. 200 mesh sieve and containing less than 4 percent organic material. The fine sand and fine sand with silt or fine sand with clay, without roots, as encountered in the borings, are suitable as fill materials and, with proper moisture control, should densify using conventional compaction methods. Soils with more than 10 to 12 percent passing the No. 200 sieve will be more difficult to compact, due to their nature to retain soil moisture, and may require drying. Typically, the material should exhibit moisture contents within ±2 percent of the modified Proctor optimum moisture content (ASTM D 1557) during the compaction operations. Compaction should continue until densities of at least 95 percent of the modified Proctor maximum dry density (ASTM D 1557) have been achieved within each lift of the compacted structural fill.

Fill materials should not be placed on excessively wet soils. Excessively wet soils should be scarified, aerated, and moisture conditioned. Proper drainage should be maintained during the earthwork phases of construction to prevent ponding of water which has a tendency to degrade subgrade soils. The contractor should minimize dusting or implement dust control measures, as required.

We recommend that the grading contractor have equipment on site during earthwork for both drying and wetting fill soils. Moisture control may be difficult during extended periods of rain. The control of moisture content of soils containing more than 10% fines may be difficult when these soils become wet. Further, such soils are easily degraded by construction traffic when the moisture content is elevated.

6.1.6 Foundation Areas

After satisfactory placement and compaction of the required structural fill, the foundation areas may be excavated to the planned bearing levels. The foundation bearing level soils, after compaction, should exhibit densities equivalent to 95 percent of the Modified Proctor maximum dry density (ASTM D 1557) to a depth of one foot below the bearing level. For confined areas, such as the footing excavations, any compactive effort should be provided by a lightweight vibratory sled or roller having a total weight on the order of 500 to 2,000 pounds.

6.1.7 Pavement Areas

After completing the clearing/stripping operations in the pavement areas, any underlying clayey sands and sandy clays that are within 2 feet of the bottom of the pavement base should be over-excavated from within the pavement areas. Structural backfill and fill required to achieve the finish pavement grades then can be placed and compacted as described Sections 6.1.4 and 6.1.5 above. As an exception, densities of at least 98 percent of the modified Proctor maximum dry density (ASTM D1557) should be obtained within the upper one foot of the materials immediately below the proposed base course.

6.2 UTILITY INSTALLATIONS

Utility Subgrades: The silty fine sand (SM) soils may be suitable if they can be compacted to the required density (typically 95 percent of the soil's Modified Proctor (AASHTO T-180) maximum dry density). If the SM soils cannot be compacted to the required density, and if clayey or clay soils (SC or CH) are encountered, then we recommend these soils be overexcavated a minimum of 12 inches

below the pipe invert elevation (24 inches below the manhole base elevation) and replaced with compacted structural fill as described in Section 6.1.5 above.

Alternatively, a medium-duty woven geotextile, such as Mirafi 600X or equivalent, may be used as a separation barrier between the compacted backfill and the silty/clayey soils. If a woven geotextile is used, then no overexcavation is necessary for the pipeline, and the depth of overexcavation for the utility manholes may be reduced to 12 inches. The geotextile should be placed in the excavation bottom and along the sides above the silty/clayey soils creating a barrier between these soils and the sand backfill to preclude contamination of the backfill.

Utility Backfilling: Backfill placed around the pipe, and to a height of 2 feet above the top of pipe, should be placed in 6-inch lifts. Each lift should be compacted with hand-held equipment to 95 percent of the soil's Modified Proctor (ASTM D 1557) maximum dry density. Backfill placed above the 2-foot zone above the top of pipe elevation may be placed in 12-inch lifts and compacted with heavier equipment. Typically, the backfill soil should exhibit moisture contents within ±2 percent of the soil's optimum moisture content as determined from the Proctor test. Care should be taken to avoid damaging the pipe during compaction operations.

Backfill placed around utility structures should be placed in 6-inch thick lifts, and compacted with hand-held equipment to the same in-place soil density stated above. Heavy equipment should not be used within 5 feet of the structures to prevent overstressing of the structure walls.

Utility Excavation Dewatering: Based on the groundwater depths encountered in our borings, groundwater will likely be encountered by utility excavations which extend below existing grades. It is expected that removal of groundwater will be required, especially for deeper utility excavations. The contractor should submit a dewatering plan prior to installing the site utilities.

6.3 GENERAL CONSTRUCTION CONSIDERATIONS

Moisture Conditioning: We anticipate that typical moisture conditioning for soils in this area should be anticipated. The sandy surface soils may require wetting during dry periods or periods of high heat. Drying of soils containing more than 10% fines or excavated from below the water table may be required to be within ±2 percentage points of the modified Proctor optimum moisture content (ASTM D 1557).

Subgrade Protection: Measures should also be taken to limit site disturbance, especially from rubber-tired heavy construction equipment, and to control and remove surface water from development areas, including structural and pavement areas. It would be advisable to designate a haul road and construction staging area to limit the areas of disturbance and to prevent construction traffic from excessively degrading sensitive subgrade soils and existing pavement areas. Haul roads and construction staging areas could be covered with excess depths of aggregate to protect those subgrades. The aggregate can later be removed and used in pavement areas.

Surface Drainage: Surface drainage conditions should be properly maintained. Surface water should be directed away from the construction area, and the work area should be sloped away from the construction area at a gradient of 1 percent or greater to reduce the potential of ponding water and the subsequent saturation of the surface soils. At the end of each work day, the subgrade soils should be sealed by rolling the surface with a smooth drum roller to minimize infiltration of surface water.

Excavation Safety: All excavations and slopes should be made and maintained in accordance with OSHA excavation safety standards. The contractor is solely responsible for designing and constructing stable, temporary excavations and slopes and should shore, slope, or bench the sides of the excavations and slopes as required to maintain stability of both the excavation sides and bottom. The contractor's responsible person, as defined in 29 CFR Part 1926, should evaluate the soil exposed in the excavations as part of the contractor's safety procedures. In no case should slope height, slope inclination, or excavation depth, including utility trench excavation depth, exceed those specified in local, state, and federal safety regulations. ECS is providing this information solely as a service to our client. ECS is not assuming responsibility for construction site safety or the contractor's activities; such responsibility is not being implied and should not be inferred.

Erosion Control: The surface soils may be erodible. Therefore, the Contractor should provide and maintain good site drainage during earthwork operations to maintain the integrity of the surface soils. All erosion and sedimentation controls should be in accordance with sound engineering practices and local requirements.

7.0 QUALITY CONTROL TESTING

ECS should be retained to perform the construction material testing and observations required for this project, to verify that our recommendations have been satisfied. We are the most qualified to address problems that may arise during construction, since we are familiar with the intent of our engineering design.

A representative number of field in-place density tests should be made in the upper 2 feet of compacted natural soils, in each lift of compacted backfill and fill, and in the upper 12 inches below the bearing levels in the footing excavations. Density tests are recommended to verify that satisfactory compaction operations have been performed. We recommend density testing be performed at frequencies presented in the table below.

Table 7.0.1 Frequency of Compaction Testing

Location	Frequency of Tests
Expanded Building Limits	1 test per 2,500 sq. ft.
Continuous Wall Footing	1 test per 50 linear ft. of footing
Isolated Column Footings	1 test at 50% of footings
Pavement Areas	1 test per 10,000 sq. ft.

8.0 CLOSING

Our geotechnical exploration has been performed, our findings obtained, and our recommendations prepared, in accordance with generally accepted geotechnical engineering principles and practices. ECS is not responsible for any independent conclusions, interpretation, opinions, or recommendations made by others based on the data contained in this report.

Our scope of services was intended to evaluate the soil conditions within the zone of soil influenced by the foundation system. Our scope of services does not address geologic conditions, such as sinkholes or soil conditions existing below the depth of the soil borings.

This report does not reflect any variations that may occur adjacent to or between soil borings. The discovery of any site or subsurface condition during construction that deviates from the data obtained during this geotechnical exploration should be reported to us for our evaluation.

If any of the project description information discussed in this report is inaccurate, either due to our interpretation of the documents provided or site or design changes that may occur later, ECS should be contacted immediately in order that we can review the report in light of the changes and provide additional or alternate recommendations as may be required to reflect the proposed construction.

We recommend that ECS be allowed to review the project's plans and specifications pertaining to our work so that we may ascertain consistency of those plans/specifications with the intent of the geotechnical report.

Field observations, monitoring, and quality assurance testing during earthwork and foundation installation are an extension of and integral to the geotechnical design recommendation. We recommend that the owner retain these quality assurance services and that ECS be allowed to continue our involvement throughout these critical phases of construction to provide general consultation as issues arise. ECS is not responsible for the conclusions, opinions, or recommendations of others based on the data in this report.

FIGURES

Figure 1 Site Location Plan
Figure 2 Boring Location Diagram
Generalized Subsurface Soil Profile Section A-A’
Generalized Subsurface Soil Profile Section B-B’

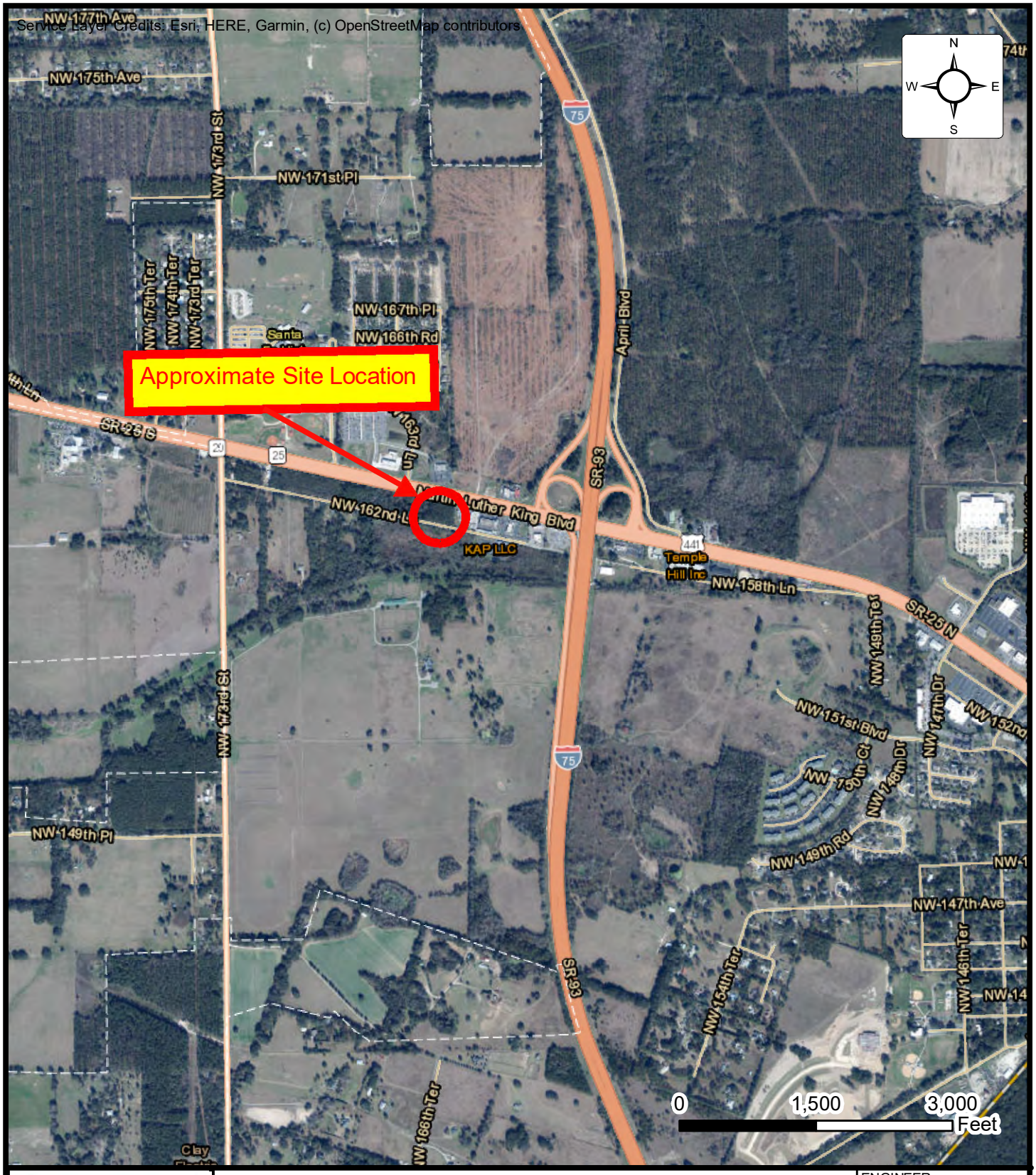


Figure 1 - Site Location Plan
QSR ALACHUA

TAX PARCEL 03053-002-001, ALACHUA, FLORIDA

DE FOODS, LLC

ENGINEER	CAD4
SCALE	AS NOTED
PROJECT NO.	35:31000
SHEET	1 OF 1
DATE	12/2/2020

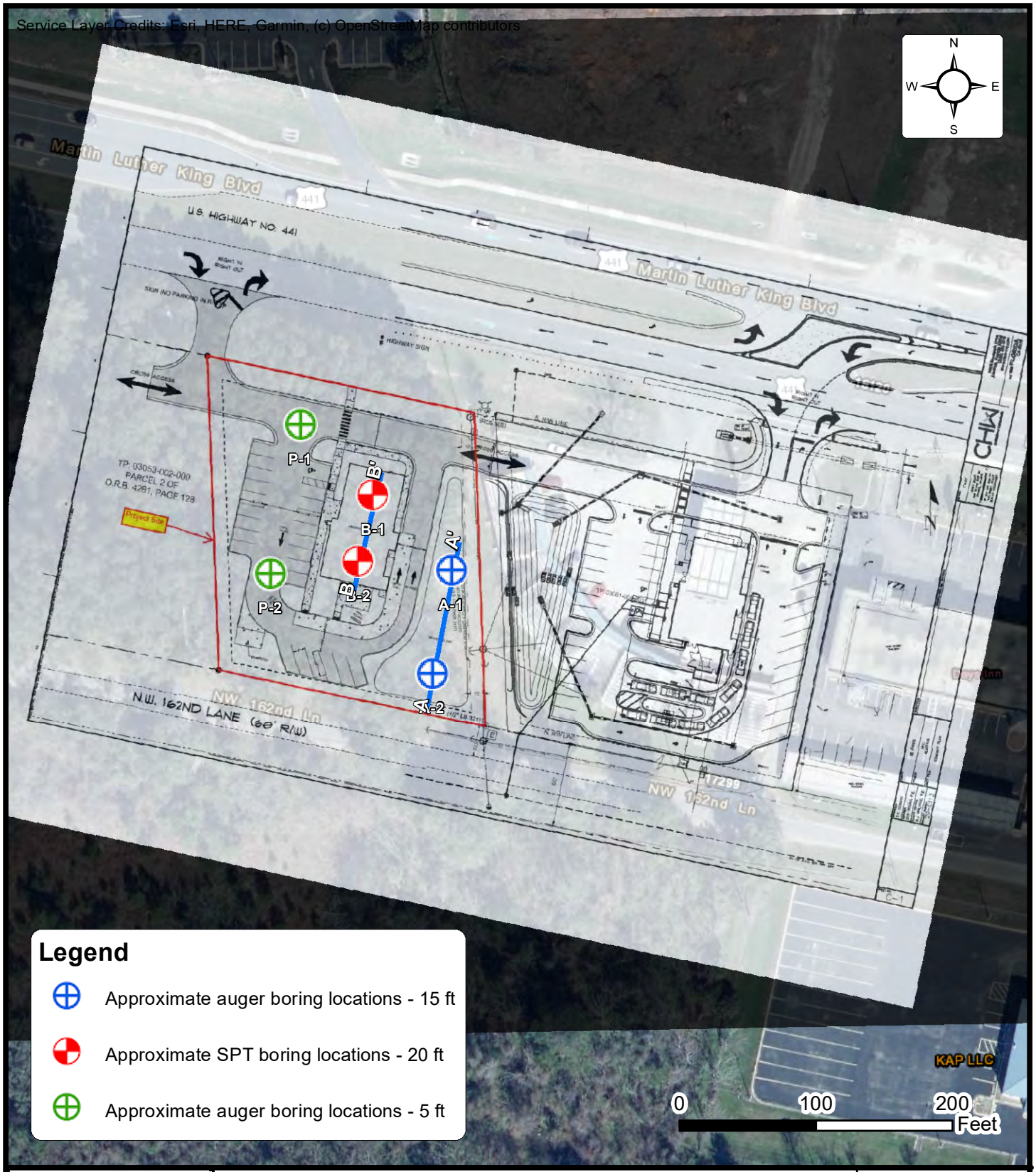
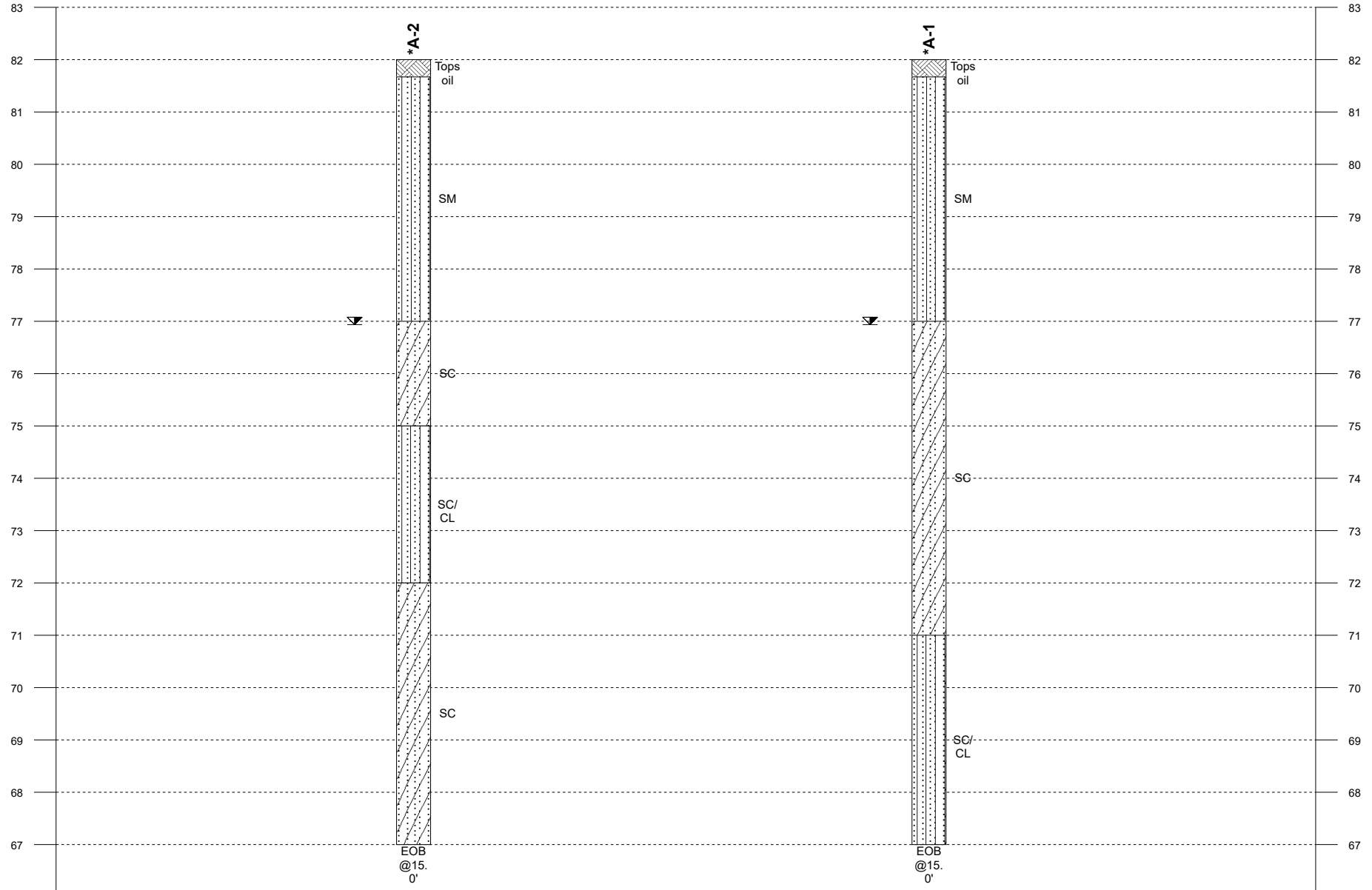


Figure 2 - Boring Location Diagram
QSR ALACHUA

TAX PARCEL 03053-002-001, ALACHUA, FLORIDA

DE FOODS, LLC

ENGINEER	CAD4
SCALE	AS NOTED
PROJECT NO.	35:31000
SHEET	1 OF 1
DATE	12/2/2020



Legend Key

- Topsoil
- SILTY SAND
- CLAYEY SAND
- Clayey sand/L…

66.00

0.0 0

26. 75

103 .53

145 .23

AR: AUGER REFUSAL

Plastic Limit Water Content
Liquid Limit
X
[Fines Content %]

WL (First Encountered)		Fill	
WL (Completion)		Possible Fill	
WL (Seasonal High Water)		Probable Fill	
WL (Stabilized)		Rock	

Notes:

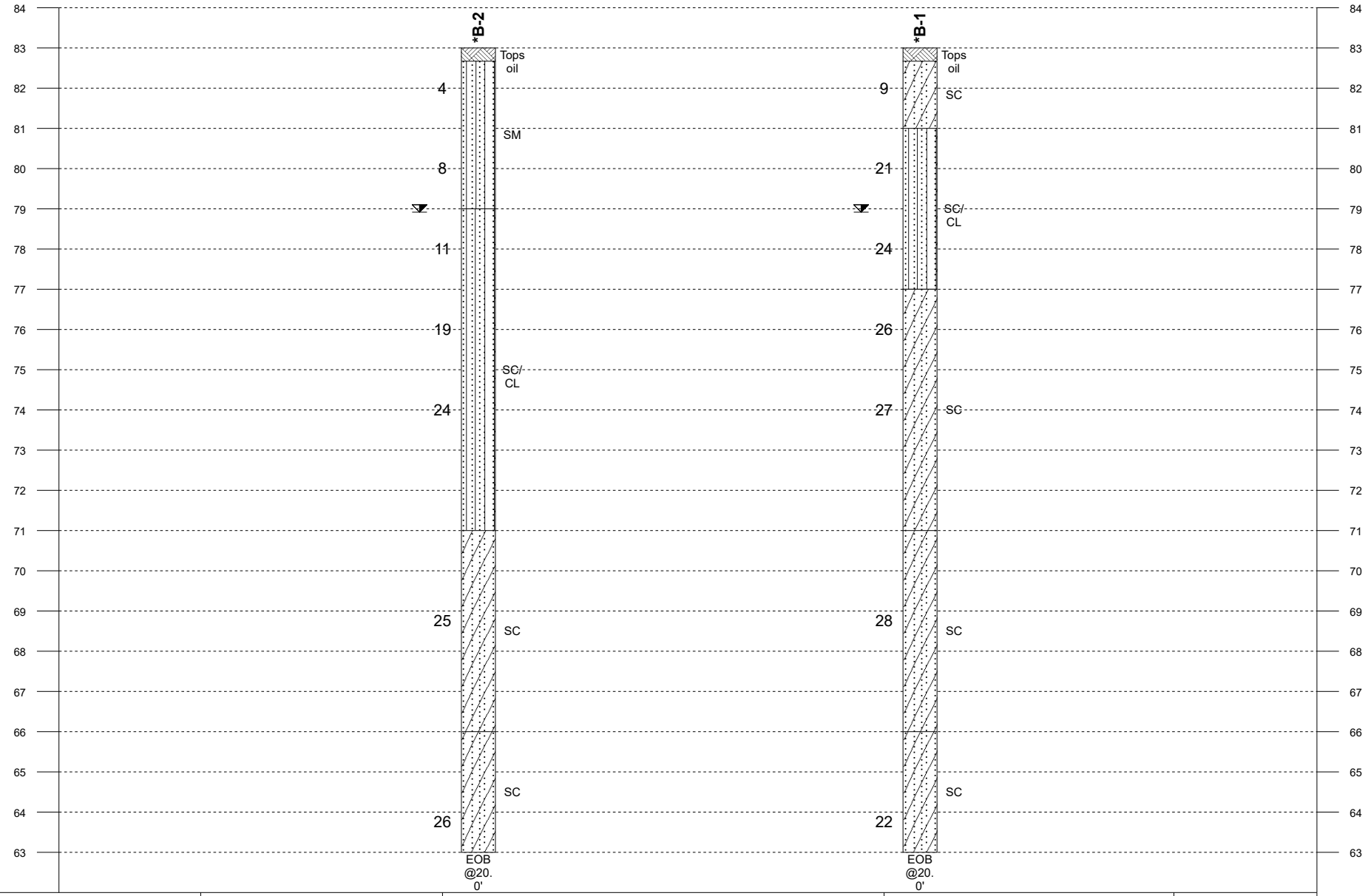
1- EOB: END OF BORING SPT: SAMPLER REFUSAL.
2- THE NUMBER BELOW THE STRIPS IS THE DISTANCE ALONG THE BASELINE.
3- SEE INDIVIDUAL BORING LOG AND GEOTECHNICAL INFORMATION.
4- STANDARD PENETRATION TEST RESISTANCE (LEFT OF BORING) IN BLOWS PER FOOT (ASTM D1586).

GENERALIZED SUBSURFACE SOIL PROFILE

Section Cut A-A'

QSR Alachua
DE Foods, LLC
Tax Parcel 03053-002-001, Alachua, Florida 32615

Project No: 35:31000 | Date: 12/02/2020



Legend Key

- Topsoil
- CLAYEY SAND
- SILTY SAND
- Clayey sand/L…

62.00

0.0 0

27. 16

76. 75

109 .30

AR: AUGER REFUSAL

Notes:

1- EOB: END OF BORING SR: SAMPLER REFUSAL.
2- THE NUMBER BELOW THE STRIPS IS THE DISTANCE ALONG THE BASELINE.
3- SEE INDIVIDUAL BORING LOG AND GEOTECHNICAL INFORMATION.
4- STANDARD PENETRATION TEST RESISTANCE (LEFT OF BORING) IN BLOWS PER FOOT (ASTM D1586).

Plastic Limit Water Content
Liquid Limit
X
[Fines Content %]

▽	WL (First Encountered)		Fill
▼	WL (Completion)		Possible Fill
▽	WL (Seasonal High Water)		Probable Fill
▽	WL (Stabilized)		Rock

GENERALIZED SUBSURFACE SOIL PROFILE

Section Cut B-B'

QSR Alachua
DE Foods, LLC
Tax Parcel 03053-002-001, Alachua, Florida 32615

Project No: 35:31000 | Date: 12/02/2020

APPENDIX A – Field Operations

Soil Boring Logs
Field Exploration Procedures
Key to Soil Classification

CLIENT:	PROJECT NO.:	BORING NO.:	SHEET:
DE Foods, LLC	**35:31000**	**A-1**	**1 of 1**
PROJECT NAME:	DRILLER/CONTRACTOR:		
QSR Alachua	**Standard Drilling Services LLC**		
SITE LOCATION:			LOSS OF CIRCULATION
Tax Parcel 03053-002-001, Alachua, Florida 32615			
NORTHING:	EASTING:	STATION:	SURFACE ELEVATION:
1991115.4	**174509.3**		**82.0**

DEPTH (FT)	SAMPLE NUMBER	SAMPLE TYPE	SAMPLE DIST. (IN)	RECOVERY (IN)	DESCRIPTION OF MATERIAL	WATER LEVELS	ELEVATION (FT)	BLOWS/6"
					Topsoil Thickness[4.00"]			
	S-1	AU	6		(SM) SILTY SAND, brown, moist			
	S-2	AU	6					
5					(SC) CLAYEY SAND, brown and orange, moist	▼	77	
	S-3	AU	6					
10							72	
	S-4	AU	6		(SC/CL) CLAYEY SAND/SANDY LEAN CLAY, brown, moist			
15					**END OF DRILLING AT 15.0 FT**		67	
20							62	
25							57	
30							52	

Plastic Limit Water Content Liquid Limit

⊗ STANDARD PENETRATION BLOWS/FT

ROCK QUALITY DESIGNATION & RECOVERY

RQD

REC

○ CALIBRATED PENETROMETER TON/SF

[FINES CONTENT %]

THE STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY LINES BETWEEN SOIL TYPES. IN-SITU THE TRANSITION MAY BE GRADUAL

▽ WL (First Encountered)		BORING STARTED:	**Nov 19 2020**	CAVE IN DEPTH:	
▼ WL (Completion)	**Dry**	BORING COMPLETED:	**Nov 19 2020**	HAMMER TYPE:	**Manual**
▼ WL (Seasonal High Water)	**5.00**	EQUIPMENT:	LOGGED BY:	DRILLING METHOD:	
▽ WL (Stabilized)		**Truck**	**KAP1**		**Mud rotary**

GEOTECHNICAL BOREHOLE LOG

CLIENT:	PROJECT NO.:	BORING NO.:	SHEET:
DE Foods, LLC	**35:31000**	**A-2**	**1 of 1**
PROJECT NAME:	DRILLER/CONTRACTOR:		
QSR Alachua	**Standard Drilling Services LLC**		
SITE LOCATION:			LOSS OF CIRCULATION
Tax Parcel 03053-002-001, Alachua, Florida 32615			
NORTHING:	EASTING:	STATION:	SURFACE ELEVATION:
1991040.3	**174493.0**		**82.0**

DEPTH (FT)	SAMPLE NUMBER	SAMPLE TYPE	SAMPLE DIST. (IN)	RECOVERY (IN)	DESCRIPTION OF MATERIAL	WATER LEVELS	ELEVATION (FT)	BLOWS/6"
					Topsoil Thickness[4.00"]			
	S-1	AU	6		(SM) SILTY SAND, brown, moist			
	S-2	AU	6					
5					(SC) CLAYEY SAND, brown, moist	▼	77	
	S-3	AU	6					
	S-4	AU	6		(SC/CL) CLAYEY SAND/SANDY LEAN CLAY, brown and gray, moist			
10					(SC) CLAYEY SAND, brown and light gray, moist		72	
	S-5	AU	6					
15					**END OF DRILLING AT 15.0 FT**		67	
20							62	
25							57	
30							52	

Plastic Limit Water Content Liquid Limit

⊗ STANDARD PENETRATION BLOWS/FT

ROCK QUALITY DESIGNATION & RECOVERY

RQD

REC

○ CALIBRATED PENETROMETER TON/SF

[FINES CONTENT %]

THE STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY LINES BETWEEN SOIL TYPES. IN-SITU THE TRANSITION MAY BE GRADUAL

▽ WL (First Encountered)		BORING STARTED: **Nov 19 2020**	CAVE IN DEPTH:
▼ WL (Completion)	**Dry**	BORING COMPLETED: **Nov 19 2020**	HAMMER TYPE:
▼ WL (Seasonal High Water)	**5.00**	EQUIPMENT: **Truck** / LOGGED BY: **KAP1**	DRILLING METHOD: **Mud rotary**
▽ WL (Stabilized)			

GEOTECHNICAL BOREHOLE LOG

CLIENT:	PROJECT NO.:	BORING NO.:	SHEET:
DE Foods, LLC	**35:31000**	**B-1**	**1 of 1**
PROJECT NAME:	DRILLER/CONTRACTOR:		
QSR Alachua	**Standard Drilling Services LLC**		
SITE LOCATION:			LOSS OF CIRCULATION
Tax Parcel 03053-002-001, Alachua, Florida 32615			
NORTHING:	EASTING:	STATION:	SURFACE ELEVATION:
1991172.0	**174454.0**		**83.0**

DEPTH (FT)	SAMPLE NUMBER	SAMPLE TYPE	SAMPLE DIST. (IN)	RECOVERY (IN)	DESCRIPTION OF MATERIAL	WATER LEVELS	ELEVATION (FT)	BLOWS/6"
0	S-1	SS	24	24	Topsoil Thickness[4.00"] (SC) CLAYEY SAND, brown, moist, loose, with trace limestone		83	2-3-6-7 (9)
	S-2	SS	24	24	(SC/CL) CLAYEY SAND/SANDY LEAN CLAY, gray and brown, moist, medium dense, with trace limestone	▼		7-11-10-12 (21)
5	S-3	SS	24	24			78	11-12-12-13 (24)
	S-4	SS	24	24	(SC) CLAYEY SAND, tan, moist, medium dense			12-13-13-14 (26)
	S-5	SS	24	24				9-13-14-16 (27)
10							73	
					(SC) CLAYEY SAND, light gray to tan, wet, medium dense			
	S-6	SS	18	18				7-14-14 (28)
15							68	
					(SC) CLAYEY SAND, light tan, moist, medium dense			
	S-7	SS	18	18				7-10-12 (22)
20					**END OF DRILLING AT 20.0 FT**		63	
25							58	
30							53	

Plastic Limit Water Content Liquid Limit

STANDARD PENETRATION BLOWS/FT

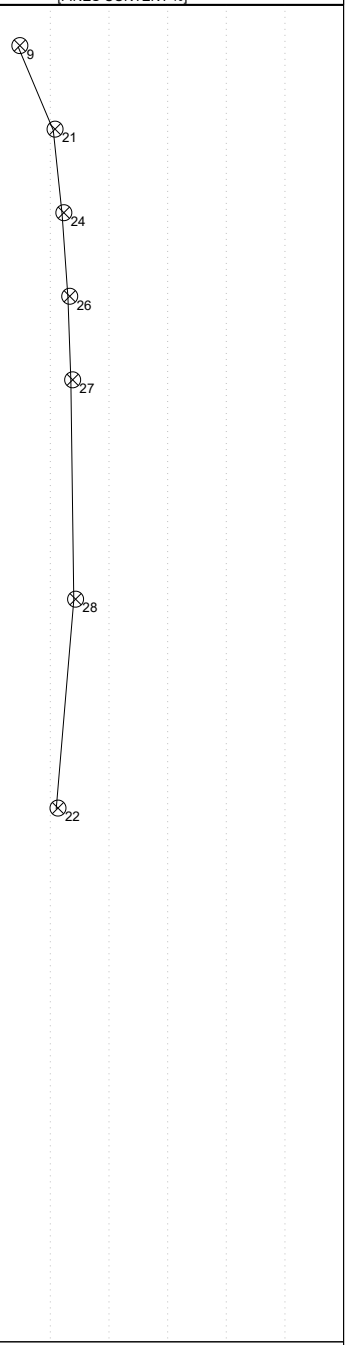
ROCK QUALITY DESIGNATION & RECOVERY

RQD

REC

CALIBRATED PENETROMETER TON/SF

[FINES CONTENT %]



THE STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY LINES BETWEEN SOIL TYPES. IN-SITU THE TRANSITION MAY BE GRADUAL

▽ WL (First Encountered)		BORING STARTED: **Nov 19 2020**		CAVE IN DEPTH:
▼ WL (Completion)	**Dry**	BORING COMPLETED: **Nov 19 2020**		HAMMER TYPE: **Manual**
▼ WL (Seasonal High Water)	**4.00**	EQUIPMENT: **Truck**	LOGGED BY: **KAP1**	DRILLING METHOD: **Mud rotary**
▽ WL (Stabilized)				

GEOTECHNICAL BOREHOLE LOG

CLIENT:	PROJECT NO.:	BORING NO.:	SHEET:
DE Foods, LLC	**35:31000**	**B-2**	**1 of 1**
PROJECT NAME:	DRILLER/CONTRACTOR:		
QSR Alachua	**Standard Drilling Services LLC**		
SITE LOCATION:			LOSS OF CIRCULATION
Tax Parcel 03053-002-001, Alachua, Florida 32615			
NORTHING:	EASTING:	STATION:	SURFACE ELEVATION:
1991123.9	**174441.5**		**83.0**

DEPTH (FT)	SAMPLE NUMBER	SAMPLE TYPE	SAMPLE DIST. (IN)	RECOVERY (IN)	DESCRIPTION OF MATERIAL	WATER LEVELS	ELEVATION (FT)	BLOWS/6"
0	S-1	SS	24	24	Topsoil Thickness[4.00"] (SM) SILTY SAND, brown, moist, loose			2-2-2-2 (4)
	S-2	SS	24	24				3-4-4-4 (8)
5	S-3	SS	24	24	(SC/CL) CLAYEY SAND,/SANDY LEAN CLAY brown, moist, medium dense		78	4-5-6-8 (11)
	S-4	SS	24	24				7-7-12-14 (19)
	S-5	SS	24	24				7-12-12-15 (24)
10							73	
					(SC) CLAYEY SAND, gray, wet, medium dense			
	S-6	SS	18	18				6-12-13 (25)
15							68	
					(SC) CLAYEY SAND, light tan, moist, medium dense			
	S-7	SS	18	18				7-12-14 (26)
20					**END OF DRILLING AT 20.0 FT**		63	
25							58	
30							53	

Plastic Limit Water Content Liquid Limit

⊗ STANDARD PENETRATION BLOWS/FT

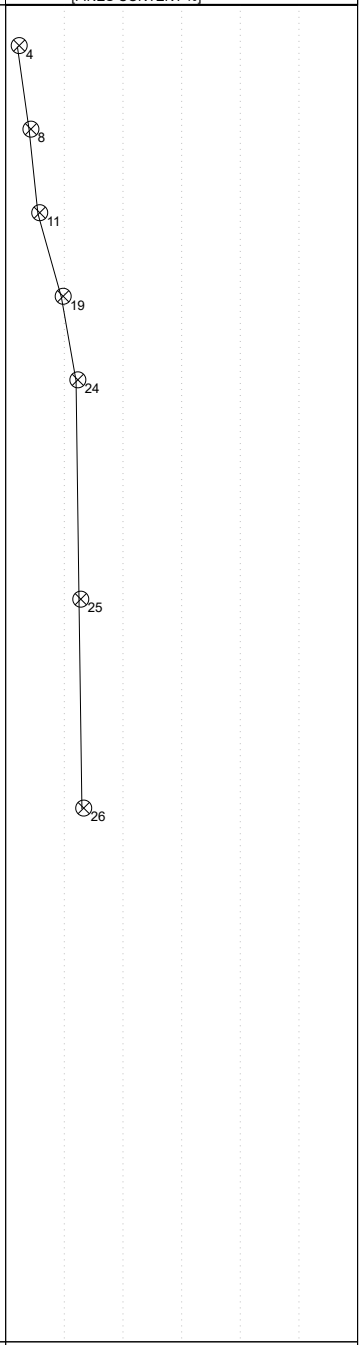
ROCK QUALITY DESIGNATION & RECOVERY

RQD

REC

○ CALIBRATED PENETROMETER TON/SF

[FINES CONTENT %]



THE STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY LINES BETWEEN SOIL TYPES. IN-SITU THE TRANSITION MAY BE GRADUAL

▽ WL (First Encountered)		BORING STARTED:	**Nov 19 2020**	CAVE IN DEPTH:	
▼ WL (Completion)	**Dry**	BORING COMPLETED:	**Nov 19 2020**	HAMMER TYPE:	**Manual**
▼ WL (Seasonal High Water)	**4.00**	EQUIPMENT:	LOGGED BY:	DRILLING METHOD:	
▽ WL (Stabilized)		**Truck**	**KAP1**		**Mud rotary**

GEOTECHNICAL BOREHOLE LOG

CLIENT:	PROJECT NO.:	BORING NO.:	SHEET:
DE Foods, LLC	**35:31000**	**P-1**	**1 of 1**
PROJECT NAME:	DRILLER/CONTRACTOR:		
QSR Alachua	**Standard Drilling Services LLC**		
SITE LOCATION:			LOSS OF CIRCULATION
Tax Parcel 03053-002-001, Alachua, Florida 32615			
NORTHING:	EASTING:	STATION:	SURFACE ELEVATION:
1991225.2	**174401.9**		**85.0**

DEPTH (FT)	SAMPLE NUMBER	SAMPLE TYPE	SAMPLE DIST. (IN)	RECOVERY (IN)	DESCRIPTION OF MATERIAL	WATER LEVELS	ELEVATION (FT)	BLOWS/6"
					Topsoil Thickness[4.00"]			
	S-1	AU	6		(SM) SILTY SAND, dark brown, moist			
	S-2	AU	6					
	S-3	AU	6		(SC) CLAYEY SAND, gray, moist			
5					**END OF DRILLING AT 5.0 FT**		80	
10							75	
15							70	
20							65	
25							60	
30							55	

Plastic Limit Water Content Liquid Limit
X——●——△

⊗ STANDARD PENETRATION BLOWS/FT

ROCK QUALITY DESIGNATION & RECOVERY

RQD

REC

○ CALIBRATED PENETROMETER TON/SF

[FINES CONTENT %]

THE STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY LINES BETWEEN SOIL TYPES. IN-SITU THE TRANSITION MAY BE GRADUAL

▽ WL (First Encountered)		BORING STARTED: **Nov 19 2020**	CAVE IN DEPTH:
▼ WL (Completion)	**Dry**	BORING COMPLETED: **Nov 19 2020**	HAMMER TYPE:
▼ WL (Seasonal High Water)	**4.00**	EQUIPMENT: **Truck** / LOGGED BY: **KAP1**	DRILLING METHOD: **Mud rotary**
▽ WL (Stabilized)			

GEOTECHNICAL BOREHOLE LOG

CLIENT:	PROJECT NO.:	BORING NO.:	SHEET:
DE Foods, LLC	**35:31000**	**P-2**	**1 of 1**
PROJECT NAME:	DRILLER/CONTRACTOR:		
QSR Alachua	**Standard Drilling Services LLC**		
SITE LOCATION:			LOSS OF CIRCULATION
Tax Parcel 03053-002-001, Alachua, Florida 32615			
NORTHING:	EASTING:	STATION:	SURFACE ELEVATION:
1991117.2	**174376.6**		**85.0**

BOTTOM OF CASING

Plastic Limit Water Content Liquid Limit

STANDARD PENETRATION BLOWS/FT

ROCK QUALITY DESIGNATION & RECOVERY

RQD

REC

CALIBRATED PENETROMETER TON/SF

[FINES CONTENT %]

DEPTH (FT)	SAMPLE NUMBER	SAMPLE TYPE	SAMPLE DIST. (IN)	RECOVERY (IN)	DESCRIPTION OF MATERIAL	WATER LEVELS	ELEVATION (FT)	BLOWS/6"
0					Topsoil Thickness[4.00"]		85	
	S-1	AU	6		(SM) SILTY SAND, brown, moist			
5					**END OF DRILLING AT 5.0 FT**		80	
10							75	
15							70	
20							65	
25							60	
30							55	

THE STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY LINES BETWEEN SOIL TYPES. IN-SITU THE TRANSITION MAY BE GRADUAL

WL (First Encountered)		BORING STARTED: **Nov 19 2020**	CAVE IN DEPTH:	
WL (Completion)	**Dry**	BORING COMPLETED: **Nov 19 2020**	HAMMER TYPE:	
WL (Seasonal High Water)	**5.00**	EQUIPMENT: **Truck**	LOGGED BY: **KAP1**	DRILLING METHOD: **Mud rotary**
WL (Stabilized)				

GEOTECHNICAL BOREHOLE LOG

FIELD EXPLORATION PROCEDURES

Standard Penetration Test (SPT) Borings

The Standard Penetration Test (SPT) borings were made in general accordance with the latest revision of ASTM D 1586, "Penetration Test and Split-Barrel Sampling of Soils". The borings were advanced by rotary (or "wash-n-chop") drilling techniques. At 2 ½ to 5 foot intervals, a split-barrel sampler inserted to the borehole bottom and driven 18 inches into the soil using a 140 pound hammer falling on the average 30 inches per hammer blow. The number of hammer blows for the final 12 inches of penetration is termed the "penetration resistance, blow count, or N-value". This value is an index to several in-place geotechnical properties of the material tested, such as relative density and Young's Modulus.

After driving the sampler 18 inches (or less if in hard rock-like material), the sampler was retrieved from the borehole and representative samples of the material within the split-barrel were containerized and sealed. After completing the drilling operations, the samples for each boring were transported to our laboratory where they were examined by our engineer in order to verify the driller's field classification. The retrieved samples will be kept in our facility for a period of sixty (60) calendar days unless directed otherwise.

Flight Auger Boring

The auger borings were performed mechanically by the use of a continuous-flight auger attached to the drill rig and in general accordance with the latest revision of ASTM D 1452, "Soil Investigation and Sampling by Auger Borings". Representative samples of the soils brought to the ground surface by the augering process were containerized and sealed, and transported to our laboratory where they were examined by our engineer to verify the driller's field classification.

REFERENCE NOTES FOR BORING LOGS

MATERIAL[1,2]	
	ASPHALT
	CONCRETE
	GRAVEL
	TOPSOIL
	VOID
	BRICK
	AGGREGATE BASE COURSE
	FILL[3] MAN-PLACED SOILS
GW	WELL-GRADED GRAVEL gravel-sand mixtures, little or no fines
GP	POORLY-GRADED GRAVEL gravel-sand mixtures, little or no fines
GM	SILTY GRAVEL gravel-sand-silt mixtures
GC	CLAYEY GRAVEL gravel-sand-clay mixtures
SW	WELL-GRADED SAND gravelly sand, little or no fines
SP	POORLY-GRADED SAND gravelly sand, little or no fines
SM	SM SILTY SAND sand-silt mixtures
SC	CLAYEY SAND sand-clay mixtures
ML	SILT non-plastic to medium plasticity
MH	ELASTIC SILT high plasticity
CL	LEAN CLAY low to medium plasticity
CH	FAT CLAY high plasticity
OL	ORGANIC SILT or CLAY non-plastic to low plasticity
OH	ORGANIC SILT or CLAY high plasticity
PT	PEAT highly organic soils

DRILLING SAMPLING SYMBOLS & ABBREVIATIONS			
SS	Split Spoon Sampler	PM	Pressuremeter Test
ST	Shelby Tube Sampler	RD	Rock Bit Drilling
WS	Wash Sample	RC	Rock Core, NX, BX, AX
BS	Bulk Sample of Cuttings	REC	Rock Sample Recovery %
PA	Power Auger (no sample)	RQD	Rock Quality Designation %
HSA	Hollow Stem Auger		

PARTICLE SIZE IDENTIFICATION		
DESIGNATION		PARTICLE SIZES
Boulders		12 inches (300 mm) or larger
Cobbles		3 inches to 12 inches (75 mm to 300 mm)
Gravel:	Coarse	¾ inch to 3 inches (19 mm to 75 mm)
	Fine	4.75 mm to 19 mm (No. 4 sieve to ¾ inch)
Sand:	Coarse	2.00 mm to 4.75 mm (No. 10 to No. 4 sieve)
	Medium	0.425 mm to 2.00 mm (No. 40 to No. 10 sieve)
	Fine	0.074 mm to 0.425 mm (No. 200 to No. 40 sieve)
Silt & Clay (“Fines”)		<0.074 mm (smaller than a No. 200 sieve)

COHESIVE SILTS & CLAYS		
UNCONFINED COMPRESSIVE STRENGTH, Q_P[4]	SPT[5] (BPF)	CONSISTENCY[7] (COHESIVE)
<0.25	<3	Very Soft
0.25 - <0.50	3 - 4	Soft
0.50 - <1.00	5 - 8	Firm
1.00 - <2.00	9 - 15	Stiff
2.00 - <4.00	16 - 30	Very Stiff
4.00 - 8.00	31 - 50	Hard
>8.00	>50	Very Hard

GRAVELS, SANDS & NON-COHESIVE SILTS	
SPT[5]	DENSITY
<5	Very Loose
5 - 10	Loose
11 - 30	Medium Dense
31 - 50	Dense
>50	Very Dense

RELATIVE AMOUNT[7]	COARSE GRAINED (%)[8]	FINE GRAINED (%)[8]
Trace	≤5	≤5
Dual Symbol (ex: SW-SM)	10	10
With	15 - 20	15 - 25
Adjective (ex: “Silty”)	≥25	≥30

WATER LEVELS[6]		
	WL	Water Level (WS)(WD) (WS) While Sampling (WD) While Drilling
	SHW	Seasonal High WT
	ACR	After Casing Removal
	SWT	Stabilized Water Table
	DCI	Dry Cave-In
	WCI	Wet Cave-In

[1]Classifications and symbols per ASTM D 2488-09 (Visual-Manual Procedure) unless noted otherwise.

[2]To be consistent with general practice, “POORLY GRADED” has been removed from GP, GP-GM, GP-GC, SP, SP-SM, SP-SC soil types on the boring logs.

[3]Non-ASTM designations are included in soil descriptions and symbols along with ASTM symbol [Ex: (SM-FILL)].

[4]Typically estimated via pocket penetrometer or Torvane shear test and expressed in tons per square foot (tsf).

[5]Standard Penetration Test (SPT) refers to the number of hammer blows (blow count) of a 140 lb. hammer falling 30 inches on a 2 inch OD split spoon sampler required to drive the sampler 12 inches (ASTM D 1586). “N-value” is another term for “blow count” and is expressed in blows per foot (bpf). SPT correlations per 7.4.2 Method B and need to be corrected if using an auto hammer.

[6]The water levels are those levels actually measured in the borehole at the times indicated by the symbol. The measurements are relatively reliable when augering, without adding fluids, in granular soils. In clay and cohesive silts, the determination of water levels may require several days for the water level to stabilize. In such cases, additional methods of measurement are generally employed.

[7]Minor deviation from ASTM D 2488-09 Note 16.

[8]Percentages are estimated to the nearest 5% per ASTM D 2488-09.

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APPENDIX B – Laboratory Testing

Laboratory Data

Laboratory Testing Summary

Sample Source	Sample Number	Start Depth (feet)	End Depth (feet)	Sample Distance (feet)	MC[1] (%)	Soil Type[2]	Atterberg Limits[3]			Percent Passing No. 200 Sieve[4]	Maximum Dry Density (pcf)	Limerock Bearing Ratio	Constant Head Permeability (ft/day)[5]
							LL	PL	PI				
B1	S-3	4.0	6.0	2.0	18.5	SC/CL			NP	40.9			
B2	S-5	8.0	10.0	2.0	26.4	SC/CL				46.7			
P1		1.0	2.0	1.0	11.6	SM				18.2	123.0	70	
P2	S-1	2.0	2.5	0.5	12.3	SM				30.4			
A1		0.0	2.0	2.0		SM							k_h = 18 k_v = 12
A1	S-4	12.0	12.5	0.5	14.8	SC/CL				34.4			

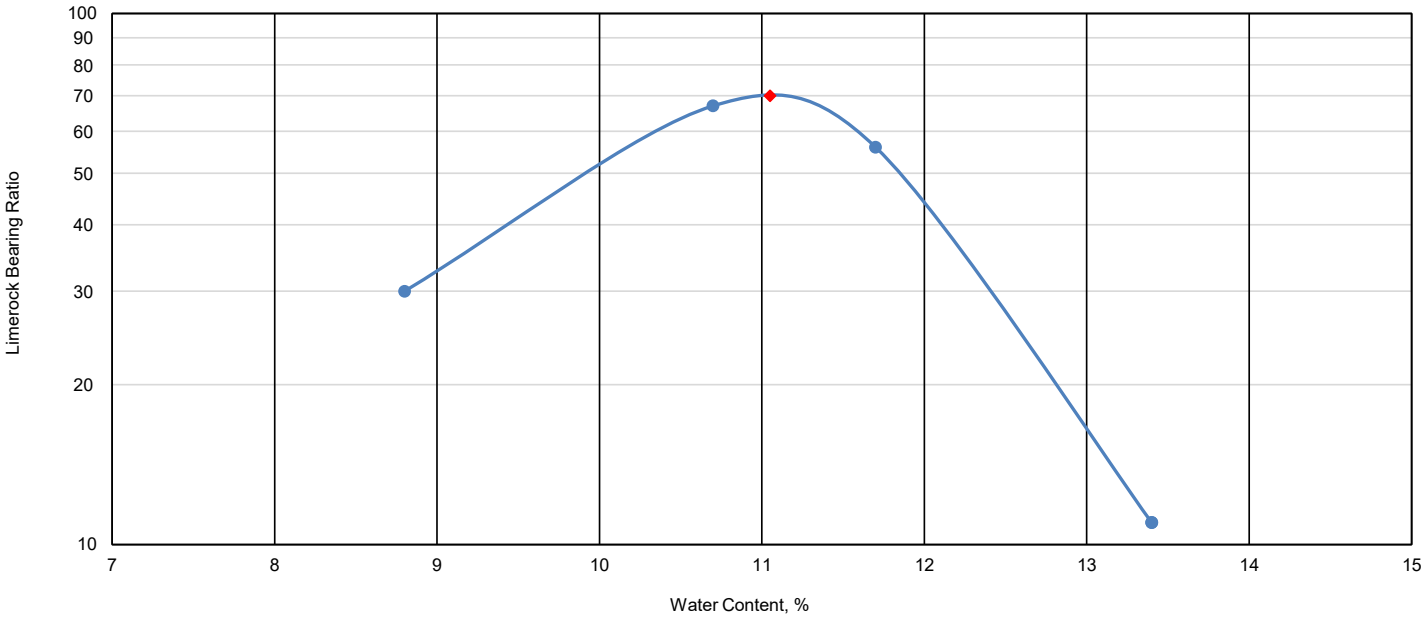
Notes: 1. ASTM D 2216, 2. ASTM D 2487, 3. ASTM D 4318, 4. ASTM D 1140, 5. ASTM D 2434

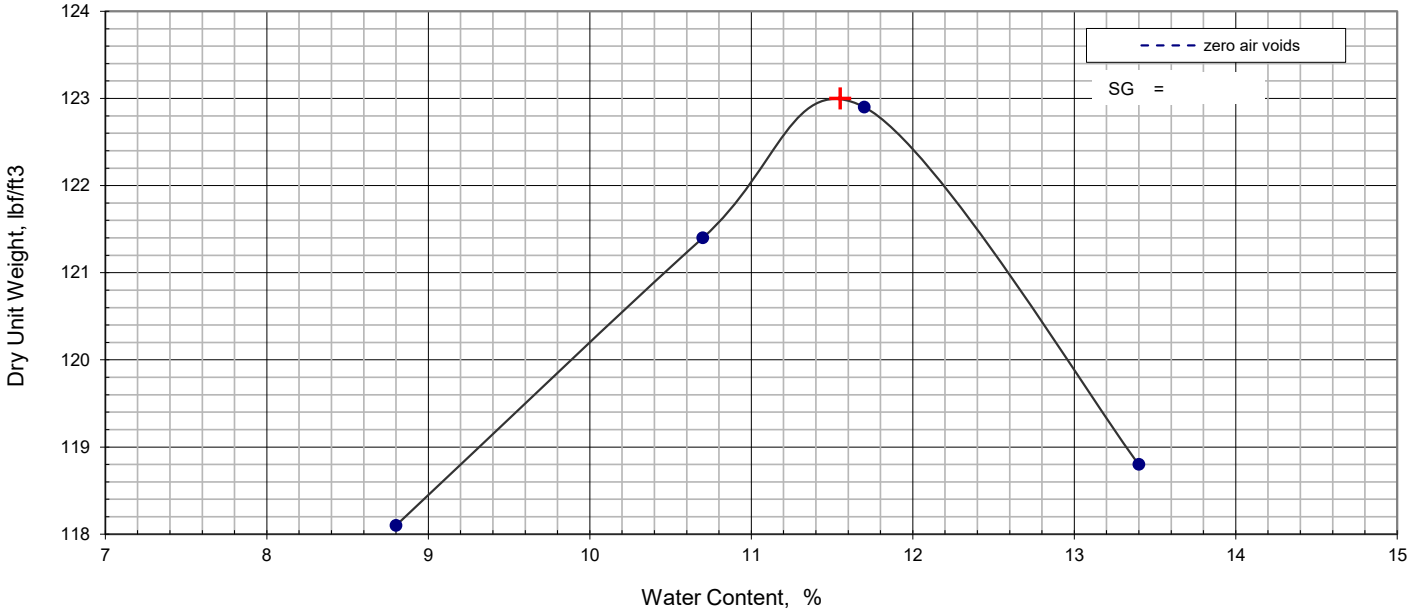
Definitions: MC: Moisture Content, Soil Type: USCS (Unified Soil Classification System), LL: Liquid Limit, PL: Plastic Limit, PI: Plasticity Index

Project No. **35:31000**
Project Name: **QSR Alachua**
PM: **C. Dunlap**
PE: **D. Spangler**
Printed On: **December 2, 2020**

ECS Florida, LLC
6342 NW 18th Drive, Unit 10
Gainesville, Florida 32653
(352) 221-9221

Limerock Bearing Ratio (LBR)





TEST RESULTS (FM 5-515)

Max. Dry Density (pcf)	Optimum Moisture Content (%)	Max. LBR	Fines Content (%)
123.0	11.6	70	18.2
Material Description		USCS	AASHTO
Brown Silty Fine Sand (SM)			

Project: QSR Alachua
Client: DE Foods, LLC
Sample / Source: P1

Project No.: 35:31000
Depth (ft): 1 - 2
Sample No.: D3S-28
Date Reported: 29.11.2020

Office / Lab	Address	Office Number / Fax
ECS Florida LLC - Jacksonville	7064 Davis Creek Road Jacksonville, FL 32256	(904)880-0960 (904)880-0970

Tested by	Checked by	Approved by	Date Received
GBarham		SEBrown	

Traffic Impact Analysis



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CHW
Professional Consultants

CONTENTS

1. Executive Summary 2

2. Study Area 2

3. Trip Generation 4

4. Data Collection 4

5. Reserved Trips 6

6. Trip Distribution 9

7. Analysis 12

7.1. Intersection Analysis 12

7.2 Access Management 14

8. Conclusions and Recommendations 17

List of Figures

- Figure 1: Study Area
- Figure 2: Existing (Peak Season) Conditions
- Figure 3: Reserved Trips
- Figure 4: Background with Reserved Trips Conditions
- Figure 5: Project Trip Distribution
- Figure 6: Build-out with Reserved Trips Conditions
- Figure 7: Access Management Map

List of Tables

- Table 1: Trip Generation
- Table 2: PM Intersection Analysis Summary
- Table 3: FDOT Spacing Standards

List of Appendices

- Appendix A: Methodology Correspondence
- Appendix B: Concept Exhibit
- Appendix C: Count Data and Background Data
- Appendix D: Reserved Trips
- Appendix E: Background Tables
- Appendix F: HCS and Synchro Reports
- Appendix G: Straight-Line Diagram
- Appendix H: Trip Generation Documentation

1. EXECUTIVE SUMMARY

A quick service restaurant (QSR) is being proposed on the south side of US HWY 441 approximately 830 FT west of the I-75 southbound off-ramp in the City of Alachua, as illustrated in the **Figure 1 Study Area**. This Traffic Impact Analysis (TIA) provides the intersection analysis and access management review, as documented in the methodology letter. The methodology letter was reviewed and approved by FDOT and the City of Alachua and is provided in **Appendix A**.

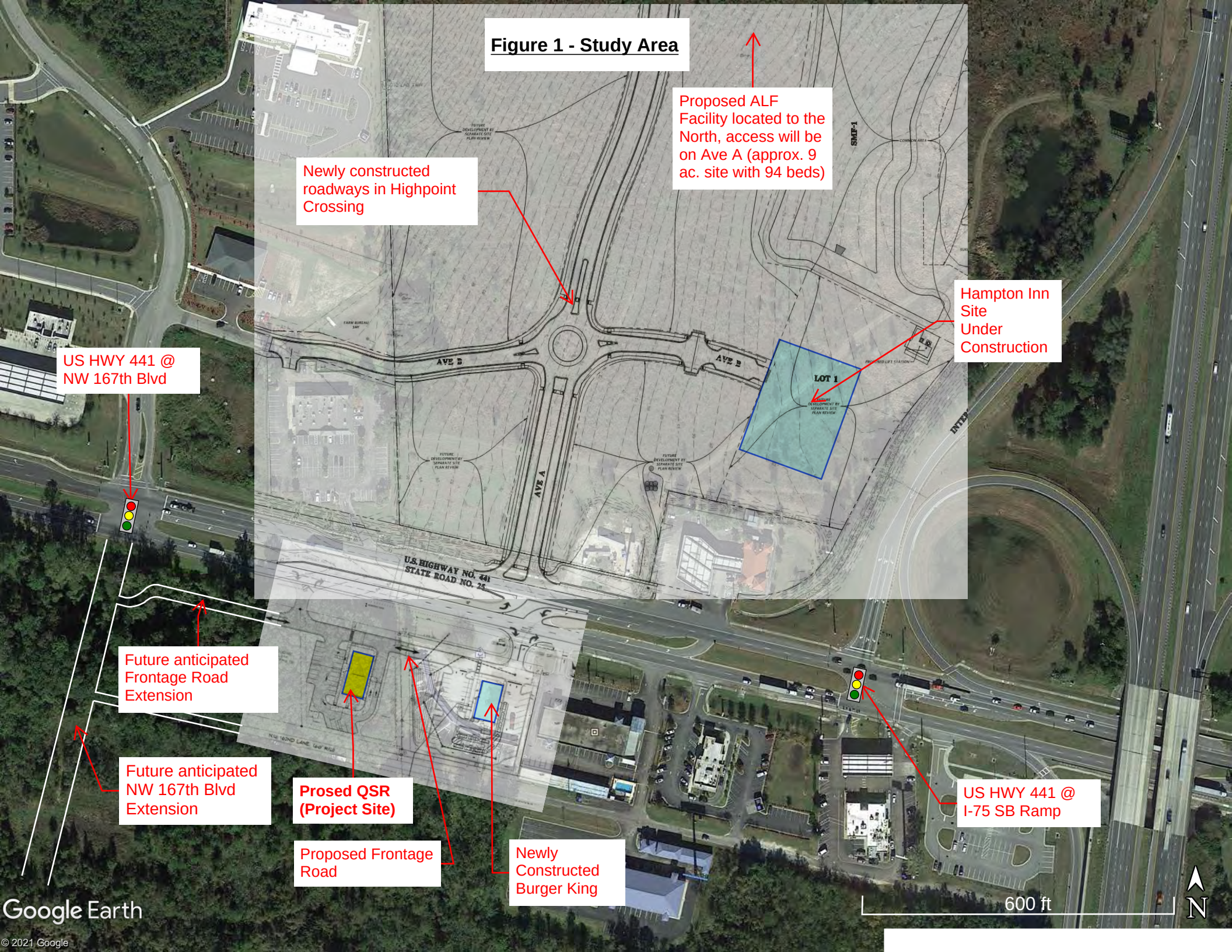
The site proposes a connection via a frontage road along US HWY 441. A right-in right-out connecting the parallel access to US HWY 441, aligned with the sites west property line is being proposed. The access locations and site layout are provided in **Appendix B**.

Based on the results of the analysis from this study, off-site transportation improvements are not required to support this site.

2. STUDY AREA

The study area consists of the following intersections:

- US HWY 441 at NW 167th Boulevard
- US HWY 441 at the proposed right-in/right-out just west of the proposed site
- US HWY 441 at the proposed directional left that will serve the future Burger King
- US HWY 441 at the eastbound directional left located just west of the I-75 southbound off-ramp


Figure 1 - Study Area
Proposed ALF Facility located to the North, access will be on Ave A (approx. 9 ac. site with 94 beds)
Newly constructed roadways in Highpoint Crossing
Hampton Inn Site Under Construction
US HWY 441 @ NW 167th Blvd
AVE B
AVE A
LOT 1
U.S. HIGHWAY NO. 441
STATE ROAD NO. 25
Future anticipated Frontage Road Extension
Future anticipated NW 167th Blvd Extension
Prosed QSR (Project Site)
Proposed Frontage Road
Newly Constructed Burger King
US HWY 441 @ I-75 SB Ramp
600 ft
N
Google Earth
© 2021 Google

3. TRIP GENERATION

The trip generation of the QSR site is based on the Institute of Transportation Engineer's (ITE) Trip Generation Manual, 10th Edition. The estimated trip generation is provided in **Table 1**. The trip generation and pass-by ITE Manual documentation are provided in **Appendix H**.

Table 1 | Trip Generation

QSR- US HWY 441 - City of Alachua						
Land Use	**ITE LU Code**	**Variable (1000 SF)**	**Daily**	**PM Peak**		
			Total	**Total**	**In**	**Out**
Fast Food Restaurant with Drive-Through Window	934	1.904	897	62	32	30
*Pass-by Trips, AM=49%, PM=50%, Weekday=49.5%			444	31	16	15
Net New Total Trips			453	31	16	15

* The ITE provides a pass-by trip rate of 49% for the AM peak hour and 50% for the PM peak hour but does not provide a pass-by rate for daily trips. The daily trip pass-by rate was estimated as the average of the AM and PM rates or 49.5%.

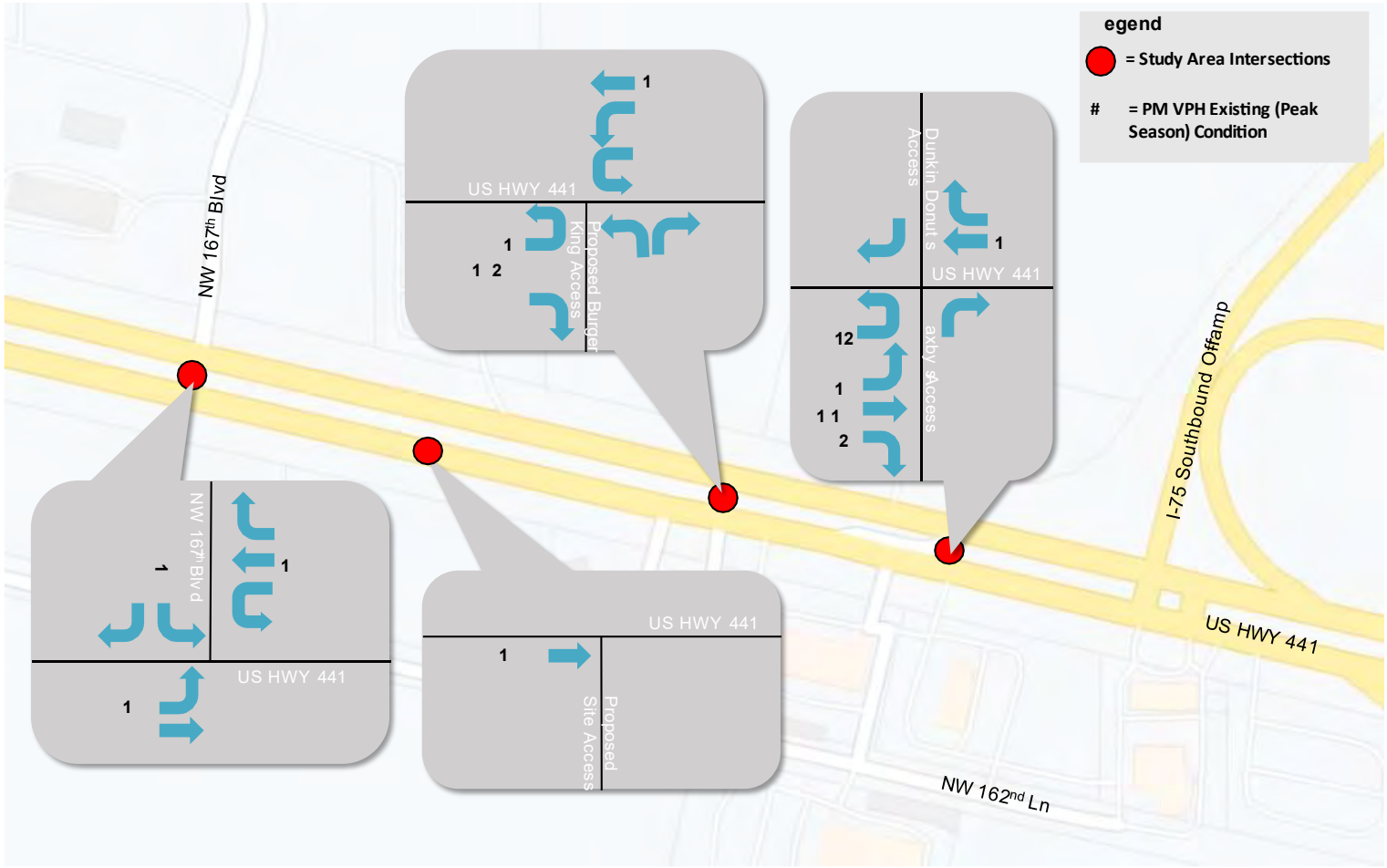
4. DATA COLLECTION

Turning movement counts at the study area intersections were performed during the PM peak hours on November 17, 2020. The raw count data is provided in **Appendix C**.

Due to the current traffic impacts due to COVID-19, a factor was considered to convert the observed traffic volumes to pre-COVID-19 volumes. However, a comparison of the observed volumes at the full median opening adjacent to the proposed Burger King, with the 2019 volumes, from FDOT count station 0461, indicates that the 2020 observed volumes are higher than the 2019 counts. Therefore, a factor is not applied to the count data. This count comparison calculation is provided in **Appendix C**.

The count data was adjusted based on the seasonal adjustment factor. **Figure 2** provides the existing (peak season) turning movements at all study area intersections. Growth rates were determined based on historical ADT where available to derive the 2021 background volumes. A minimum 1.0% yearly growth rate was used for roadways with historical growth rates less than 1.0%.

Figure 2 | Existing (Peak Season) Conditions



5. RESERVED TRIPS

The City of Alachua provided reserved trips for developments currently being constructed or permitted and planned for construction. The reserved trip volumes are provided in **Appendix D**. The estimated trip distribution of the reserved trips is provided in **Figure** . The background turning movements, with the reserved trips applied, are provided in tabular form in **Appendix E** and illustrated in **Figure** .

Figure 3 | Reserved Trips

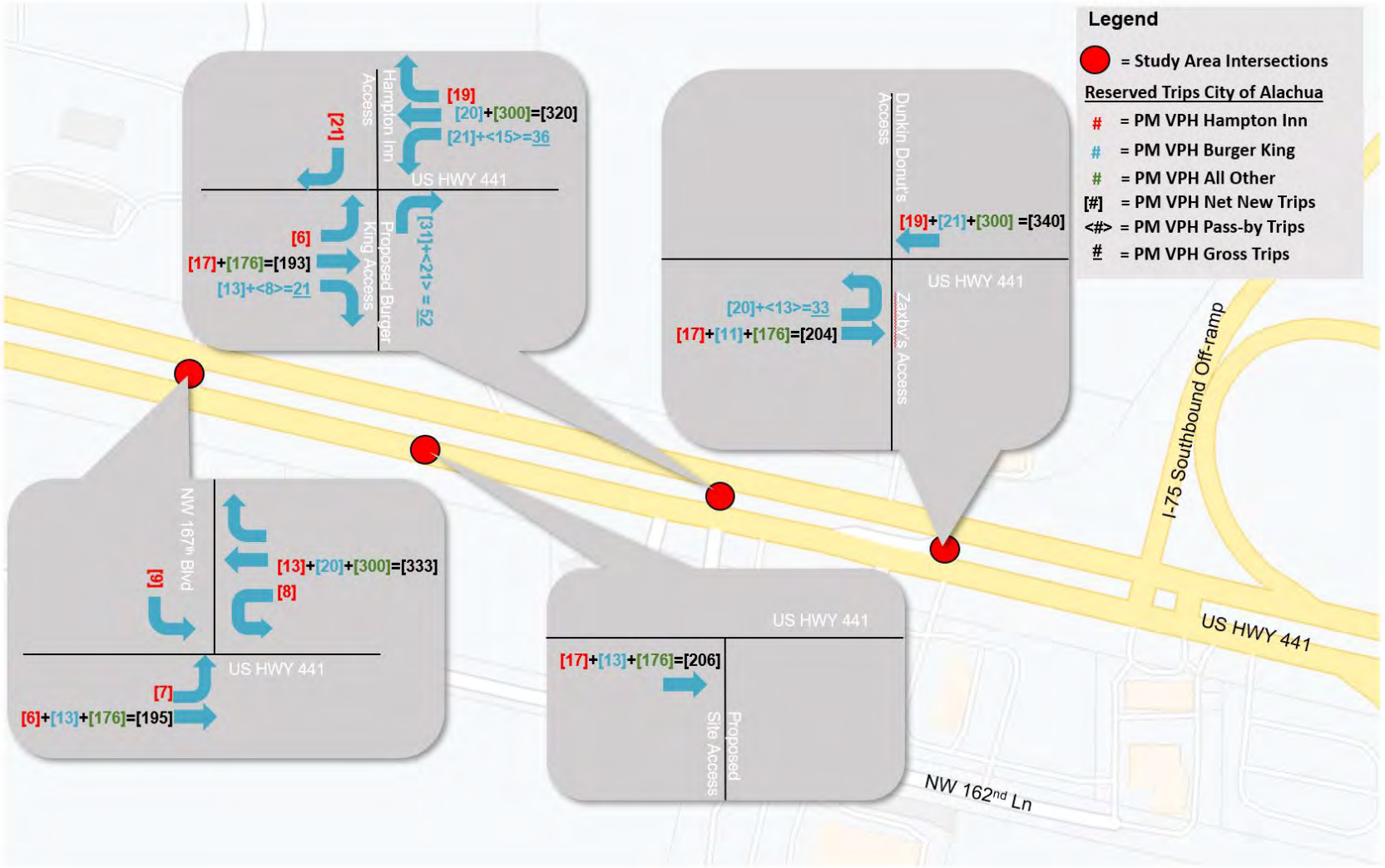
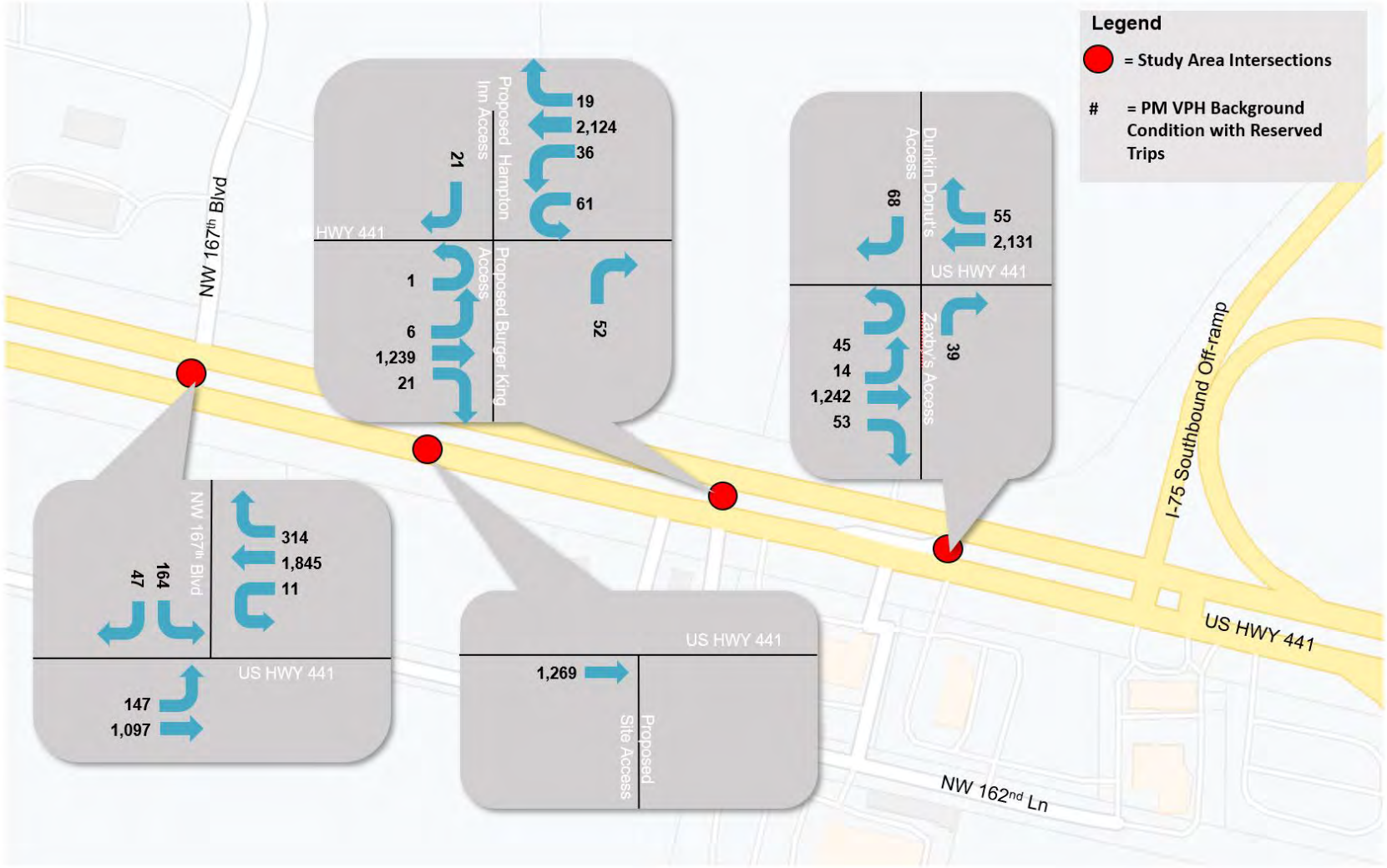


Figure 4 | Background with Reserved Trips Conditions



6. TRIP DISTRIBUTION

The project trip distribution for this site is expected to mimic the existing traffic patterns along US HWY 441. Therefore, the trip distribution was based on the US HWY 441 directional distribution derived from the turning movement count data. **Figure** provides the expected trip distribution where, 63% of the traffic is heading westbound and 37% of the traffic is heading eastbound. **Figure** provides the build-out condition which includes the background trips, reserved trips, and project trips.

Figure 5 | Project Trip Distribution

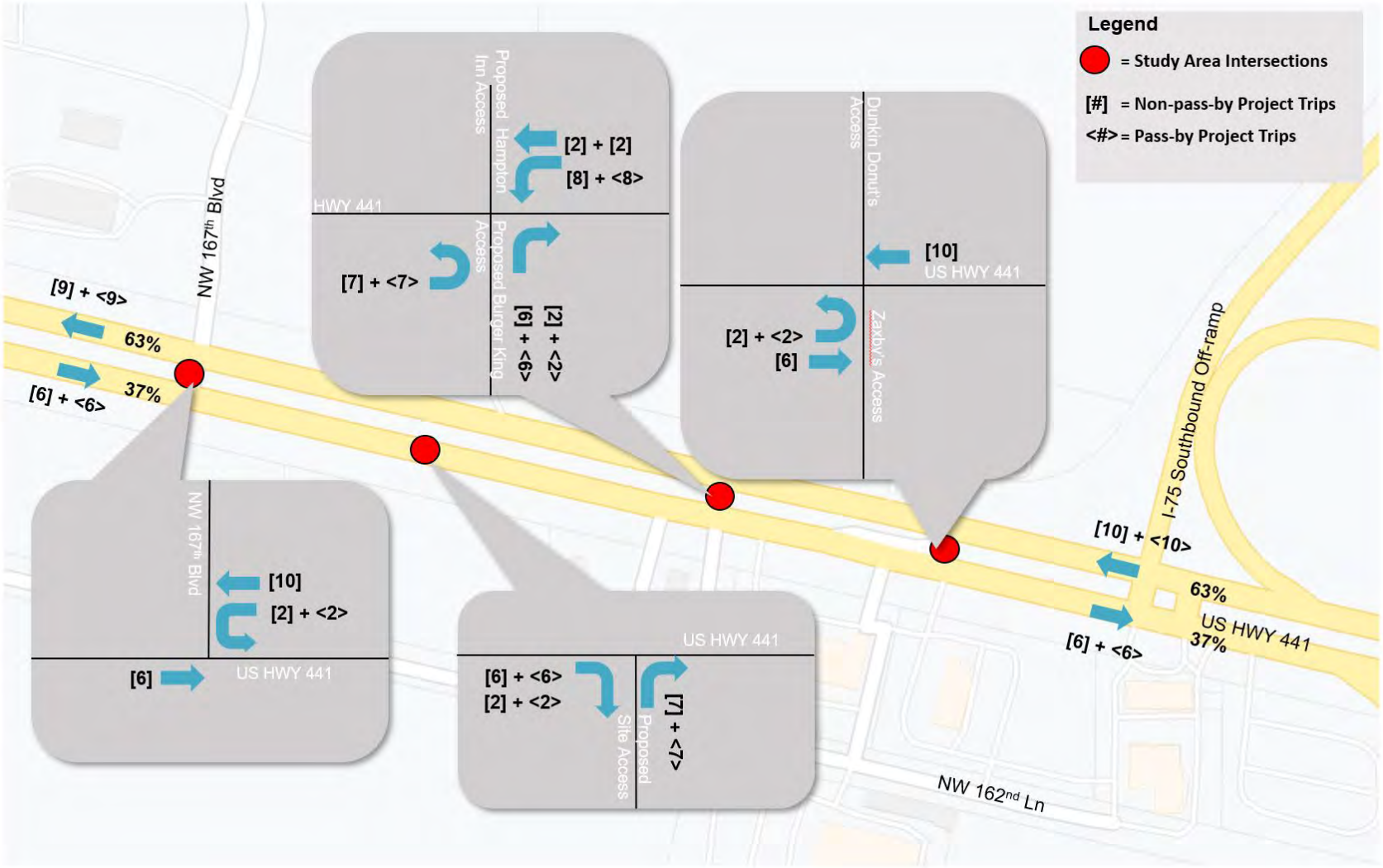
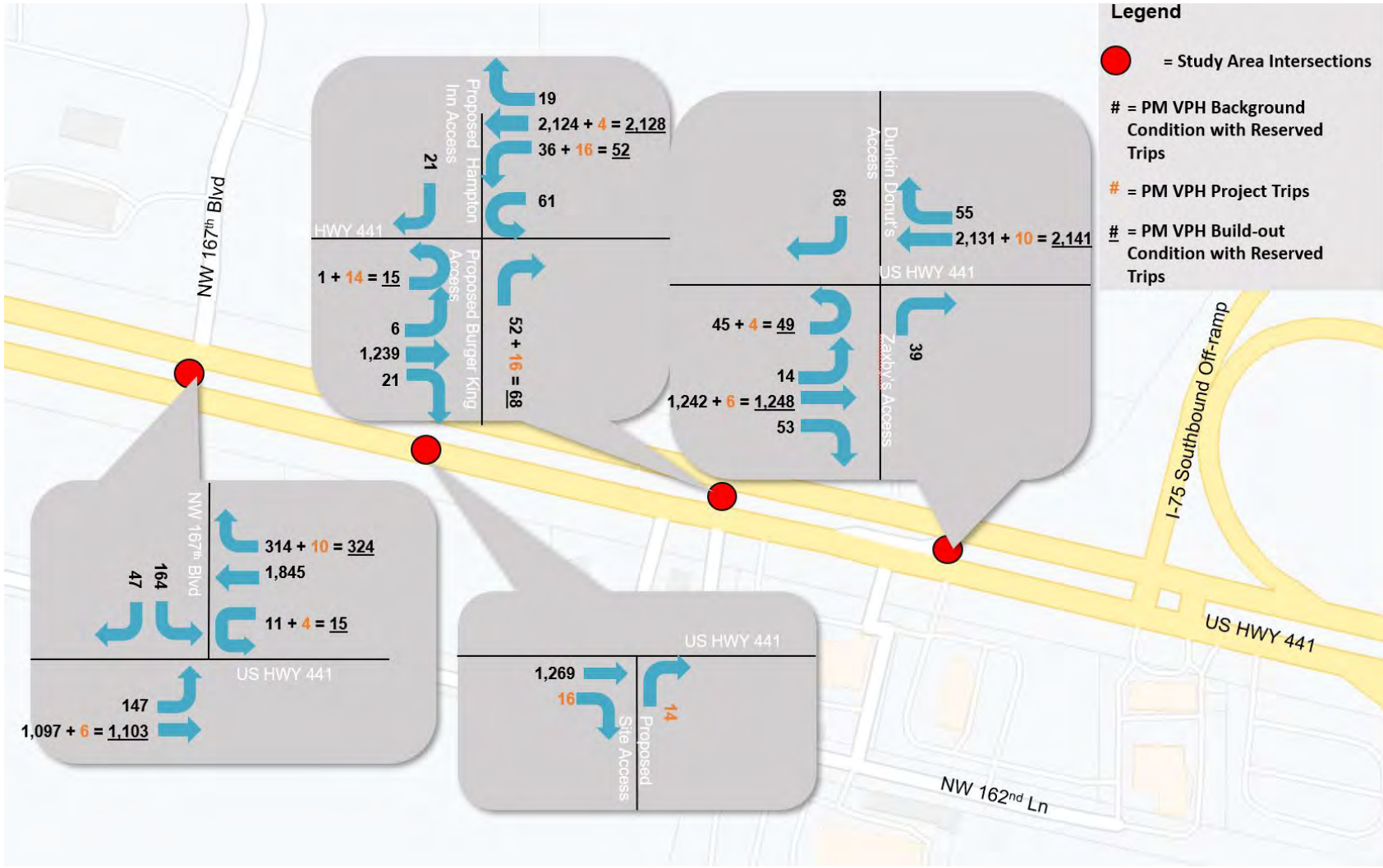


Figure 6 | Build-Out Condition With Reserved Trips



An intersection analysis was performed at the intersection provided in the study area under the following scenarios:

- Existing (peak season)
- Background with reserved trips condition
- Build-out with reserved trips condition

Additionally, access management was reviewed comparing the proposed access spacing with the FDOT spacing standards.

7.1. Intersection Analysis

All Study Area intersections were analyzed during the PM peak hour. HCS 7 was used to analyze the stop-controlled intersections and Synchro 9 was used to analyze the signalized intersection of US HWY 441 at NW 167th Blvd. **Table 2** provides the LOS, delay, v/c ratio, and 95th percentile queue length of the study area intersection movements.

The following intersection movement deficiencies are highlighted in **Table 2**:

- Movements operating with a LOS of F
- v/c ratios exceeding 1.0
- 95th percentile queue lengths exceeding the available storage

Both the Synchro and HCS reports are provided in **Appendix F**

Table 2: PM Intersection Analysis Summary															
Intersection	Movement	Available Storage (ft)	Existing (Peak Season)				Background with Reserved Trips				Build-out with Reserved Trips				
			LOS	Delay(s)	v/c	95% Queue (FT)	LOS	Delay(s)	v/c	95% Queue (FT)	LOS	Delay(s)	v/c	95% Queue (FT)	
Stop Controlled Intersections															
HWY 441 and proposed Right-in/Right-out	NBR	N/A									B		14.1	0.03	
	Intersection Total												0.2		
HWY 441 and Burger King / Hampton Inn	EBL	275	E	36.3	0.01	0	C	18.4	0.03	25	C	24.5	0.11	25	
	WBL	275	C	18.1	0.20	25	D	25.1	0.37	50	D	25.5	0.41	50	
	NB(LT)R	N/A	F	494.2	1.39	125	B	11.8	0.1	25	B	12.1	0.13	25	
	SBR	N/A					C	18.3	0.08	25	C	18.3	0.08	25	
	Intersection Total			7.2				1.0				1.3			
HWY 441 and Dunkin Donuts	EBL	250	D	31.3	0.17	25	E	35.7	0.35	50	E	40.0	0.40	50	
	NBR	N/A	B	11.2	0.07	25	B	11.7	0.07	25	B	11.7	0.07	25	
	SBR	N/A	B	10.9	0.11	25	C	21.3	0.25	25	C	21.3	0.25	25	
	Intersection Total			0.7				1.1				1.2			
Signalized Intersections															
HWY 441 and NW 167th Blvd	EBL	270	C	25.9	0.63	102	E	69.3	0.78	186	E	69.3	0.78	186	
	EBT	N/A	A	6.1	0.34	211	A	7.9	0.43	284	A	9.0	0.44	289	
	WBU	250	A	1.7	0.01	0	A	2.1	0.04	25	A	2.1	0.05	25	
	WBT	N/A	B	10.9	0.71	303	B	19.5	0.91	447	B	19.5	0.91	456	
	WBR	290	A	1.2	0.30	0	A	1.4	0.32	0	A	1.4	0.33	0	
	SBL/R	150	E	65.2	0.54	150	E	66.7	0.57	158	E	66.7	0.57	158	
	Intersection Total		B	12.9			B	19.3			B	19.5			

Yellow highlight = deficiency occurring without project traffic

Red highlight = deficiency occurs due to added project trips

As illustrated in **Table 2** there are no deficiencies caused by the addition of project trips, therefore no transportation improvements are recommended. The deficiencies highlighted in yellow occur before project trips are added to the network. The northbound movement deficiency at the HWY 441 and Burger King/Hampton Inn connection is corrected during the 2021 year, due to the full median opening becoming a directional opening.

7.2 Access Management

Based on FDOT's straight-line diagram, as provided in **Appendix G**, US HWY 441 has a Class 3 access classification and a posted speed of 45 MPH. The connection spacing standard for a Class 3 roadway is 440 FT, as provided in **Table 3**. The proposed right-in/right-out connection on US HWY 441 is located centered between the intersections of NW 167th Blvd and the proposed directional opening that will serve Burger King. This provides a connection spacing of approximately 330 FT to the adjacent signalized intersection to the west and a spacing of approximately 405 FT to the directional intersection to the east, as illustrated in **Figure 7**. Though the proposed right-in/right-out falls slightly short of meeting the connection spacing, the access causes no deficiencies and allows exiting vehicles from the QSR to make a u-turn at the directional opening at the proposed Burger King access rather than a u-turn at the directional opening at the Dunkin Donuts access, which has less available capacity according to the results from Table 2. Therefore, this study supports the right-in/right-out access as proposed.

The frontage road that provides access to the project site, is anticipated to connect to a NW 167th Boulevard extension as development continues in this area. This will provide signal control for drivers desiring to exit to the west or enter from the east to gain access to this and other projects along the frontage road.

Table 3 | FDOT Spacing Standards

Access Class	Median Type	Connection Spacing (feet)		Median Opening Spacing (feet)		Signal Spacing (feet)
		>45 mph	≤45 mph	Directional	Full	
2	Restrictive with Service Roads	1320	660	1320	2640	2640
3	Restrictive	660	440	1320	2640	2640
4	Non-Restrictive	660	440			2640
5	Restrictive	440	245	660	2640 >45 mph 1320 ≤ 45 mph	
6	Non-Restrictive	440	245			1320
7	Both Median Types	125		330	660	1320

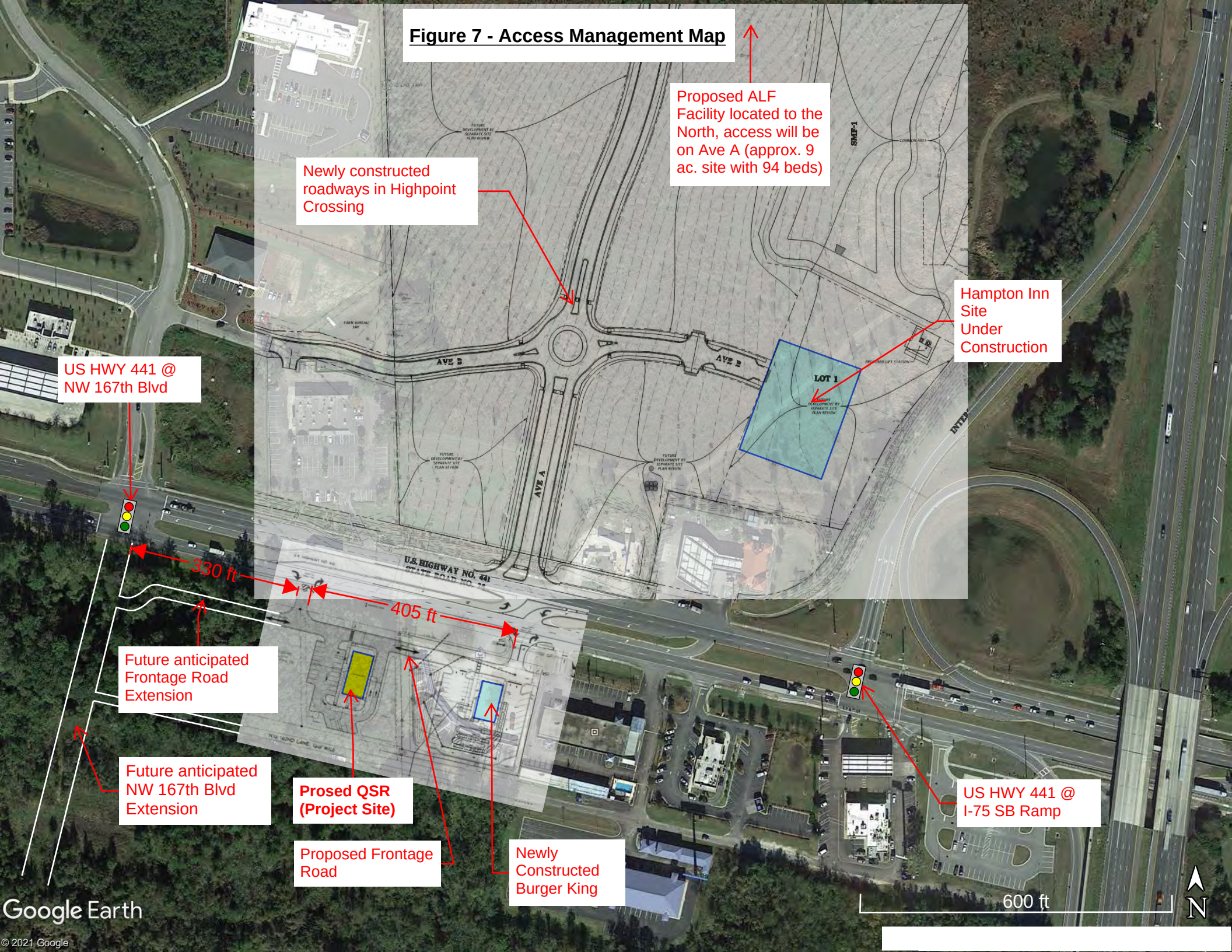
Notes:

1. "Restrictive" physically prevent vehicle crossing.
2. "Non-Restrictive" allow turns across at any point.
3. Speeds shown in this table are posted speeds.

Connection Spacing Near Interchange Ramps:

Connections and median openings located within 1,320 feet of interchange ramps require the following spacing (measured from the ramp furthest from the interchange):

- 440 feet ≤ 45 mph
- 660 feet > 45 mph
- 1,320 feet on Access Class 2 Facilities > 45 mph


Figure 7 - Access Management Map
Proposed ALF Facility located to the North, access will be on Ave A (approx. 9 ac. site with 94 beds)
Newly constructed roadways in Highpoint Crossing
Hampton Inn Site Under Construction
US HWY 441 @ NW 167th Blvd
330 ft
405 ft
Future anticipated Frontage Road Extension
Future anticipated NW 167th Blvd Extension
Prosed QSR (Project Site)
Proposed Frontage Road
Newly Constructed Burger King
US HWY 441 @ I-75 SB Ramp
600 ft
N
Google Earth
© 2021 Google

8. CONCLUSIONS AND RECOMMENDATIONS

This study's operational analysis indicates that the added project traffic causes no deficiencies at the study area intersections, therefore no transportation improvements are recommended.

The proposed access at US HWY 441 was reviewed with FDOT's spacing standards indicating that the proposed right-in/right-out falls slightly short (approximately 50 FT) of meeting the FDOT connection spacing standards. However, the proposed access is supported as it allows exiting vehicles from the QSR to make a u-turn at the directional opening at the proposed Burger King rather than a u-turn at the directional opening at the Dunkin Donuts access, which has less capacity.

Appendix A
Methodology Correspondence

October 30, 2020

Mr. Tom Cavin, PE
Jacksonville Studies Engineer
Florida Department of Transportation
2198 Edison Avenue, MS2815
Jacksonville, FL 32204

Re: Traffic Impact Analysis Methodology, QSR, US HWY 441, Alachua, FL

Dear Tom,

CHW is preparing a Traffic Impact Analysis (TIA) for a proposed KFC Quick Service Restaurant site located on parcel 03053-002-001, just east of NW 167th Boulevard, in the City of Alachua, FL. The restaurant will provide a drive-thru window and up to 30 seats. The site will be accessed via a cross access along US HWY 441. A right-in/right-out driveway is proposed to connect the cross access to US HWY 441 just west of the site. And a directional left on US HWY 441, aligned with the Burger King currently being constructed, will connect the cross access with US HWY 441, just east of the site.

Please review the methodology and either provide comment for the purposes of modification/ clarification or provide acceptance.

Study Area

The study area will consist of the following intersections:

- US HWY 441 at the proposed right-in/right-out just west of the proposed site
- US HWY 441 at the proposed directional left that will serve the future Burger King
- US HWY 441 at the eastbound directional left located just west of the I-75 southbound off-ramp
- US HWY 441 at NW 167th Boulevard

Land Use and Trip Generation

The trip generation for the site is based on the Institute of Transportation Engineer's (ITE) Trip Generation Manual, 10th Edition. The estimated trip generation is provided below.

KFC - US HWY 441 - City of Alachua						
Land Use	ITE LU Code	Variable (1000 SF)	Daily	PM Peak		
			Total	Total	In	Out
Fast Food Restaurant with Drive-Through Window	934	2.123	1000	69	36	33
*Pass-by Trips, AM=49%, PM=50%, Weekday=49.5%			495	35	18	17
Net New Total Trips			505	34	18	16

* The ITE provides a pass-by trip rate of 49 for the AM peak hour and 50 for the PM peak hour but does not provide a pass-by rate for daily trips. The daily trip pass-by rate was estimated as the average of the AM and PM rates or 49.5%.

Data Collection

Turning movement counts during the PM peak two hours are proposed at the following intersections:

- US HWY 441 at the proposed directional left that will serve the future Burger King (this intersection is currently a full median opening)
- US HWY 441 at the eastbound directional left located just west of the I-75 southbound off-ramp
- US HWY 441 at NW 167th Boulevard

Due to the traffic volume impacts of COVID-19, CHW will use available traffic count data from a 2019 count provided by FDOT traffic online to derive a factor that converts the turning movement count data into typical Non-COVID-19 traffic volumes. A peak season factor will be applied to the count data as well as a yearly growth rate derived from FDOT historical ADT, to forecast background volumes.

Trip Distribution

The site's trip distribution is expected to mimic directional distribution along US HWY 441 as approximately half of the trips are estimated to be pass-by trips according to the ITE Trip Generation Manual 10th Edition. Therefore, the US HWY 441 directional distribution percentages will be applied to the project trips.

Reserved trips, provided by the City of Alachua, will be included in the analysis.

Analysis

The analysis will be performed at the intersections provided in the study area under the following scenarios, during the PM peak hour of adjacent street traffic:

- Existing conditions
- Background conditions
- Build-out conditions
- Build-out with improvements condition (if applicable)

The intersection operation analysis will review the delay, LOS, V/C, and 95th percentile queue length of each intersection movement as well as the intersection as a whole. The intersections will be analyzed using HCS 7. The analysis reports will be provided in the appendices.

The proposed right-in/right-out access location will be reviewed based on FDOT standards to ensure that the location is acceptable to FDOT.

Any needed roadway modifications based on this analysis will be identified and modeled to demonstrate that the improvement mitigates the deficiency.

Report

A draft report, including the following at a minimum, will be submitted for review and comment.

- Study Area Map
- Trip Generation
- Data Collection
- Trip Distribution
- Analysis
  - AM and PM Intersection LOS, Delay, V/C and 95th Percentile Queue Length Analysis
  - Access Management
- Conclusion and Recommendations
- Appendices with Supporting Documentation

Following any revisions, based upon staff comments, we will prepare a final signed and sealed report.

If your office concurs with this methodology, please find the appropriate signature block below and sign as such. Please return a signed copy to our office for our records.

Thank you for your review and consideration.

Sincerely,
CHW



Brian Snyder, P.E.
Project Manager

I concur with the traffic study methodology presented for the proposed KFC Quick Service Restaurant in Alachua, FL.

Tom Cavin, PE Date

N:\2020\20-0113\Departments\04_Engineering\04_Traffic\REPORT\METHODOLOGY\LETTER 201007_KFC City of Alachua Traffic Study Methodology.docx

City of Alachua
Correspondence

Brian Snyder

From: Justin Tabor <jtabor@cityofalachua.org>
Sent: Thursday, October 29, 2020 10:41 AM
To: Brian Snyder
Cc: Tom Cavin; Adam Doyle; Kathy Winburn; Adam Hall
Subject: Re: City of Alachua QSR
Attachments: CoA_October_2020_Development_Monitoring_Report.xlsx

Brian,

A traffic study would not be required by our code but since one will be performed I would like to request it be included with your site plan application.

I would offer the following suggestions on the methodology:

- The study area should include the intersection of US 441 and NW 167th Blvd, since some US 441 WB traffic may access the site by making a u-turn at this intersection.
- Turning movement counts should also be taken at the US 441/NW 167th Blvd intersection.
- The analysis should include reserved trips. I have attached the most current development monitoring report with this information.

Sincerely,

Justin Tabor, AICP
Principal Planner
City of Alachua
15100 NW 142nd Terrace | PO Box 9
Alachua, Florida 32616
386.418.6100 x 107 | fax: 386.418.6130
jtabor@cityofalachua.com

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From: "Brian Snyder" <brians@chw-inc.com>
To: "Tom Cavin" <Tom.Cavin@dot.state.fl.us>, "Justin Tabor" <jtabor@cityofalachua.org>
Cc: "Adam Doyle" <Adam.Doyle@dot.state.fl.us>
Sent: Thursday, October 29, 2020 10:27:21 AM
Subject: FW: City of Alachua QSR

Tom,

Please see the attached methodology letter for your review.

Justin,

If your staff will be reviewing the study also I can add your name to the methodology. Please let me know.

Thank you,

BRIAN SNYDER | Project Manager
d: (386) 518-5154 | **c:** (352) 339-2938
e: brians@chw-inc.com
w: www.chw-inc.com

JACKSONVILLE.GAINESVILLE.OCALA
t: (904) 619-6521 | 8563 Argyle Business Loop, Suite 3, Jacksonville, FL 32244
t: (352) 331-1976 | 11801 Research Drive, Alachua, FL 32615
t: (352) 414-4621 | 101 NE 1st Avenue, Ocala, FL 34470
PLANNING.DESIGN.SURVEYING.ENGINEERING.CONSTRUCTION.

From: Brian Snyder
Sent: Wednesday, October 28, 2020 9:31 AM
To: Cavin, Tom <Tom.Cavin@dot.state.fl.us>; Justin Tabor <jtabor@cityofalachua.org>
Cc: Doyle, Adam <Adam.Doyle@dot.state.fl.us>
Subject: RE: City of Alachua QSR

Tom, Justin,

Please see the adjusted trip generation based on SF. I removed AM, as they are not open at that time.

KFC - US HWY 441 - City of Alachua						
Land Use	ITE LU Code	Variable (1000 SF)	Daily	PM Peak		
			Total	Total	In	Out
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Net New Total Trips			505	34	18	16

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Thank you,

BRIAN SNYDER | Project Manager
d: (386) 518-5154 | **c:** (352) 339-2938
e: brians@chw-inc.com
w: www.chw-inc.com

JACKSONVILLE.GAINESVILLE.OCALA
t: (904) 619-6521 | 8563 Argyle Business Loop, Suite 3, Jacksonville, FL 32244
t: (352) 331-1976 | 11801 Research Drive, Alachua, FL 32615
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From: Cavin, Tom <Tom.Cavin@dot.state.fl.us>
Sent: Wednesday, October 28, 2020 9:06 AM
To: Justin Tabor <jtabor@cityofalachua.org>
Cc: Doyle, Adam <Adam.Doyle@dot.state.fl.us>; Brian Snyder <brians@chw-inc.com>
Subject: FW: City of Alachua QSR

To all,

I should have replied “to all” with my comment.

Tom Cavin, P.E.
Jacksonville Studies Engineer
Florida Department of Transportation
2198 Edison Avenue, MS2815
Tom.cavin@dot.state.fl.us
904-360-5641



From: Cavin, Tom
Sent: Wednesday, October 28, 2020 8:48 AM

To: Brian Snyder <brians@chw-inc.com>
Subject: RE: City of Alachua QSR

Brian,

Instead of seats, please use square footage. There is no way for us to confirm seats—square footage is the norm.

I should be in the meeting too

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Jacksonville Studies Engineer
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2198 Edison Avenue, MS2815
Tom.cavin@dot.state.fl.us
904-360-5641

From: Brian Snyder <brians@chw-inc.com>
Sent: Tuesday, October 27, 2020 5:30 PM
To: Cavin, Tom <Tom.Cavin@dot.state.fl.us>; Justin Tabor <jtabor@cityofalachua.org>
Subject: City of Alachua QSR

EXTERNAL SENDER: Use caution with links and attachments.

Tom, Justin,

I would like to confirm that the KFC proposed on US HWY 441 with a maximum of 30 seats will not require a traffic study, see trip generation provided below.

Tom, CHW has a meeting proposed with FDOT tomorrow to discuss the site. Not sure if you will be attending that meeting.

Please let me know if you need more info or would like to discuss.

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FDOT
Correspondence

Brian Snyder

From: Cavin, Tom <Tom.Cavin@dot.state.fl.us>
Sent: Monday, November 2, 2020 10:00 AM
To: Brian Snyder
Cc: Doyle, Adam
Subject: FW: City of Alachua QSR
Attachments: LETTER 201030_KFC City of Alachua Traffic Study Methodology.pdf

Brian,

I am ok with your study methodology. At this point, the Department is not in approving the right in/right out connection to the west of the property. The study can consider it and make recommendations as to why the Department should allow the connection even though it violates FAC access management spacing requirements. As we discussed last week, FDOT and the City of Alachua are meeting to discuss another property this week. This additional access will be discussed with them at that time too.

I will let you know what we decide.

Tom Cavin, P.E.
Jacksonville Studies Engineer
Florida Department of Transportation
2198 Edison Avenue, MS2815
Tom.cavin@dot.state.fl.us
904-360-5641

From: Brian Snyder <brians@chw-inc.com>
Sent: Monday, November 2, 2020 6:57 AM
To: Cavin, Tom <Tom.Cavin@dot.state.fl.us>
Subject: FW: City of Alachua QSR

Tom,

Just checking to see whether you have had a chance to look at this methodology?

Thank you,

BRIAN SNYDER | Project Manager
d: (386) 518-5154 | **c:** (352) 339-2938
e: brians@chw-inc.com
w: www.chw-inc.com

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t: (352) 414-4621 | 101 NE 1st Avenue, Ocala, FL 34470

From: Brian Snyder
Sent: Friday, October 30, 2020 8:34 AM
To: Tom Cavin <Tom.Cavin@dot.state.fl.us>; Justin Tabor <jtabor@cityofalachua.org>
Subject: FW: City of Alachua QSR

Tom, Justin,

Please see the revised methodology based on Justin's recommendations.

Thanks

BRIAN SNYDER | Project Manager
d: (386) 518-5154 | **c:** (352) 339-2938
e: brians@chw-inc.com
w: www.chw-inc.com

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PLANNING.DESIGN.SURVEYING.ENGINEERING.CONSTRUCTION.

From: Justin Tabor <jtabor@cityofalachua.org>
Sent: Thursday, October 29, 2020 10:41 AM
To: Brian Snyder <brians@chw-inc.com>
Cc: Tom Cavin <Tom.Cavin@dot.state.fl.us>; Adam Doyle <Adam.Doyle@dot.state.fl.us>; Kathy Winburn <kwinburn@cityofalachua.org>; Adam Hall <ad_hall@cityofalachua.org>
Subject: Re: City of Alachua QSR

Brian,

A traffic study would not be required by our code but since one will be performed I would like to request it be included with your site plan application.

I would offer the following suggestions on the methodology:

- The study area should include the intersection of US 441 and NW 167th Blvd, since some US 441 WB traffic may access the site by making a u-turn at this intersection.
- Turning movement counts should also be taken at the US 441/NW 167th Blvd intersection.
- The analysis should include reserved trips. I have attached the most current development monitoring report with this information.

Sincerely,

Justin Tabor, AICP
Principal Planner
City of Alachua
15100 NW 142nd Terrace | PO Box 9

Alachua, Florida 32616
386.418.6100 x 107 | fax: 386.418.6130
jtabor@cityofalachua.com

City Hall Hours of Operation
Monday - Thursday, 7:30 AM - 6:00 PM

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From: "Brian Snyder" <brians@chw-inc.com>
To: "Tom Cavin" <Tom.Cavin@dot.state.fl.us>, "Justin Tabor" <jtabor@cityofalachua.org>
Cc: "Adam Doyle" <Adam.Doyle@dot.state.fl.us>
Sent: Thursday, October 29, 2020 10:27:21 AM
Subject: FW: City of Alachua QSR

Tom,

Please see the attached methodology letter for your review.

Justin,

If your staff will be reviewing the study also I can add your name to the methodology. Please let me know.

Thank you,

BRIAN SNYDER | Project Manager
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Appendix B
Concept Exhibit

U.S. HIGHWAY NO. 441

RIGHT IN
RIGHT OUT

SIGN (NO PARKING IN R/W)

HIGHWAY SIGN

RIGHT IN
RIGHT OUT

S. R/W LINE

HYD

(RLS 509)

CROSS ACCESS

CROSS ACCESS

TP: 03053-002-000
PARCEL 2 OF
O.R.B. 4281, PAGE 128

PROPOSED BUILDING

DRIVE THRU

DRIVE THRU

BY-PASS

MENU BOARD

PROPOSED SMF

DUMPSTER

(U) PUBLIC UTILITY EASEMENT FOR NATURAL GAS FACILITIES (O.R.B. 411, PAGE 211)

TP: 03061-004-001

ONE-STORY BUILDING

ONE-STORY BUILDING

ASPHALT

RAILING

OVERHANG

(1/2" LB 021)

N. R/W LINE

N.W. 162ND LANE (60' R/W)

Ex. 15" P.V.C. San. Sew. Line

18' 24' 18' 24' 11.5' 10' 10' 14' 9' 18' 10' 10' 22.9' 60'

TRM #1
EL.= 76.54'

TRM #2
EL.= 76.74'

MW#6 MW#2 MW#13 MW#17 DW#2 MW#1 MW#4 MW#3 MW#5 MW#10

DA-1 DA-2 DA-3 DA-4 DA-5 DA-6 DA-7 DA-8 DA-9 DA-10

OHW PP TP

1801 Research Drive
Alachua, FL 32615
(352) 331-1976
www.chw-inc.com

CHW
Professional Consultants

est. 1988 FLORIDA CA-5975

SCALE: 1"=20'

VERIFY SCALE
BAR IS ONE INCH ON
ORIGINAL DRAWING
0 1"
IF NOT ONE INCH ON
THIS SHEET ADJUST
SCALES ACCORDINGLY.

CONSTRUCTION/BID REVISIONS:

SUBMITTALS:

CLIENT: DE FOODS

PROJECT: KFC
ALACHUA

SHEET TITLE: CONCEPT PLAN

TECHNICIAN: T.F. COWART
DESIGNER: D.L. YOUNG, P.E.
QUALITY CONTROL: R.J. WALPOLE, P.E.

PROJECT NUMBER: 20-0113

SHEET NO.: C-1

Date: thomasc Plot Date: Oct 19,2020 2:41pm Filename: N:\2020\V20-0113\Department\06_Engineering\06_Design\Base Files\Concept\20-0113_C-1.dwg

Appendix C
Count Data and Background Data

COUNTY: 26
STATION: 0461
DESCRIPTION: SR 20 .2 MI. NW OF SR 93
START DATE: 08/27/2019
START TIME: 1100

	DIRECTION: N					DIRECTION: S					COMBINED
TIME	1ST	2ND	3RD	4TH	TOTAL	1ST	2ND	3RD	4TH	TOTAL	TOTAL
0000	15	16	22	27	80	24	18	18	18	78	158
0100	16	9	7	15	47	17	29	16	17	79	126
0200	6	8	7	22	43	11	12	20	26	69	112
0300	19	19	58	44	140	9	16	18	28	71	211
0400	36	56	50	74	216	37	30	53	54	174	390
0500	83	96	124	173	476	58	53	76	67	254	730
0600	184	308	316	341	1149	73	74	94	110	351	1500
0700	447	475	419	234	1575	105	158	162	202	627	2202
0800	279	450	426	318	1473	169	205	215	166	755	2228
0900	241	234	225	216	916	156	150	148	178	632	1548
1000	220	209	196	180	805	190	173	189	159	711	1516
1100	204	177	248	240	869	219	224	214	231	888	1757
1200	213	237	248	178	876	244	220	218	226	908	1784
1300	206	222	205	217	850	245	222	227	230	924	1774
1400	203	197	207	229	836	240	245	247	265	997	1833
1500	213	298	252	203	966	324	299	272	343	1238	2204
1600	208	202	214	228	852	327	356	378	431	1492	2344
1700	253	174	224	200	851	437	448	464	437	1786	2637
1800	177	179	141	146	643	354	321	274	238	1187	1830
1900	127	133	112	108	480	206	192	175	162	735	1215
2000	101	79	108	83	371	152	150	148	133	583	954
2100	72	60	49	101	282	102	91	80	100	373	655
2200	70	50	48	35	203	85	63	78	51	277	480
2300	27	20	19	7	73	34	24	36	28	122	195
24-HOUR TOTALS:					15072					15311	30383

PEAK VOLUME INFORMATION

	DIRECTION: N		DIRECTION: S		COMBINED DIRECTIONS	
	HOUR	VOLUME	HOUR	VOLUME	HOUR	VOLUME
A.M.	645	1682	745	791	800	2228
P.M.	1445	992	1700	1786	1645	2659
DAILY	645	1682	1700	1786	1645	2659

TMC 11/17/2020

HOUR	VOLUME
16:45	2,704

* SINCE THE 2020 VOLUMES ARE GREATER THAN 2019 VOLUMES, NO COVID FACTOR IS APPLIED.

GENERATED BY SPS 5.0.53P

Peggy Malone & Associates
(888) 247-8602

File Name : 1- US 441 & NW 167th Blvd
Site Code :
Start Date : 11/17/2020
Page No : 1

Groups Printed- Cars

	NW 167th Blvd Southbound				US 441 Westbound					US 441 Eastbound				
Start Time	Right	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Thru	Left	Peds	App. Total	Int. Total
04:15 PM	8	34	0	42	51	282	0	0	333	175	21	0	196	571
04:30 PM	7	31	0	38	60	296	1	0	357	189	24	0	213	608
04:45 PM	9	33	0	42	76	329	0	0	405	194	30	0	224	671
Total	24	98	0	122	187	907	1	0	1095	558	75	0	633	1850
			0					0				0		
05:00 PM	9	33	0	42	48	301	3	0	352	195	29	0	224	618
05:15 PM	11	38	0	49	90	384	0	0	474	204	35	0	239	762
05:30 PM	14	40	0	54	74	339	0	0	413	193	34	0	227	694
05:45 PM	8	48	0	56	66	282	0	0	348	167	32	0	199	603
Total	42	159	0	201	278	1306	3	0	1587	759	130	0	889	2677
06:00 PM	12	46	0	58	60	270	1	0	331	170	27	0	197	586
Grand Total	78	303	0	381	525	2483	5	0	3013	1487	232	0	1719	5113
Apprch %	20.5	79.5	0		17.4	82.4	0.2	0		86.5	13.5	0		
Total %	1.5	5.9	0	7.5	10.3	48.6	0.1	0	58.9	29.1	4.5	0	33.6	

	NW 167th Blvd Southbound			US 441 Westbound				US 441 Eastbound			
Start Time	Right	Left	App. Total	Right	Thru	Left	App. Total	Thru	Left	App. Total	Int. Total
Peak Hour Analysis From 04:15 PM to 06:00 PM - Peak 1 of 1											
Peak Hour for Entire Intersection Begins at 04:45 PM											
04:45 PM	9	33	42	76	329	0	405	194	30	224	671
05:00 PM	9	33	42	48	301	**3**	352	195	29	224	618
05:15 PM	11	38	49	**90**	**384**	0	**474**	**204**	**35**	**239**	**762**
05:30 PM	**14**	**40**	**54**	74	339	0	413	193	34	227	694
Total Volume	43	144	187	288	1353	3	1644	786	128	914	2745
% App. Total	23	77		17.5	82.3	0.2		86	14		
PHF	.768	.900	.866	.800	.881	.250	.867	.963	.914	.956	.901

Peggy Malone & Associates
(888) 247-8602

File Name : 1- US 441 & NW 167th Blvd
Site Code :
Start Date : 11/17/2020
Page No : 1

Groups Printed- Trucks

	NW 167th Blvd Southbound				US 441 Westbound					US 441 Eastbound				
Start Time	Right	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Thru	Left	Peds	App. Total	Int. Total
04:15 PM	0	0	0	0	1	12	0	0	13	7	0	0	7	20
04:30 PM	0	2	0	2	0	22	0	0	22	16	1	0	17	41
04:45 PM	1	1	0	2	1	10	0	0	11	18	1	0	19	32
Total	1	3	0	4	2	44	0	0	46	41	2	0	43	93
				3					50				50	
05:00 PM	0	0	0	0	2	17	0	0	19	14	0	0	14	33
05:15 PM	0	1	0	1	0	12	0	0	12	8	0	0	8	21
05:30 PM	0	0	0	0	0	8	0	0	8	9	0	0	9	17
05:45 PM	0	0	0	0	1	9	0	0	10	10	0	0	10	20
Total	0	1	0	1	3	46	0	0	49	41	0	0	41	91
06:00 PM	0	0	0	0	0	16	0	0	16	7	0	0	7	23
Grand Total	1	4	0	5	5	106	0	0	111	89	2	0	91	207
Apprch %	20	80	0		4.5	95.5	0	0		97.8	2.2	0		
Total %	0.5	1.9	0	2.4	2.4	51.2	0	0	53.6	43	1	0	44	

	NW 167th Blvd Southbound			US 441 Westbound				US 441 Eastbound			
Start Time	Right	Left	App. Total	Right	Thru	Left	App. Total	Thru	Left	App. Total	Int. Total
Peak Hour Analysis From 04:15 PM to 06:00 PM - Peak 1 of 1											
Peak Hour for Entire Intersection Begins at 04:30 PM											
04:30 PM	0	**2**	**2**	0	**22**	0	**22**	16	**1**	17	**41**
04:45 PM	**1**	1	2	1	10	0	11	**18**	1	**19**	32
05:00 PM	0	0	0	**2**	17	0	19	14	0	14	33
05:15 PM	0	1	1	0	12	0	12	8	0	8	21
Total Volume	1	4	5	3	61	0	64	56	2	58	127
% App. Total	20	80		4.7	95.3	0		96.6	3.4		
PHF	.250	.500	.625	.375	.693	.000	.727	.778	.500	.763	.774

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File Name : 1- US 441 & NW 167th Blvd
Site Code :
Start Date : 11/17/2020
Page No : 1

Groups Printed- Combined

	NW 167th Blvd Southbound				US 441 Westbound					US 441 Eastbound				
Start Time	Right	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Thru	Left	Peds	App. Total	Int. Total
04:15 PM	8	34	0	42	52	294	0	0	346	182	21	0	203	591
04:30 PM	7	33	0	40	60	318	1	0	379	205	25	0	230	649
04:45 PM	10	34	0	44	77	339	0	0	416	212	31	0	243	703
Total	25	101	0	126	189	951	1	0	1141	599	77	0	676	1943
05:00 PM	9	33	0	42	50	318	3	0	371	209	29	0	238	651
05:15 PM	11	39	0	50	90	396	0	0	486	212	35	0	247	783
05:30 PM	14	40	0	54	74	347	0	0	421	202	34	0	236	711
05:45 PM	8	48	0	56	67	291	0	0	358	177	32	0	209	623
Total	42	160	0	202	281	1352	3	0	1636	800	130	0	930	2768
06:00 PM	12	46	0	58	60	286	1	0	347	177	27	0	204	609
Grand Total	79	307	0	386	530	2589	5	0	3124	1576	234	0	1810	5320
Apprch %	20.5	79.5	0		17	82.9	0.2	0		87.1	12.9	0		
Total %	1.5	5.8	0	7.3	10	48.7	0.1	0	58.7	29.6	4.4	0	34	

	NW 167th Blvd Southbound			US 441 Westbound				US 441 Eastbound			
Start Time	Right	Left	App. Total	Right	Thru	Left	App. Total	Thru	Left	App. Total	Int. Total
Peak Hour Analysis From 04:15 PM to 06:00 PM - Peak 1 of 1											
Peak Hour for Entire Intersection Begins at 04:45 PM											
04:45 PM	10	34	44	77	339	0	416	**212**	31	243	703
05:00 PM	9	33	42	50	318	**3**	371	209	29	238	651
05:15 PM	11	39	50	**90**	**396**	0	**486**	212	**35**	**247**	**783**
05:30 PM	**14**	**40**	**54**	74	347	0	421	202	34	236	711
Total Volume	44	146	190	291	1400	3	1694	835	129	964	2848
% App. Total	23.2	76.8		17.2	82.6	0.2		86.6	13.4		
PHF	.786	.913	.880	.808	.884	.250	.871	.985	.921	.976	.909

TRUCK%	3/190 = 2%	50/1694 = 3%	50/964 = 5%
PEDS	0	0	0

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File Name : 2- US 441 & Median Cut West of I-75
Site Code :
Start Date : 11/17/2020
Page No : 1

Groups Printed- Cars

	US 441 Westbound				Driveway Northbound				US 441 Eastbound					
Start Time	Thru	Left	Peds	App. Total	Right	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
04:15 PM	328	16	0	344	1	7	0	8	4	192	0	0	196	548
04:30 PM	363	16	0	379	0	3	0	3	2	224	0	0	226	608
04:45 PM	382	16	0	398	1	12	0	13	1	214	1	0	216	627
Total	1073	48	0	1121	2	22	0	24	7	630	1	0	638	1783
			1				0					0		
05:00 PM	370	12	1	383	2	5	0	7	1	235	0	0	236	626
05:15 PM	452	11	0	463	3	5	0	8	1	240	0	0	241	712
05:30 PM	413	20	0	433	0	10	0	10	0	226	0	0	226	669
05:45 PM	329	13	0	342	1	8	0	9	0	244	0	0	244	595
Total	1564	56	1	1621	6	28	0	34	2	945	0	0	947	2602
06:00 PM	319	20	0	339	1	10	0	11	5	202	1	0	208	558
Grand Total	2956	124	1	3081	9	60	0	69	14	1777	2	0	1793	4943
Apprch %	95.9	4	0		13	87	0		0.8	99.1	0.1	0		
Total %	59.8	2.5	0	62.3	0.2	1.2	0	1.4	0.3	35.9	0	0	36.3	

	US 441 Westbound			Driveway Northbound			US 441 Eastbound				
Start Time	Thru	Left	App. Total	Right	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Analysis From 04:15 PM to 06:00 PM - Peak 1 of 1											
Peak Hour for Entire Intersection Begins at 04:45 PM											
04:45 PM	382	16	398	1	**12**	**13**	**1**	214	**1**	216	627
05:00 PM	370	12	382	2	5	7	1	235	0	236	625
05:15 PM	**452**	11	**463**	**3**	5	8	1	**240**	0	**241**	**712**
05:30 PM	413	**20**	433	0	10	10	0	226	0	226	669
Total Volume	1617	59	1676	6	32	38	3	915	1	919	2633
% App. Total	96.5	3.5		15.8	84.2		0.3	99.6	0.1		
PHF	.894	.738	.905	.500	.667	.731	.750	.953	.250	.953	.925

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File Name : 2- US 441 & Median Cut West of I-75 w Uturns
Site Code :
Start Date : 11/17/2020
Page No : 1

Groups Printed- Cars

	US 441 Westbound				Driveway Northbound				US 441 Eastbound				
Start Time	Thru	Left	U-Turn	App. Total	Right	Left	U-Turn	App. Total	Right	Thru	U-Turn	App. Total	Int. Total
04:15 PM	328	0	16	344	1	7	0	8	4	192	0	196	548
04:30 PM	363	1	15	379	0	3	0	3	2	224	0	226	608
04:45 PM	382	1	15	398	1	12	0	13	1	214	1	216	627
Total	1073	2	46	1121	2	22	0	24	7	630	1	638	1783
05:00 PM	370	0	12	382	2	5	0	7	1	235	0	236	625
05:15 PM	452	1	10	463	3	5	0	8	1	240	0	241	712
05:30 PM	413	1	19	433	0	10	0	10	0	226	0	226	669
05:45 PM	329	1	12	342	1	8	0	9	0	244	0	244	595
Total	1564	3	53	1620	6	28	0	34	2	945	0	947	2601
06:00 PM	319	0	20	339	1	10	0	11	5	202	1	208	558
Grand Total	2956	5	119	3080	9	60	0	69	14	1777	2	1793	4942
Apprch %	96	0.2	3.9		13	87	0		0.8	99.1	0.1		
Total %	59.8	0.1	2.4	62.3	0.2	1.2	0	1.4	0.3	36	0	36.3	

	US 441 Westbound				Driveway Northbound				US 441 Eastbound				
Start Time	Thru	Left	U-Turn	App. Total	Right	Left	U-Turn	App. Total	Right	Thru	U-Turn	App. Total	Int. Total
Peak Hour Analysis From 04:15 PM to 06:00 PM - Peak 1 of 1													
Peak Hour for Entire Intersection Begins at 04:45 PM													
04:45 PM	382	**1**	15	398	1	**12**	0	**13**	**1**	214	**1**	216	627
05:00 PM	370	0	12	382	2	5	0	7	1	235	0	236	625
05:15 PM	**452**	1	10	**463**	**3**	5	0	8	1	**240**	0	**241**	**712**
05:30 PM	413	1	**19**	433	0	10	0	10	0	226	0	226	669
Total Volume	1617	3	56	1676	6	32	0	38	3	915	1	919	2633
% App. Total	96.5	0.2	3.3		15.8	84.2	0		0.3	99.6	0.1		
PHF	.894	.750	.737	.905	.500	.667	.000	.731	.750	.953	.250	.953	.925

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File Name : 2- US 441 & Median Cut West of I-75 w Uturns
Site Code :
Start Date : 11/17/2020
Page No : 1

Groups Printed- Trucks

	US 441 Westbound				Driveway Northbound				US 441 Eastbound				
Start Time	Thru	Left	U-Turn	App. Total	Right	Left	U-Turn	App. Total	Right	Thru	U-Turn	App. Total	Int. Total
04:15 PM	13	0	1	14	0	0	0	0	0	8	0	8	22
04:30 PM	23	0	0	23	0	0	0	0	0	19	0	19	42
04:45 PM	11	0	0	11	0	0	0	0	0	18	0	18	29
Total	47	0	1	48	0	0	0	0	0	45	0	45	93
				54				0				53	
05:00 PM	19	0	0	19	0	0	0	0	0	13	0	13	32
05:15 PM	12	0	0	12	0	0	0	0	0	12	0	12	24
05:30 PM	11	0	1	12	0	0	0	0	0	10	0	10	22
05:45 PM	11	0	0	11	0	1	0	1	0	9	0	9	21
Total	53	0	1	54	0	1	0	1	0	44	0	44	99
06:00 PM	13	0	0	13	0	1	0	1	0	6	0	6	20
Grand Total	113	0	2	115	0	2	0	2	0	95	0	95	212
Apprch %	98.3	0	1.7		0	100	0		0	100	0		
Total %	53.3	0	0.9	54.2	0	0.9	0	0.9	0	44.8	0	44.8	

	US 441 Westbound				Driveway Northbound				US 441 Eastbound				
Start Time	Thru	Left	U-Turn	App. Total	Right	Left	U-Turn	App. Total	Right	Thru	U-Turn	App. Total	Int. Total
Peak Hour Analysis From 04:15 PM to 06:00 PM - Peak 1 of 1													
Peak Hour for Entire Intersection Begins at 04:30 PM													
04:30 PM	**23**	0	0	**23**	0	0	0	0	0	**19**	0	**19**	**42**
04:45 PM	11	0	0	11	0	0	0	0	0	18	0	18	29
05:00 PM	19	0	0	19	0	0	0	0	0	13	0	13	32
05:15 PM	12	0	0	12	0	0	0	0	0	12	0	12	24
Total Volume	65	0	0	65	0	0	0	0	0	62	0	62	127
% App. Total	100	0	0		0	0	0		0	100	0		
PHF	.707	.000	.000	.707	.000	.000	.000	.000	.000	.816	.000	.816	.756

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File Name : 2- US 441 & Median Cut West of I-75 w Uturns
Site Code :
Start Date : 11/17/2020
Page No : 1

Groups Printed- Cars - Trucks

	US 441 Westbound				Driveway Northbound				US 441 Eastbound				
Start Time	Thru	Left	U-Turn	App. Total	Right	Left	U-Turn	App. Total	Right	Thru	U-Turn	App. Total	Int. Total
04:15 PM	341	0	17	358	1	7	0	8	4	200	0	204	570
04:30 PM	386	1	15	402	0	3	0	3	2	243	0	245	650
04:45 PM	393	1	15	409	1	12	0	13	1	232	1	234	656
Total	1120	2	47	1169	2	22	0	24	7	675	1	683	1876
05:00 PM	389	0	12	401	2	5	0	7	1	248	0	249	657
05:15 PM	464	1	10	475	3	5	0	8	1	252	0	253	736
05:30 PM	424	1	20	445	0	10	0	10	0	236	0	236	691
05:45 PM	340	1	12	353	1	9	0	10	0	253	0	253	616
Total	1617	3	54	1674	6	29	0	35	2	989	0	991	2700
06:00 PM	332	0	20	352	1	11	0	12	5	208	1	214	578
Grand Total	3069	5	121	3195	9	62	0	71	14	1872	2	1888	5154
Apprch %	96.1	0.2	3.8		12.7	87.3	0		0.7	99.2	0.1		
Total %	59.5	0.1	2.3	62	0.2	1.2	0	1.4	0.3	36.3	0	36.6	
Cars	2956	5	119	3080	9	60	0	69	14	1777	2	1793	4942
% Cars	96.3	100	98.3	96.4	100	96.8	0	97.2	100	94.9	100	95	95.9
Trucks	113	0	2	115	0	2	0	2	0	95	0	95	212
% Trucks	3.7	0	1.7	3.6	0	3.2	0	2.8	0	5.1	0	5	4.1

	US 441 Westbound				Driveway Northbound				US 441 Eastbound				
Start Time	Thru	Left	U-Turn	App. Total	Right	Left	U-Turn	App. Total	Right	Thru	U-Turn	App. Total	Int. Total
Peak Hour Analysis From 04:15 PM to 06:00 PM - Peak 1 of 1													
Peak Hour for Entire Intersection Begins at 04:45 PM													
04:45 PM	393	**1**	15	409	1	**12**	0	**13**	**1**	232	**1**	234	656
05:00 PM	389	0	12	401	2	5	0	7	1	248	0	249	657
05:15 PM	**464**	1	10	**475**	**3**	5	0	8	1	**252**	0	**253**	**736**
05:30 PM	424	1	**20**	445	0	10	0	10	0	236	0	236	691
Total Volume	1670	3	57	1730	6	32	0	38	3	968	1	972	2740
% App. Total	96.5	0.2	3.3		15.8	84.2	0		0.3	99.6	0.1		
PHF	.900	.750	.713	.911	.500	.667	.000	.731	.750	.960	.250	.960	.931

TRUCK %	54/1730 = 3%	0/38 = 0%	53/972 = 5%
PEDS	1	0	0

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File Name : 3- US 441 & EB Left Turn to Mobil_Dunkin Donuts
Site Code :
Start Date : 11/17/2020
Page No : 1

Groups Printed- Cars

	Mobil Driveway Southbound					US 441 Westbound					Zaxby's Driveway Northbound					US 441 Eastbound					
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
04:15 PM	20	0	0	0	20	20	326	0	0	346	10	0	0	0	10	2	208	5	0	215	591
04:30 PM	20	0	0	0	20	17	360	0	0	377	6	0	0	0	6	11	222	3	0	236	639
04:45 PM	11	0	0	0	11	11	377	0	0	388	9	0	0	0	9	15	209	12	0	236	644
Total	51	0	0	0	51	48	1063	0	0	1111	25	0	0	0	25	28	639	20	0	687	1874
05:00 PM	19	0	0	0	19	17	358	0	0	375	7	0	0	0	7	9	237	4	0	250	651
05:15 PM	18	0	0	0	18	12	434	0	0	446	11	0	0	0	11	12	233	3	0	248	723
05:30 PM	14	0	0	0	14	11	439	0	0	450	10	0	0	0	10	13	228	5	0	246	720
05:45 PM	11	0	1	0	12	9	324	0	0	333	10	0	0	0	10	14	221	13	0	248	603
Total	62	0	1	0	63	49	1555	0	0	1604	38	0	0	0	38	48	919	25	0	992	2697
06:00 PM	13	0	0	0	13	11	310	0	0	321	6	0	0	0	6	15	209	10	0	234	574
Grand Total	126	0	1	0	127	108	2928	0	0	3036	69	0	0	0	69	91	1767	55	0	1913	5145
Apprch %	99.2	0	0.8	0		3.6	96.4	0	0		100	0	0	0		4.8	92.4	2.9	0		
Total %	2.4	0	0	0	2.5	2.1	56.9	0	0	59	1.3	0	0	0	1.3	1.8	34.3	1.1	0	37.2	

	Mobil Driveway Southbound				US 441 Westbound				Zaxby's Driveway Northbound				US 441 Eastbound				
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Analysis From 04:15 PM to 06:00 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:45 PM																	
04:45 PM	11	0	0	11	11	377	0	388	9	0	0	9	**15**	209	**12**	236	644
05:00 PM	**19**	0	0	**19**	**17**	358	0	375	7	0	0	7	9	**237**	4	**250**	651
05:15 PM	18	0	0	18	12	434	0	446	**11**	0	0	**11**	12	233	3	248	**723**
05:30 PM	14	0	0	14	11	**439**	0	**450**	10	0	0	10	13	228	5	246	720
Total Volume	62	0	0	62	51	1608	0	1659	37	0	0	37	49	907	24	980	2738
% App. Total	100	0	0		3.1	96.9	0		100	0	0		5	92.6	2.4		
PHF	.816	.000	.000	.816	.750	.916	.000	.922	.841	.000	.000	.841	.817	.957	.500	.980	.947

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File Name : 3- US 441 & EB Left Turn to Mobil_Dunkin Donuts w Uturns
Site Code :
Start Date : 11/17/2020
Page No : 1

Groups Printed- Cars

	Mobil Driveway Southbound					US 441 Westbound					Zaxby's Driveway Northbound					US 441 Eastbound					
Start Time	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Int. Total
04:15 PM	20	0	0	0	20	20	326	0	0	346	10	0	0	0	10	2	208	2	3	215	591
04:30 PM	20	0	0	0	20	17	360	0	0	377	6	0	0	0	6	11	222	2	1	236	639
04:45 PM	11	0	0	0	11	11	377	0	0	388	9	0	0	0	9	15	209	5	7	236	644
Total	51	0	0	0	51	48	1063	0	0	1111	25	0	0	0	25	28	639	9	11	687	1874
05:00 PM	19	0	0	0	19	17	358	0	0	375	7	0	0	0	7	9	237	3	1	250	651
05:15 PM	18	0	0	0	18	12	434	0	0	446	11	0	0	0	11	12	233	2	1	248	723
05:30 PM	14	0	0	0	14	11	439	0	0	450	10	0	0	0	10	13	228	3	2	246	720
05:45 PM	11	0	0	1	12	9	324	0	0	333	10	0	0	0	10	14	221	3	10	248	603
Total	62	0	0	1	63	49	1555	0	0	1604	38	0	0	0	38	48	919	11	14	992	2697
06:00 PM	13	0	0	0	13	11	310	0	0	321	6	0	0	0	6	15	209	3	7	234	574
Grand Total	126	0	0	1	127	108	2928	0	0	3036	69	0	0	0	69	91	1767	23	32	1913	5145
Apprch %	99.2	0	0	0.8		3.6	96.4	0	0		100	0	0	0		4.8	92.4	1.2	1.7		
Total %	2.4	0	0	0	2.5	2.1	56.9	0	0	59	1.3	0	0	0	1.3	1.8	34.3	0.4	0.6	37.2	

	Mobil Driveway Southbound					US 441 Westbound					Zaxby's Driveway Northbound					US 441 Eastbound					
Start Time	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Int. Total
Peak Hour Analysis From 04:15 PM to 06:00 PM - Peak 1 of 1																					
Peak Hour for Entire Intersection Begins at 04:45 PM																					
04:45 PM	11	0	0	0	11	11	377	0	0	388	9	0	0	0	9	**15**	209	**5**	**7**	236	644
05:00 PM	**19**	0	0	0	**19**	**17**	358	0	0	375	7	0	0	0	7	9	**237**	3	1	**250**	651
05:15 PM	18	0	0	0	18	12	434	0	0	446	**11**	0	0	0	**11**	12	233	2	1	248	**723**
05:30 PM	14	0	0	0	14	11	**439**	0	0	**450**	10	0	0	0	10	13	228	3	2	246	720
Total Volume	62	0	0	0	62	51	1608	0	0	1659	37	0	0	0	37	49	907	13	11	980	2738
% App. Total	100	0	0	0		3.1	96.9	0	0		100	0	0	0		5	92.6	1.3	1.1		
PHF	.816	.000	.000	.000	.816	.750	.916	.000	.000	.922	.841	.000	.000	.000	.841	.817	.957	.650	.393	.980	.947

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File Name : 3- US 441 & EB Left Turn to Mobil_Dunkin Donuts w Uturns
Site Code :
Start Date : 11/17/2020
Page No : 1

Groups Printed- Trucks

	Mobil Driveway Southbound					US 441 Westbound					Zaxby's Driveway Northbound					US 441 Eastbound					
Start Time	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Int. Total
04:15 PM	0	0	0	0	0	2	14	0	0	16	0	0	0	0	0	0	8	0	0	8	24
04:30 PM	1	0	0	0	1	0	23	0	0	23	0	0	0	0	0	0	16	0	0	16	40
04:45 PM	0	0	0	0	0	0	11	0	0	11	0	0	0	0	0	0	19	0	0	19	30
Total	1	0	0	0	1	2	48	0	0	50	0	0	0	0	0	0	43	0	0	43	94
					2					50					0					53	
05:00 PM	1	0	0	0	1	0	18	0	0	18	0	0	0	0	0	0	12	0	0	12	31
05:15 PM	1	0	0	0	1	0	11	0	0	11	0	0	0	0	0	0	12	0	0	12	24
05:30 PM	0	0	0	0	0	0	10	0	0	10	0	0	0	0	0	0	10	0	0	10	20
05:45 PM	0	0	0	0	0	0	12	0	0	12	0	0	0	0	0	0	10	0	0	10	22
Total	2	0	0	0	2	0	51	0	0	51	0	0	0	0	0	0	44	0	0	44	97
06:00 PM	0	0	0	0	0	0	13	0	0	13	0	0	0	0	0	0	6	0	0	6	19
Grand Total	3	0	0	0	3	2	112	0	0	114	0	0	0	0	0	0	93	0	0	93	210
Apprch %	100	0	0	0		1.8	98.2	0	0		0	0	0	0		0	100	0	0		
Total %	1.4	0	0	0	1.4	1	53.3	0	0	54.3	0	0	0	0	0	0	44.3	0	0	44.3	

	Mobil Driveway Southbound					US 441 Westbound					Zaxby's Driveway Northbound					US 441 Eastbound					
Start Time	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Int. Total
Peak Hour Analysis From 04:15 PM to 06:00 PM - Peak 1 of 1																					
Peak Hour for Entire Intersection Begins at 04:15 PM																					
04:15 PM	0	0	0	0	0	**2**	14	0	0	16	0	0	0	0	0	0	8	0	0	8	24
04:30 PM	**1**	0	0	0	**1**	0	**23**	0	0	**23**	0	0	0	0	0	0	16	0	0	16	**40**
04:45 PM	0	0	0	0	0	0	11	0	0	11	0	0	0	0	0	0	**19**	0	0	**19**	30
05:00 PM	1	0	0	0	1	0	18	0	0	18	0	0	0	0	0	0	12	0	0	12	31
Total Volume	2	0	0	0	2	2	66	0	0	68	0	0	0	0	0	0	55	0	0	55	125
% App. Total	100	0	0	0		2.9	97.1	0	0		0	0	0	0		0	100	0	0		
PHF	.500	.000	.000	.000	.500	.250	.717	.000	.000	.739	.000	.000	.000	.000	.000	.000	.724	.000	.000	.724	.781

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File Name : 3- US 441 & EB Left Turn to Mobil_Dunkin Donuts w Uturns
Site Code :
Start Date : 11/17/2020
Page No : 1

Groups Printed- Cars - Trucks

	Mobil Driveway Southbound					US 441 Westbound					Zaxby's Driveway Northbound					US 441 Eastbound					
Start Time	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Int. Total
04:15 PM	20	0	0	0	20	22	340	0	0	362	10	0	0	0	10	2	216	2	3	223	615
04:30 PM	21	0	0	0	21	17	383	0	0	400	6	0	0	0	6	11	238	2	1	252	679
04:45 PM	11	0	0	0	11	11	388	0	0	399	9	0	0	0	9	15	228	5	7	255	674
Total	52	0	0	0	52	50	1111	0	0	1161	25	0	0	0	25	28	682	9	11	730	1968
05:00 PM	20	0	0	0	20	17	376	0	0	393	7	0	0	0	7	9	249	3	1	262	682
05:15 PM	19	0	0	0	19	12	445	0	0	457	11	0	0	0	11	12	245	2	1	260	747
05:30 PM	14	0	0	0	14	11	449	0	0	460	10	0	0	0	10	13	238	3	2	256	740
05:45 PM	11	0	0	1	12	9	336	0	0	345	10	0	0	0	10	14	231	3	10	258	625
Total	64	0	0	1	65	49	1606	0	0	1655	38	0	0	0	38	48	963	11	14	1036	2794
06:00 PM	13	0	0	0	13	11	323	0	0	334	6	0	0	0	6	15	215	3	7	240	593
Grand Total	129	0	0	1	130	110	3040	0	0	3150	69	0	0	0	69	91	1860	23	32	2006	5355
Apprch %	99.2	0	0	0.8		3.5	96.5	0	0		100	0	0	0		4.5	92.7	1.1	1.6		
Total %	2.4	0	0	0	2.4	2.1	56.8	0	0	58.8	1.3	0	0	0	1.3	1.7	34.7	0.4	0.6	37.5	
Cars	126	0	0	1	127	108	2928	0	0	3036	69	0	0	0	69	91	1767	23	32	1913	5145
% Cars	97.7	0	0	100	97.7	98.2	96.3	0	0	96.4	100	0	0	0	100	100	95	100	100	95.4	96.1
Trucks	3	0	0	0	3	2	112	0	0	114	0	0	0	0	0	0	93	0	0	93	210
% Trucks	2.3	0	0	0	2.3	1.8	3.7	0	0	3.6	0	0	0	0	0	0	5	0	0	4.6	3.9

	Mobil Driveway Southbound					US 441 Westbound					Zaxby's Driveway Northbound					US 441 Eastbound					
Start Time	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Int. Total
Peak Hour Analysis From 04:15 PM to 06:00 PM - Peak 1 of 1																					
Peak Hour for Entire Intersection Begins at 04:45 PM																					
04:45 PM	11	0	0	0	11	11	388	0	0	399	9	0	0	0	9	**15**	228	**5**	**7**	255	674
05:00 PM	**20**	0	0	0	**20**	**17**	376	0	0	393	7	0	0	0	7	9	**249**	3	1	**262**	682
05:15 PM	19	0	0	0	19	12	445	0	0	457	**11**	0	0	0	**11**	12	245	2	1	260	**747**
05:30 PM	14	0	0	0	14	11	**449**	0	0	**460**	10	0	0	0	10	13	238	3	2	256	740
Total Volume	64	0	0	0	64	51	1658	0	0	1709	37	0	0	0	37	49	960	13	11	1033	2843
% App. Total	100	0	0	0		3	97	0	0		100	0	0	0		4.7	92.9	1.3	1.1		
PHF	.800	.000	.000	.000	.800	.750	.923	.000	.000	.929	.841	.000	.000	.000	.841	.817	.964	.650	.393	.986	.951

Truck%	2/64 = 3%	50/1709 = 3%	0/37= 0%	53/1033 = 5%
Peds	0	0	0	0

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File Name : 2- US 441 & Median Cut West of I-75 w Uturns
Site Code :
Start Date : 11/17/2020
Page No : 1

Groups Printed- Cars

	US 441 Westbound				Driveway Northbound				US 441 Eastbound				
Start Time	Thru	Left	U-Turn	App. Total	Right	Left	U-Turn	App. Total	Right	Thru	U-Turn	App. Total	Int. Total
04:15 PM	328	0	16	344	1	7	0	8	4	192	0	196	548
04:30 PM	363	1	15	379	0	3	0	3	2	224	0	226	608
04:45 PM	382	1	15	398	1	12	0	13	1	214	1	216	627
Total	1073	2	46	1121	2	22	0	24	7	630	1	638	1783
05:00 PM	370	0	12	382	2	5	0	7	1	235	0	236	625
05:15 PM	452	1	10	463	3	5	0	8	1	240	0	241	712
05:30 PM	413	1	19	433	0	10	0	10	0	226	0	226	669
05:45 PM	329	1	12	342	1	8	0	9	0	244	0	244	595
Total	1564	3	53	1620	6	28	0	34	2	945	0	947	2601
06:00 PM	319	0	20	339	1	10	0	11	5	202	1	208	558
Grand Total	2956	5	119	3080	9	60	0	69	14	1777	2	1793	4942
Apprch %	96	0.2	3.9		13	87	0		0.8	99.1	0.1		
Total %	59.8	0.1	2.4	62.3	0.2	1.2	0	1.4	0.3	36	0	36.3	

	US 441 Westbound				Driveway Northbound				US 441 Eastbound				
Start Time	Thru	Left	U-Turn	App. Total	Right	Left	U-Turn	App. Total	Right	Thru	U-Turn	App. Total	Int. Total
Peak Hour Analysis From 04:15 PM to 06:00 PM - Peak 1 of 1													
Peak Hour for Entire Intersection Begins at 04:45 PM													
04:45 PM	382	**1**	15	398	1	**12**	0	**13**	**1**	214	**1**	216	627
05:00 PM	370	0	12	382	2	5	0	7	1	235	0	236	625
05:15 PM	**452**	1	10	**463**	**3**	5	0	8	1	**240**	0	**241**	**712**
05:30 PM	413	1	**19**	433	0	10	0	10	0	226	0	226	669
Total Volume	1617	3	56	1676	6	32	0	38	3	915	1	919	2633
% App. Total	96.5	0.2	3.3		15.8	84.2	0		0.3	99.6	0.1		
PHF	.894	.750	.737	.905	.500	.667	.000	.731	.750	.953	.250	.953	.925

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File Name : 2- US 441 & Median Cut West of I-75 w Uturns
Site Code :
Start Date : 11/17/2020
Page No : 1

Groups Printed- Trucks

	US 441 Westbound				Driveway Northbound				US 441 Eastbound				
Start Time	Thru	Left	U-Turn	App. Total	Right	Left	U-Turn	App. Total	Right	Thru	U-Turn	App. Total	Int. Total
04:15 PM	13	0	1	14	0	0	0	0	0	8	0	8	22
04:30 PM	23	0	0	23	0	0	0	0	0	19	0	19	42
04:45 PM	11	0	0	11	0	0	0	0	0	18	0	18	29
Total	47	0	1	48	0	0	0	0	0	45	0	45	93
				54				0				53	
05:00 PM	19	0	0	19	0	0	0	0	0	13	0	13	32
05:15 PM	12	0	0	12	0	0	0	0	0	12	0	12	24
05:30 PM	11	0	1	12	0	0	0	0	0	10	0	10	22
05:45 PM	11	0	0	11	0	1	0	1	0	9	0	9	21
Total	53	0	1	54	0	1	0	1	0	44	0	44	99
06:00 PM	13	0	0	13	0	1	0	1	0	6	0	6	20
Grand Total	113	0	2	115	0	2	0	2	0	95	0	95	212
Apprch %	98.3	0	1.7		0	100	0		0	100	0		
Total %	53.3	0	0.9	54.2	0	0.9	0	0.9	0	44.8	0	44.8	

	US 441 Westbound				Driveway Northbound				US 441 Eastbound				
Start Time	Thru	Left	U-Turn	App. Total	Right	Left	U-Turn	App. Total	Right	Thru	U-Turn	App. Total	Int. Total
Peak Hour Analysis From 04:15 PM to 06:00 PM - Peak 1 of 1													
Peak Hour for Entire Intersection Begins at 04:30 PM													
04:30 PM	**23**	0	0	**23**	0	0	0	0	0	**19**	0	**19**	**42**
04:45 PM	11	0	0	11	0	0	0	0	0	18	0	18	29
05:00 PM	19	0	0	19	0	0	0	0	0	13	0	13	32
05:15 PM	12	0	0	12	0	0	0	0	0	12	0	12	24
Total Volume	65	0	0	65	0	0	0	0	0	62	0	62	127
% App. Total	100	0	0		0	0	0		0	100	0		
PHF	.707	.000	.000	.707	.000	.000	.000	.000	.000	.816	.000	.816	.756

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File Name : 2- US 441 & Median Cut West of I-75 w Uturns
Site Code :
Start Date : 11/17/2020
Page No : 1

Groups Printed- Cars - Trucks

	US 441 Westbound				Driveway Northbound				US 441 Eastbound				
Start Time	Thru	Left	U-Turn	App. Total	Right	Left	U-Turn	App. Total	Right	Thru	U-Turn	App. Total	Int. Total
04:15 PM	341	0	17	358	1	7	0	8	4	200	0	204	570
04:30 PM	386	1	15	402	0	3	0	3	2	243	0	245	650
04:45 PM	393	1	15	409	1	12	0	13	1	232	1	234	656
Total	1120	2	47	1169	2	22	0	24	7	675	1	683	1876
05:00 PM	389	0	12	401	2	5	0	7	1	248	0	249	657
05:15 PM	464	1	10	475	3	5	0	8	1	252	0	253	736
05:30 PM	424	1	20	445	0	10	0	10	0	236	0	236	691
05:45 PM	340	1	12	353	1	9	0	10	0	253	0	253	616
Total	1617	3	54	1674	6	29	0	35	2	989	0	991	2700
06:00 PM	332	0	20	352	1	11	0	12	5	208	1	214	578
Grand Total	3069	5	121	3195	9	62	0	71	14	1872	2	1888	5154
Apprch %	96.1	0.2	3.8		12.7	87.3	0		0.7	99.2	0.1		
Total %	59.5	0.1	2.3	62	0.2	1.2	0	1.4	0.3	36.3	0	36.6	
Cars	2956	5	119	3080	9	60	0	69	14	1777	2	1793	4942
% Cars	96.3	100	98.3	96.4	100	96.8	0	97.2	100	94.9	100	95	95.9
Trucks	113	0	2	115	0	2	0	2	0	95	0	95	212
% Trucks	3.7	0	1.7	3.6	0	3.2	0	2.8	0	5.1	0	5	4.1

	US 441 Westbound				Driveway Northbound				US 441 Eastbound				
Start Time	Thru	Left	U-Turn	App. Total	Right	Left	U-Turn	App. Total	Right	Thru	U-Turn	App. Total	Int. Total
Peak Hour Analysis From 04:15 PM to 06:00 PM - Peak 1 of 1													
Peak Hour for Entire Intersection Begins at 04:45 PM													
04:45 PM	393	**1**	15	409	1	**12**	0	**13**	**1**	232	**1**	234	656
05:00 PM	389	0	12	401	2	5	0	7	1	248	0	249	657
05:15 PM	**464**	1	10	**475**	**3**	5	0	8	1	**252**	0	**253**	**736**
05:30 PM	424	1	**20**	445	0	10	0	10	0	236	0	236	691
Total Volume	1670	3	57	1730	6	32	0	38	3	968	1	972	2740
% App. Total	96.5	0.2	3.3		15.8	84.2	0		0.3	99.6	0.1		
PHF	.900	.750	.713	.911	.500	.667	.000	.731	.750	.960	.250	.960	.931

TRUCK %	54/1730 = 3%	0/38 = 0%	53/972 = 5%
PEDS	1	0	0

Distribution Westbound = 1670/(1670+968) = 63%

Eastbound = 968/(1670+968) = 37%

Volumes for COVID-19 Factor comparison 1670 + 3 + 57 + 6 + 968 =2,704

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File Name : 2- US 441 & Median Cut West of I-75
Site Code :
Start Date : 11/17/2020
Page No : 1

Groups Printed- Cars

	US 441 Westbound				Driveway Northbound				US 441 Eastbound					
Start Time	Thru	Left	Peds	App. Total	Right	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
04:15 PM	328	16	0	344	1	7	0	8	4	192	0	0	196	548
04:30 PM	363	16	0	379	0	3	0	3	2	224	0	0	226	608
04:45 PM	382	16	0	398	1	12	0	13	1	214	1	0	216	627
Total	1073	48	0	1121	2	22	0	24	7	630	1	0	638	1783
			1				0					0		
05:00 PM	370	12	1	383	2	5	0	7	1	235	0	0	236	626
05:15 PM	452	11	0	463	3	5	0	8	1	240	0	0	241	712
05:30 PM	413	20	0	433	0	10	0	10	0	226	0	0	226	669
05:45 PM	329	13	0	342	1	8	0	9	0	244	0	0	244	595
Total	1564	56	1	1621	6	28	0	34	2	945	0	0	947	2602
06:00 PM	319	20	0	339	1	10	0	11	5	202	1	0	208	558
Grand Total	2956	124	1	3081	9	60	0	69	14	1777	2	0	1793	4943
Apprch %	95.9	4	0		13	87	0		0.8	99.1	0.1	0		
Total %	59.8	2.5	0	62.3	0.2	1.2	0	1.4	0.3	35.9	0	0	36.3	

	US 441 Westbound			Driveway Northbound			US 441 Eastbound				
Start Time	Thru	Left	App. Total	Right	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Analysis From 04:15 PM to 06:00 PM - Peak 1 of 1											
Peak Hour for Entire Intersection Begins at 04:45 PM											
04:45 PM	382	16	398	1	**12**	**13**	**1**	214	**1**	216	627
05:00 PM	370	12	382	2	5	7	1	235	0	236	625
05:15 PM	**452**	11	**463**	**3**	5	8	1	**240**	0	**241**	**712**
05:30 PM	413	**20**	433	0	10	10	0	226	0	226	669
Total Volume	1617	59	1676	6	32	38	3	915	1	919	2633
% App. Total	96.5	3.5		15.8	84.2		0.3	99.6	0.1		
PHF	.894	.738	.905	.500	.667	.731	.750	.953	.250	.953	.925

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File Name : 3- US 441 & EB Left Turn to Mobil_Dunkin Donuts w Uturns
Site Code :
Start Date : 11/17/2020
Page No : 1

Groups Printed- Cars

	Mobil Driveway Southbound					US 441 Westbound					Zaxby's Driveway Northbound					US 441 Eastbound					
Start Time	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Int. Total
04:15 PM	20	0	0	0	20	20	326	0	0	346	10	0	0	0	10	2	208	2	3	215	591
04:30 PM	20	0	0	0	20	17	360	0	0	377	6	0	0	0	6	11	222	2	1	236	639
04:45 PM	11	0	0	0	11	11	377	0	0	388	9	0	0	0	9	15	209	5	7	236	644
Total	51	0	0	0	51	48	1063	0	0	1111	25	0	0	0	25	28	639	9	11	687	1874
05:00 PM	19	0	0	0	19	17	358	0	0	375	7	0	0	0	7	9	237	3	1	250	651
05:15 PM	18	0	0	0	18	12	434	0	0	446	11	0	0	0	11	12	233	2	1	248	723
05:30 PM	14	0	0	0	14	11	439	0	0	450	10	0	0	0	10	13	228	3	2	246	720
05:45 PM	11	0	0	1	12	9	324	0	0	333	10	0	0	0	10	14	221	3	10	248	603
Total	62	0	0	1	63	49	1555	0	0	1604	38	0	0	0	38	48	919	11	14	992	2697
06:00 PM	13	0	0	0	13	11	310	0	0	321	6	0	0	0	6	15	209	3	7	234	574
Grand Total	126	0	0	1	127	108	2928	0	0	3036	69	0	0	0	69	91	1767	23	32	1913	5145
Apprch %	99.2	0	0	0.8		3.6	96.4	0	0		100	0	0	0		4.8	92.4	1.2	1.7		
Total %	2.4	0	0	0	2.5	2.1	56.9	0	0	59	1.3	0	0	0	1.3	1.8	34.3	0.4	0.6	37.2	

	Mobil Driveway Southbound					US 441 Westbound					Zaxby's Driveway Northbound					US 441 Eastbound					
Start Time	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Int. Total
Peak Hour Analysis From 04:15 PM to 06:00 PM - Peak 1 of 1																					
Peak Hour for Entire Intersection Begins at 04:45 PM																					
04:45 PM	11	0	0	0	11	11	377	0	0	388	9	0	0	0	9	**15**	209	**5**	**7**	236	644
05:00 PM	**19**	0	0	0	**19**	**17**	358	0	0	375	7	0	0	0	7	9	**237**	3	1	**250**	651
05:15 PM	18	0	0	0	18	12	434	0	0	446	**11**	0	0	0	**11**	12	233	2	1	248	**723**
05:30 PM	14	0	0	0	14	11	**439**	0	0	**450**	10	0	0	0	10	13	228	3	2	246	720
Total Volume	62	0	0	0	62	51	1608	0	0	1659	37	0	0	0	37	49	907	13	11	980	2738
% App. Total	100	0	0	0		3.1	96.9	0	0		100	0	0	0		5	92.6	1.3	1.1		
PHF	.816	.000	.000	.000	.816	.750	.916	.000	.000	.922	.841	.000	.000	.000	.841	.817	.957	.650	.393	.980	.947

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File Name : 3- US 441 & EB Left Turn to Mobil_Dunkin Donuts w Uturns
Site Code :
Start Date : 11/17/2020
Page No : 1

Groups Printed- Trucks

	Mobil Driveway Southbound					US 441 Westbound					Zaxby's Driveway Northbound					US 441 Eastbound					
Start Time	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Int. Total
04:15 PM	0	0	0	0	0	2	14	0	0	16	0	0	0	0	0	0	8	0	0	8	24
04:30 PM	1	0	0	0	1	0	23	0	0	23	0	0	0	0	0	0	16	0	0	16	40
04:45 PM	0	0	0	0	0	0	11	0	0	11	0	0	0	0	0	0	19	0	0	19	30
Total	1	0	0	0	1	2	48	0	0	50	0	0	0	0	0	0	43	0	0	43	94
					2					50					0					53	
05:00 PM	1	0	0	0	1	0	18	0	0	18	0	0	0	0	0	0	12	0	0	12	31
05:15 PM	1	0	0	0	1	0	11	0	0	11	0	0	0	0	0	0	12	0	0	12	24
05:30 PM	0	0	0	0	0	0	10	0	0	10	0	0	0	0	0	0	10	0	0	10	20
05:45 PM	0	0	0	0	0	0	12	0	0	12	0	0	0	0	0	0	10	0	0	10	22
Total	2	0	0	0	2	0	51	0	0	51	0	0	0	0	0	0	44	0	0	44	97
06:00 PM	0	0	0	0	0	0	13	0	0	13	0	0	0	0	0	0	6	0	0	6	19
Grand Total	3	0	0	0	3	2	112	0	0	114	0	0	0	0	0	0	93	0	0	93	210
Apprch %	100	0	0	0		1.8	98.2	0	0		0	0	0	0		0	100	0	0		
Total %	1.4	0	0	0	1.4	1	53.3	0	0	54.3	0	0	0	0	0	0	44.3	0	0	44.3	

	Mobil Driveway Southbound					US 441 Westbound					Zaxby's Driveway Northbound					US 441 Eastbound					
Start Time	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Int. Total
Peak Hour Analysis From 04:15 PM to 06:00 PM - Peak 1 of 1																					
Peak Hour for Entire Intersection Begins at 04:15 PM																					
04:15 PM	0	0	0	0	0	**2**	14	0	0	16	0	0	0	0	0	0	8	0	0	8	24
04:30 PM	**1**	0	0	0	**1**	0	**23**	0	0	**23**	0	0	0	0	0	0	16	0	0	16	**40**
04:45 PM	0	0	0	0	0	0	11	0	0	11	0	0	0	0	0	0	**19**	0	0	**19**	30
05:00 PM	1	0	0	0	1	0	18	0	0	18	0	0	0	0	0	0	12	0	0	12	31
Total Volume	2	0	0	0	2	2	66	0	0	68	0	0	0	0	0	0	55	0	0	55	125
% App. Total	100	0	0	0		2.9	97.1	0	0		0	0	0	0		0	100	0	0		
PHF	.500	.000	.000	.000	.500	.250	.717	.000	.000	.739	.000	.000	.000	.000	.000	.000	.724	.000	.000	.724	.781

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File Name : 3- US 441 & EB Left Turn to Mobil_Dunkin Donuts w Uturns
Site Code :
Start Date : 11/17/2020
Page No : 1

Groups Printed- Cars - Trucks

	Mobil Driveway Southbound					US 441 Westbound					Zaxby's Driveway Northbound					US 441 Eastbound					
Start Time	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Int. Total
04:15 PM	20	0	0	0	20	22	340	0	0	362	10	0	0	0	10	2	216	2	3	223	615
04:30 PM	21	0	0	0	21	17	383	0	0	400	6	0	0	0	6	11	238	2	1	252	679
04:45 PM	11	0	0	0	11	11	388	0	0	399	9	0	0	0	9	15	228	5	7	255	674
Total	52	0	0	0	52	50	1111	0	0	1161	25	0	0	0	25	28	682	9	11	730	1968
05:00 PM	20	0	0	0	20	17	376	0	0	393	7	0	0	0	7	9	249	3	1	262	682
05:15 PM	19	0	0	0	19	12	445	0	0	457	11	0	0	0	11	12	245	2	1	260	747
05:30 PM	14	0	0	0	14	11	449	0	0	460	10	0	0	0	10	13	238	3	2	256	740
05:45 PM	11	0	0	1	12	9	336	0	0	345	10	0	0	0	10	14	231	3	10	258	625
Total	64	0	0	1	65	49	1606	0	0	1655	38	0	0	0	38	48	963	11	14	1036	2794
06:00 PM	13	0	0	0	13	11	323	0	0	334	6	0	0	0	6	15	215	3	7	240	593
Grand Total	129	0	0	1	130	110	3040	0	0	3150	69	0	0	0	69	91	1860	23	32	2006	5355
Apprch %	99.2	0	0	0.8		3.5	96.5	0	0		100	0	0	0		4.5	92.7	1.1	1.6		
Total %	2.4	0	0	0	2.4	2.1	56.8	0	0	58.8	1.3	0	0	0	1.3	1.7	34.7	0.4	0.6	37.5	
Cars	126	0	0	1	127	108	2928	0	0	3036	69	0	0	0	69	91	1767	23	32	1913	5145
% Cars	97.7	0	0	100	97.7	98.2	96.3	0	0	96.4	100	0	0	0	100	100	95	100	100	95.4	96.1
Trucks	3	0	0	0	3	2	112	0	0	114	0	0	0	0	0	0	93	0	0	93	210
% Trucks	2.3	0	0	0	2.3	1.8	3.7	0	0	3.6	0	0	0	0	0	0	5	0	0	4.6	3.9

	Mobil Driveway Southbound					US 441 Westbound					Zaxby's Driveway Northbound					US 441 Eastbound					
Start Time	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Int. Total
Peak Hour Analysis From 04:15 PM to 06:00 PM - Peak 1 of 1																					
Peak Hour for Entire Intersection Begins at 04:45 PM																					
04:45 PM	11	0	0	0	11	11	388	0	0	399	9	0	0	0	9	**15**	228	**5**	**7**	255	674
05:00 PM	**20**	0	0	0	**20**	**17**	376	0	0	393	7	0	0	0	7	9	**249**	3	1	**262**	682
05:15 PM	19	0	0	0	19	12	445	0	0	457	**11**	0	0	0	**11**	12	245	2	1	260	**747**
05:30 PM	14	0	0	0	14	11	**449**	0	0	**460**	10	0	0	0	10	13	238	3	2	256	740
Total Volume	64	0	0	0	64	51	1658	0	0	1709	37	0	0	0	37	49	960	13	11	1033	2843
% App. Total	100	0	0	0		3	97	0	0		100	0	0	0		4.7	92.9	1.3	1.1		
PHF	.800	.000	.000	.000	.800	.750	.923	.000	.000	.929	.841	.000	.000	.000	.841	.817	.964	.650	.393	.986	.951

Truck%	2/64 = 3%	50/1709 = 3%	0/37= 0%	53/1033 = 5%
Peds	0	0	0	0

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File Name : 3- US 441 & EB Left Turn to Mobil_Dunkin Donuts
Site Code :
Start Date : 11/17/2020
Page No : 1

Groups Printed- Cars

	Mobil Driveway Southbound					US 441 Westbound					Zaxby's Driveway Northbound					US 441 Eastbound					
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
04:15 PM	20	0	0	0	20	20	326	0	0	346	10	0	0	0	10	2	208	5	0	215	591
04:30 PM	20	0	0	0	20	17	360	0	0	377	6	0	0	0	6	11	222	3	0	236	639
04:45 PM	11	0	0	0	11	11	377	0	0	388	9	0	0	0	9	15	209	12	0	236	644
Total	51	0	0	0	51	48	1063	0	0	1111	25	0	0	0	25	28	639	20	0	687	1874
05:00 PM	19	0	0	0	19	17	358	0	0	375	7	0	0	0	7	9	237	4	0	250	651
05:15 PM	18	0	0	0	18	12	434	0	0	446	11	0	0	0	11	12	233	3	0	248	723
05:30 PM	14	0	0	0	14	11	439	0	0	450	10	0	0	0	10	13	228	5	0	246	720
05:45 PM	11	0	1	0	12	9	324	0	0	333	10	0	0	0	10	14	221	13	0	248	603
Total	62	0	1	0	63	49	1555	0	0	1604	38	0	0	0	38	48	919	25	0	992	2697
06:00 PM	13	0	0	0	13	11	310	0	0	321	6	0	0	0	6	15	209	10	0	234	574
Grand Total	126	0	1	0	127	108	2928	0	0	3036	69	0	0	0	69	91	1767	55	0	1913	5145
Apprch %	99.2	0	0.8	0		3.6	96.4	0	0		100	0	0	0		4.8	92.4	2.9	0		
Total %	2.4	0	0	0	2.5	2.1	56.9	0	0	59	1.3	0	0	0	1.3	1.8	34.3	1.1	0	37.2	

	Mobil Driveway Southbound				US 441 Westbound				Zaxby's Driveway Northbound				US 441 Eastbound				
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Analysis From 04:15 PM to 06:00 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:45 PM																	
04:45 PM	11	0	0	11	11	377	0	388	9	0	0	9	**15**	209	**12**	236	644
05:00 PM	**19**	0	0	**19**	**17**	358	0	375	7	0	0	7	9	**237**	4	**250**	651
05:15 PM	18	0	0	18	12	434	0	446	**11**	0	0	**11**	12	233	3	248	**723**
05:30 PM	14	0	0	14	11	**439**	0	**450**	10	0	0	10	13	228	5	246	720
Total Volume	62	0	0	62	51	1608	0	1659	37	0	0	37	49	907	24	980	2738
% App. Total	100	0	0		3.1	96.9	0		100	0	0		5	92.6	2.4		
PHF	.816	.000	.000	.816	.750	.916	.000	.922	.841	.000	.000	.841	.817	.957	.500	.980	.947

2019 PEAK SEASON FACTOR CATEGORY REPORT - REPORT TYPE: ALL

CATEGORY: 2600 ALACHUA COUNTYWIDE

MOCF: 0.97

WEEK	DATES	SF	PSCF
1	01/01/2019 - 01/05/2019	1.05	1.08
2	01/06/2019 - 01/12/2019	1.04	1.07
3	01/13/2019 - 01/19/2019	1.03	1.06
4	01/20/2019 - 01/26/2019	1.02	1.05
5	01/27/2019 - 02/02/2019	1.01	1.04
6	02/03/2019 - 02/09/2019	0.99	1.02
* 7	02/10/2019 - 02/16/2019	0.98	1.01
* 8	02/17/2019 - 02/23/2019	0.97	1.00
* 9	02/24/2019 - 03/02/2019	0.97	1.00
*10	03/03/2019 - 03/09/2019	0.96	0.99
*11	03/10/2019 - 03/16/2019	0.96	0.99
*12	03/17/2019 - 03/23/2019	0.96	0.99
*13	03/24/2019 - 03/30/2019	0.96	0.99
*14	03/31/2019 - 04/06/2019	0.96	0.99
*15	04/07/2019 - 04/13/2019	0.96	0.99
*16	04/14/2019 - 04/20/2019	0.96	0.99
*17	04/21/2019 - 04/27/2019	0.97	1.00
*18	04/28/2019 - 05/04/2019	0.98	1.01
*19	05/05/2019 - 05/11/2019	0.99	1.02
20	05/12/2019 - 05/18/2019	1.00	1.03
21	05/19/2019 - 05/25/2019	1.00	1.03
22	05/26/2019 - 06/01/2019	1.01	1.04
23	06/02/2019 - 06/08/2019	1.02	1.05
24	06/09/2019 - 06/15/2019	1.03	1.06
25	06/16/2019 - 06/22/2019	1.03	1.06
26	06/23/2019 - 06/29/2019	1.04	1.07
27	06/30/2019 - 07/06/2019	1.04	1.07
28	07/07/2019 - 07/13/2019	1.05	1.08
29	07/14/2019 - 07/20/2019	1.06	1.09
30	07/21/2019 - 07/27/2019	1.04	1.07
31	07/28/2019 - 08/03/2019	1.03	1.06
32	08/04/2019 - 08/10/2019	1.01	1.04
33	08/11/2019 - 08/17/2019	1.00	1.03
34	08/18/2019 - 08/24/2019	0.99	1.02
35	08/25/2019 - 08/31/2019	0.99	1.02
36	09/01/2019 - 09/07/2019	0.99	1.02
37	09/08/2019 - 09/14/2019	0.99	1.02
38	09/15/2019 - 09/21/2019	0.99	1.02
39	09/22/2019 - 09/28/2019	0.99	1.02
40	09/29/2019 - 10/05/2019	0.98	1.01
41	10/06/2019 - 10/12/2019	0.98	1.01
42	10/13/2019 - 10/19/2019	0.98	1.01
43	10/20/2019 - 10/26/2019	0.99	1.02
44	10/27/2019 - 11/02/2019	1.00	1.03
45	11/03/2019 - 11/09/2019	1.01	1.04
46	11/10/2019 - 11/16/2019	1.03	1.06
47	11/17/2019 - 11/23/2019	1.03	1.06
48	11/24/2019 - 11/30/2019	1.04	1.07
49	12/01/2019 - 12/07/2019	1.04	1.07
50	12/08/2019 - 12/14/2019	1.05	1.08
51	12/15/2019 - 12/21/2019	1.05	1.08
52	12/22/2019 - 12/28/2019	1.04	1.07
53	12/29/2019 - 12/31/2019	1.03	1.06

* PEAK SEASON

FLORIDA DEPARTMENT OF TRANSPORTATION
TRANSPORTATION STATISTICS OFFICE
2019 HISTORICAL AADT REPORT

COUNTY: 26 - ALACHUA

SITE: 0461 - SR 20 .2 MI. NW OF SR 93

YEAR	AADT	DIRECTION 1	DIRECTION 2	*K FACTOR	D FACTOR	T FACTOR
2019	30000 C	N 15000	S 15000	9.50	58.00	5.60
2018	28000 C	N 14000	S 14000	9.50	57.90	4.90
2017	29500 C	N 14500	S 15000	9.50	53.80	4.60
2016	26000 C	N 13000	S 13000	9.50	53.60	4.90
2015	24500 C	N 12500	S 12000	9.50	57.00	5.20
2014	23500 C	N 11500	S 12000	9.50	57.40	5.40
2013	23000 C	N 11500	S 11500	9.50	57.80	5.00
2012	21000 C	N 10500	S 10500	9.50	58.40	4.90
2011	21500 C	N 10500	S 11000	9.50	58.80	5.50
2010	21000 C	N 10500	S 10500	10.13	59.87	5.10
2009	24000 C	N 12000	S 12000	10.04	57.81	6.20
2008	22500 C	N 11000	S 11500	10.17	57.73	7.30
2007	26000 C	N 13000	S 13000	10.22	58.44	5.70
2006	24500 C	N 12000	S 12500	9.98	59.05	6.70
2005	21000 C	N 10500	S 10500	10.10	58.20	19.60
2004	22500 C	N 11500	S 11000	10.20	62.30	9.10

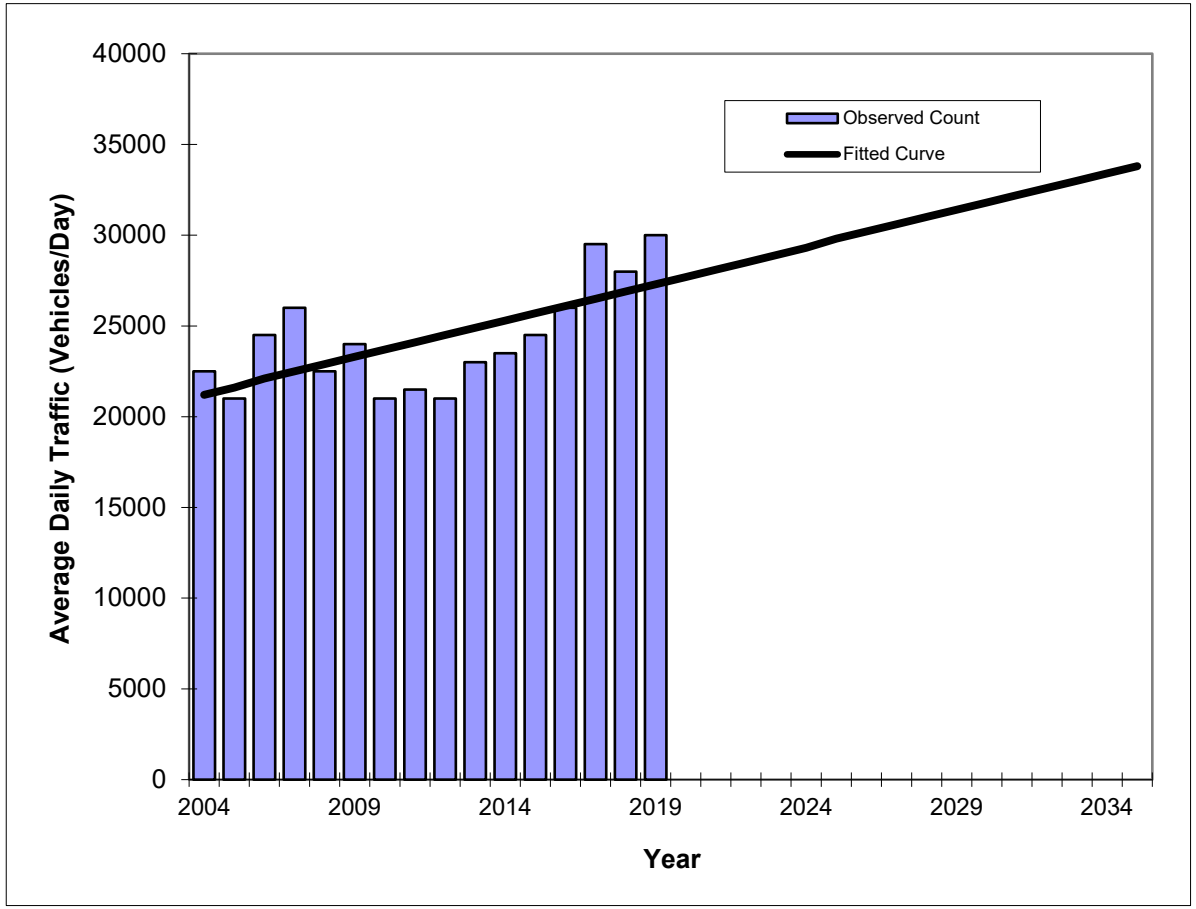
AADT FLAGS: C = COMPUTED; E = MANUAL ESTIMATE; F = FIRST YEAR ESTIMATE
S = SECOND YEAR ESTIMATE; T = THIRD YEAR ESTIMATE; R = FOURTH YEAR ESTIMATE
V = FIFTH YEAR ESTIMATE; 6 = SIXTH YEAR ESTIMATE; X = UNKNOWN
*K FACTOR: STARTING WITH YEAR 2011 IS STANDARDK, PRIOR YEARS ARE K30 VALUES

Traffic Trends - V03.a

US HWY 441 --

FIN#	429193-1
Location	1

County:	Alachua (26)
Station #:	0461
Highway:	US HWY 441



Year	Traffic (ADT/AADT) Count*	Trend**
2004	22500	21200
2005	21000	21600
2006	24500	22100
2007	26000	22500
2008	22500	22900
2009	24000	23300
2010	21000	23700
2011	21500	24100
2012	21000	24500
2013	23000	24900
2014	23500	25300
2015	24500	25700
2016	26000	26100
2017	29500	26500
2018	28000	26900
2019	30000	27300
2022 Opening Year Trend		
2022	N/A	28500
2023 Mid-Year Trend		
2023	N/A	28900
2024 Design Year Trend		
2024	N/A	29300
TRANPLAN Forecasts/Trends		

**** Annual Trend Increase:**	405
Trend R-squared:	43.44%
Trend Annual Historic Growth Rate:	1.92%
Trend Growth Rate (2019 to Design Year):	1.47%
Printed:	11-Dec-20

Straight Line Growth Option

***Axle-Adjusted**

Appendix D
Reserved Trips

Table 8.Traffic Impacts Segment by Segment

Segment Name/Number	Segment 6
Roadway	US 441
Roadway Description	From I-75 to CR 235A
	Peak Hour
Total Impact (Preliminary Development Orders)	313
Total Impact (Final Development Orders)	290
Total Impact (All Development Orders)	603
Preliminary Development Orders - Certificates of LOS Standard	
Tara Forest East Preliminary Plat	197
Tara Baywood SEP & Preliminary Plat	116
Tolosa PD-R Preliminary Plat & Final PD Plan	0
Final Development Orders - Certificates of Concurrency Compliance	
San Felasco Tech City SP	0
Ellianos SP	28
Legacy Park, Phase 2A SP	0
Hampton Inn SP	62
San Felasco Hammock State Park Shop Building SP	0
Upland Industrial Park Lots 2 - 5 SP	0
Alachua Towne Centre Parcel D-2 SP	11
Foundation Park Phase 2 SP	0
San Felasco Tech City Phase 2 SP	0
Hague Fire Station #25 SP	0
Alchem Building 3 SP	0
Innovation One SP	0
Pilot Forest Lot 29 Replat	0
Kolomyiko SP	0
Alachua Retail Center SP	66
San Felasco Tech City Phase 3 Special Exception Permit & SP	0
Savannah Station Phase II-A 1 Final Plat	0
Tara Village Final Plat	0
Golf Plaza SP	0
Sheth SP	0
Dreyer's DKI SP	0
Turkey Creek (3 Lots) Final Plat	0
Southern Wrecker & Recovery SP	13
Progress Park Parcels G & H SP	0
Burger King SP	65
Hal Brady Recreation Center SP	0
Copeland Park (2020) SP	0
Briarwood Phase 1 Final Plat	45
Copeland Park PDL SP	0
Black Flag Metalsmith SP	0
PTI SP	0
Ray's Metal Works SP	0

Notes: Peak Hour trip distribution was not provided for all projects prior to November 2008. A

This table is not automatically updated; please add trips to the appropriate segments

Total Reserved Trips minus Hampton Inn and Burger King = 476 VPH
Westbound (63%) = 300
Eastbound (37%) = 176

Burger King Project Trips

Table 3. Trip Generation

Land Use	AADT (Enter/Exit)[1]	AM Peak Hour (Enter/Exit)[1]	PM Peak Hour (Enter/Exit)[1]
Fast-Food Restaurant with Drive-Through (ITE Code 934)	1,578 (789 / 789)	135 (69 / 66)	109 (57 / 52)
Less Pass-By Trips (40%)[2]	631 (315 / 316)	54 (28 / 26)	44 (23 / 21)
NET TRIP GENERATION	947 (473 / 474)	81 (41 / 40)	65 (34 / 31)

1 Source: ITE Trip Generation, 10th Edition. Formulas: AADT –470.95 trips per 1,000 square feet x 3,349 square feet (50% entering/50% exiting); AM Peak Hour – 40.19 trips per 1,000 square feet x 3,349 square feet (51% entering/49% exiting); PM Peak Hour –32.67 trips per 1,000 square feet x 3,349 square feet (52% entering/48% exiting).

2 ITE Trip Generation Handbook, 3rd Edition. Average Pass-By Trip Percentage of 50% reduced by 10%, resulting in a pass-by rate of 40%.

PM Project Trips

Entering Westbound (63%) = [21] + <15> = 36
Entering Eastbound (37%) = [13] + <8> = 21
Exiting Westbound (63%) = [20] + <13> = 33
Exiting Eastbound (37%) = [11] + <8> = 19

Where;
[#] = Net New Trips
<#> = Pass-by Trips
= Gross Trips

Hampton Inn Project Trips

Entering Eastbound (42%) = 13
Entering Westbound (58%) = 19
Exiting Westbound (42%) = 13
Exiting Eastbound (58%) = 17

From ITE Trip Generation Manual 10th Edition, 51% entering (32), 49% exiting (30)

Directional Distribution based on Highpoint Crossing Traffic Study Dated 6/1/2017

Brian Snyder

From: Justin Tabor <jtabor@cityofalachua.org>
Sent: Wednesday, December 2, 2020 10:08 AM
To: Brian Snyder
Subject: Re: City of Alachua QSR

Brian,

Yes. The 65 reserved peak hr trips for Burger King accounts for a pass-by rate of 40%. See the table below from the staff report which provides the calculations.

Table 3. Trip Generation

Land Use	AADT (Enter/Exit)[1]	AM Peak Hour (Enter/Exit)[1]	PM Peak H (Enter/Ex
Fast-Food Restaurant with Drive-Through (ITE Code 934)	1,578 (789 / 789)	135 (69 / 66)	109 (57 / 52
Less Pass-By Trips (40%)[2]	631 (315 / 316)	54 (28 / 26)	44 (23 / 21
NET TRIP GENERATION	947 (473 / 474)	81 (41 / 40)	65 (34 / 31

1 Source: ITE Trip Generation, 10th Edition. Formulas: AADT –470.95 trips per 1,000 square feet x 3,349 square feet (50% entering/50% exiting); AM Peak Ho trips per 1,000 square feet x 3,349 square feet (51% entering/49% exiting); PM Peak Hour –32.67 trips per 1,000 square feet x 3,349 square feet (52% enter exiting).
2 ITE Trip Generation Handbook, 3rd Edition. Average Pass-By Trip Percentage of 50% reduced by 10%, resulting in a pass-by rate of 40%.

Sincerely,

Justin Tabor, AICP
Principal Planner
City of Alachua
15100 NW 142nd Terrace | PO Box 9
Alachua, Florida 32616
386.418.6100 x 107 | fax: 386.418.6130
jtabor@cityofalachua.com

City Hall Hours of Operation
Monday - Thursday, 7:30 AM - 6:00 PM

Under Florida law, e-mail addresses are public records. If you do not want your e-mail address released in response to a public records request, do not send electronic mail to this entity. Instead, contact this office by phone or in writing.

From: "Brian Snyder" <brians@chw-inc.com>
To: "Justin Tabor" <jtabor@cityofalachua.org>
Sent: Wednesday, December 2, 2020 9:42:55 AM
Subject: FW: City of Alachua QSR

Justin,

I am incorporating the reserved trips that you provided into the study. There are 65 reserved peak hour trips for Burger King. Can you let me know whether these are net new trips with the pass-by trips removed? Since the access will be shared by both sites I would like to use the total Burger King trips (pass-by included) to analyze the intersection.

Thank you,

BRIAN SNYDER | Project Manager
d: (386) 518-5154 | **c:** (352) 339-2938
e: brians@chw-inc.com
w: www.chw-inc.com

JACKSONVILLE.GAINESVILLE.OCALA
t: (904) 619-6521 | 8563 Argyle Business Loop, Suite 3, Jacksonville, FL 32244

t: (352) 331-1976 | 11801 Research Drive, Alachua, FL 32615
t: (352) 414-4621 | 101 NE 1st Avenue, Ocala, FL 34470

PLANNING.DESIGN.SURVEYING.ENGINEERING.CONSTRUCTION.

From: Brian Snyder
Sent: Friday, October 30, 2020 8:34 AM
To: Tom Cavin <Tom.Cavin@dot.state.fl.us>; Justin Tabor <jtabor@cityofalachua.org>
Subject: FW: City of Alachua QSR

Tom, Justin,

Please see the revised methodology based on Justin's recommendations.

Thanks

Appendix E
Background Tables

US HWY 441 at NW 167th Blvd

Growth Rate Data - US HWY 441 at NW 167th Blvd			
Peak Season Factor			1.06
Growth Rate			
Approach	**Historical Growth Rate**	**Growth Rate Used**	**2021 Growth Factor**
US HWY 441	1.92%	1.92%	1.019

PM Peak Hour Background Traffic Volumes - US HWY 441 at NW 167th Blvd							
Scenario	US HWY 441 Eastbound		US HWY 441 Westbound			NW 167th Blvd Southbound	
	Left	Thru	U-turn	Thru	Right	Left	Right
Observed Turning Movement Counts	129	835	3	1400	291	146	44
Peak Season Volume (Existing)	137	885	3	1484	308	155	47
2021 Background Volumes	140	902	3	1512	314	155	47
Reserved Trips	7	195	8	333	0	9	0
2021 Background Volumes with Reserved Trips	147	1097	11	1845	314	164	47
PHF (Total Intersection = 0.91)	0.98		0.87			0.88	
Truck %	5%		3%			2%	
Pedestrian Crossings	0		0			0	

US HWY 441 at Proposed Right-in/Right-out Access

PM Pk Hour Background Traffic Volumes - US HWY 441 at RI/RO Site Access	
Scenario	US HWY 441 Eastbound
	Thru
Observed Turning Movement Counts	984
Peak Season Volume (Existing)	1043
2021 Background Volumes	1063
Reserved Trips	189
2021 Background Volumes with Reserved Trips	1252
PHF (Total Intersection = 0.98)	0.98
Truck %	5%
Pedestrian Crossings	0

US HWY 441 at Directional Left serving the proposed Burger King

Growth Rate Data - US HWY 441 at Directional Left serving the proposed Burger King			
Peak Season Factor			1.06
Growth Rate			
Approach	**Historical Growth Rate**	**Growth Rate Used**	**2021 Growth Factor**
US HWY 441	1.92%	1.92%	1.019

PM Peak Hour Background Traffic Volumes - US HWY 441 at Directional Left serving the proposed Burger King											
Scenario	US HWY 441				US HWY 441				Driveway		Driveway
	Eastbound				Westbound				Northbound		Southbound
	U-turn	Left	Thru	Right	U-turn	Left	Thru	Right	Left	Right	Right
Observed Turning Movement Counts	1	0	968	3	57	3	1670	0	32	6	0
Peak Season Volume (Existing)	1	0	1026	3	60	3	1770	0	34	6	0
2021 Background Volumes	1	0	1046	0	61	0	1804	0	0	0	0
Reserved Trips	0	6	193	21	0	36	320	19	0	52	21
2021 Background Volumes with Reserved	1	6	1239	21	61	36	2124	19	0	52	21
PHF (Total Intersection = 0.93)	0.96				0.91				0.73		0.92
Truck %	5%				3%				0%		2%
Pedestrian Crossings	0				1				0		0

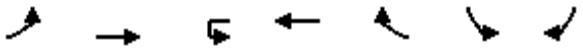
US HWY 441 at Directional Opening West of I-75 SB Off-ramp

Growth Rate Data - US HWY 441 at Directional Opening West of I-75 SB Off-ramp			
Peak Season Factor			1.06
Growth Rate			
Approach	**Historical Growth Rate**	**Growth Rate Used**	**2021 Growth Factor**
US HWY 441	1.92%	1.92%	1.019

PM Peak Hour Background Traffic Volumes - US HWY 441 at Directional Opening West of I-75 SB Off-ramp								
Scenario	US HWY 441				US HWY 441		Zaxby's DW	Dunkin Donut's DW
	Eastbound				Westbound		Northbound	Southbound
	U-turns	Left	Thru	Right	Thru	Right	Right	Right
Observed Turning Movement Counts	11	13	960	49	1658	51	37	64
Peak Season Volume (Existing)	12	14	1018	52	1757	54	39	68
2021 Background Volumes	12	14	1038	53	1791	55	39	68
Reserved Trips	33	0	204	0	340	0	0	0
2021 Background Volumes with Reserved Trips	45	14	1242	53	2131	55	39	68
PHF (Total Intersection = 0.95)	0.99				0.93		0.84	0.80
Truck %	5%				3%		0%	3%
Pedestrian Crossings	0				0		0	0

Appendix F
HCS and Synchro Reports

PM Existing
(Peak Season) Conditions



Lane Group	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations							
Traffic Volume (vph)	137	885	3	1484	308	155	47
Future Volume (vph)	137	885	3	1484	308	155	47
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12
Grade (%)		0%		0%		0%	
Storage Length (ft)	270		250		290	150	0
Storage Lanes	1		1		1	1	0
Taper Length (ft)	25		25			25	
Lane Util. Factor	1.00	0.95	1.00	0.95	1.00	0.97	0.95
Ped Bike Factor							
Frt					0.850	0.965	
Flt Protected	0.950		0.950			0.963	
Satd. Flow (prot)	1719	3438	1752	3505	1568	3358	0
Flt Permitted	0.079		0.316			0.963	
Satd. Flow (perm)	143	3438	583	3505	1568	3358	0
Right Turn on Red					Yes		Yes
Satd. Flow (RTOR)					315	21	
Link Speed (mph)		25		45		45	
Link Distance (ft)		939		1449		667	
Travel Time (s)		25.6		22.0		10.1	
Confl. Peds. (#/hr)							
Confl. Bikes (#/hr)							
Peak Hour Factor	0.98	0.98	0.87	0.87	0.87	0.88	0.88
Growth Factor	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	5%	5%	3%	3%	3%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0	0
Parking (#/hr)							
Mid-Block Traffic (%)		0%		0%		0%	
Adj. Flow (vph)	140	903	3	1706	354	176	53
Shared Lane Traffic (%)							
Lane Group Flow (vph)	140	903	3	1706	354	229	0
Turn Type	pm+pt	NA	pm+pt	NA	Perm	Prot	
Protected Phases	1	6	5	2		3	
Permitted Phases	6		2		2		
Detector Phase	1	6	5	2	2	3	
Switch Phase							
Minimum Initial (s)	4.0	10.0	4.0	10.0	10.0	4.0	
Minimum Split (s)	12.2	39.2	11.8	38.8	38.8	24.7	
Total Split (s)	24.0	116.0	18.0	110.0	110.0	26.0	
Total Split (%)	15.0%	72.5%	11.3%	68.8%	68.8%	16.3%	
Maximum Green (s)	16.8	108.8	11.2	103.2	103.2	19.3	
Yellow Time (s)	5.2	5.2	4.8	4.8	4.8	3.8	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.9	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	7.2	7.2	6.8	6.8	6.8	6.7	
Lead/Lag	Lead	Lag	Lead	Lag	Lag		
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes		
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	

Lane Group	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Minimum Gap (s)	3.0	3.0	3.0	3.0	3.0	3.0	
Time Before Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	
Time To Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	
Recall Mode	None	C-Max	None	C-Max	C-Max	Max	
Walk Time (s)				7.0	7.0		
Flash Dont Walk (s)				25.0	25.0		
Pedestrian Calls (#/hr)				0	0		
Act Effct Green (s)	126.8	124.3	114.6	109.1	109.1	19.3	
Actuated g/C Ratio	0.79	0.78	0.72	0.68	0.68	0.12	
v/c Ratio	0.63	0.34	0.01	0.71	0.30	0.54	
Control Delay	25.9	6.1	1.7	10.9	1.2	65.2	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	25.9	6.1	1.7	10.9	1.2	65.2	
LOS	C	A	A	B	A	E	
Approach Delay		8.8		9.2		65.2	
Approach LOS		A		A		E	
Queue Length 50th (ft)	29	122	0	540	35	107	
Queue Length 95th (ft)	102	211	m0	303	0	150	
Internal Link Dist (ft)		859		1369		587	
Turn Bay Length (ft)	270		250		290	150	
Base Capacity (vph)	278	2670	519	2388	1169	423	
Starvation Cap Reductn	0	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	
Reduced v/c Ratio	0.50	0.34	0.01	0.71	0.30	0.54	

Intersection Summary			
Area Type:	Other		
Cycle Length: 160			
Actuated Cycle Length: 160			
Offset: 96 (60%), Referenced to phase 2:WBTU and 6:EBTL, Start of Yellow			
Natural Cycle: 90			
Control Type: Actuated-Coordinated			
Maximum v/c Ratio: 0.71			
Intersection Signal Delay: 12.9		Intersection LOS: B	
Intersection Capacity Utilization 71.8%		ICU Level of Service C	
Analysis Period (min) 15			
m Volume for 95th percentile queue is metered by upstream signal.			

Splits and Phases: 3: HWY 441 & NW 167th Blvd



HCS7 Two-Way Stop-Control Report

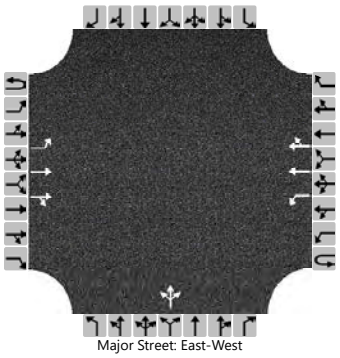
General Information

Analyst	Brian Snyder
Agency/Co.	CHW
Date Performed	12/8/2020
Analysis Year	2020
Time Analyzed	PM Peak
Intersection Orientation	East-West
Project Description	Existing Pk Season

Site Information

Intersection	HWY 441 at BK DW
Jurisdiction	City of Alachua
East/West Street	HWY 441
North/South Street	Burger King/Hampton DW
Peak Hour Factor	0.93
Analysis Time Period (hrs)	0.25

Lanes


Major Street: East-West

Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	2	0	0	1	2	0		0	1	0		0	0	0
Configuration		L	T	TR		L	T	TR			LTR					
Volume (veh/h)	1	0	1026	3	60	3	1770	0		34	0	6				
Percent Heavy Vehicles (%)	5	5			3	3				3	3	3				
Proportion Time Blocked		0.290				0.090				0.290	0.290	0.090				
Percent Grade (%)										0						
Right Turn Channelized																
Median Type | Storage	Undivided															

Critical and Follow-up Headways

Base Critical Headway (sec)	6.4	4.1			6.4	4.1				7.5	6.5	6.9				
Critical Headway (sec)	6.50	4.20			6.46	4.16				7.56	6.56	6.96				
Base Follow-Up Headway (sec)	2.5	2.2			2.5	2.2				3.5	4.0	3.3				
Follow-Up Headway (sec)	2.55	2.25			2.53	2.23				3.53	4.03	3.33				

Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)		1				68					43					
Capacity, c (veh/h)		116				342					31					
v/c Ratio		0.01				0.20					1.39					
95% Queue Length, Q_{95} (veh)		0.0				0.7					4.8					
Control Delay (s/veh)		36.3				18.1					494.2					
Level of Service (LOS)		E				C					F					
Approach Delay (s/veh)	0.0				0.6				494.2							
Approach LOS									F							

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HCS7 Two-Way Stop-Control Report

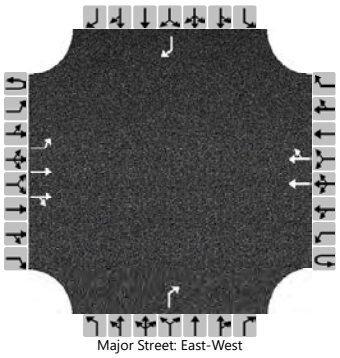
General Information

Analyst	Brian Snyder
Agency/Co.	CHW
Date Performed	12/6/2020
Analysis Year	2020
Time Analyzed	PM Peak
Intersection Orientation	East-West
Project Description	Existing Pk Season

Site Information

Intersection	HWY 441 at Dunkin D
Jurisdiction	City of Alachua
East/West Street	US HWY 441
North/South Street	Dunkin Donuts Access
Peak Hour Factor	0.95
Analysis Time Period (hrs)	0.25

Lanes


Major Street: East-West

Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	2	0	0	0	2	0		0	0	1		0	0	1
Configuration		L	T	TR			T	TR				R				R
Volume (veh/h)	12	14	1018	52			1757	54				39				68
Percent Heavy Vehicles (%)	5	5										0				3
Proportion Time Blocked		0.300										0.090				0.300
Percent Grade (%)									0				0			
Right Turn Channelized									No				No			
Median Type | Storage	Undivided															

Critical and Follow-up Headways

Base Critical Headway (sec)	6.4	4.1										6.9				6.9
Critical Headway (sec)	6.50	4.20										6.90				6.96
Base Follow-Up Headway (sec)	2.5	2.2										3.3				3.3
Follow-Up Headway (sec)	2.55	2.25										3.30				3.33

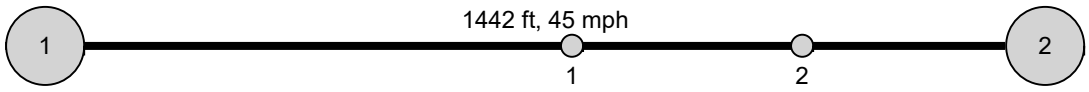
Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)		27										41				72
Capacity, c (veh/h)		164										618				677
v/c Ratio		0.17										0.07				0.11
95% Queue Length, Q_{95} (veh)		0.6										0.2				0.4
Control Delay (s/veh)		31.3										11.2				10.9
Level of Service (LOS)		D										B				B
Approach Delay (s/veh)	0.7								11.2				10.9			
Approach LOS									B				B			

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HCS7 Urban Street Access Point Report

General Information				Streets Information	
Agency	CHW			Number of Intersections	2
Analyst	Brian Snyder	Analysis Date	12/7/2020	Number of Segments	1
Jurisdiction	City of Alachua	Time Period	PM Peak	Number of Iterations	15
File Name	Corridor Signal Timings_Existing	Analysis Year	2020	System Cycle Length, s	160
Intersections	HWY 441 at NW 167th Blvd	HWY 441 at I-75 SB		Analysis Period	1> 7:00
Project Description	Existing				



Basic Segment Information

Segment	Speed Limit		Through Lanes		Segment Length		Intersection Wid		Length of RM		Percent Curb		Other Delay	
	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB
1	45	45	2	2	1442	1442	50	50	0	0	70	70	0.0	0.0

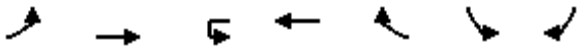
Number of Access Points	2

#1 (Burger King DW)	Peak Hour Factor			0.93			Location, ft			800		
	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lanes	1	2	0	1	2	0	0	0	1	0	0	1
Input volume, veh/h	1	1026	3	63	1770	0	34	0	6	0	0	0
Balanced flow, veh/h	1	1137	3	66	1862	0	37	0	6	0	0	0
Proportion time blocked	0.29	0.00	0.00	0.09	0.00	0.00	0.29	0.29	0.09	0.29	0.29	0.29
Thru vehicle delay		0.0			0.0							
Probability inside blocked		0.00			0.00							

#2 (Dunkin Donuts DW)	Peak Hour Factor			0.95			Location, ft			1132		
	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lanes	1	2	0	0	2	0	0	0	0	0	1	0
Input volume, veh/h	26	1018	52	0	1757	54	0	0	39	0	0	68
Balanced flow, veh/h	27	1062	54	0	1857	57	0	0	41	0	0	72
Proportion time blocked	0.30	0.00	0.00	0.09	0.00	0.00	0.30	0.30	0.09	0.30	0.30	0.30
Thru vehicle delay		0.1			0.1							
Probability inside blocked		0.00			0.00							

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PM Background with Reserved Trips Conditions



Lane Group	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations							
Traffic Volume (vph)	147	1097	11	1845	314	164	47
Future Volume (vph)	147	1097	11	1845	314	164	47
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12
Grade (%)		0%		0%		0%	
Storage Length (ft)	270		250		290	150	0
Storage Lanes	1		1		1	1	0
Taper Length (ft)	25		25			25	
Lane Util. Factor	1.00	0.95	1.00	0.95	1.00	0.97	0.95
Ped Bike Factor							
Frt					0.850	0.967	
Flt Protected	0.950		0.950			0.963	
Satd. Flow (prot)	1719	3438	1752	3505	1568	3365	0
Flt Permitted	0.035		0.251			0.963	
Satd. Flow (perm)	63	3438	463	3505	1568	3365	0
Right Turn on Red					Yes		Yes
Satd. Flow (RTOR)					258	19	
Link Speed (mph)		25		45		45	
Link Distance (ft)		939		1449		667	
Travel Time (s)		25.6		22.0		10.1	
Confl. Peds. (#/hr)							
Confl. Bikes (#/hr)							
Peak Hour Factor	0.98	0.98	0.87	0.87	0.87	0.88	0.88
Growth Factor	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	5%	5%	3%	3%	3%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0	0
Parking (#/hr)							
Mid-Block Traffic (%)		0%		0%		0%	
Adj. Flow (vph)	150	1119	13	2121	361	186	53
Shared Lane Traffic (%)							
Lane Group Flow (vph)	150	1119	13	2121	361	239	0
Turn Type	pm+pt	NA	pm+pt	NA	Perm	Prot	
Protected Phases	1	6	5	2		3	
Permitted Phases	6		2		2		
Detector Phase	1	6	5	2	2	3	
Switch Phase							
Minimum Initial (s)	4.0	10.0	4.0	10.0	10.0	4.0	
Minimum Split (s)	12.2	39.2	11.8	38.8	38.8	24.7	
Total Split (s)	24.0	116.0	18.0	110.0	110.0	26.0	
Total Split (%)	15.0%	72.5%	11.3%	68.8%	68.8%	16.3%	
Maximum Green (s)	16.8	108.8	11.2	103.2	103.2	19.3	
Yellow Time (s)	5.2	5.2	4.8	4.8	4.8	3.8	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.9	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	7.2	7.2	6.8	6.8	6.8	6.7	
Lead/Lag	Lead	Lag	Lead	Lag	Lag		
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes		
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	

Lane Group	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Minimum Gap (s)	3.0	3.0	3.0	3.0	3.0	3.0	
Time Before Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	
Time To Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	
Recall Mode	None	C-Max	None	C-Max	C-Max	Max	
Walk Time (s)				7.0	7.0		
Flash Dont Walk (s)				25.0	25.0		
Pedestrian Calls (#/hr)				0	0		
Act Effct Green (s)	126.8	121.7	112.0	106.2	106.2	19.3	
Actuated g/C Ratio	0.79	0.76	0.70	0.66	0.66	0.12	
v/c Ratio	0.78	0.43	0.04	0.91	0.32	0.57	
Control Delay	69.3	7.9	2.1	19.5	1.4	66.7	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	69.3	7.9	2.1	19.5	1.4	66.7	
LOS	E	A	A	B	A	E	
Approach Delay		15.2		16.8		66.7	
Approach LOS		B		B		E	
Queue Length 50th (ft)	105	165	2	960	41	113	
Queue Length 95th (ft)	186	284	m1	447	0	158	
Internal Link Dist (ft)		859		1369		587	
Turn Bay Length (ft)	270		250		290	150	
Base Capacity (vph)	223	2614	430	2326	1127	422	
Starvation Cap Reductn	0	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	
Reduced v/c Ratio	0.67	0.43	0.03	0.91	0.32	0.57	

Intersection Summary			
Area Type:	Other		
Cycle Length: 160			
Actuated Cycle Length: 160			
Offset: 96 (60%), Referenced to phase 2:WBTU and 6:EBTL, Start of Yellow			
Natural Cycle: 110			
Control Type: Actuated-Coordinated			
Maximum v/c Ratio: 0.91			
Intersection Signal Delay: 19.3		Intersection LOS: B	
Intersection Capacity Utilization 82.6%		ICU Level of Service E	
Analysis Period (min) 15			
m Volume for 95th percentile queue is metered by upstream signal.			

Splits and Phases: 3: HWY 441 & NW 167th Blvd



HCS7 Two-Way Stop-Control Report

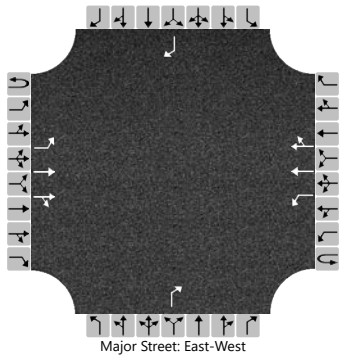
General Information

Analyst	Brian Snyder
Agency/Co.	CHW
Date Performed	12/8/2020
Analysis Year	2021
Time Analyzed	PM Peak
Intersection Orientation	East-West
Project Description	Background with reserved trips

Site Information

Intersection	HWY 441 at BK DW
Jurisdiction	City of Alachua
East/West Street	HWY 441
North/South Street	Burger King/Hampton DW
Peak Hour Factor	0.93
Analysis Time Period (hrs)	0.25

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	2	0	0	1	2	0		0	0	1		0	0	1
Configuration		L	T	TR		L	T	TR				R				R
Volume (veh/h)	1	6	1239	21	61	36	2124	19				52				21
Percent Heavy Vehicles (%)	5	5			3	3						0				2
Proportion Time Blocked		0.730				0.130						0.130				0.730
Percent Grade (%)										0				0		
Right Turn Channelized										No				No		
Median Type | Storage			Undivided													

Critical and Follow-up Headways

Base Critical Headway (sec)	6.4	4.1			6.4	4.1						6.9				6.9
Critical Headway (sec)	6.50	4.20			6.46	4.16						6.90				6.94
Base Follow-Up Headway (sec)	2.5	2.2			2.5	2.2						3.3				3.3
Follow-Up Headway (sec)	2.55	2.25			2.53	2.23						3.30				3.32

Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)		8				104						56				23
Capacity, c (veh/h)		277				282						582				293
v/c Ratio		0.03				0.37						0.10				0.08
95% Queue Length, Q_{95} (veh)		0.1				1.6						0.3				0.2
Control Delay (s/veh)		18.4				25.1						11.8				18.3
Level of Service (LOS)		C				D						B				C
Approach Delay (s/veh)	0.1				1.1				11.8				18.3			
Approach LOS									B				C			

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HCS7 Two-Way Stop-Control Report

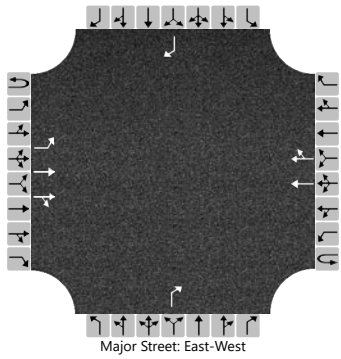
General Information

Analyst	Brian Snyder
Agency/Co.	CHW
Date Performed	12/6/2020
Analysis Year	2021
Time Analyzed	PM Peak
Intersection Orientation	East-West
Project Description	Background with reserved trips

Site Information

Intersection	HWY 441 at Dunkin D
Jurisdiction	City of Alachua
East/West Street	US HWY 441
North/South Street	Dunkin Donuts Access
Peak Hour Factor	0.95
Analysis Time Period (hrs)	0.25

Lanes


Major Street: East-West

Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	2	0	0	0	2	0		0	0	1		0	0	1
Configuration		L	T	TR			T	TR				R				R
Volume (veh/h)	45	14	1242	53			2131	55				39				68
Percent Heavy Vehicles (%)	5	5										0				3
Proportion Time Blocked		0.730										0.130				0.730
Percent Grade (%)										0				0		
Right Turn Channelized										No				No		
Median Type | Storage			Undivided													

Critical and Follow-up Headways

Base Critical Headway (sec)	6.4	4.1										6.9				6.9
Critical Headway (sec)	6.50	4.20										6.90				6.96
Base Follow-Up Headway (sec)	2.5	2.2										3.3				3.3
Follow-Up Headway (sec)	2.55	2.25										3.30				3.33

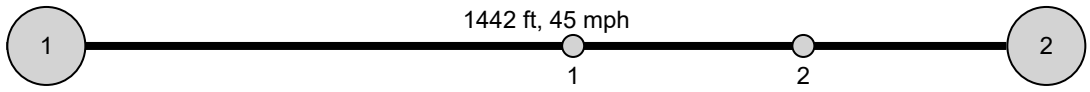
Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)		62										41				72
Capacity, c (veh/h)		178										580				292
v/c Ratio		0.35										0.07				0.25
95% Queue Length, Q_{95} (veh)		1.5										0.2				0.9
Control Delay (s/veh)		35.7										11.7				21.3
Level of Service (LOS)		E										B				C
Approach Delay (s/veh)		1.6								11.7				21.3		
Approach LOS										B				C		

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HCS7 Urban Street Access Point Report

General Information				Streets Information	
Agency	CHW			Number of Intersections	2
Analyst	Brian Snyder	Analysis Date	12/7/2020	Number of Segments	1
Jurisdiction	City of Alachua	Time Period	PM Peak	Number of Iterations	15
File Name	Corridor Signal Timings_Backgr	Analysis Year	2021	System Cycle Length, s	160
Intersections	HWY 441 at NW 167th Blvd	HWY 441 at I-75 SB		Analysis Period	1> 7:00
Project Description	Background				



Basic Segment Information

Segment	Speed Limit		Through Lanes		Segment Length		Intersection Wid		Length of RM		Percent Curb		Other Delay	
	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB
1	45	45	2	2	1442	1442	50	50	0	0	70	70	0.0	0.0

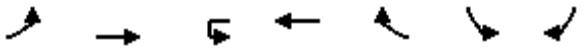
Number of Access Points	2

#1 (Burger King DW)	Peak Hour Factor			0.93			Location, ft			800		
	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lanes	1	2	0	1	2	0	0	0	1	0	0	1
Input volume, veh/h	7	1239	21	97	2124	19	0	0	52	0	0	21
Balanced flow, veh/h	8	1354	23	101	2213	20	0	0	56	0	0	23
Proportion time blocked	0.73	0.00	0.00	0.14	0.00	0.00	0.73	0.73	0.14	0.73	0.73	0.73
Thru vehicle delay		0.0			0.0							
Probability inside blocked		0.00			0.00							

#2 (Directional Left ...)	Peak Hour Factor			0.95			Location, ft			1132		
	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lanes	1	2	0	0	2	0	0	0	0	0	1	0
Input volume, veh/h	59	1242	53	0	2131	55	0	0	39	0	0	68
Balanced flow, veh/h	61	1293	55	0	2262	58	0	0	41	0	0	72
Proportion time blocked	0.73	0.00	0.00	0.14	0.00	0.00	0.73	0.73	0.14	0.73	0.73	0.73
Thru vehicle delay		0.1			0.1							
Probability inside blocked		0.00			0.00							

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PM Build-out with Reserved Trips Conditions



Lane Group	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Lane Configurations							
Traffic Volume (vph)	147	1103	15	1845	324	164	47
Future Volume (vph)	147	1103	15	1845	324	164	47
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12
Grade (%)		0%		0%		0%	
Storage Length (ft)	270		250		290	150	0
Storage Lanes	1		1		1	1	0
Taper Length (ft)	25		25			25	
Lane Util. Factor	1.00	0.95	1.00	0.95	1.00	0.97	0.95
Ped Bike Factor							
Frt					0.850	0.967	
Flt Protected	0.950		0.950			0.963	
Satd. Flow (prot)	1719	3438	1752	3505	1568	3365	0
Flt Permitted	0.035		0.244			0.963	
Satd. Flow (perm)	63	3438	450	3505	1568	3365	0
Right Turn on Red					Yes		Yes
Satd. Flow (RTOR)					266	19	
Link Speed (mph)		25		45		45	
Link Distance (ft)		939		1449		667	
Travel Time (s)		25.6		22.0		10.1	
Confl. Peds. (#/hr)							
Confl. Bikes (#/hr)							
Peak Hour Factor	0.98	0.98	0.87	0.87	0.87	0.88	0.88
Growth Factor	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	5%	5%	3%	3%	3%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0	0
Parking (#/hr)							
Mid-Block Traffic (%)		0%		0%		0%	
Adj. Flow (vph)	150	1126	17	2121	372	186	53
Shared Lane Traffic (%)							
Lane Group Flow (vph)	150	1126	17	2121	372	239	0
Turn Type	pm+pt	NA	pm+pt	NA	Perm	Prot	
Protected Phases	1	6	5	2		3	
Permitted Phases	6		2		2		
Detector Phase	1	6	5	2	2	3	
Switch Phase							
Minimum Initial (s)	4.0	10.0	4.0	10.0	10.0	4.0	
Minimum Split (s)	12.2	39.2	11.8	38.8	38.8	24.7	
Total Split (s)	24.0	116.0	18.0	110.0	110.0	26.0	
Total Split (%)	15.0%	72.5%	11.3%	68.8%	68.8%	16.3%	
Maximum Green (s)	16.8	108.8	11.2	103.2	103.2	19.3	
Yellow Time (s)	5.2	5.2	4.8	4.8	4.8	3.8	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.9	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	7.2	7.2	6.8	6.8	6.8	6.7	
Lead/Lag	Lead	Lag	Lead	Lag	Lag		
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes		
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	

Lane Group	EBL	EBT	WBU	WBT	WBR	SBL	SBR
Minimum Gap (s)	3.0	3.0	3.0	3.0	3.0	3.0	
Time Before Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	
Time To Reduce (s)	0.0	0.0	0.0	0.0	0.0	0.0	
Recall Mode	None	C-Max	None	C-Max	C-Max	Max	
Walk Time (s)				7.0	7.0		
Flash Dont Walk (s)				25.0	25.0		
Pedestrian Calls (#/hr)				0	0		
Act Effct Green (s)	126.8	119.1	112.1	106.2	106.2	19.3	
Actuated g/C Ratio	0.79	0.74	0.70	0.66	0.66	0.12	
v/c Ratio	0.78	0.44	0.05	0.91	0.33	0.57	
Control Delay	69.3	9.0	2.1	19.5	1.4	66.7	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	69.3	9.0	2.1	19.5	1.4	66.7	
LOS	E	A	A	B	A	E	
Approach Delay		16.0		16.7		66.7	
Approach LOS		B		B		E	
Queue Length 50th (ft)	105	243	2	961	42	113	
Queue Length 95th (ft)	186	289	m2	456	0	158	
Internal Link Dist (ft)		859		1369		587	
Turn Bay Length (ft)	270		250		290	150	
Base Capacity (vph)	223	2559	421	2326	1130	422	
Starvation Cap Reductn	0	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	
Reduced v/c Ratio	0.67	0.44	0.04	0.91	0.33	0.57	

Intersection Summary			
Area Type:	Other		
Cycle Length: 160			
Actuated Cycle Length: 160			
Offset: 96 (60%), Referenced to phase 2:WBTU and 6:EBTL, Start of Yellow			
Natural Cycle: 110			
Control Type: Actuated-Coordinated			
Maximum v/c Ratio: 0.91			
Intersection Signal Delay: 19.5		Intersection LOS: B	
Intersection Capacity Utilization 82.6%		ICU Level of Service E	
Analysis Period (min) 15			
m Volume for 95th percentile queue is metered by upstream signal.			

Splits and Phases: 3: HWY 441 & NW 167th Blvd



HCS7 Two-Way Stop-Control Report

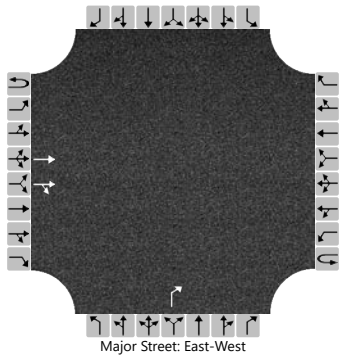
General Information

Analyst	Brian Snyder
Agency/Co.	CHW
Date Performed	12/8/2020
Analysis Year	2021
Time Analyzed	PM Peak
Intersection Orientation	East-West
Project Description	Build-out with reserved trips

Site Information

Intersection	HWY 441 at Proposed RI/RO
Jurisdiction	City of Alachua
East/West Street	US HWY 441
North/South Street	Proposed RightIn/RightOut
Peak Hour Factor	0.98
Analysis Time Period (hrs)	0.25

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	2	0	0	0	0	0		0	0	1		0	0	0
Configuration			T	TR								R				
Volume (veh/h)			1269	16								14				
Percent Heavy Vehicles (%)												2				
Proportion Time Blocked																
Percent Grade (%)										0						
Right Turn Channelized										No						
Median Type | Storage			Undivided													

Critical and Follow-up Headways

Base Critical Headway (sec)												6.9				
Critical Headway (sec)												6.94				
Base Follow-Up Headway (sec)												3.3				
Follow-Up Headway (sec)												3.32				

Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)												14				
Capacity, c (veh/h)												408				
v/c Ratio												0.03				
95% Queue Length, Q_{95} (veh)												0.1				
Control Delay (s/veh)												14.1				
Level of Service (LOS)												B				
Approach Delay (s/veh)										14.1						
Approach LOS										B						

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HCS7 Two-Way Stop-Control Report

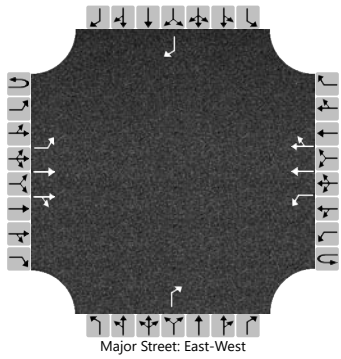
General Information

Analyst	Brian Snyder
Agency/Co.	CHW
Date Performed	12/8/2020
Analysis Year	2021
Time Analyzed	PM Peak
Intersection Orientation	East-West
Project Description	Build-out with reserved trips

Site Information

Intersection	HWY 441 at BK DW
Jurisdiction	City of Alachua
East/West Street	HWY 441
North/South Street	Burger King/Hampton DW
Peak Hour Factor	0.93
Analysis Time Period (hrs)	0.25

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	2	0	0	1	2	0		0	0	1		0	0	1
Configuration		L	T	TR		L	T	TR				R				R
Volume (veh/h)	15	6	1239	21	61	52	2128	19				68				21
Percent Heavy Vehicles (%)	5	5			3	3						0				2
Proportion Time Blocked		0.730				0.130						0.130				0.730
Percent Grade (%)										0				0		
Right Turn Channelized										No				No		
Median Type | Storage			Undivided													

Critical and Follow-up Headways

Base Critical Headway (sec)	6.4	4.1			6.4	4.1						6.9				6.9
Critical Headway (sec)	6.50	4.20			6.46	4.16						6.90				6.94
Base Follow-Up Headway (sec)	2.5	2.2			2.5	2.2						3.3				3.3
Follow-Up Headway (sec)	2.55	2.25			2.53	2.23						3.30				3.32

Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)		23				122						73				23
Capacity, c (veh/h)		207				295						582				293
v/c Ratio		0.11				0.41						0.13				0.08
95% Queue Length, Q_{95} (veh)		0.4				1.9						0.4				0.2
Control Delay (s/veh)		24.5				25.5						12.1				18.3
Level of Service (LOS)		C				D						B				C
Approach Delay (s/veh)	0.4				1.3				12.1				18.3			
Approach LOS									B				C			

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HCS7 Two-Way Stop-Control Report

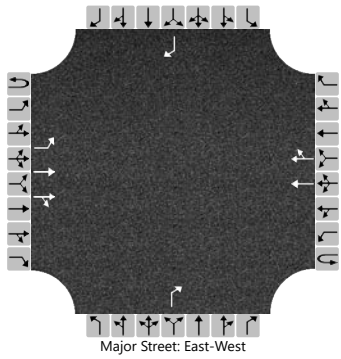
General Information

Analyst	Brian Snyder
Agency/Co.	CHW
Date Performed	12/6/2020
Analysis Year	2021
Time Analyzed	PM Peak
Intersection Orientation	East-West
Project Description	Build-out with reserved trips

Site Information

Intersection	HWY 441 at Dunkin D
Jurisdiction	City of Alachua
East/West Street	US HWY 441
North/South Street	Dunkin Donuts Access
Peak Hour Factor	0.95
Analysis Time Period (hrs)	0.25

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	2	0	0	0	2	0		0	0	1		0	0	1
Configuration		L	T	TR			T	TR				R				R
Volume (veh/h)	49	14	1248	53			2141	55				39				68
Percent Heavy Vehicles (%)	5	5										0				3
Proportion Time Blocked		0.730										0.130				0.730
Percent Grade (%)										0				0		
Right Turn Channelized										No				No		
Median Type | Storage			Undivided													

Critical and Follow-up Headways

Base Critical Headway (sec)	6.4	4.1										6.9				6.9
Critical Headway (sec)	6.50	4.20										6.90				6.96
Base Follow-Up Headway (sec)	2.5	2.2										3.3				3.3
Follow-Up Headway (sec)	2.55	2.25										3.30				3.33

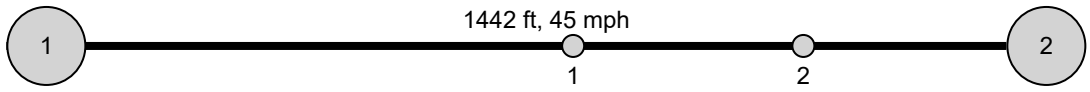
Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)		66										41				72
Capacity, c (veh/h)		167										577				292
v/c Ratio		0.40										0.07				0.25
95% Queue Length, Q_{95} (veh)		1.7										0.2				0.9
Control Delay (s/veh)		40.0										11.7				21.3
Level of Service (LOS)		E										B				C
Approach Delay (s/veh)		1.8								11.7				21.3		
Approach LOS										B				C		

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HCS7 Urban Street Access Point Report

General Information				Streets Information	
Agency	CHW			Number of Intersections	2
Analyst	Brian Snyder	Analysis Date	12/7/2020	Number of Segments	1
Jurisdiction	City of Alachua	Time Period	PM Peak	Number of Iterations	15
File Name	Corridor Signal Timings_Build-ou	Analysis Year	2021	System Cycle Length, s	160
Intersections	HWY 441 at NW 167th Blvd	HWY 441 at I-75 SB		Analysis Period	1> 7:00
Project Description	Build-out				



Basic Segment Information

Segment	Speed Limit		Through Lanes		Segment Length		Intersection Wid		Length of RM		Percent Curb		Other Delay	
	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB
1	45	45	2	2	1442	1442	50	50	0	0	70	70	0.0	0.0

Number of Access Points	2

#1 (Burger King DW)	Peak Hour Factor			0.93			Location, ft			800		
	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lanes	1	2	0	1	2	0	0	0	1	0	0	1
Input volume, veh/h	21	1239	21	113	2128	19	0	0	68	0	0	21
Balanced flow, veh/h	23	1346	23	117	2209	20	0	0	73	0	0	23
Proportion time blocked	0.73	0.00	0.00	0.14	0.00	0.00	0.73	0.73	0.14	0.73	0.73	0.73
Thru vehicle delay		0.0			0.0							
Probability inside blocked		0.00			0.00							

#2 (Directional Left ...)	Peak Hour Factor			0.95			Location, ft			1132		
	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lanes	1	2	0	0	2	0	0	0	0	0	1	0
Input volume, veh/h	63	1248	53	0	2141	55	0	0	39	0	0	68
Balanced flow, veh/h	66	1298	55	0	2275	58	0	0	41	0	0	72
Proportion time blocked	0.73	0.00	0.00	0.14	0.00	0.00	0.73	0.73	0.14	0.73	0.73	0.73
Thru vehicle delay		0.1			0.1							
Probability inside blocked		0.00			0.00							

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Appendix G
Straight Line Diagram

	5 YR INV	SLD REV
DATE	03/29/2017	04/20/2017
BY	METRIC	METRIC

BMP	EMP	INV	SLD REV
19.685	26.493	08/06/2019 METRIC	08/14/2019 METRIC
5.082	25.370	03/13/2018 FDOT	04/02/2018 Metric
2.890	2.890	08/24/2017 METRIC	08/24/2017 METRIC

FLORIDA DEPARTMENT OF TRANSPORTATION

STRAIGHT LINE DIAGRAM OF ROAD INVENTORY

SECTION STATUS	INT. or US ROUTE NO.	STATE ROAD NO.	COUNTY	DISTRICT	ROADWAY ID	SHEET NO:
12	US 441/US 41	CR 329/SR 20/SR 25	ALACHUA	02	26020000	4 OF 5

18.0 | 19.0 | 20.0 | 21.0

ROADWAY FEATURES

INSIDE CITY, NOT URBAN
* ALACHUA
*|<=US-441 N/MLK MEM HWY
*|<SR 20/SR 25
*|<US 441

18.000 N MAIN ST 18.075 | 18.075 NW 141 ST 18.222 | NW 154TH AVE 18.180 | NW 142 TER 18.075

18.075 88.0' - 48.0'
4 - 12.0' RDWY
20.0 CB&VEG MED
2 - 2.0' CRG INSHLD1
2 - 6.0' PVD SHLD1
6.0' LWN SHLD2 - LT
2.0' C&G SHLD2 - RT

NW 144 WAY 18.294 | NW 144 ST 18.294

18.294 96.0' - 48.0'
4 - 12.0' RDWY
20.0 CB&VEG MED
2 - 2.0' CRG INSHLD1
2 - 4.0' PVD SHLD1
2 - 10.0' LWN SHLD2

DR 18.481 | NW 147TH 18.481

NW 149 TER 18.676 | NW 158 LN 18.676

18.676 112.0' - 48.0'
4 - 12.0' RDWY
40.0 VEG MED
2 - 4.0' PVD SHLD1
2 - 8.0' LWN SHLD2

SR-93 19.337 | I-75/ | 26260033 SB ON | 26260030 SB ON | 26260032 NB OFF | 26260031 NB ON | 26260029 SB OFF

APRIL BLVD 19.097 | 19.169 | 19.217 | 19.373 | 19.407

SITE

19.097 105.0' - 48.0'
4 - 12.0' RDWY
41.0 CB&VEG MED
2 - 2.0' CRG INSHLD1
2 - 4.0' PVD SHLD1
2 - 4.0' LWN SHLD2

INSIDE CITY, NOT URBAN
19.677 * ALACHUA
*|<=US-441 N
*|<SR 20/SR 25
*|<US 441

NW 167TH BLVD 19.677 | SHOPPING ENT 19.747 | SCHOOL ENT 19.828

19.520 112.0' - 48.0'
4 - 12.0' RDWY
40.0 VEG MED
2 - 2.0' PVD INSHLD1
2 - 4.0' PVD SHLD1
2 - 8.0' LWN SHLD2

CR-235A/ NW 173 ST 20.076 | NW CR 235A 20.076 | NW 174 DR 20.217 | NW 164 LN 20.432 | FROM NW 164 LN 20.476

LANE WIDTHS ARE AVERAGED

ROADWAY COMPOSITION

18.000 28/FC-5

18.000 28/FC-5

HORIZONTAL ALIGNMENT

CURVE DATA NOT FIELD VERIFIED

Δ=23°10'00.00"
D=2°00'
PC=18.544
PI=18.656
PT=18.764

B=N55°50'42"W | B=N79°00'42"W

STRUCTURE DESCRIPTION

18.580 1 - 15" X 52' CC
18.661 1 - 24" X 135' CC - LT
1 - 24" X 135' CC - RT
18.704 1 - 24" X 147' CC - LT
1 - 24" X 147' CC - RT
18.842 1 - 15" X 72' CC
19.078 1 - 30" X 144' CC - LT
1 - 30" X 14' CC - RT
19.174 1 - 15" X 41' CC
19.283 #0073 63.4' UP | 19.295 | 19.297 | #0065 63.4' UP | 19.309
19.362 1 - 36" X 209' CC
19.422 2 - 24" X 333' CC
20.100 1 - 18" X 71' CC
20.207 1 - 18" X 73' CC
20.341 1 - 18" X 73' CC
20.423 1 - 18" X 76' CC
20.620 1 - 24" X 149' CC - LT
1 - 24" X 149' CC - RT
20.678 1 - 18" X 74' CC
20.909 1 - 18" X 74' CC

DISTRICT USE

SIS

FUN CLASS: 18.000 RURAL PRIN ART OTHER

SPEED LIMIT: 18.000 45MPH | 18.610 45MPH | 19.748 45MPH | 20.178 55MPH | 20.339 65MPH

AC MAN CLS: 18.000 ACCESS CLASS05 | ACCESS CLASS05 18.294 | 18.294 ACCESS CLASS03

21.0 | 22.0 | 23.0 | 24.0

ROADWAY FEATURES

21.000 INSIDE CITY, NOT URBAN
* ALACHUA
*|<=US-441 N
*|<SR 20/SR 25
*|<US 441

21.137 INSIDE CITY, NOT URBAN
* HIGH SPRINGS
*|<=US-441 N
*|<SR 20/SR 25
*|<US 441

ST 21.138 | NW 188 21.138 | OAK RIDGE 21.356 | NW 194TH ST 21.553 | FIRE DEPT ENT 21.921 | NW 170 LN 21.985

ST 22.135 | NW 202 22.135 | NW 207 TER 22.475 | MEMORIAL PARK 22.711

22.475 INSIDE CITY, NOT URBAN
* HIGH SPRINGS
*|<=E SANTA FE BLVD.
*|<SR 20/SR 25
*|<US 441

NW 218TH TER 23.222 | NW 222 ST 23.462

LANE WIDTHS ARE AVERAGED

21.000 112.0' - 48.0'
4 - 12.0' RDWY
40.0 VEG MED
2 - 2.0' PVD INSHLD1
2 - 4.0' PVD SHLD1
2 - 8.0' LWN SHLD2

23.957 89.0' - 48.0'
4 - 12.0' RDWY
17.0 CB&VEG MED
2 - 2.0' C&G INSHLD1
2 - 4.0' PVD SHLD1
2 - 8.0' LWN SHLD2

ROADWAY COMPOSITION

21.000 28/FC-5

21.000 28/FC-5

HORIZONTAL ALIGNMENT

CURVE DATA NOT FIELD VERIFIED

PC=22.072
PI=22.200
PT=22.328
Δ=13°41'00.00"
D=1°00'

PC=23.908
PI=24.174
PT=24.451
Δ=28°39'00.00"
D=1°00'

B=N65°19'42"W

STRUCTURE DESCRIPTION

21.121 1 - 18" X 78' CC
21.330 1 - 18" X 146' CC - LT
1 - 18" X 146' CC - RT
21.362 1 - 18" X 71' CC
21.550 1 - 18" X 70' CC
21.584 1 - 24" X 139' CC - LT
1 - 24" X 139' CC - RT
21.760 1 - 18" X 74' CC
21.838 1 - 24" X 146' CC - LT
1 - 24" X 146' CC - RT
22.003 1 - 18" X 66' CC
22.149 1 - 18" X 65' CC
22.257 1 - 18" X 66' CC
22.391 1 - 18" X 73' CC
22.793 1 - 18" X 74' CC
23.020 1 - 18" X 61' CC
23.197 1 - 24" X 142' CC - LT
1 - 24" X 142' CC - RT
23.262 1 - 18" X 64' CC
23.551 1 - 18" X 144' CC
23.675 1 - 18" X 64' CC
23.745 1 - 36" X 142' CC - LT
1 - 36" X 142' CC - RT
23.823 1 - 18" X 64' CC
23.930 1 - 18" X 65' CC

DISTRICT USE

SIS

FUN CLASS: 21.000 RURAL PRIN ART OTHER

SPEED LIMIT: 21.000 65MPH | 23.190 55MPH | 23.890 45MPH

AC MAN CLS: 21.000 ACCESS CLASS03 | ACCESS CLASS03 23.476 | 23.476 ACCESS CLASS05

Version: 1.4.2.27 08/14/2019

Appendix H
Trip Generation Documentation

Fast-Food Restaurant with Drive-Through Window
(934)

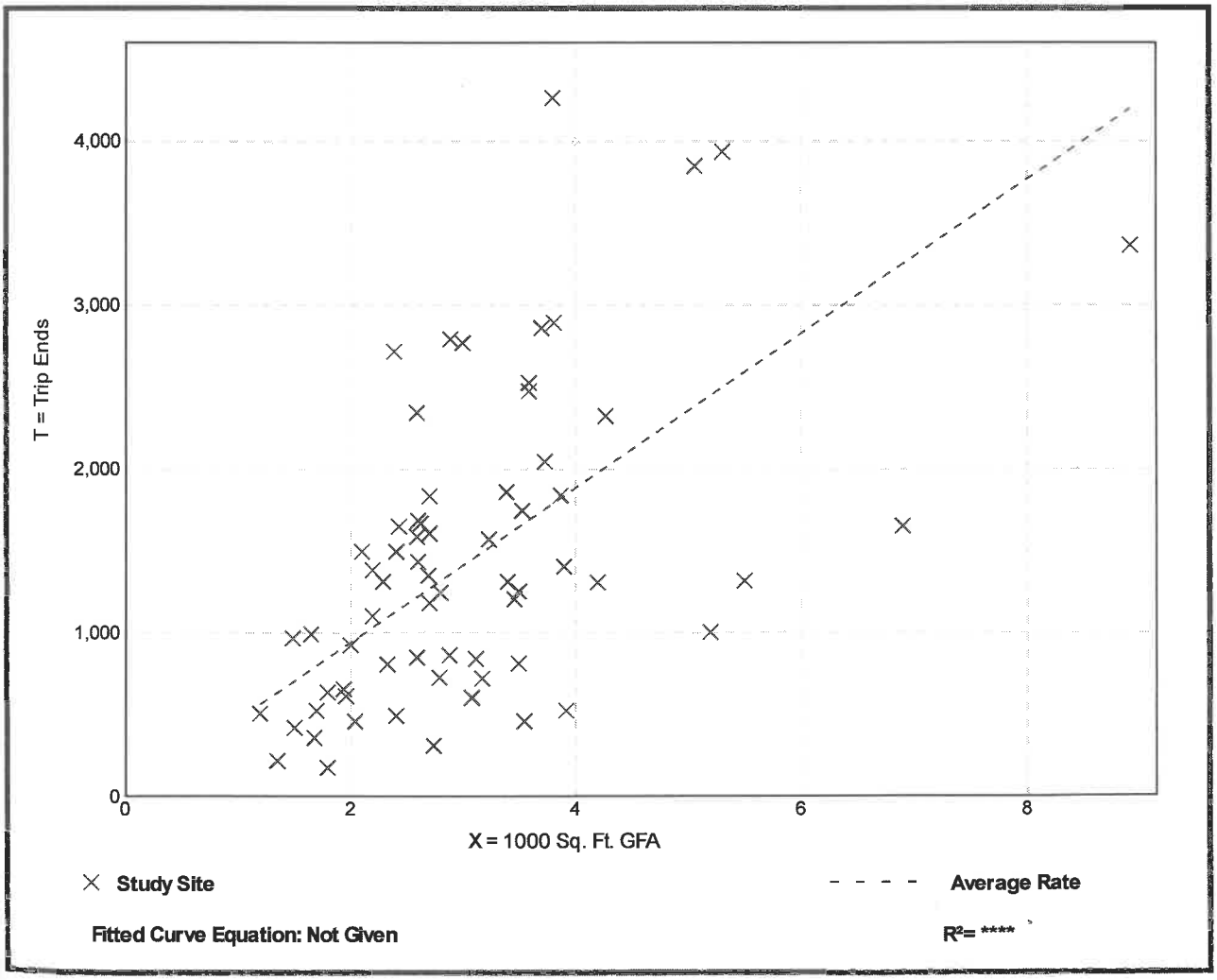
Vehicle Trip Ends vs: **1000 Sq. Ft. GFA**
On a: **Weekday**

Setting/Location: **General Urban/Suburban**
Number of Studies: 67
1000 Sq. Ft. GFA: 3
Directional Distribution: 50% entering, 50% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
470.95	98.89 - 1137.66	244.44

Data Plot and Equation



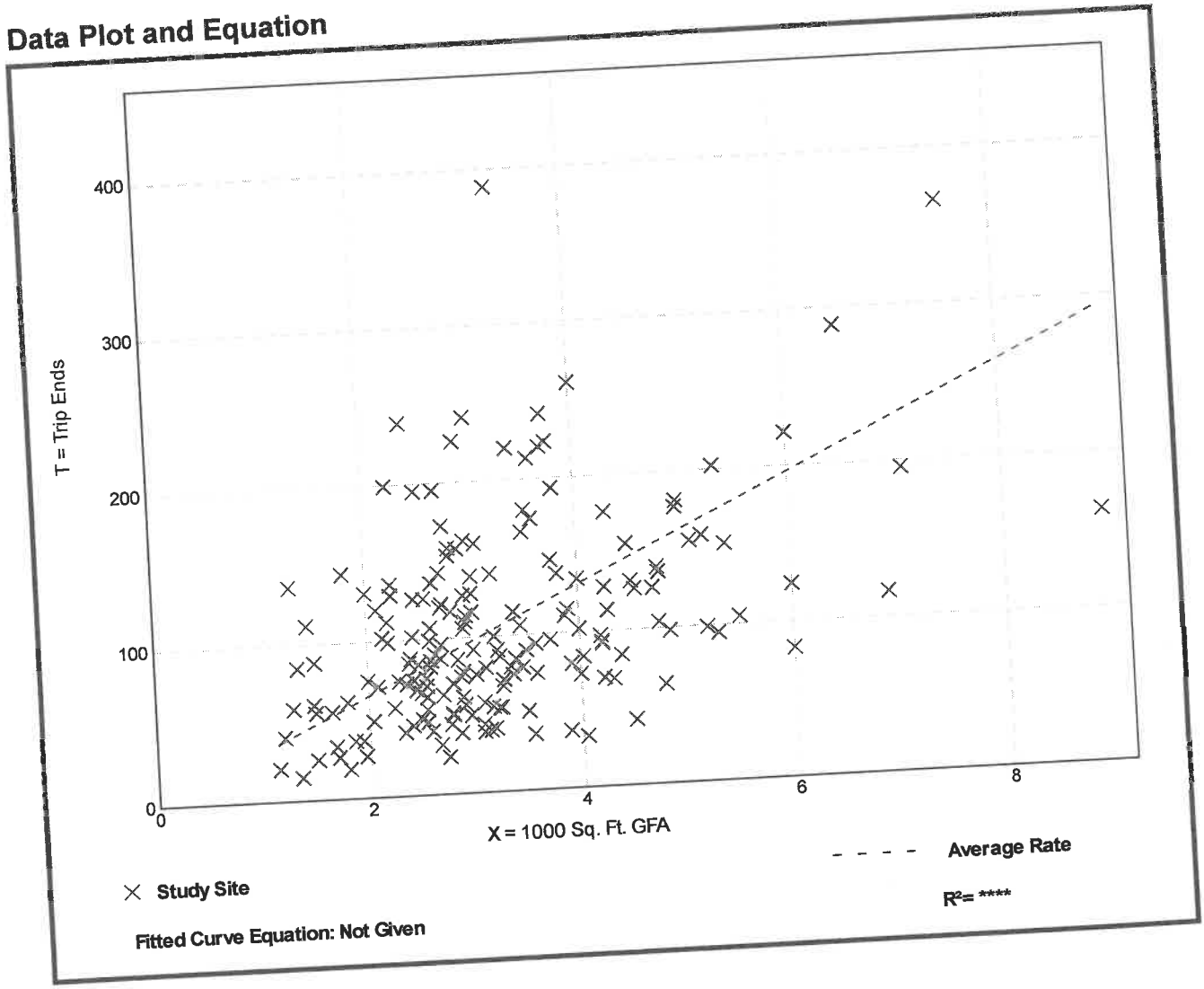
Fast-Food Restaurant with Drive-Through Window
(934)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA
On a: Weekday,
Peak Hour of Adjacent Street Traffic,
One Hour Between 4 and 6 p.m.
Setting/Location: General Urban/Suburban
Number of Studies: 185
1000 Sq. Ft. GFA: 3
Directional Distribution: 52% entering, 48% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
32.67	8.17 - 117.22	17.87

Data Plot and Equation



Fitted Curve Equation: Not Given

Table E.32 Pass-By and Non-Pass-By Trips Weekday, PM Peak Period
Land Use Code 934—Fast-Food Restaurant with Drive-Through Window

SEATS	SIZE (1,000 SQ FT GFA)	LOCATION	WEEKDAY SURVEY DATE	NO. OF INTERVIEWS	TIME PERIOD	PASS-BY TRIP (%)	NON-PASS-BY TRIPS (%)			ADJ. STREET PEAK HOUR VOLUME	SOURCE
							PRIMARY	DIVERTED	TOTAL		
—	~2.6	Minn-St. Paul, MN	1987	50	3:00–7:00 p.m.	25	27	48	75	—	—
—	<5.0	Chicago suburbs, IL	1987	80	3:00–6:00 p.m.	38	—	—	62	—	Kenig, O'Hara, Humes, Flock
—	<5.0	Chicago suburbs, IL	1987	100	3:00–6:00 p.m.	55	—	—	45	—	Kenig, O'Hara, Humes, Flock
—	<5.0	Chicago suburbs, IL	1987	159	3:00–6:00 p.m.	56	—	—	44	—	Kenig, O'Hara, Humes, Flock
—	<5.0	Chicago suburbs, IL	1987	225	3:00–6:00 p.m.	48	—	—	52	—	Kenig, O'Hara, Humes, Flock
—	<5.0	Chicago suburbs, IL	1987	88	3:00–6:00 p.m.	35	—	—	65	—	Kenig, O'Hara, Humes, Flock
—	<5.0	Chicago suburbs, IL	1987	84	3:00–6:00 p.m.	44	—	—	56	—	Kenig, O'Hara, Humes, Flock
88	1.3	Louisville area, KY	1993	—	4:00–6:00 p.m.	68	22	10	32	2,055	Barton-Aschman Assoc.
120	1.9	Louisville area, KY	1993	33	4:00–6:00 p.m.	67	24	9	33	2,447	Barton-Aschman Assoc.
87	4.2	New Albany, IN	1993	—	4:00–6:00 p.m.	56	25	19	44	1,632	Barton-Aschman Assoc.
150	3.0	Louisville area, KY	1993	—	4:00–6:00 p.m.	31	31	38	69	4,250	Barton-Aschman Assoc.
—	3.1	Kissimmee, FL	1995	28	2:00–6:00 p.m.	71	—	—	29	—	TPD Inc.
—	3.1	Apopka, FL	1996	29	2:00–6:00 p.m.	38	—	—	62	—	TPD Inc.
—	2.8	Winter Springs, FL	1995	47	2:00–6:00 p.m.	66	—	—	34	—	TPD Inc.
—	4.3	Longwood, FL	1994	304	2:00–6:00 p.m.	62	—	—	38	—	TPD Inc.
—	3.2	Altamonte Springs, FL	1996	202	2:00–6:00 p.m.	40	39	21	60	—	TPD Inc.
—	2.9	Winter Park, FL	1996	271	2:00–6:00 p.m.	41	41	18	59	—	TPD Inc.
—	3.3*	several	1996	varies	4:00–6:00 p.m.	62	—	—	38	—	Oracle Engineering

*Average of several combined studies.
Average Pass-By Trip Percentage: 50
"—" means no data were provided

Table E.31 Pass-By and Non-Pass-By Trips Weekday, AM Peak Period Land Use Code 934—Fast-Food Restaurant with Drive-Through Window

SEATS	SIZE (1,000 SQ FT GFA)	LOCATION	WEEKDAY SURVEY DATE	NO. OF INTERVIEWS	TIME PERIOD	PASS-BY TRIP (%)	NON-PASS-BY TRIPS (%)			ADJ. STREET PEAK HOUR VOLUME	SOURCE
							PRIMARY	DIVERTED	TOTAL		
—	<5	Chicago suburbs, IL	1987	84	7:00–9:00 a.m.	44	—	—	56	—	Kenig, O'Hara, Humes, Flock
88	1.4	Louisville area, KY	1993	—	7:00–9:00 a.m.	62	22	16	38	1,407	Barton-Aschman Assoc.
100	3.6	Louisville, KY	1993	—	7:00–9:00 a.m.	32	47	21	68	437	Barton-Aschman Assoc.
87	4.2	New Albany, IN	1993	—	7:00–9:00 a.m.	46	23	31	54	1,049	Barton-Aschman Assoc.
150	3.0	Louisville area, KY	1993	—	7:00–9:00 a.m.	43	14	43	57	2,903	Barton-Aschman Assoc.
—	3.3	varies	1996	—	6:00–9:00 a.m.	68	—	—	32	—	Oracle Engineering

Average Pass-By Trip Percentage: 49
"—" means no data were provided

l u-turn caused a crash

u-turn, pulled into W. U.S. Highway 27 heading eastbound into oncoming raffic and collided with a 016 Chevrolet Silverado. The Silverado driver, 9-year-old Alice Knight, lso of Fort White, had riginally been behind the fitsubishi.

The two drivers and firage passenger 19-year-ld Jacob Petrovich of Fort /hite were transported to ea hospitals with neck, oulder and possible back juries.

Both vehicles were operable and were towed om the scene.

According to the High Springs Police Department, alcohol was not suspected to be related to the crash.

# # #

Email cwalker@alachuatoday.com

y of
ACHUA

BOARD
CY

accepting applications
wing Board:
gency (CRA) Advisory
s of the CRAAB is to
CRA Board regarding
t Area, including any
dance with the CRA
ership qualifications

other municipal office;
within the Community
ap of the Community
the CRA's webpage at

serve for a three-year

nd monthly meetings
third Monday of every

ll be available on the
.com, and at City Hall
a submission deadline
planned that the CRA
oint an individual for
10, 2021, at 5:00 p.m.
d June 12, 2024.

cation process may be
Coordinator, at

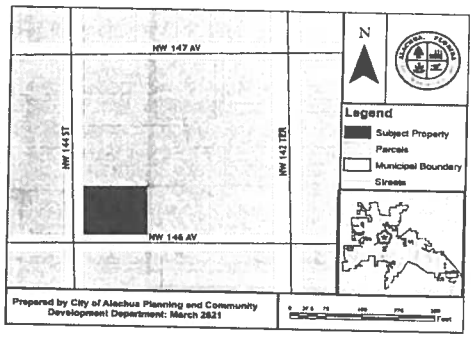
April 08 & 15, 2021)

y Today
ews - 386.462.3355

City of ALACHUA

NOTICE OF PUBLIC HEARING BEFORE THE PLANNING AND ZONING BOARD OF THE CITY OF ALACHUA, FLORIDA

Notice is hereby given that the Planning and Zoning Board of the City of Alachua will hold a public hearing on April 20, 2021, at 6:00 p.m. The hearing will be held in the James A. Lewis Commission Chambers in City Hall, located at 15100 NW 142nd Terrace, Alachua, Florida, to consider the following: a request by Raghu Consbruck and Govinda Cordua, property owners, for the approval of a Certificate of Appropriateness for the following alterations to a property located within the Historic Overlay District: construction of a new one story single family residence. Tax Parcel No. 03679-001-000; FLUM: Medium Density Residential; Zoning: Residential Single Family – 3 (RSF-3).



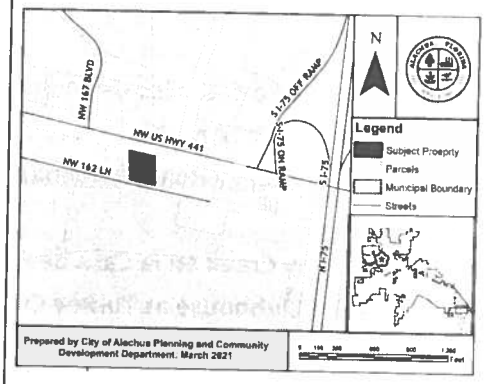
At the public hearing, all interested parties may appear and be heard with respect to the application. Copies of the application are available for public inspection at the Planning and Community Development Department, 15100 NW 142nd Terrace, Alachua, Florida, on any regular business day between the hours of 7:30 a.m. to 6:00 p.m. Written comments on the application may be sent to the following address: City of Alachua, Planning and Community Development, P.O. Box 9, Alachua, FL 32616. Notice is given pursuant to Section 286.0105, Florida Statutes, that, in order to appeal any decision made at the public hearing, you will need a record of the proceedings, and that, for such purpose, you may need to ensure that a verbatim record of the proceedings is made, which includes the testimony and evidence upon which the appeal is to be based. In accordance with the Americans with Disabilities Act, any persons with a disability requiring reasonable accommodation in order to participate in this meeting should call the City Clerk at (386) 418-6100 x 1501 at least 48 hours prior to the public hearing

(Published: Alachua County Today - April 08, 2021)

City of ALACHUA

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At the public hearing, all interested parties may appear and be heard with respect to the application. Copies of the application are available for public inspection at the Planning and Community Development Department, 15100 NW 142nd Terrace, Alachua, Florida, on any regular business day between the hours of 7:30 a.m. to 6:00 p.m. Written comments on the application may be sent to the following address: City of Alachua, Planning and Community Development, P.O. Box 9, Alachua, FL 32616. Notice is given pursuant to Section 286.0105, Florida Statutes, that, in order to appeal any decision made at the public hearing, you will need a record of the proceedings, and that, for such purpose, you may need to ensure that a verbatim record of the proceedings is made, which includes the testimony and evidence upon which the appeal is to be based. In accordance with the Americans with Disabilities Act, any persons with a disability requiring reasonable accommodation in order to participate in this meeting should call the City Clerk at (386) 418-6100 x 1501 at least 48 hours prior to the public hearing.

(Published: Alachua County Today - April 08, 2021)

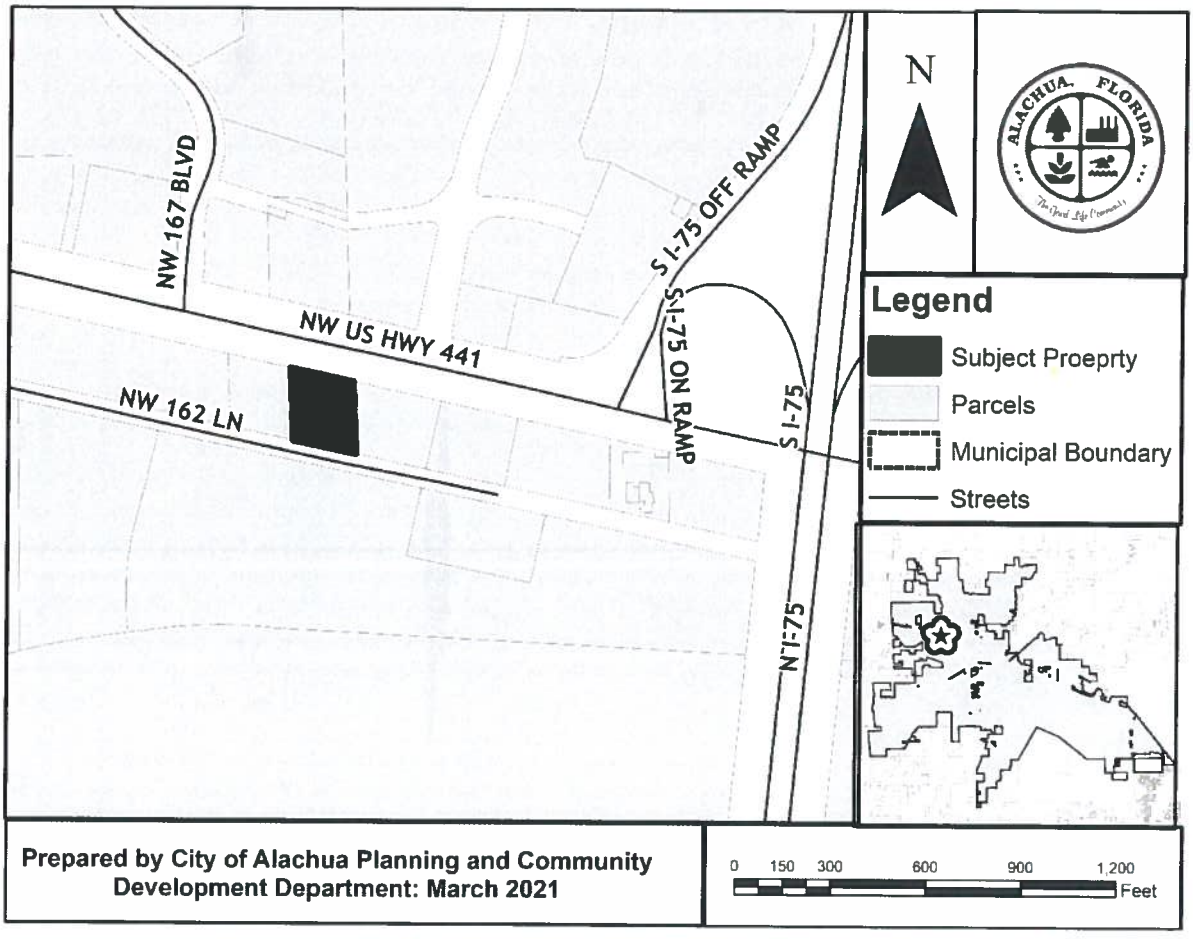
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City of ALACHUA
THE GOOD LIFE COMMUNITY

Mailed 4/5/2024 KC

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Statutes, that, in order to appeal any decision made at the public hearing, you will need a record of the proceedings, and that, for such purpose, you may need to ensure that a verbatim record of the proceedings is made, which includes the testimony and evidence upon which the appeal is to be based. In accordance with the Americans with Disabilities Act, any persons with a disability requiring reasonable accommodation in order to participate in this meeting should call the City Clerk at (386) 418-6100 x 101 at least 48 hours prior to the public hearing.

ALACHUA A ONE LLC
15260 NW 147TH DR STE 100
ALACHUA, FL 32615

ALACHUA GATEWAY CENTER SURFACE
JAMES D SALTER ESQ
3940 NW 16TH BLVD BLDG B
GAINESVILLE, FL 32605

CAVACEPPI SHARLEEN O TRUSTEE
PO BOX 1325
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DAYTONA BEACH, FL 32118

SARCAV LLC
2567 RIVER ENCLAVE LN
JACKSONVILLE, FL 32226

T & H PROPERTY GROUP LLC
3580 N MAIN ST
GAINESVILLE, FL 32609

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Tamara Robbins
PO Box 2317
Alachua, FL 32616

Dan Rhine
288 Turkey Creek
Alachua, FL 32615

Linda Dixon, AICP
PO Box 115050
Gainesville, FL 32611

Michele L. Lieberman
County Manager
12 SE 1st Street
Gainesville, FL 32601

Tom Gorman
9210 NW 59th Street
Alachua, FL 32653

Craig Parenteau
FDEP
4801 Camp Ranch Road
Gainesville, FL 32641

Bonnie Flynn
16801 NW 166th Drive
Alachua, FL 32615

Richard Gorman
5716 NW 93rd Avenue
Alachua, FL 32653

Jeannette Hinsdale
P.O. Box 1156
Alachua, FL 32616

Hugh & Jean Calderwood
P.O. Box 2307
Alachua, FL. 32616

Peggy Arnold
410 Turkey Creek
Alachua, FL 32615

Lynn Coullias
7406 NW 126th Ave
Alachua, FL 32615

Lisia Jenkins
P.O. Box 1071
Alachua, FL. 32616

David Forest
23 Turkey Creek
Alachua, FL 32615

Lynda Coon
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AFFIDAVIT FOR POSTED LAND USE SIGN

I Cole Menhennett, POSTED THE LAND USE
(Name)
SIGN ON 4/1/2021 FOR THE KFC Site Plan
(Date) (State type of action and project name)
LAND USE ACTION.

AS PER ARTICLE 2.2.9 D OF THE LAND DEVELOPMENT REGULATIONS.

THIS WILL BE INCLUDED IN THE STAFF REPORT.

(Signature)

1
(Number of signs)

City of Alachua
THE GOOD LIFE COMMUNITY

Board/Committee Agenda Item

MEETING DATE: 4/20/2021
SUBJECT: Site Plan – Alchem Building 4: A request by Claudia Vega, P.E., of EDA Consultants, Inc., applicant and agent for Alchem Laboratories Corporation, property owner, for consideration of a Site Plan to construct a ±3,788 square foot building and associated site improvements on a ±0.76 acre property; Tax Parcel Numbers 03231-003-000 and 03231-004-001 (Quasi-Judicial Hearing).

PREPARED BY: Justin Tabor, AICP, Principal Planner

RECOMMENDED ACTION:

Staff recommends that the Planning & Zoning Board approve the Site Plan for Alchem Building 4, subject to the three (3) conditions provided in Exhibit "A" and located on page 19 of the April 20, 2021 Staff Report to the Planning & Zoning Board upon making the following motion:

Based upon the competent substantial evidence presented at this hearing, the presentation before this Board, and Staff's recommendation, this Board finds the application to be consistent with the City of Alachua Comprehensive Plan and in compliance with the Land Development Regulations and approves the Site Plan Alchem Building 4, subject to the three (3) conditions provided in Exhibit "A" and located on page 19 of the April 20, 2021 Staff Report to the Planning & Zoning Board.

Summary

This application is a request by Claudia Vega, P.E., of EDA Consultants, Inc., applicant and agent for Alchem Laboratories Corporation, property owner, for consideration of a Site Plan which proposes the construction of a ±3,788 square foot building with associated site improvements. The proposed use of the building is for light manufacturing.

The subject property is ±0.76 acres in area and is located at 13305 Rachael Boulevard (also known as County Road 2054). There are two (2) buildings currently located on the subject property. The larger of these two buildings, which is ±7,191 square feet, will remain. The second existing building is a modular building (±700 square feet) and will be demolished as part of this project.

Access to the site will be provided through the redesign of an existing connection to Rachael Boulevard. The proposed development will convey the development's stormwater runoff to a stormwater basin located in the northeast corner of the subject property.

ATTACHMENTS:

Description

- Staff Report & Staff Supporting Materials
- Site Plan - Alchem Building 4
- Application & Supporting Materials
- Public Notice Materials for the 4/20/21 PZB Hearing

- Public Notice Materials for the 4/20/21 PZB Hearing

City of Alachua
Planning & Community Development Department
Staff Report

Planning & Zoning Board Hearing Date: April 20, 2021
Quasi-Judicial Hearing

SUBJECT:	A request for a Site Plan proposing the construction of a ±3,788 square foot building with associated site improvements
APPLICANT/AGENT:	Claudia Vega, P.E. EDA Consultants, Inc.
PROPERTY OWNER:	Alchem Laboratories Corporation
LOCATION:	13305 Rachael Boulevard
PARCEL ID NUMBERS:	03231-003-000 and 03231-004-000
FLUM DESIGNATION:	Industrial
ZONING:	Light & Warehouse Industrial (ILW)
OVERLAY:	N/A
ACREAGE:	±0.76 acres
PROJECT PLANNER:	Justin Tabor, AICP
RECOMMENDATION:	Staff recommends that the Planning & Zoning Board approve the Site Plan for Alchem Building 4, subject to the three (3) conditions provided in Exhibit "A" and located on page 19 of the April 20, 2021 Staff Report to the Planning & Zoning Board.
RECOMMENDED MOTION:	*Based upon the competent substantial evidence presented at this hearing, the presentation before this Board, and Staff's recommendation, this Board finds the application to be consistent with the City of Alachua Comprehensive Plan and in compliance with the Land Development Regulations and approves the Site Plan for Alchem Building 4, subject to the three (3) conditions provided in Exhibit "A" and located on page 19 of the April 20, 2021 Staff Report to the Planning & Zoning Board.*

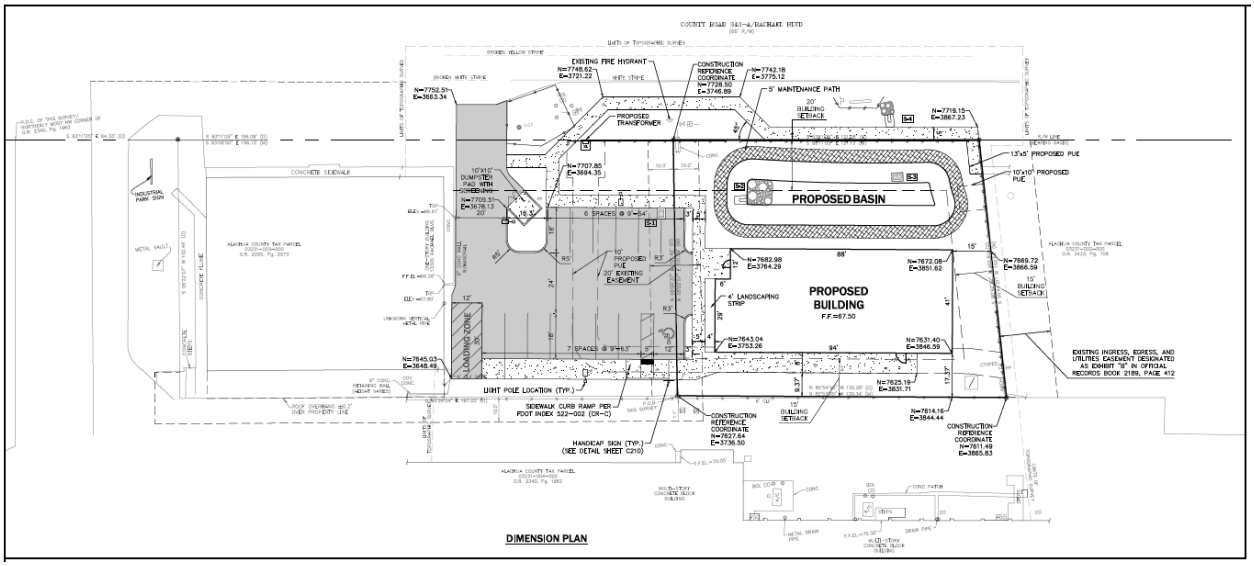
SUMMARY

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Illustration 1. Alchem Building 4 – Site & Dimension Plan



SURROUNDING USES

The existing uses, Future Land Use Map (FLUM) Designations, and zoning districts of the surrounding area are identified in Table 1. Map 1 provides an overview of the vicinity of the subject property. (NOTE: The information below is intended to provide a general overview of the area surrounding the subject property and to generally orient the reader. It is not intended to be all-inclusive, and may not identify all existing uses, FLUM Designations, and/or zoning districts surrounding the subject property.)

Map 1. Vicinity Map

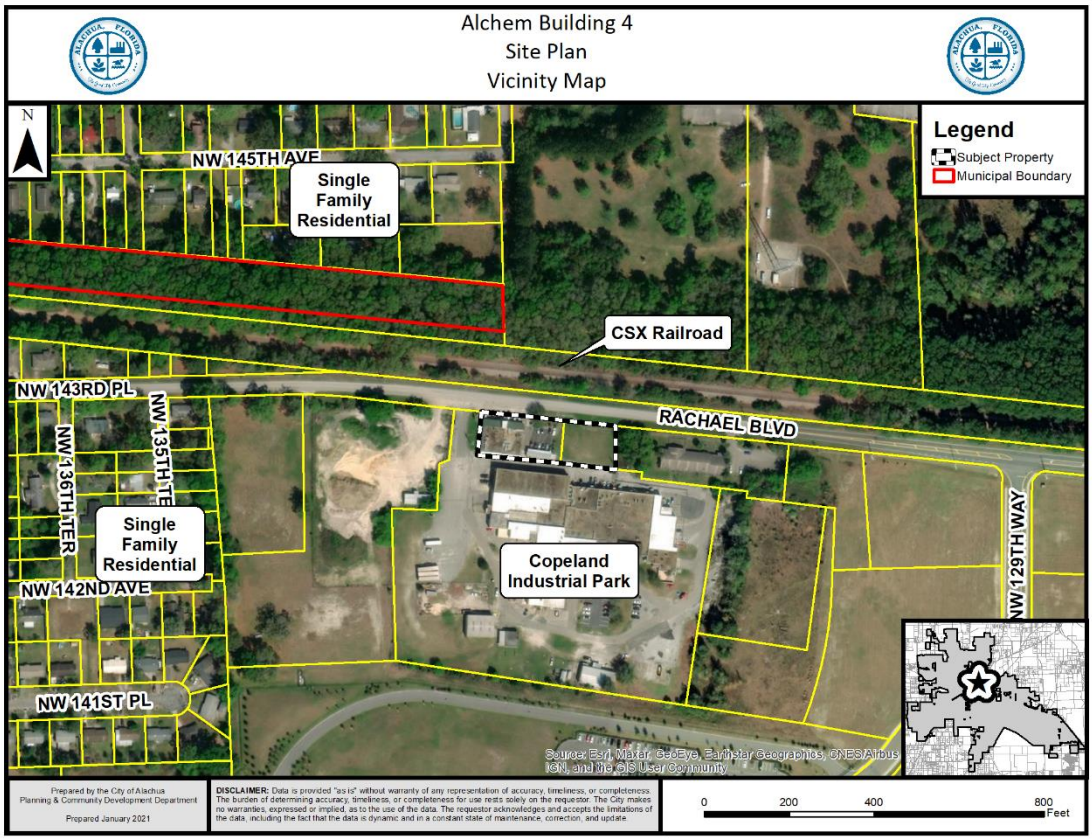


Table 1. Surrounding Land Uses

Direction	Existing Use(s)	FLUM Designation(s)	Zoning District(s)
North	Rachael Boulevard; CSX Railroad Right-of-Way	N/A	N/A
South	Copeland Industrial Park	Industrial	Light & Warehouse Industrial (ILW)
East	Metalloy Industries, Inc.	Industrial	Light & Warehouse Industrial (ILW)
West	Copeland Industrial Park	Industrial	Light & Warehouse Industrial (ILW)

NEIGHBORHOOD MEETING

The purpose of a Neighborhood Meeting is to educate the owners of nearby land and any other interested members of the public about the project and to receive comments regarding the project. As required by Section 2.2.4 of the LDRs, all property owners within 400 feet of the subject property were notified of the meeting and notice of the meeting was published in a newspaper of general circulation.

A Neighborhood Meeting was held virtually on October 26, 2020. The applicant was available to answer questions. Materials submitted by the applicant indicate that the meeting was unattended.

CONSISTENCY WITH THE COMPREHENSIVE PLAN

The Goals, Objectives, and Policies (GOPs) identified below are provided to establish a basis of the application's consistency with the Comprehensive Plan. There may be additional GOPs which the application is consistent with that are not identified within this report. An evaluation and findings of consistency with the identified GOPs is also provided below.

Future Land Use Element

GOAL 1: Future Land Use Map 2025:

The City shall maintain a Future Land Use Map in order to effectively guide development in a sustainable manner and to ensure economic prosperity and stability while maintaining a high quality of life for all of its present and future citizens, businesses, and visitors.

Objective 1.5: Industrial

The City shall establish one industrial land use category: Industrial. This land use category shall provide a broad range of clean industry, warehousing, research, and technology industries, to provide a variety of job opportunities to the citizens of Alachua and the North Central Florida Region.

Policy 1.5.a: Industrial: Industrial uses are generally intense uses that require large land area and convenient access to transportation facilities, such as roads, highways, and rail lines, or specialized uses that require specialized laboratory space and utility services. Industrial uses, such as warehousing and manufacturing, shall be located and designed in such a manner as to prevent unwanted impacts to adjacent properties.

2. The Industrial land use category may include manufacturing facilities subject to the following standards:

Type of manufacturing	Maximum Building Size	Maximum Area: Manufacturing	Maximum Area: Warehousing	Maximum Building Area Coverage
Manufacturing/ Assembly	300,000 sq. ft.	75% of total area	No Maximum	40%

3. The Industrial land use category may include flex facilities subject to the following standards:

Type of flex	Maximum Building Size	Maximum Area: Manufacturing	Maximum Area: Warehousing	Maximum Building Area Coverage
Research and Development	150,000 sq. ft.	75% of total area (may include labs and offices)	No Maximum	50%
Office Showroom	150,000 sq. ft.	None permitted	60% warehousing/ 20% retail showroom	50%
Multitenant	120,000 sq. ft.	None permitted	60%retail/ 40%warehousing	50%

Evaluation and Findings: The subject property has an Industrial FLUM Designation, which permits a variety of potential uses within the buildings, including research and development uses, multitenant uses, and manufacturing and assembly. The proposed building will be consistent with the provisions of Goal 1, Objective 1.5, and Policy 1.5.a.

Objective 2.4: Landscaping and Tree Protection Standards: The City shall adopt landscaping and tree protection standards in order to achieve the aesthetic design values of the community and preserve tree canopies, as well as to protect exceptional specimens and champion trees.

Policy 2.4.a: Landscaping: General – The City shall require landscaping plans to be submitted with each nonresidential and multiple family residential site plan. The minimum landscaped area shall be 10% of the development site, not inclusive of any designated open space areas. Landscaping designs shall incorporate principles of xeriscaping, where feasible. The City shall develop a list of preferred planting materials to assist in the landscape design. Landscape plans shall include perimeter and internal site landscaping.

Policy 2.4.b: Landscaping: Buffering – A buffer consists of horizontal space (land) and vertical elements (plants, berms, fences, walls) that physically separate and visually screen adjacent land uses. The City shall establish buffer yard requirements that are based on the compatibility of the adjacent uses and the desired result of the buffer.

Evaluation and Findings: The site plan includes a landscaping plan which demonstrates that the proposed development will comply with all applicable landscaping and buffering standards required by the City's Comprehensive Plan and Land Development Regulations. Upon completion of the development, approximately 25.67% of the site will consist of landscaped areas which exceeds the minimum 10% area required by Policy 2.4.a.

Objective 2.5: Open Space Standards: The City shall utilize open space requirements to preserve the rural character of Alachua, protect natural resources, and provide spaces for people to recreate and gather.

Policy 2.5.a: There shall be a minimum of 10% percent open space required. The City shall establish incentives for the provision of open space beyond minimum requirements.

Evaluation and Findings: The site plan indicates that following completion of the development, the subject property will consist of 10.2% open space, exceeding the minimum 10% open space requirement.

Objective 5.1: Natural Features: The City shall coordinate land use categories with appropriate topography, soils, areas of seasonal flooding, wetlands and habitat during review of proposed amendments to the Future Land Use Map and the development review process. Natural features may be included as amenities within a development project.

Evaluation and Findings: An environmental conditions and site suitability analysis has been provided in this report, and indicates that the development will not adversely affect natural features.

Objective 5.2: Availability of facilities and services: The City shall utilize a concurrency management system to ensure that the adopted level of service standards are maintained.

Policy 5.2.a: All new development shall meet level of service requirements for roadways, potable water, sanitary sewer, stormwater, solid waste, and public schools in accordance with LOS standards adopted in the elements addressing these facilities.

Evaluation and Findings: An analysis of the development's impact to public facilities has been provided within this report. This analysis demonstrates that the development will not adversely affect the level of service (LOS) standard of any monitored public facilities.

Policy 9.1: Any new development or redevelopment within a Commercial or Industrial land use category within the corporate limits, where potable water and wastewater service are available, as defined in Policy 1.2.a and Policy 4.2.b of the Community Facilities Element of the City's

Comprehensive Plan, shall connect to the City's potable water and wastewater system.

Evaluation and Findings: The development is located within the City's utility service area and will connect to potable water and wastewater facilities.

Transportation Element

Objective 1.1: Level of Service

The City shall establish a safe, convenient and efficient level of service standard for all motorized and non-motorized transportation systems.

Evaluation and Findings: An analysis of the development's impacts to transportation facilities is provided within this report. The development will not adversely affect the level of service for transportation facilities.

Policy 1.3.a: The City shall establish minimum and maximum parking standards in order to avoid excessive amounts of underutilized parking areas.

Policy 1.3.d: The City shall require landscaping within parking areas, with an emphasis on canopy trees. The City shall consider establishing incentives for landscaping in excess of minimum standards.

Policy 1.3.g: The City shall require spaces to accommodate persons with physical disabilities as required by the Americans with Disabilities Act.

Evaluation and Findings: The site plan demonstrates that the development will comply with Objective 1.1 and Policies 1.3.a, 1.3.d, and 1.3.g, which are implemented by the standards found within Section 6.1 of the City's Land Development Regulations. Required landscaping materials will be provided within parking areas. The site plan also provides the minimum number of required accessible parking spaces.

Community Facilities Element

Policy 1.1.d:

The City hereby establishes the following level of service standards for sanitary sewer facilities:

Wastewater System Levels of Service

a. Quality: Compliance with all applicable standards of the U.S. Environmental Protection Agency (EPA) and the Florida Department of Environmental Protection (FDEP).
b. Quantity: System-wide wastewater collection and treatment will be sufficient to provide a minimum of 250 gallons per day per equivalent residential unit (ERU) on an average annual basis. Plant expansion shall be planned in accordance with Rule 62-600.405, Florida Administrative Code, or subsequent provision.

c. System capacity: If the existing demand and reserved capacity of the committed use of the City's wastewater facility reaches 85% of the permitted capacity design, no further development orders for projects without reserved capacity will be issued until additional capacity becomes available or funds to increase facility capacity are committed in accordance with a development agreement.

Evaluation and Findings: An analysis of the development's impacts to sanitary sewer facilities is provided within this report. The development will not adversely affect the level of service for sanitary sewer facilities.

Policy 1.2.a: The City shall establish a Community Wastewater Service Area, which shall include all areas where wastewater service is available. Wastewater service shall be deemed available if:

3. A gravity wastewater main, wastewater pumping station, or force main exists within 2,640 feet of the property line of any proposed residential subdivision comprised of more than 5 units, or any multi-family residential development, or any commercial development, or any industrial development and the gravity wastewater system, wastewater pumping station, or force main is accessible through public utility easements or right of ways. The distance shall be measured as required for construction of the infrastructure along public utility easements and right of ways.

Evaluation and Findings: The development is located within the City's utility service area and will connect to the City's wastewater system.

Policy 2.1.a: The City hereby establishes the following level of service standards for solid waste disposal facilities for residential uses:

FACILITY TYPE	LEVEL OF SERVICE STANDARD
Solid Waste Landfill	.73 tons per capita per year

Evaluation and Findings: An analysis of the development's impacts to solid waste facilities is provided within this report. The development will not adversely affect the level of service for solid waste facilities.

Objective 3.1: *Drainage and Stormwater Level of Service Standards and Implementation*
The City shall ensure the provision of drainage and stormwater retention through level of service standards and design requirements to minimize flooding and to protect and improve water quality.

Evaluation and Findings: The proposed development will convey the development's stormwater runoff to a stormwater basin located in the northeast corner of the subject property. The proposed basin has been designed to meet the applicable

requirements of the Suwannee River Water Management District (SRWMD) and the City of Alachua LDRs.

Policy 4.1.b: The City shall establish a Community Potable Water Service Area, which includes all areas where potable water service is available. Water service shall be deemed available if:

3. A water main exists within 2,640 feet of any proposed residential subdivision with more than 5 units, or any multi-family residential development, or any commercial development, or any industrial development and water service is accessible through public utility easements or right of ways. The distance shall be measured as required for construction of the infrastructure along public utility easements and right of ways.

Evaluation and Findings: The development is located within the City's utility service area and will connect to the City's potable water system.

Policy 4.1.c: The City establishes the following level of service standards for potable water:

1. Quality: Compliance with all applicable standards of the U.S. Environmental Protection Agency (EPA) and the Florida Department of Environmental Protection.
2. Quantity: System-wide potable water distribution and treatment will be sufficient to provide a minimum of 275 gallons per day per equivalent residential unit (ERU) on an average annual basis.
3. Capacity: Plant expansion shall be planned in accordance with Florida Administrative Code. Additionally, if the existing demand and reserved capacity of the City's potable water facility reaches 85% of the permitted design capacity, no further development orders or permits for projects without reserved capacity will be issued until additional capacity becomes available or funds to increase facility capacity are committed in accordance with a development agreement.

Evaluation and Findings: An analysis of the development's impacts to potable water facilities is provided within this report. The development will not adversely affect the level of service for potable water facilities.

Conservation & Open Space Element

Policy 1.2.a:

The City shall ensure that land use designations, development practices and regulations protect native communities and ecosystems, and environmentally sensitive lands.

Policy 1.3.e:

The City's land use designations shall offer the best possible protection to threatened and endangered species.

Evaluation and Findings: The development will have minimal environmental impacts. Please reference the Environmental Conditions Analysis provided within this report for further review of specific features and environmental features.

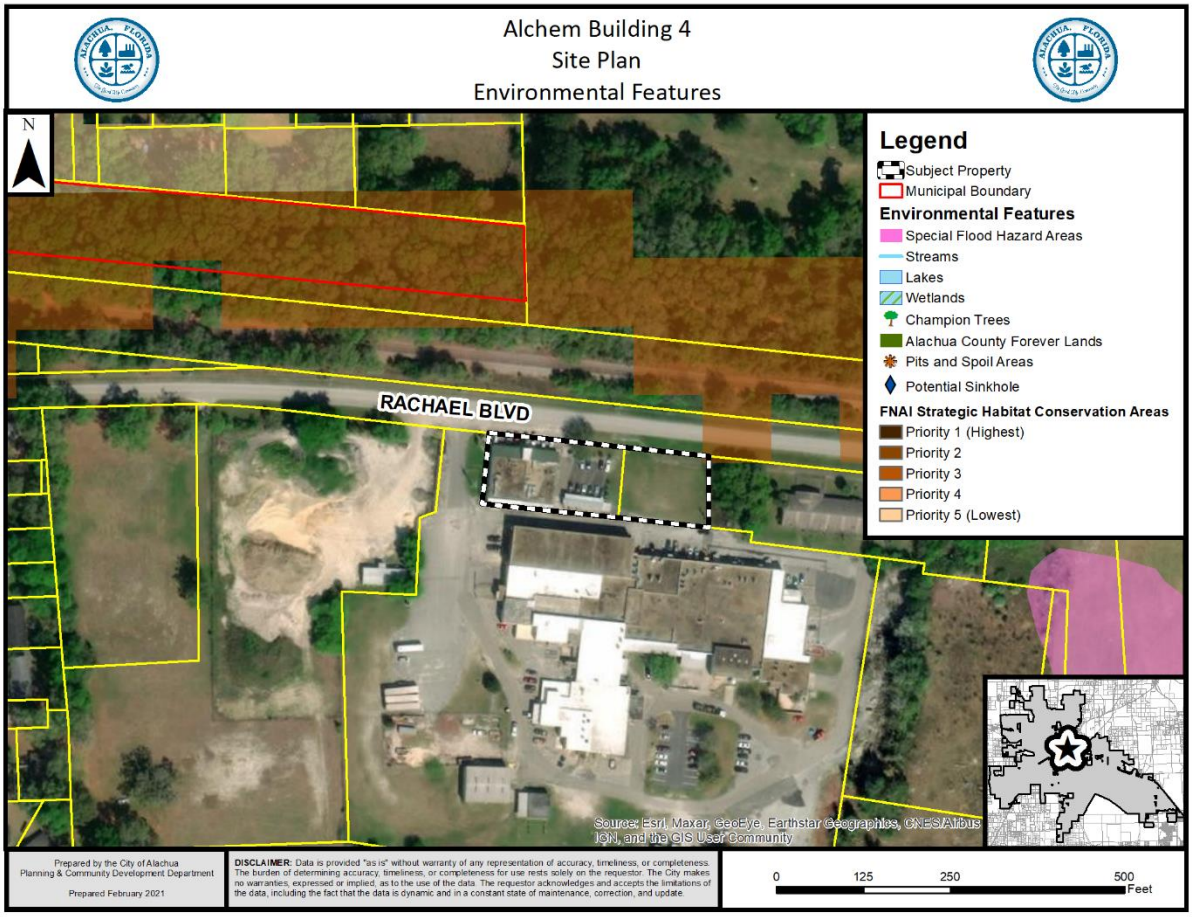
ENVIRONMENTAL CONDITIONS ANALYSIS

Wetlands

According to best available data, there are no wetlands located on the subject property. If any wetlands are identified on the subject property at a later time, these areas will be subject to the applicable protection standards of the City of Alachua Comprehensive Plan and the Land Development Regulations (LDRs.)

Evaluation: No wetlands have been identified on subject property therefore, there are no issues related to wetland protection.

Map 2. Environmental Features



Strategic Ecosystems

Strategic Ecosystems were identified by an ecological inventory project in a report prepared for Alachua County Department of Growth Management in 1987 and updated in 1996. The purpose of the inventory was to identify, inventory, map, describe, and evaluate the most significant natural biological communities in private ownership in Alachua County.

Evaluation: The subject property is not located within or adjacent to a Strategic Ecosystem, therefore, the development will have no impact upon any Strategic Ecosystem(s) identified within the ecological inventory report.

Regulated Plant & Animal Species

The subject property is not known to contain any species identified as endangered, threatened, or of special concern. The Florida Natural Areas Inventory (FNAI) has identified areas throughout the State of Florida which may contain good quality natural communities. This data layer is known as the Potential Natural Areas (PNA) data layer, and identifies privately owned lands that are not managed or listed for conservation purposes. These areas were delineated by FNAI scientific staff through interpretation of natural vegetation from 1988-1993 FDOT aerial photographs and from input received during Regional Ecological Workshops held for each regional planning council. These workshops were attended by experts familiar with natural areas in the region. Potential Natural Areas were assigned ranks of Priority 1 through Priority 5 based on size, perceived quality, and type of natural community present. The areas included in Priority 5 are exceptions to the above criteria. These areas were identified through the same process of aerial photographic interpretation and regional workshops as the PNA 1 through 4 ranked sites, but do not meet the standard criteria.

Evaluation: No species identified as endangered, threatened, or of special concern are known to exist on the subject property. The subject property does not contain any areas identified within the PNA data layer. While the FNAI PNA data layer provides an indicator of potential of lands to feature habitat which could support species identified as endangered, threatened, or of special concern, this data is not intended for use in a regulatory decision making process. The data must be referenced only as a resource to indicate the potential of land to support wildlife.

If a regulated plant or animal species is identified during development, the applicant must adhere to the applicable standards in the City of Alachua Comprehensive Plan and the Land Development Regulations.

Soil Survey

The hydrologic soil group is an indicator of potential soil limitations. The hydrologic soil group, as defined for each specific soil, refers to a group of soils which have been categorized according to their runoff-producing characteristics. These hydrologic groups are defined by the Soil Survey of Alachua County, Florida, dated August 1985. The chief consideration with respect to runoff potential is the capacity of each soil to permit infiltration (the slope and kind of plant cover are not considered, but are separate factors in predicting runoff.) There

are four hydrologic groups: A, B, C, and D. "Group A" soils have a higher infiltration rate when thoroughly wet and therefore have a lower runoff potential. "Group D" soils have very lower infiltration rates and therefore a higher runoff potential.

There is one (1) soil type found on the subject property:

Urban Land
Areas mapped as Urban Land are comprised of 85 percent or more of impervious surfaces such as parking lots, buildings, streets, sidewalks, and related facilities. The natural soil cannot be observed.

Evaluation: The soil type present on the subject property is not known to pose any significant limitations for development.

Flood Potential

Panel 0140D of the Federal Emergency Management Agency (FEMA) Flood Insurance Rate Map (FIRM) Series, dated June 16, 2006, indicates that the subject property is in Flood Zone X (areas determined to be outside of the 500-year floodplain).

Evaluation: Since the subject property is located in Flood Zone X (areas determined to be outside of the 500-year floodplain), there are no issues related to flood potential.

Karst-Sensitive Features

Karst sensitive areas include geologic features, such as fissures, sinkholes, underground streams, and caverns, and are generally the result of irregular limestone formations. The subject property is located within an area where sinkholes may potentially allow hydrologic access to the Floridan Aquifer System, however, best available data indicates that no sinkholes or known indicators of sinkhole activity are located on the subject property.

Evaluation: There are no known geologic features located on the subject property which could indicate an increased potential for karst sensitivity.

Wellfield Protection Zones

Policy 7.2.1 of the Future Land Use Element of the City's Comprehensive Plan establishes a 500 foot radius area around each city-owned potable water well.

Evaluation: The subject property is not located within a City of Alachua wellhead protection zone as identified on the City of Alachua Wellfield Primary Protection Zones Map of the City's Comprehensive Plan, therefore, there are no issues related to wellfield protection.

Historic Structures/Markers and Historic Features

The subject property does not contain any historic structures as determined by the State of Florida and the Alachua County Historic Resources Inventory. Additionally, the subject

property is not located within the City's Historic Overlay District, as established by Section 3.7 of the City's Land Development Regulations.

Evaluation: There are no issues related to historic structures or markers.

FINDINGS OF FACT: COMPLIANCE WITH LAND DEVELOPMENT REGULATIONS

SITE PLAN STANDARDS

Section 2.4.9(E) of the City's Land Development Regulations (LDRs) establishes the standards with which all site plans must be found to be compliant. The application has been reviewed for compliance with the standards of Section 2.4.9(E.) An evaluation and findings of the application's compliance with the standards of Section 2.4.9(E) is provided below.

(E) Site Plan Standards

A Site Plan shall be approved only upon a finding the applicant demonstrates all of the following standards are met:

(1) ***Consistency with Comprehensive Plan***
The development and uses in the Site Plan comply with the Goals, Objectives and Policies of the Comprehensive Plan.

Evaluation & Findings: An analysis of the application's consistency with the Comprehensive Plan has been provided in this report.

(2) ***Use Allowed in Zone District***
The use is allowed in the zone district in accordance with Article 4: *Use Regulations*.

Evaluation & Findings: The proposed development will be utilized for light manufacturing. Article 10 of the City's LDRs defines "light manufacturing" as: "the mechanical transformation of predominantly previously prepared materials into new products, including assembly of component parts and the creation of products for sale to the wholesale or retail markets or directly to consumers. Such uses are wholly confined within an enclosed building, do not include processing of hazardous gases and chemicals, and do not emit noxious noise, smoke, vapors, fumes, dust, glare, odor or vibration…" Table 4.1-1 of the City's LDRs establishes the allowable uses within each zoning district, and indicates that the ILW zoning district permits light manufacturing uses.

(3) ***Zone District Use-Specific Standards***
The development and uses in the Site Plan comply with Section 4.3, *Use-Specific Standards*.

Evaluation & Findings: The use type proposed by the development is the "light manufacturing" use type. There are no Use-Specific Standards which apply to the "light manufacturing" use type.

(4) ***Development and Design Standards***

The development proposed in the Site Plan and its general layout and design comply with all appropriate standards in Article 6: *Development Standards*.

Evaluation & Findings: The application has been reviewed for and is found to be in compliance with all relevant provisions of Article 6, *Development Standards*, including but not limited to *Section 6.1*, Off Street Parking & Loading Standards, *Section 6.2*, Tree Protection/Landscape/Xeriscape Standards, *Section 6.3*, Fencing Standards, *Section 6.4*, Exterior Lighting Standards, *Section 6.7*, Open Space Standards, and Section 6.9, Environmental Protection Standards.

(5) ***Subdivision Standards***

In cases where a subdivision has been approved or is pending, the development proposed in the Site Plan and its general layout and design comply with all appropriate standards in Article 7: *Subdivision Standards*.

Evaluation & Findings: No subdivision of land is proposed, therefore, compliance with this standard is not applicable.

(6) ***Complies with All Other Relevant Laws and Ordinances***

The proposed site plan development and use complies with all other relevant City laws and ordinances, state and federal laws, and regulations.

Evaluation & Findings: The application is consistent with all other relevant City ordinances and regulations.

ALTERNATIVE COMPLIANCE: LANDSCAPING

Section 6.2.2(D)(10) of the LDRs establishes the procedure for the approval of an alternative landscape plan. An alternative landscape plan may be used where unreasonable or impractical situations would result from application of the landscape standards as set forth in Section 6.2.2. Alternative plans, materials or methods may be justified from natural conditions, such as streams, natural rock formations, topography and physical conditions related to the site. Lot configuration and utility easements may support a request for an alternative landscape plan.

Alternative landscape plans may be approved by the LDR Administrator. Allowable deviations from the standards of this section include, but are not limited to the following:

(i) a reduction in the total number of required trees and/or alteration of the spacing requirements between trees when underground connections to public facilities or public utilities, or public easements or rights-of-way, are located upon or in close proximity to the parcel or whenever a fewer number of trees would be more desirable in terms of good landscape planning practice;

(ii) a reduction in the count, spacing, or species diversity standards which would be more desirable in terms of good landscape planning practice considering the nature of the parcel and adjacent parcels; and,
(iii) up to a 33 percent reduction in the total number of required trees provided that the cumulative caliper size of all trees to be planted meets or exceeds the total caliper inches that would have been provided otherwise.

A request for consideration of an alternative landscape plan was received from the applicant, and is described in a letter from Marsha Swider of EDA Consultants, Inc. to the City Manager/LDR Administrator, dated March 25, 2021.

The proposed deviations from the landscape requirements established in Section 6.2.2 are as follows:

- **East Perimeter Buffer Landscaping:** Due to the location of existing overhead and underground utilities along the eastern side of the site, the east perimeter buffer requirement cannot be met. To mitigate, the number of canopy and understory trees that would be required to fulfill the east perimeter buffer requirement are proposed to be located near the new stormwater management facility.
- **Site Landscaping Requirements:** Due to the location of existing overhead and underground utilities along the eastern side of the site, the east site landscaping requirement cannot be met. To mitigate, the number of canopy trees that would be required to fulfill the east site landscaping requirement are proposed to be located near the new stormwater management facility.

Based upon the description of existing site conditions and the constraints present on the property, and the allowable deviations defined in Section 6.2.2(D)(10)(b), the applicant's request for the approval an alternative landscape plan has been found to be consistent with the relevant provisions of LDRs and was approved by the LDR Administrator. The LDR Administrator's approval letter has been included within Exhibit "B", Staff Supporting Materials, attached to this Staff Report.

PUBLIC FACILITIES IMPACT

Traffic Impact

Table 2. Affected Comprehensive Plan Roadway Segments[1]

Segment Number[2, 3]	Segment Description	Lanes	Functional Classification	Area Type	LOS
8	SR 235 (from NW 143rd Place to US 441)	2/U	Major Collector	Comm	D
11	CR 235 (from NW 143rd Place to South City Limit)	2/U	Major Collector	Comm	D
15	CR 2054 (from SR 235 to US 441)	2/U	Major Collector	Comm	D

1 Source: City of Alachua Comprehensive Plan, Transportation Element.
2 For developments generating less than 1,000 trips, affected roadway segments are identified as all those wholly or partially located within ½ mile of the development's ingress/egress, or to the nearest intersecting major street, whichever is greater [Section 2.4.14(H)(2)(a)of the LDRs].
3 FDOT roadway segment number shown in parenthesis (when applicable.) For the purposes of concurrency management, COA Comprehensive Plan segments that make up a portion of a larger FDOT roadway segment will be evaluated together when determining post development roadway capacity.

Table 3. Trip Generation[1]

Land Use	AADT (Enter/Exit)[2]	AM Peak Hour (Enter/Exit)[2]	PM Peak Hour (Enter/Exit)[2]
General Light Industrial (ITE Code 110)	19 (9 / 10)	3 (3 / 0)	2 (0 / 2)

1 Source: ITE Trip Generation, 10th Edition.
2 Formulas: AADT –4.96 trips per 1,000 square feet x 3,788 square feet (50% entering/50% exiting); AM Peak Hour – 0.70 trips per 1,000 square feet x 3,788 square feet (88% entering/12% exiting); PM Peak Hour –0.63 trips per 1,000 square feet x 3,788 square feet (13% entering/87% exiting.)

Table 4a. Projected Impact on Affected Comprehensive Plan Roadway Segments (AADT)

Traffic System Category	Segment 8 SR 235 (from NW 143rd Place to US 441)[1]	Segment 11 CR 235 (from NW 143rd Place to South City Limit)[1]	Segment 15 CR 2054 (from SR 235 to US 441)[1]
Average Annual Daily Trips			
Maximum Service Volume[2]	14,400	15,120	15,120
Existing Traffic[3]	9,792	5,867	3,007
Reserved Trips[4]	152	616	2,138
Available Capacity[4]	4,456	8,637	9,975
Increase/Decrease in Daily Trips Generated by Development	5	5	19
Residual Capacity After Development's Impacts[5]	4,451	8,632	9,956

1 FDOT roadway segment number shown in parenthesis. For the purposes of concurrency management, COA Comprehensive Plan segments that make up a portion of a larger FDOT roadway segment will be evaluated together when determining post development roadway capacity.
2 AADT & Peak Hour MSVs calculated using LOSPLAN 2012. County Facilities reflect a 10 percent reduction in the MSV calculated within LOSPLAN 2012 as set forth in the Generalized Tables for AADT / Peak Hour Volumes, FDOT 2018 Q/LOS Handbook.
3 Florida State Highway System Level of Service Report, Florida Department of Transportation, District Two.
4 Source: City of Alachua March 2021 Development Monitoring Report.
5 The application is for a Final Development Order. Facility capacity and concurrency will be reserved.

Table 4b. Projected Impact on Affected Comprehensive Plan Roadway Segments (Peak Hour)

Traffic System Category	Segment 8 SR 235 (from NW 143rd Place to US 441)[1]	Segment 11 CR 235 (from NW 143rd Place to South City Limit)[1]	Segment 15 CR 2054 (from SR 235 to US 441)[1]
PM Peak Hour Trips			
Maximum Service Volume[2]	1,290	1,359	1,359
Existing Traffic[3]	930	558	354
Reserved Trips[4]	40	70	156
Available Capacity[4]	320	731	849
Increase/Decrease in PM Peak Hour Trips Generated by Development	1	1	2
Residual Capacity After Development's Impacts[5]	319	730	847

1 FDOT roadway segment number shown in parenthesis. For the purposes of concurrency management, COA Comprehensive Plan segments that make up a portion of a larger FDOT roadway segment will be evaluated together when determining post development roadway capacity.
2 AADT & Peak Hour MSVs calculated using LOSPLAN 2012. County Facilities reflect a 10 percent reduction in the MSV calculated within LOSPLAN 2012 as set forth in the Generalized Tables for AADT / Peak Hour Volumes, FDOT 2018 Q/LOS Handbook.
3 Florida State Highway System Level of Service Report, Florida Department of Transportation, District Two.
4 Source: City of Alachua March 2021 Development Monitoring Report.
5 The application is for a Final Development Order. Facility capacity and concurrency will be reserved.

Evaluation: The impacts generated by the development will not adversely affect the Level of Service (LOS) of the roadway segment identified above. The impacts that will be generated by the development are acceptable.

Potable Water Impacts

Table 5. Potable Water Impacts

System Category	Gallons Per Day
Current Permitted Capacity[1]	2,300,000
Less Actual Potable Water Flows[1]	1,295,000
Reserved Capacity[2]	181,743
Available Capacity	823,257
Increase/Decrease in Potable Water Demand from Application[3]	568
Residual Capacity	**822,689**
Percentage of Permitted Design Capacity Utilized	**64.23%**

Sources:
1 City of Alachua Public Services Department, April 2020
2 City of Alachua March 2021 Development Monitoring Report
3 Source: Chapter 64E-6, Florida Administrative Code; Formula: 15 gallons per day per 100 square feet x 3,788 square feet.

Evaluation: The impacts to the potable water system that will be generated by the development will not adversely affect the Level of Service (LOS) for potable water facilities; therefore, the impacts that will be generated by the development are acceptable.

Sanitary Sewer Impacts

Table 6. Sanitary Sewer Impacts

System Category	Gallons Per Day
Treatment Plant Current Permitted Capacity	1,500,000
Less Actual Treatment Plant Flows[1]	675,000
Reserved Capacity[2]	161,357
Available Capacity	663,643
Increase/Decrease in Sanitary Sewer Demand from Application[3]	568
Residual Capacity	**663,075**
Percentage of Permitted Design Capacity Utilized	**55.80%**

Sources:
1 *City of Alachua Public Services Department, April 2020*
2 *City of Alachua March 2021 Development Monitoring Report*
3 *Source: Chapter 64E-6, Florida Administrative Code; Formula: 15 gallons per day per 100 square feet x 3,788 square feet.*

Evaluation: The impacts to the potable water system that will be generated by the development will not adversely affect the Level of Service (LOS) for sanitary sewer facilities; therefore, the impacts that will be generated by the development are acceptable.

Solid Waste Impacts

Table 7. Solid Waste Impacts

System Category	Pounds Per Day	Tons Per Year
Demand from Existing Development[1]	41,192.00	7,517.54
Reserved Capacity[2]	18,923.62	3,453.56
Increase/Decrease in Demand Generated by Application[3]	45.40	8.29
New River Solid Waste Facility Capacity[4]	**50 years**	

Sources:
1 *University of Florida, Bureau of Economic & Business Research, Estimates of Population by County and City in Florida, 2019; Policy 2.1.a, CFNGAR Element (Formula: 10,298 persons x 0.73 tons per person per year).*
2 *City of Alachua March 2021 Development Monitoring Report.*
3 *Sincero and Sincero; Environmental Engineering: A Design Approach. Prentice Hall, New Jersey, 1996*
4 *New River Solid Waste Facility, April 2020.*

Evaluation: The impacts to the solid waste system that will be generated by the development will not adversely affect the Level of Service (LOS) for solid waste facilities. The impacts that will be generated by the development are therefore acceptable.

Recreation Facilities

The proposed development is a nonresidential development. Therefore, there are no impacts to recreation facilities. The development will have no impact to the Level of Service (LOS) of recreation facilities.

Public School Facilities

The proposed development is a nonresidential development. Therefore, there are no impacts to public school facilities. The development will have no impact to the Level of Service (LOS) of public school facilities.

EXHIBIT "A"
TO
ALCHEM BUILDING 4
SITE PLAN
STAFF REPORT

CONDITIONS:

1. The applicant agrees it shall provide Public Utilities Easements as depicted on the Site Plan. All Public Utilities Easements shall be approved by the City and recorded in the Public Records of Alachua County prior to applying for a building permit. The applicant shall incur all costs associated with preparing and recording such Public Utilities Easements.
2. The applicant agrees it shall obtain all other applicable local, state, and federal permits before the commencement of the development.
3. The applicant agrees that Conditions 1 – 2 as stated above do not inordinately burden the land and shall be binding upon the property owner, including any subsequent property owners, successors, or assigns, and that the development shall comply with Conditions 1 – 3 as stated herein.

EXHIBIT "B"

TO

ALCHEM BUILDING 4
SITE PLAN
STAFF REPORT

SUPPORTING APPLICATION MATERIALS SUBMITTED BY CITY STAFF TO THE PLANNING AND ZONING BOARD

JBrown Professional Group
CIVIL ENGINEERING • LAND SURVEYING • PLANNING
3530 NW 43rd Street • Gainesville, FL 32606 • 352.375.8999 • JBProGroup.com

April 5, 2021

Mr. Justin Tabor, AICP
Planner
City of Alachua
Office of Planning & Community Development
P.O. Box 9
Alachua, FL 32616-0009

Re: Alchem Laboratories Building 4

Dear Mr. Tabor:

As you requested, we have reviewed the Alchem Laboratories Building 4 submittal drawings and other materials provided to us for the above referenced project. The drawings reviewed were created by EDA and dated 03/29/2021. These latest plans have addressed all of our comments, and we find the project approvable from an engineering review standpoint.

Sincerely,

A. J. "Jay" Brown, Jr., PE
President, JBrown Professional Group Inc.

Cc: Claudia Vega

City of Alachua

ADAM BOUKARI
CITY MANAGER

PLANNING & COMMUNITY DEVELOPMENT
DIRECTOR KATHY WINBURN, AICP

April 1, 2021

Sent by electronic mail to cvega@edafl.com

Claudia Vega, P.E.
EDA Consultants, Inc.
720 SW 2nd Avenue
South Tower, Suite 300
Gainesville, FL 32601

RE: Planning & Zoning Board (PZB) Public Hearing: Alchem Building 4 Site Plan

Dear Ms. Vega:

On March 25, 2021, the City of Alachua received your revised application and materials for a Site Plan on behalf of Alchem Laboratories Corporation for Alchem Building 4. Based upon a review of the revised application, the City has determined that the application can now be scheduled for a hearing before the Planning and Zoning Board (PZB).

You must provide two (2) *double-sided, three-hole punched, color sets* of the **complete** application package, seven (7) sets of plans, and a digital copy of all materials in PDF format on a CD or by emailing a Cloud / FTP link to download the materials to planning@cityofalachua.com *no less than 10 business days prior to the PZB Meeting at which your application is scheduled to be heard.* The application has been scheduled for the **April 20, 2021** PZB Meeting, therefore, the above referenced materials must be submitted to the City no later than ***Monday, April 6, 2021.*** Materials may be submitted earlier than this date.

In addition, Section 2.2.9(D) of the Land Development Regulations requires the applicant to place posted notice signs on the subject property at least 14 days prior to the public hearing. Therefore, posted notice signs must be placed on the property no later than ***Monday, April 6, 2021***. Staff will contact notify you when the signs are available for pick up at City Hall.

If you plan to utilize a PowerPoint presentation or would like other materials to be available for reference during the public hearing, please submit the presentation or materials no later than 12:00 PM on the last business day prior the PZB meeting (no later than ***Monday, April 19, 2021***). Any presentation or materials may be submitted by emailing them to planning@cityofalachua.com.

Should you have any questions, please feel free to contact me at (386) 418-6100, x 107 or via email at jtabor@cityofalachua.com.

Sincerely,

Justin Tabor, AICP
Principal Planner

c: Adam Boukari, City Manager (*by electronic mail*)
Mike DaRoza, Assistant City Manager (*by electronic mail*)
Kamal Latham, Assistant City Manager (*by electronic mail*)
Kathy Winburn, AICP, Planning & Community Development Director (*by electronic mail*)
Adam Hall, AICP, Planner (*by electronic mail*)
Project File

City of Alachua

ADAM BOUKARI
CITY MANAGER

PLANNING & COMMUNITY DEVELOPMENT
DIRECTOR KATHY WINBURN, AICP

April 1, 2021

Sent by electronic mail to cvega@edafl.com

Claudia Vega, P.E.
EDA Consultants, Inc.
720 SW 2nd Avenue
South Tower, Suite 300
Gainesville, FL 32601

RE: Alternative Landscape Plan Request: Alchem Building 4 Site Plan

Dear Ms. Vega:

The City has received your request for the approval of an alternative landscape plan for the Alchem Site Plan project submitted on behalf of Alchem Laboratories Corportation. This request has been submitted pursuant to provisions established in Section 6.2.2(D)(10) of the Land Development Regulations (LDRs). Section 6.2.2(D)(10)(a) of the LDRs states, "[a]n alternative landscape plan may be used where unreasonable or impractical situations would result from the application of this section... Alternative plans may be justified from natural conditions, such as streams, natural rock formations, topography, and physical conditions related to the site. Also, the lot configuration and utility easements may justify an alternative landscape plan."

Section 6.2.2(D)(10)(b) of the LDRs states, "[t]he LDR Administrator shall approve an alternative landscape plan. Allowable deviations from the standards of this section include, but are not limited to the following:

(i) A reduction in the total number of required trees and/or alteration of the spacing requirements between trees when underground connections to public facilities or public utilities, or public easements or rights-of-way, are located upon or in close proximity to the parcel or whenever a fewer number of trees would be more desirable in terms of good landscape planning practice.

(ii) A reduction in the count, spacing, or species diversity standards which would be more desirable in terms of good landscape planning practice considering the nature of the parcel and adjacent parcels.

(iii) Up to a 33 percent reduction in the total number of required trees provided that the cumulative caliper size of all trees to be planted meets or exceeds the total caliper inches that would have been provided otherwise."

This request for an alternative landscape plan is described in a letter from Marsha Swider of EDA Consultants, Inc. to the City Manager/LDR Administrator, dated March 25, 2021.

The proposed deviations from the landscape requirements established in Section 6.2.2 are as follows:

- **East Perimeter Buffer Landscaping:** Due to the location of existing overhead and underground utilities along the eastern side of the site, the east perimeter buffer requirement cannot be met. To mitigate, the number of canopy and understory trees that would be required to fulfill the east perimeter buffer requirement are proposed to be located near the new stormwater management facility.
- **Site Landscaping Requirements:** Due to the location of existing overhead and underground utilities along the eastern side of the site, the east site landscaping requirement cannot be met. To mitigate, the number of canopy trees that would be required to fulfill the east site landscaping requirement are proposed to be located near the new stormwater management facility.

Based upon the description of existing site conditions and the constraints present on the property, and the allowable deviations defined in Section 6.2.2(D)(10)(b), this request for the approval an alternative landscape plan is found to be consistent with the relevant provisions of LDRs. As such, the alternative landscape plan is hereby approved.

If you have any questions regarding this approval, please contact the Planning & Community Development Department at 386-418-6121.

Sincerely,

Adam Boukari
LDR Administrator/City Manager

c: Mike DaRoza, Assistant City Manager (*by electronic mail*)
Kamal Latham, Assistant City Manager (*by electronic mail*)
Kathy Winburn, AICP, Planning & Community Development Director (*by electronic mail*)
Justin Tabor, AICP, Principal Planner (*by electronic mail*)
Adam Hall, AICP, Planner (*by electronic mail*)
File

City of Alachua

ADAM BOUKARI
CITY MANAGER

RODOLFO VALLADARES, P.E.
PUBLIC SERVICES DIRECTOR

INTER-OFFICE COMMUNICATION

DATE: **Mar 25, 2021**

TO: **Kathy Winburn, AICP**
Planning & Community Development Director

FROM: **Rodolfo Valladares, P.E.**
Public Services Director
Tom Ridgik, P.E.
Engineering Supervisor

TR 3/25/2021

RE: **Alchem Bldg 4 Site Plan**

Public Services has reviewed the subject project (Mar 25 Submittal) and offer the following comments. Review was specific to the Public Services Utilities.

NO.	COMMENTS
1.	**Comment on Jan 28 Submittal** **General** The attached markup of C400 is provided as a convenience in addressing the submittal comments, which are listed below. **FYI - APPROVED**
2.	**Comment on Jan 28 Submittal** **Sheet C400** To the east of the proposed basin, there is a note "TIE INTO EXISTING WATER LINE". There does not appear to an existing water line in the area. Note that the subject rerouted water line appears to connect at a point northwest of the proposed basin. **Comment on Mar 09 Submittal** **APPROVED**
3	**Comment on Jan 28 Submittal**

NO.	COMMENTS
	Sheet C400 Keyed note 4 southeast of the proposed basin. Keyed note 4 refers to a fire hydrant assembly. The fire hydrant is being supplied by a small 2-inch line, which is undersized to supply a fire hydrant. Please evaluate. **Comment on Mar 09 Submittal** **APPROVED**
4	**Comment on Jan 28 Submittal** **Sheet C400** There is also keyed note 4 northeast the proposed basin. Please confirm that the design intent is to install a fire hydrant at this location. If so, what is its water supply? **Comment on Mar 09 Submittal** **APPROVED**
5	**Comment on Jan 28 Submittal** **Sheet C400** Northeast of the proposed basin, there is a fitting (45-degree elbow) in the new, rerouted 2-inch water main. Suggest calling out this fitting. **Comment on Mar 09 Submittal** **APPROVED**
6	**Comment on Jan 28 Submittal** **Sheet C400** Please call out a PUE for the underground electric utilities and the transformer. **Comment on Mar 09 Submittal** A PUE has been added around the transformer. Also, please add a PUE around the UGE from the existing power pole to the transformer. **Submit** applicable documents. **Comment on Mar 24 Submittal** **APPROVED**
7	**Comment on Jan 28 Submittal** **Sheet C400** Identify the required voltage type (460-volt, 3-phase, etc.) for the load side of the transformer. Provide electrical load calculations to assist CoA Electrical Department in sizing the

NO.	COMMENTS
	transformer. **Comment on Mar 09 Submittal** The transformer voltage and phase requirements have been added. Electrical load calculations have not yet been submitted. Please **submit** the electrical load calculations, which are a prerequisite for CoA Electrical Department to size the transformer. **Comment on Mar 24 Submittal** **APPROVED**
8	**Comment on Jan 28 Submittal** **Sheet C400** The UGE power circuit is shown connecting to an existing power pole. Please contact CoA Electrical Department to confirm that this pole exists, or whether there is another preferred point of connection. CoA Electrical Department contact information is (1) Andy Lindsey, work cell – (352) 258-8915 and (2) Billy Shiskin, work cell – (352) 316-8182. **Comment on Mar 09 Submittal** Please confirm that the CoA Electrical Department has been contacted. Either confirm that the existing power pole will be the point of power connection or show the point of power connection at the location proposed by CoA. **Comment on Mar 24 Submittal** **APPROVED**
9	**Comment on Jan 28 Submittal** **Sheet C400** Water Fitting Schedule, Keyed Note 1: This is to confirm that CoA will perform the tap, but site conditions will dictate whether the tap is wet or dry. Thus, please change part of the note to: "1 – 2" X 1" TAP, WET OR DRY, DEPENDING ON SITE CONDITIONS" **Comment on Mar 09 Submittal** **APPROVED**
10	**Comment on Jan 28 Submittal** **Sheet C400** Please change General Utility Note 1 to read: "1. ALL CONSTRUCTION WORK SHALL BE IN ACCORDANCE WITH THE LATEST EDITON CITY OF ALACHUA, PUBLIC SERVICE DESIGN AND CONSTRUCTION MANUAL, INCLUDING STANDARD DETAILS, & FDOT, ETC." **Comment on Mar 09 Submittal** **APPROVED**

NO.	COMMENTS
11	**Comment on Jan 28 Submittal** **Sheet C410** • Replace Detail 300 with CoA Detail 200 (attached). • Replace Detail 127 with updated CoA Detail (attached). • Add Detail 501 to documents (attached) . This detail is for a single-phase load side transformer. If it turns out a 3-phase load size transformer is needed, CoA will forward the required detail. **Comment on Mar 09 Submittal** The required transformer is 3-phase. Thus, please delete Detail 501 & replace with attached details 584B, 584C & 584D. **Comment on Mar 24 Submittal** Note: Design concept has changed to a single-phase transformer. Accordingly, responses to comments have been adjusted. **APPROVED**
12	
13	
14	
15	
END OF COMMENTS	

Please advise if you have any questions or require additional information.
cc: Justin Tabor – AICP Principal Planner
Adam Hall – AICP Planner
Harry Dillard – Lead Engineering Technician

March 25, 2021

Justin Tabor, AICP
Principal Planner
City of Alachua
PO Box 9
Alachua, FL 32616

Re: Development Review Team (DRT) Comments – Resubmittal #1
Alchem Building 4 Site Plan

Dear Mr. Tabor:

The applicant's responses to the completeness review comments issued on March 17, 2021 are below.

Previous Comments – February 18, 2021 Letter
1. *Section 6.2, Tree Protection / Landscape / Xeriscape Standards*

a. The existing vegetation noted as fulfilling the east perimeter buffer requirement is located on an adjacent property that is not in common ownership with the subject property. A perimeter buffer must be provided on the subject property along the east property line.

Remaining Issues: Alternative compliance is requested in a memorandum from Marsha Swider to the City Manager/LDR Administrator (Adam Boukari) dated 3/9/2021. The memorandum notes that the existing trees on the adjacent property are proposed as mitigation to address the physical constraints present along the east project boundary. Section 6.2.2(D)(10) does not contemplate the proposed method of mitigation. Additionally, Section 6.2.2(D)(3) does not allow existing trees on an adjacent parcel to fulfill perimeter buffer requirements. The property owner has no control over the adjacent property and cannot guarantee that the buffer will remain as noted on Sheet L202. The alternative compliance request must be revised to propose a form of mitigation as allowed within Sections 6.2.2(D)(3) and (10) and all applicable notes/callouts referencing the existing buffer on the adjacent property should be removed. A potential option to mitigate through alternative compliance may include increasing the size of proposed landscaping near the east property line in order to provide the total number of caliper inches that would be required for the perimeter buffer.

RESPONSE: The Alternative Compliance Request Memo has been updated to reflect the City of Alachua proposed mitigation approach. See Memo.

b. Alternative landscape plan request will be presented to the LDR Administrator upon fully addressing all review comments.

Remaining Issues: All review comments must be fully addressed prior to consideration of the alternative landscape plan request by the LDR Administrator.

RESPONSE: All review comments have been addressed, the Alternative Compliance Request Memo has been updated to reflect the City of Alachua proposed mitigation approach. See Memo.

2. *Utilities*

a. The proposed connection to the existing sanitary sewer manhole is located on the adjacent property to the east of the site. A Public Utility Easement is required over the existing line.

Remaining Issues: Comment not addressed. It was discussed during the February 22, 2021 DRT Meeting a PUE is believed to be in existence over the existing sanitary sewer line on the adjacent property. Staff requested the recording information for the existing PUE be added to the plan. Add recording information or coordinate with the adjacent owner to obtain the PUE.

RESPONSE: The adjacent PUE is now shown.

3. *Miscellaneous*

a. A sidewalk with a minimum width of 5 feet shall be provided along the subject property's frontage of CR 2054. Reference supporting plans/reports adopted by the City Commission, including:

i. Comprehensive Plan: GOPs, including but not limited to: Vision Element: Goal 3; Future Land Use Element: Policy 1.3.d; Transportation Element: Objective 1.4; Policy 1.5.c; Conservation & Open Space Element: Policy 1.13.b;

ii. Alachua Market and Economic Impact Development Implementation Plan, dated July 16, 2016 (see page 46).

b. The access drive to the proposed dumpster pad is 10' in width. This is inadequate to provide access to a garbage truck without interfering with the loading area.

c. Dimension a typical parking space width on the dimension plan.

Remaining Issues: Provide a PUE over the sidewalk where located on the subject property.

RESPONSE: A PUE is now provided over the sidewalk located on the subject property.

New Comments – March 9, 2021 Submittal

4. *Section 4.4, Accessory Uses and Structures*

a. Section 4.4.4(E) states that no accessory structure (except for permitted fences or walls erected on a property line) may occupy a required front. Additionally, Section 4.4.4(E) states that no accessory structure may project beyond the front building line of the principal structure or site. The dumpster pad has been relocated to be within the front setback area and in front of the front building line of the proposed building. Please address.

RESPONSE: The dumpster has been relocated to be outside of the setback and behind the front building line of the principle existing structure on the project site.

5. *Section 6.2, Tree Protection / Landscape / Xeriscape Standards*

a. Alternative compliance is proposed for two landscape requirements: (1) site landscaping; and (2) east perimeter buffer. Information about the request concerning site landscaping requirements is addressed on Sheet L201, but the information concerning the east perimeter buffer requirements is addressed in a memorandum dated 3/9/2021 from Marsha Swider to the City Manager/LDR Administrator. The alternative landscape plan request should be summarized in one location, either on the plans or in a separate document (such as a letter or memorandum). If summarized in a separate document, reference the document on the plans.

RESPONSE: The Alternative Compliance Request Memo has been updated to include the alternative compliance request for both (1) Site Landscaping Requirements, Canopy Tree Requirements Per 6.2.2.D.(1)(d)(i) and (2) Site Perimeter Landscape Buffer Requirements Per 6.2.2.D.(3). The document has been referenced on Sheet L201 and L202.

6. *Public Services / Fire Rescue / Engineering Review Comments*

a. The applicant must address the comments provided by Tom Ridgik, P.E., Engineering Supervisor, in a memorandum dated March 16, 2021.

b. The applicant must address the comments provided by A.J. "Jay" Brown, P.E., of JBrown Professional Group, Inc., as provided in a memorandum dated March 17, 2021.

RESPONSE: All comments provided by Tom Ridgik, P.E. and JBrown Professional Group, Inc. have been addressed.

7. *Miscellaneous*

a. There are inconsistencies in the proposed building area. Sheet C100 indicates the proposed building will be 3,788 square feet (Impervious Area Calculations, line 6), but the landscape plans (Sheet L201) indicates the proposed building will be 3,683 square feet. Please correct the discrepancies.

RESPONSE: Sheet L201 of the Landscape Plans have been updated to reflect the corrected proposed building expansion of 3,788 square feet. See Sheet L201 for updates.

b. When addressing the requirements of Section 8.7.2, two different percentages are used for parking (48%) and landscaping (47%). Please ensure the application of Section 8.7.2 is consistent.

RESPONSE: All percentages have been updated to reflect the corrected percentage of 48%. See Sheet L201 for updates.

c. Sheet C120: Portions of the existing sidewalk within the CR 2054 ROW are noted to be removed but are not hatched.

RESPONSE: All portions of the sidewalk to be removed are now hatched.

Public Services Comments

6. Sheet C400: Please add a PUE around the UGE from the existing power pole to the transformer. Submit applicable documents.

RESPONSE: A PUE has been added over the UGE to the existing power pole.

7. Sheet C400: Electrical load calculations have not yet been submitted. Please submit the electrical load calculations, which are a prerequisite for CoA Electrical Department to size the transformer.

RESPONSE: Electrical load calculations have been included with this submittal.

8. Sheet C400: The UGE power circuit is shown connecting to an existing power pole. Please contact CoA Electrical Department to confirm that this pole exists, or whether there is another preferred point of connection.

Please confirm that the CoA Electrical Department has been contacted. Either confirm that the existing power pole will be the point of power connection or show the point of power connection at the location proposed by CoA.

RESPONSE: We have been in contact with Andy Lindsey of the CoA Electrical Department and confirmed the point of connection will be the existing power pole.

11. Sheet C410: The required transformer is 3-phase. Thus, please delete Detail 501 & replace with attached details 584B, 584C & 584D.

RESPONSE: The plans have been revised to call for a single-phase transformer.

JBrown Comments

C200 – Paving Grading & Drainage Plan
1. Please show additional grading contours and spot elevations on the south side of the building. Concerns of flooding from stormwater runoff coming from the south of the property.

RESPONSE: Additional contours and spot elevations have been added.

2. Please show S-2 coming into the basin perpendicular to the side slopes.

RESPONSE: S-2 is shown as perpendicular to the basin side slopes as possible.

3. Please revise grading on the sidewalk near the southwest corner of the building. Concerns of ponding of water. Please verify to ensure that there is positive discharge away from the building.

RESPONSE: Grading has been revised near the southwest corner of the building.

City of Alachua

ADAM BOUKARI
CITY MANAGER

PLANNING & COMMUNITY DEVELOPMENT
DIRECTOR KATHY WINBURN, AICP

March 17, 2021

Sent by electronic mail to cvega@edafl.com

Claudia Vega, P.E.
EDA Consultants, Inc.
720 SW 2nd Avenue
South Tower, Suite 300
Gainesville, FL 32601

RE: Development Review Team (DRT) Comments – Resubmittal #1: Alchem Building 4 Site Plan

Dear Ms. Vega:

On March 9, 2021, the City of Alachua received your application for a Site Plan on behalf of the Alchem Laboratories Corporation. The application proposes the construction of a ±3,788 square foot building and associated site improvements on a ±0.76 acre property, located at 13305 Rachael Boulevard and consisting of Tax Parcel Number 03231-003-000 and 03231-004-001. The revised application received on March 9, 2021 was submitted to address the Development Review Team (DRT) comments issued to you in a letter dated February 18, 2021 and as discussed at a DRT Meeting held on February 22, 2021.

Upon review of the revised application and materials, additional revisions must be made to the application before the application may be scheduled for a hearing before the Planning & Zoning Board (PZB).

Please address all insufficiencies in writing and provide an indication as to how they have been addressed by **5:00 PM** on **Thursday, March 25, 2021**. A total of four (4) copies of the application package, plans, and a digital copy of all materials in PDF format on a CD or sent by emailing a Cloud / FTP link must be provided by this date. If all comments are addressed by the resubmission date above, the application may be scheduled for the **April 20, 2021 Planning & Zoning Board (PZB) Meeting.**

Please address the following:

Previous Comments – February 18, 2021 Letter

1. *Section 6.2, Tree Protection / Landscape / Xeriscape Standards*
   a. The existing vegetation noted as fulfilling the east perimeter buffer requirement is located on an adjacent property that is not in common ownership with the subject property. A perimeter buffer must be provided on the subject property along the east property line.

      Remaining Issues: Alternative compliance is requested in a memorandum from Marsha Swider to the City Manager/LDR Administrator (Adam Boukari) dated 3/9/2021. The memorandum notes that the existing trees on the adjacent property are proposed as mitigation to address the physical constraints present along the east project boundary.

Section 6.2.2(D)(10) does not contemplate the proposed method of mitigation. Additionally, Section 6.2.2(D)(3) does not allow existing trees on an adjacent parcel to fulfill perimeter buffer requirements. The property owner has no control over the adjacent property and cannot guarantee that the buffer will remain as noted on Sheet L202. The alternative compliance request must be revised to propose a form of mitigation as allowed within Sections 6.2.2(D)(3) and (10) and all applicable notes/callouts referencing the existing buffer on the adjacent property should be removed. A potential option to mitigate through alternative compliance may include increasing the size of proposed landscaping near the east property line in order to provide the total number of caliper inches that would be required for the perimeter buffer.

b. Alternative landscape plan request will be presented to the LDR Administrator upon fully addressing all review comments.

Remaining Issues: All review comments must be fully addressed prior to consideration of the alternative landscape plan request by the LDR Administrator.

2. *Utilities*
   a. The proposed connection to the existing sanitary sewer manhole is located on the adjacent property to the east of the site. A Public Utility Easement is required over the existing line.

   Remaining Issues: Comment not addressed. It was discussed during the February 22, 2021 DRT Meeting a PUE is believed to be in existence over the existing sanitary sewer line on the adjacent property. Staff requested the recording information for the existing PUE be added to the plan. Add recording information or coordinate with the adjacent owner to obtain the PUE.

3. *Miscellaneous*
   a. A sidewalk with a minimum width of 5 feet shall be provided along the subject property's frontage of CR 2054. Reference supporting plans/reports adopted by the City Commission, including:
      i. Comprehensive Plan: GOPs, including but not limited to: Vision Element: Goal 3; Future Land Use Element: Policy 1.3.d; Transportation Element: Objective 1.4; Policy 1.5.c; Conservation & Open Space Element: Policy 1.13.b;
      ii. Alachua Market and Economic Impact Development Implementation Plan, dated July 16, 2016 (see page 46).
   b. The access drive to the proposed dumpster pad is 10' in width. This is inadequate to provide access to a garbage truck without interfering with the loading area.
   c. Dimension a typical parking space width on the dimension plan.

   Remaining Issues: Provide a PUE over the sidewalk where located on the subject property.

New Comments – March 9, 2021 Submittal

4. *Section 4.4, Accessory Uses and Structures*
   a. Section 4.4.4(E) states that no accessory structure (except for permitted fences or walls erected on a property line) may occupy a required front. Additionally, Section 4.4.4(E) states that no accessory structure may project beyond the front building line of the principal structure or site. The dumpster pad has been relocated to be within the front setback area and in front of the front building line of the proposed building. Please address.

5. *Section 6.2, Tree Protection / Landscape / Xeriscape Standards*
   a. Alternative compliance is proposed for two landscape requirements: (1) site landscaping; and (2) east perimeter buffer. Information about the request concerning site landscaping requirements is addressed on Sheet L201, but the information concerning the east perimeter buffer requirements is addressed in a memorandum dated 3/9/2021 from Marsha Swider to the City Manager/LDR Administrator. The alternative landscape plan request should be summarized in one location, either on the plans or in a separate document (such as a letter or memorandum). If summarized in a separate document, reference the document on the plans.

6. *Public Services / Fire Rescue / Engineering Review Comments*
   a. The applicant must address the comments provided by Tom Ridgik, P.E., Engineering Supervisor, in a memorandum dated March 16, 2021.
   b. The applicant must address the comments provided by A.J. "Jay" Brown, P.E., of JBrown Professional Group, Inc., as provided in a memorandum dated March 17, 2021.

7. *Miscellaneous*
   a. There are inconsistencies in the proposed building area. Sheet C100 indicates the proposed building will be 3,788 square feet (Impervious Area Calculations, line 6), but the landscape plans (Sheet L201) indicates the proposed building will be 3,683 square feet. Please correct the discrepancies.
   b. When addressing the requirements of Section 8.7.2, two different percentages are used for parking (48%) and landscaping (47%). Please ensure the application of Section 8.7.2 is consistent.
   c. Sheet C120: Portions of the existing sidewalk within the CR 2054 ROW are noted to be removed but are not hatched.

If you have any questions regarding the information above, please contact me at 386-418-6100 x 1602 or via e-mail at jtabor@cityofalachua.com. We look forward to receiving your revised application.

Sincerely,

Justin Tabor, AICP
Principal Planner

Attachments: Memorandum from Tom Ridgik, P.E., Engineering Supervisor, dated March 16, 2021
Memorandum from A.J. "Jay" Brown, P.E., of JBrown Professional Group, Inc., dated March 17, 2021

c: Adam Boukari, City Manager *(by electronic mail)*
Mike DaRoza, Assistant City Manager *(by electronic mail)*
Kamal Latham, Assistant City Manager *(by electronic mail)*
Kathy Winburn, AICP, Planning & Community Development Director *(by electronic mail)*
Adam Hall, AICP, Planner *(by electronic mail)*
Project File

City of Alachua

ADAM BOUKARI
CITY MANAGER

RODOLFO VALLADARES, P.E.
PUBLIC SERVICES DIRECTOR

INTER-OFFICE COMMUNICATION

DATE: **Mar 16, 2021**

TO: **Kathy Winburn, AICP**
Planning & Community Development Director

FROM: **Rodolfo Valladares, P.E.**
Public Services Director
Tom Ridgik, P.E.
Engineering Supervisor

TR 3/16/2021

RE: **Alchem Bldg 4 Site Plan**

Public Services has reviewed the subject project (Mar 09 Submittal) and offer the following comments. Review was specific to the Public Services Utilities.

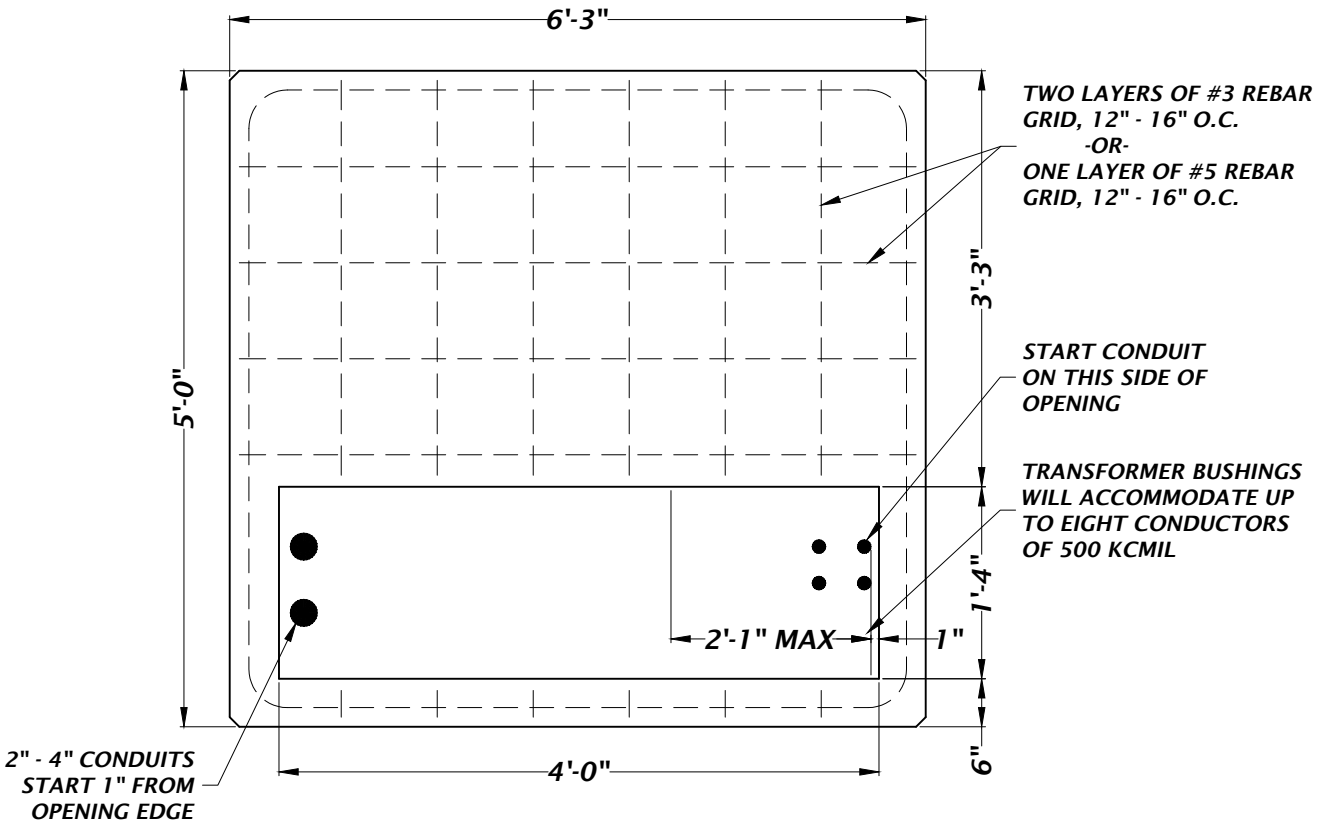
NO.	COMMENTS
1.	**Comment on Jan 28 Submittal** **General** The attached markup of C400 is provided as a convenience in addressing the submittal comments, which are listed below. **FYI - APPROVED**
2.	**Comment on Jan 28 Submittal** **Sheet C400** To the east of the proposed basin, there is a note "TIE INTO EXISTING WATER LINE". There does not appear to an existing water line in the area. Note that the subject rerouted water line appears to connect at a point northwest of the proposed basin. **Comment on Mar 09 Submittal** **APPROVED**
3	**Comment on Jan 28 Submittal**

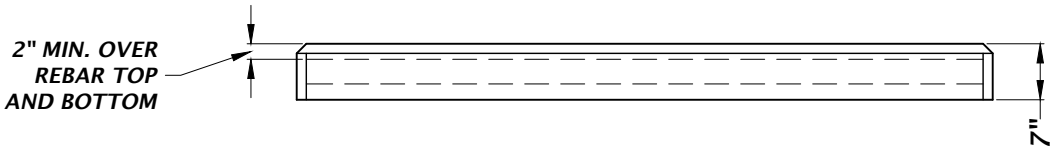
NO.	COMMENTS
	Sheet C400 Keyed note 4 southeast of the proposed basin. Keyed note 4 refers to a fire hydrant assembly. The fire hydrant is being supplied by a small 2-inch line, which is undersized to supply a fire hydrant. Please evaluate. **Comment on Mar 09 Submittal** **APPROVED**
4	**Comment on Jan 28 Submittal** **Sheet C400** There is also keyed note 4 northeast the proposed basin. Please confirm that the design intent is to install a fire hydrant at this location. If so, what is its water supply? **Comment on Mar 09 Submittal** **APPROVED**
5	**Comment on Jan 28 Submittal** **Sheet C400** Northeast of the proposed basin, there is a fitting (45-degree elbow) in the new, rerouted 2-inch water main. Suggest calling out this fitting. **Comment on Mar 09 Submittal** **APPROVED**
6	**Comment on Jan 28 Submittal** **Sheet C400** Please call out a PUE for the underground electric utilities and the transformer. **Comment on Mar 09 Submittal** A PUE has been added around the transformer. Also, please add a PUE around the UGE from the existing power pole to the transformer. **Submit** applicable documents.
7	**Comment on Jan 28 Submittal** **Sheet C400** Identify the required voltage type (460-volt, 3-phase, etc.) for the load side of the transformer. Provide electrical load calculations to assist CoA Electrical Department in sizing the transformer. **Comment on Mar 09 Submittal**

NO.	COMMENTS
	The transformer voltage and phase requirements have been added. Electrical load calculations have not yet been submitted. Please **submit** the electrical load calculations, which are a prerequisite for CoA Electrical Department to size the transformer.
8	**Comment on Jan 28 Submittal** **Sheet C400** The UGE power circuit is shown connecting to an existing power pole. Please contact CoA Electrical Department to confirm that this pole exists, or whether there is another preferred point of connection. CoA Electrical Department contact information is (1) Andy Lindsey, work cell – (352) 258-8915 and (2) Billy Shiskin, work cell – (352) 316-8182. **Comment on Mar 09 Submittal** Please confirm that the CoA Electrical Department has been contacted. Either confirm that the existing power pole will be the point of power connection or show the point of power connection at the location proposed by CoA.
9	**Comment on Jan 28 Submittal** **Sheet C400** Water Fitting Schedule, Keyed Note 1: This is to confirm that CoA will perform the tap, but site conditions will dictate whether the tap is wet or dry. Thus, please change part of the note to: "1 – 2" X 1" TAP, WET OR DRY, DEPENDING ON SITE CONDITIONS" **Comment on Mar 09 Submittal** **APPROVED**
10	**Comment on Jan 28 Submittal** **Sheet C400** Please change General Utility Note 1 to read: "1. ALL CONSTRUCTION WORK SHALL BE IN ACCORDANCE WITH THE LATEST EDITON CITY OF ALACHUA, PUBLIC SERVICE DESIGN AND CONSTRUCTION MANUAL, INCLUDING STANDARD DETAILS, & FDOT, ETC." **Comment on Mar 09 Submittal** **APPROVED**
11	**Comment on Jan 28 Submittal** **Sheet C410** • Replace Detail 300 with CoA Detail 200 (attached). • Replace Detail 127 with updated CoA Detail (attached). • Add Detail 501 to documents (attached) . This detail is for a single-phase load side transformer. If it turns out a 3-phase load size transformer is needed, CoA will

NO.	COMMENTS
	forward the required detail. **Comment on Mar 09 Submittal** The required transformer is 3-phase. Thus, please delete Detail 501 & replace with attached details 584B, 584C & 584D.
12	
13	
14	
15	
	END OF COMMENTS

Please advise if you have any questions or require additional information.
cc: Justin Tabor – AICP Principal Planner
Adam Hall – AICP Planner
Harry Dillard – Lead Engineering Technician





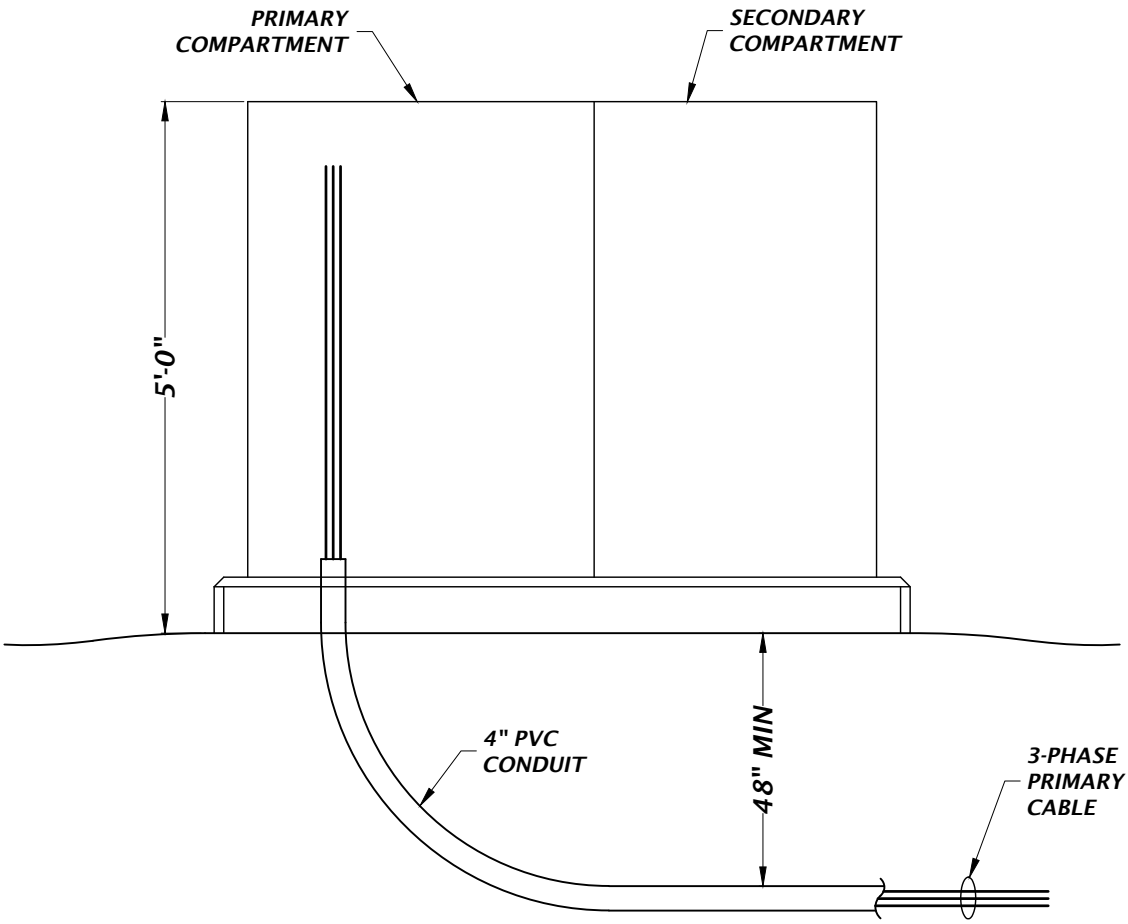
NOTES:

1. *LOCATE TRANSFORMER FRONT 8' TO 15' FROM PAVING FOR LINE TRUCK ACCESS.*
2. *LOCATE TRANSFORMER A MINIMUM OF 4' FROM THE BUILDING DRIP LINE AND 10' FROM ANY WINDOW OR DOOR.*
3. *MAINTAIN 10' SEPARATION FROM FIRE HYDRANT, TRASH DUMPSTER OR TRASH COMPACTOR.*
4. *TERMINATE CONDUITS FLUSH WITH TOP OF PAD.*
5. *CONCRETE SHALL BE 3500 PSI POURED IN PLACE OR COA-APPROVED PRECAST*

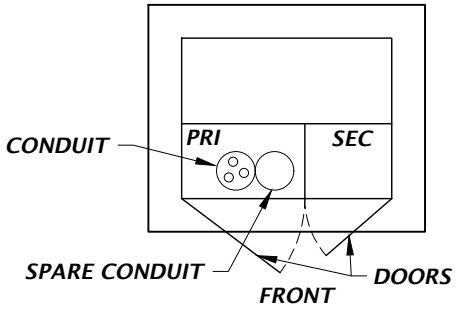
584B

CONCRETE MOUNTING PAD AND CONDUIT PLACEMENT FOR THREE-PHASE TRANSFORMERS, 75-500 KVA

NTS



VIEW LOOKING TOWARD THE TRANSFORMER FRONT
ELEVATION 3-PHASE PAD MOUNT TRANSFORMER



PLAN VIEW

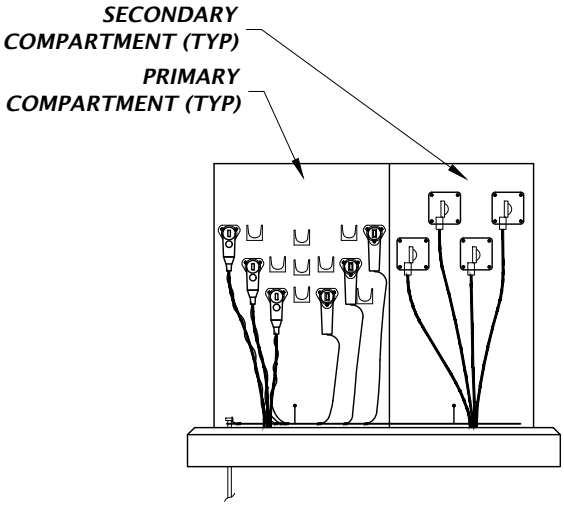
NOTES:

1. *STUB UP 3-PHASE PRIMARY CABLE 5'-0" ABOVE GROUND AND IN THE CENTER OF THE PRIMARY TRANSFORMER COMPARTMENT.*
2. *CAP OR TAPE ALL CUT CABLE ENDS.*
3. *3-PHASE PRIMARY REQUIRES 4" PVC CONDUIT.*
4. *CONDUIT AND CONDUCTORS ARE SHOWN FOR THE RADIAL FEED/LOOP TRANSFORMER CONFIGURATION. FOR LOOP FEED AND OPEN LOOP FEED TRANSFORMERS, PROVIDE CONDUIT AND CONDUCTORS AS REQUIRED.*

584C

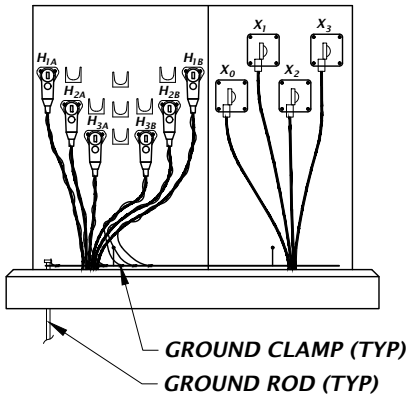
THREE-PHASE PAD MOUNT CONDUIT PLACEMENT

NTS



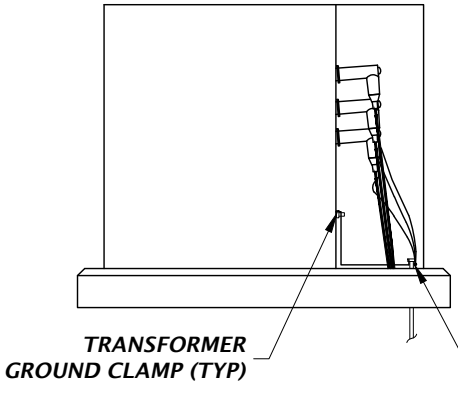
RADIAL FEED (LOOP TRANSFORMER)

FRONT ELEVATION, COVER REMOVED

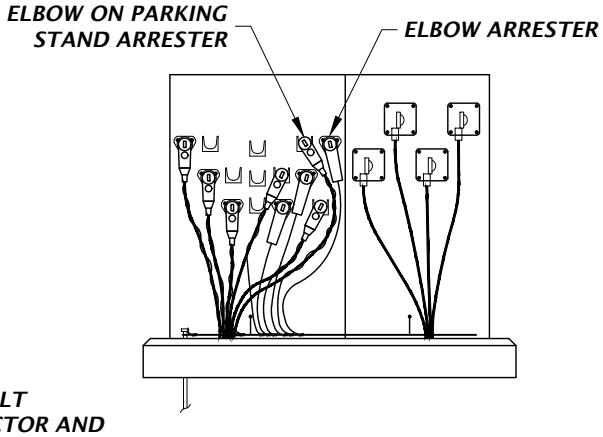


LOOP FEED

FRONT ELEVATION, COVER REMOVED



SIDE ELEVATION, COVER REMOVED



OPEN LOOP FEED

FRONT ELEVATION, COVER REMOVED

584D

THREE-PHASE PAD MOUNT TRANSFORMER CONFIGURATIONS

NTS

JBrown Professional Group
CIVIL ENGINEERING • LAND SURVEYING • PLANNING
3530 NW 43rd Street • Gainesville, FL 32606 • 352.375.8999 • JBProGroup.com

March 17, 2021

Mr. Justin Tabor, AICP
Planner
City of Alachua
Office of Planning & Community Development
P.O. Box 9
Alachua, FL 32616-0009

Re: Alchem Laboratories Building 4

Dear Mr. Tabor:

As you requested, we have reviewed the Alchem Laboratories Building 4 submittal drawings and other materials provided to us for the above referenced project. The drawings reviewed were created by EDA and dated 03/09/2021. Our review generated a few minor comments and recommendations that are outlined below.

C200 – Paving Grading & Drainage Plan

1. Please show additional grading contours and spot elevations on the south side of the building. Concerns of flooding from stormwater runoff coming from the south of the property.
2. Please show S-2 coming into the basin perpendicular to the side slopes.
3. Please revise grading on the sidewalk near the southwest corner of the building. Concerns of ponding of water. Please verify to ensure that there is positive discharge away from the building.

Sincerely,

A. J. "Jay" Brown, Jr., PE
President, JBrown Professional Group Inc.

Cc: Claudia Vega

March 9, 2021

Justin Tabor, AICP
Principal Planner
City of Alachua
PO Box 9
Alachua, FL 32616

Re: DRT Summary: Alchem Building 4 Site Plan Application

Dear Mr. Tabor:

The applicant's responses to the completeness review comments issued on February 18, 2021 are below.

1. *Section 6.1, Off-Street Parking and Loading*

a. Section 8.7.2 states expansions occurring over any continuous 5-year period that result in a greater than 50% increase of the gross square footage shall require the entire parcel of land to meet all parking and landscaping standards required by the LDRs. The proposed building addition represents an increase in building area of 52.46% (3,788 square feet proposed / 7,220 square feet existing), therefore full compliance with the parking standards of Section 6.1 are applicable. Several existing parking spaces are located partially within the ROW of CR 2054.

i. These spaces are not considered 'on-site parking spaces' and are not able to be used to meet minimum parking requirements. Reference Section 6.1.4(A)(1).

ii. Required off-street parking areas for three or more automobiles shall be designed, maintained, and regulated so that no parking or maneuvering incidental to parking shall be on a public street or walk. Reference Section 6.1.4(A)(1).

RESPONSE: Parking calculations have been revised.

b. Parking calculations shown on Sheet C0.00 should deduct the area the existing building to be removed from the area shown under 'existing building minimum required spaces'.

RESPONSE: Parking calculations have been revised.

2. *Section 6.2, Tree Protection / Landscape / Xeriscape Standards*

a. The square footage of the existing building to remain and building to be removed shown in the tables on Sheet L201 and used to evaluate compliance with Section 8.7.2 are inconsistent square footage of each building shown on Sheet C000. Based upon the square footage of each building shown on Sheet C000, the proposed building would exceed 50% of existing building area, thus requiring full compliance with Section 6.2.

RESPONSE: The square footage of the existing building to remain and building to be removed has been updated so the areas are consistent between Sheet L201 and C000.

b. The existing vegetation noted as fulfilling the east perimeter buffer requirement is located on an adjacent property that is not in common ownership with the subject property. A perimeter buffer must be provided on the subject property along the east property line.

RESPONSE: Photographs of trees to remain in the buffer on the adjacent property have been added to Sheet L202. See Tree Memo for Additional Images, quantities and sizes of existing trees to remain.

c. The calculations of required canopy trees for the building façade canopy tree requirement and the parking lot landscaping should be rounded to the nearest whole number.

RESPONSE: Required canopy tree calculations for building façade canopy tree requirement and parking lot landscaping have been rounded to the nearest whole number. See Sheet L201.

d. One additional canopy tree will be needed along the south building façade to fulfill south building façade canopy tree requirement.

RESPONSE: There are 2 canopy trees proposed along the south building façade and property line. See Sheet L202.

e. The site landscaping understory tree requirement is not met for the east side of the building.

RESPONSE: Per the 47% requirement, no understory trees are required on the east side of the building. See Sheet L201 for calculations.

f. The loading area and dumpster pad encroach into the required perimeter buffer and parking lot perimeter buffer (minimum 5', average 7'). See Sections 6.2.2(D)(2)(b)(iii) and 6.2.2(D)(3)(f)(1).

RESPONSE: The loading area and dumpster pad have been moved and no longer interfere with the required buffers. See Sheet L202.

g. The parking lot interior canopy tree requirement is not met.

RESPONSE: Per the 47% requirement, the interior canopy trees requirement is met. See Sheet L201 for calculations and Sheet L202 for proposed planting.

h. The parking lot perimeter canopy tree requirement is not met.

RESPONSE: Per the 47% requirement, the parking lot perimeter canopy tree requirement is met. See Sheet L201 for calculations and Sheet L202 for proposed planting.

i. Shrubs shall be required at the minimum rate of ten shrubs per landscaped island. Ten shrubs are not provided in the two western landscape islands.

RESPONSE: At least 10 shrubs have been proposed in each island. The islands have also been supplemented with additional groundcover plantings due to space and utility constraints. See Sheet L202.

j. The perimeter buffer requirements are not met for any of the required perimeter buffers.

RESPONSE: Per the 47% requirement, the perimeter buffer requirements are met. See Sheet L201 for calculations and Sheet L202 for proposed planting.

k. Add a calculation of landscaped areas as required per Policy 2.4.a of the Comprehensive Plan Future Land Use Element.

RESPONSE: Calculation of landscaped areas as required per Policy 2.4.a of the Comprehensive Plan Future Land Use Element has been added. See Sheet L202.

l. Add a calculation of open space areas as required per Section 6.7.3.

RESPONSE: Calculation of open space areas as required per Section 6.7.3. has been added. See Sheet L202.

m. Alternative landscape plan request will be presented to the LDR Administrator upon fully addressing all review comments.

RESPONSE: An alternative landscape plan request has been added to Sheet L201 for review.

3. *Section 6.4, Exterior Lighting*
   a. The photometric plan shows a data point of 6.7 footcandles near fixture PL4 north of the parking lot area. The maximum illumination in parking lot areas is 5 footcandles.

   RESPONSE: Output of fixture type PL4 reduced to comply with maximum 5.0fc requirement in parking areas.

4. *Utilities*
   a. The proposed connection to the existing sanitary sewer manhole is located on the adjacent property to the east of the site. A Public Utility Easement is required over the existing line.
   RESPONSE: A PUE has been added over the sanitary cleanout located on the property line.

   b. Show the location of all proposed Public Utility Easements on Sheet C400 and provide legal descriptions and sketches of such easements.
   RESPONSE: All proposed PUEs are shown. Legal descriptions and sketches will be provided as soon as they are available.

5. *Concurrency Impact Analysis*
   a. There are inconsistencies between the trip generation worksheet and the Concurrency Impact Analysis. The ITE land use codes used to calculate the number of trips generated by the proposed building is inconsistent between the two documents. Please resolve.
   RESPONSE: The inconsistencies have been resolved.

   b. The traffic segment identified in the Concurrency Impact Analysis is not an affected roadway segment. The correct roadway segments are:
      i. 15 (CR 2054 from SR 235 to US Highway 441);
      ii. 8 (SR 235 from NW 143rd Place to US 441); and
      iii. 8 (SR 235 from NW 143rd Place to US 441); and
      iv. 11 (CR 235 from NW 143rd Place to South City Limit).
   RESPONSE: Noted.

6. *Miscellaneous*
   a. A sidewalk with a minimum width of 5 feet shall be provided along the subject property's frontage of CR 2054. Reference supporting plans/reports adopted by the City Commission, including:
      i. Comprehensive Plan: GOPs, including but not limited to: Vision Element: Goal 3; Future Land Use Element: Policy 1.3.d; Transportation Element: Objective 1.4; Policy 1.5.c; Conservation & Open Space Element: Policy 1.13.b;
      ii. Alachua Market and Economic Impact Development Implementation Plan, dated July 16, 2016 (see page 46).
   RESPONSE: A sidewalk has been added at the property frontage.

   b. The access drive to the proposed dumpster pad is 10' in width. This is inadequate to provide access to a garbage truck without interfering with the loading area.
   RESPONSE: The dumpster pad has been relocated. Please see revised sheet C200.

   c. Dimension a typical parking space width on the dimension plan.
   RESPONSE: Dimensions for all parking spaces are shown.

7. *Public Services / Fire Rescue / Engineering Review Comments*
   a. The applicant must address the comments provided by Tom Ridgik, P.E., Engineering Supervisor, in a memorandum dated February 16, 2021.
   RESPONSE: All comments provided by Tom Ridgik, P.E. have been addressed.

   b. The applicant must address the comments provided by A.J. "Jay" Brown, P.E., of JBrown Professional Group, Inc., as provided in a memorandum dated February 10, 2021.
   RESPONSE: All comments provided by Jay Brown, P.E. have been addressed.

8. *Minor Revisions*
   a. Sheet C010: Construction Notes, #7 – Plan is approved by the Planning & Zoning Board (PZB). Revise accordingly.
   RESPONSE: All references to DRC have been revised to state PZB.

   b. Sheet C120: Tree Barricade Fencing Detail, Note 2 – note states the minimum height of the barricade shall be 3 feet. Minimum height required is 4 feet per Section 6.2.1(D)(2)(a).
   RESPONSE: Note 2 has been revised to state 4 feet as required.

9. *General Information*
   a. Alachua County Environmental Protection Department (EPD) requires the review and approval of all new commercial automatic irrigation systems. It is recommended that the applicant contact Alachua County EPD to coordinate this review and approval. Additional information is available at:
   https://alachuacounty.us/Depts/epd/WaterResources/WaterConservation/Pages/Irrigation-Efficiency-Design-and-Maintenance-Code.aspx
   RESPONSE: Noted.

   b. Alachua County Environmental Protection Department (EPD) requires the review and approval of stormwater management systems, unless otherwise exempt. It is recommended that the applicant contact Alachua County EPD to coordinate this review and approval. Additional information is available at:
   https://library.municode.com/fl/alachua_county/codes/code_of_ordinances?nodeId=PTIIADCO_TIT7HESA_CH77WAQUSTMAPR_ARTIIISTTRCO
   RESPONSE: Noted.

Public Services Comments

1. The attached markup of C400 is provided as a convenience in addressing the submittal comments, which are listed below.
   RESPONSE: Noted.

2. Sheet C400
   To the east of the proposed basin, there is a note "TIE INTO EXISTING WATER LINE". There does not appear to an existing water line in the area. Note that the subject rerouted water line appears to connect at a point northwest of the proposed basin.
   RESPONSE: Per the latest survey there is an existing water line in this area just south of the telephone pedestal and sanitary manhole.

3. Sheet C400
   Keyed note 4 southeast of the proposed basin. Keyed note 4 refers to a fire hydrant assembly. The fire hydrant is being supplied by a small 2-inch line, which is undersized to supply a fire hydrant. Please evaluate.
   RESPONSE: This callout has been corrected.

4. Sheet C400
   There is also keyed note 4 northeast the proposed basin. Please confirm that the design intent is to install a fire hydrant at this location. If so, what is its water supply?
   RESPONSE: This callout has been removed.

5. Sheet C400
   Northeast of the proposed basin, there is a fitting (45-degree elbow) in the new, rerouted 2- inch water main. Suggest calling out this fitting.
   RESPONSE: This fitting is now being called out.

6. Sheet C400
   Please call out a PUE for the underground electric utilities and the transformer.
   RESPONSE: A PUE has been added over the proposed transformer.

7. **Sheet C400**
   Identify the required voltage type (460-volt, 3-phase, etc.) for the load side of the transformer. Provide electrical load calculations to assist CoA Electrical Department in sizing the transformer.
   RESPONSE: The voltage and phase for the transformer are now specified.

8. Sheet C400
   The UGE power circuit is shown connecting to an existing power pole. Please contact CoA Electrical Department to confirm that this pole exists, or whether there is another preferred point of connection.
   CoA Electrical Department contact information is (1) Andy Lindsey, work cell - (352) 258- 8915 and (2) Billy Shiskin, work cell- (352) 316-8182.
   RESPONSE: Noted.

9. Sheet C400
   Water Fitting Schedule, Keyed Note 1: This is to confirm that CoA will perform the tap, but site conditions will dictate whether the tap is wet or dry. Thus, please change part of the note to:
   "1 - 2" X 1" TAP, WET OR DRY, DEPENDING ON SITE CONDITIONS"
   RESPONSE: Keyed Note 1 has been revised as requested.

10. Sheet C400
    Please change General Utility Note 1 to read:
    "1. ALL CONSTRUCTION WORK~ SHALL BE IN ACCORDANCE WITH THE LATEST EDITON CITY OF ALACHUA, PUBLIC SERVICE DESIGN AND CONSTRUCTION 1v1ANUAL, INCLUDING STANDARD DETAILS, & FDOT, ETC."
    RESPONSE: General Utility Note 1 has been revised as requested.

11. Sheet C410
    • Replace Detail 300 with CoA Detail 200 (attached) .
    • Replace Detail 127 with updated CoA Detail (attached) .
    • Add Detail 501 to documents (attached) . This detail is for a single-phase load side transformer. If it turns out a 3-phase load size transformer is needed, CoA will forward the required detail.
    RESPONSE: Details have been added and updated as requested.

JBrown Comments

C000 – Cover Sheet

1. The parking calculations table show that there is one bike rack provided in the new development. This does not appear to be shown on the plans.
   RESPONSE: The reference to a bike rack has been removed.

C120 – Demo and Erosion Control Plan

1. The 17" Palm just north of the parking lot is called out to be removed on the landscape plans. Please show this to be removed if this is the intention.
   RESPONSE: The 17" palm is now shown to be removed.

2. Please specify a measure of sediment control on the existing 24" RCP inlet, such as silt fence around the invert.
   RESPONSE: Silt fence has been added around the inlet.

3. Please revise the "unknown" linetype to show that it is a sanitary line.
   RESPONSE: The "unknown" linetype has been revised to a "wastewater" linetype.

4. Please show inlet control measures for all proposed storm structures.
   RESPONSE: Silt sacks are now specified to be installed on all proposed inlets.

C200 – Dimension Plan

1. Concerns regarding accessibility to dumpster pad as currently oriented. Truck would need to pull in at the proposed angle and then back out towards 8" concrete wall. Suggest placing dumpster pad adjacent to the loading zone. This would allow dump truck easier access. Additionally, sidewalk can be extended to the front of the dumpster pad.
   RESPONSE: Dumpster pad has been relocated. Please see revised sheet C200.

2. Consider widening dumpster pad to 20'. This will be beneficial if client needs additional recycling containers.
   RESPONSE: Noted.

3. Please show the fencing around the dumpster pad. Per detail, this fencing will be 2' offset of the concrete pad. This would likely put the fence on the nearby sidewalk.
   RESPONSE: The fencing around the dumpster pad is now shown.

4. Will a bike rack be provided on site as mentioned on cover sheet?
   RESPONSE: No bike racks are proposed at this time.

5. Consider placing a stop bar and stop sign and crosswalk at the entrance to the site
   RESPONSE: Noted.

6. Please show the radii on curves on this plan.
   RESPONSE: Radii on curves are now shown.

7. There is an unlabeled line from the southwest corner of the south parking bay to near the northwest corner of the north parking bay. Please provide label or remove.
   RESPONSE: This line has been removed.

C200 – Paving Grading & Drainage Plan

1. Please provide an additional spot elevation at the northwest corner of the north parking bay.
   RESPONSE: Additional spot elevations have been added.

2. Please provide additional spot elevations at the pavement markings in the handicap space to demonstrate ADA compliance.
   RESPONSE: Additional spot elevations have been added.

3. Suggest providing additional spot elevations near the curb at the entrance of the parking lot. Concerns of steep slopes from the entry drive to the parking area.
   RESPONSE: Sufficient spot elevations have been provided to calculate the slope. The slope at the driveway is approximately 4.5%.

4. Suggest providing additional spot elevations in the center of the parking lot drive aisle.
   RESPONSE: Additional spot elevations have been added.

5. Please provide concrete energy dissipation pad at S-4 outfall.
   RESPONSE: Rip-rap has been added at the S-4 outfall.

6. Suggest revising invert of S-2 to the bottom of the pond to prevent additional erosion. Please ensure that this structure comes in perpendicular to basin side slopes.
   RESPONSE: S-2 has been revised to have an invert at the bottom of the proposed basin.

7. Suggest grading a swale from the south of the building around the east of the building into the basin. Concerns of offsite drainage coming into the building.
   RESPONSE: Noted.

8. Please revise grading at southeast corner of site. The 66.5 spot elevation is a low point, and water will pond here, potentially getting into the building.
   RESPONSE: Grading around the southeast corner has been revised.

9. Please provide spot grades at the door on the south side of the building.
   RESPONSE: A spot elevation has been added at the door on the south side of the building.

10. Suggest lower 67.40 spot on the sidewalk on the west side of the building. Concerns of flooding here.
    RESPONSE: Noted.

11. Looking at the street view, it appears that there may be a small stormwater pipe in the north side of the swale across from the 24" RCP inlet. Please verify that, if this pipe is there, there is no conflict with any portions of the development.
    RESPONSE: A note has been added for the contractor to field verify location of the potential existing storm pipe.

C210 – PGD Details

1. Please revise Sidewalk Ramp Detail to show 5' wide handicap ramp as shown in plan view.
   RESPONSE: Sidewalk ramp detail has been revised to show 5' width.

C220 – Basin Detail Plan

1. Please show cross section in plan view used for basin typical section.
   RESPONSE: The cross section has been labeled in plan view.

2. Please show max slopes of maintenance path and whether it will be sloped up or down.
   RESPONSE: Labels have been added for the maximum slope of the maintenance path.

3. Consider using flatter back slope than 2:1 on the tiebacks from the proposed basin to prevent additional erosion.
   RESPONSE: A flatter than 2:1 Slope is being utilized everywhere possible.

4. Please show a section of the basin which shows the grading back up to the sidewalk on the west side of the basin.
   RESPONSE: An additional cross section of the basin has been added. Please see section B-B.

5. Please label the orifice on both portions of the S-3 detail.
   RESPONSE: The orifice is now labeled on both portions of the S-3 detail.

6. Suggest using a 6" wide slot weir rather than a 6"x3" weir. If the 100yr flood plain is 63.98, not having the top on the weir should not have an effect on this. This will prevent clogging of the smaller opening.
   RESPONSE: The weir has been revised to be a 6" slot weir as suggested.

C300 – SWPPP

1. Please revise Project Name / Owner line in the inspection form.
   RESPONSE: The project name is correct in the inspection form.

C400 – Utility Plan

1. Please confirm that the City of Alachua will allow a relocated fire hydrant rather than a new one.
   RESPONSE: The existing fire hydrant is no longer being relocated and is just being removed.

2. On Water Fitting Schedule #4, please specify that this is a relocated fire hydrant if this is the intention.
   RESPONSE: Fitting schedule item #4 has been revised.

3. Suggest eliminating WWCO-4 and going directly from WWCO-5 to WWCO-3.
   RESPONSE: Wastewater layout has been revised the reduce the number of cleanouts and lengths of the pipes as possible. WWCO-4 is still utilized in order to cross the rerouted water line and other utilities.

4. The max length for a 6" wastewater line is 100'. Please add an additional cleanout in between WWCO-2 and WWCO-3.
   RESPONSE: The wastewater line has been revised accordingly.

5. Please revise water fitting schedule callouts on the west side of the property. They appear to be mislabeled.
   RESPONSE: Water fitting callouts have been corrected.

Stormwater Report

1. The permeability's used in the ICPR model submitted do not match those in the geotechnical report. Please revise or show how these permeability's were obtained.
   RESPONSE: The permeability values used in the model do not match the geotechnical report because of the factor of safety used in the design. A factor of safety of 2 was used therefore the rates in the model are half of those found in the report.

2. Please provide pipe calculations to show that a 12" pvc pipe will be large enough for the max flow out of the basin.
   RESPONSE: Please see the pipe calculations in Attachment I of the Drainage Design Notes.

City of Alachua

ADAM BOUKARI
CITY MANAGER

PLANNING & COMMUNITY DEVELOPMENT
DIRECTOR KATHY WINBURN, AICP

February 18, 2021

Sent by electronic mail to cvega@edafl.com

Claudia Vega, P.E.
EDA Consultants, Inc.
720 SW 2nd Avenue
South Tower, Suite 300
Gainesville, FL 32601

RE: Development Review Team (DRT) Summary: Alchem Building 4 Site Plan

Dear Ms. Vega:

On January 28, 2021, the City of Alachua received your application for a Site Plan on behalf of the Alchem Laboratories Corporation. The application proposes the construction of a ±3,788 square foot building and associated site improvements on a ±0.76 acre property, located at 13305 Rachael Boulevard and consisting of Tax Parcel Number 03231-003-000 and a portion of 03231-004-000. A completeness review was performed on February 2, 2021 and the application was determined to be complete on the same date.

The application has been reviewed by the City's Development Review Team (DRT). Upon review of the application and materials, the following insufficiencies must be addressed. A meeting is scheduled for **2:30 PM** on **Monday, February 22, 2021** to review these comments.

Please address all insufficiencies in writing and provide an indication as to how they have been addressed by **5:00 PM** on **Tuesday, March 9, 2021**. A total of four (4) copies of the application package, plans, and a digital copy of all materials in PDF format on a CD or sent by emailing a Cloud / FTP link must be provided by this date. If all comments are addressed by the resubmission date above, the application may be scheduled for the **April 20, 2021 Planning & Zoning Board (PZB) Meeting.**

Please address the following:

1. *Section 6.1, Off-Street Parking and Loading*
   a. Section 8.7.2 states expansions occurring over any continuous 5-year period that result in a greater than 50% increase of the gross square footage shall require the entire parcel of land to meet all parking and landscaping standards required by the LDRs. The proposed building addition represents an increase in building area of 52.46% (3,788 square feet proposed / 7,220 square feet existing), therefore full compliance with the parking standards of Section 6.1 are applicable. Several existing parking spaces are located partially within the ROW of CR 2054.
      i. These spaces are not considered 'on-site parking spaces' and are not able to be used to meet minimum parking requirements. Reference Section 6.1.4(A)(1).

   ii. Required off-street parking areas for three or more automobiles shall be designed, maintained, and regulated so that no parking or maneuvering incidental to parking shall be on a public street or walk. Reference Section 6.1.4(A)(1).
b. Parking calculations shown on Sheet C0.00 should deduct the area the existing building to be removed from the area shown under 'existing building minimum required spaces'.

2. *Section 6.2, Tree Protection / Landscape / Xeriscape Standards*
   a. The square footage of the existing building to remain and building to be removed shown in the tables on Sheet L201 and used to evaluate compliance with Section 8.7.2 are inconsistent square footage of each building shown on Sheet C000. Based upon the square footage of each building shown on Sheet C000, the proposed building would exceed 50% of existing building area, thus requiring full compliance with Section 6.2.
   b. The existing vegetation noted as fulfilling the east perimeter buffer requirement is located on an adjacent property that is not in common ownership with the subject property. A perimeter buffer must be provided on the subject property along the east property line.
   c. The calculations of required canopy trees for the building façade canopy tree requirement and the parking lot landscaping should be rounded to the nearest whole number.
   d. One additional canopy tree will be needed along the south building façade to fulfill south building façade canopy tree requirement.
   e. The site landscaping understory tree requirement is not met for the east side of the building.
   f. The loading area and dumpster pad encroach into the required perimeter buffer and parking lot perimeter buffer (minimum 5', average 7'). See Sections 6.2.2(D)(2)(b)(iii) and 6.2.2(D)(3)(f)(1).
   g. The parking lot interior canopy tree requirement is not met.
   h. The parking lot perimeter canopy tree requirement is not met.
   i. Shrubs shall be required at the minimum rate of ten shrubs per landscaped island. Ten shrubs are not provided in the two western landscape islands.
   j. The perimeter buffer requirements are not met for any of the required perimeter buffers.
   k. Add a calculation of landscaped areas as required per Policy 2.4.a of the Comprehensive Plan Future Land Use Element
   l. Add a calculation of open space areas as required per Section 6.7.3.
   m. Alternative landscape plan request will be presented to the LDR Administrator upon fully addressing all review comments.

3. *Section 6.4, Exterior Lighting*
   a. The photometric plan shows a data point of 6.7 footcandles near fixture PL4 north of the parking lot area. The maximum illumination in parking lot areas is 5 footcandles.

4. *Utilities*
   a. The proposed connection to the existing sanitary sewer manhole is located on the adjacent property to the east of the site. A Public Utility Easement is required over the existing line.
   b. Show the location of all proposed Public Utility Easements on Sheet C400 and provide legal descriptions and sketches of such easements.

5. *Concurrency Impact Analysis*
   a. There are inconsistencies between the trip generation worksheet and the Concurrency Impact Analysis. The ITE land use codes used to calculate the number of trips generated by the proposed building is inconsistent between the two documents. Please resolve.
   b. The traffic segment identified in the Concurrency Impact Analysis is not an affected roadway segment. The correct roadway segments are:
      i. 15 (CR 2054 from SR 235 to US Highway 441);

ii. 8 (SR 235 from NW 143rd Place to US 441); and
iii. 11 (CR 235 from NW 143rd Place to South City Limit).

6. *Miscellaneous*
   a. A sidewalk with a minimum width of 5 feet shall be provided along the subject property's frontage of CR 2054. Reference supporting plans/reports adopted by the City Commission, including:
      i. Comprehensive Plan: GOPs, including but not limited to: Vision Element: Goal 3; Future Land Use Element: Policy 1.3.d; Transportation Element: Objective 1.4; Policy 1.5.c; Conservation & Open Space Element: Policy 1.13.b;
      ii. Alachua Market and Economic Impact Development Implementation Plan, dated July 16, 2016 (see page 46).
   b. The access drive to the proposed dumpster pad is 10' in width. This is inadequate to provide access to a garbage truck without interfering with the loading area.
   c. Dimension a typical parking space width on the dimension plan.

7. *Public Services / Fire Rescue / Engineering Review Comments*
   a. The applicant must address the comments provided by Tom Ridgik, P.E., Engineering Supervisor, in a memorandum dated February 16, 2021.
   b. The applicant must address the comments provided by A.J. "Jay" Brown, P.E., of JBrown Professional Group, Inc., as provided in a memorandum dated February 10, 2021.

8. *Minor Revisions*
   a. Sheet C010: Construction Notes, #7 – Plan is approved by the Planning & Zoning Board (PZB). Revise accordingly.
   b. Sheet C120: Tree Barricade Fencing Detail, Note 2 – note states the minimum height of the barricade shall be 3 feet. Minimum height required is 4 feet per Section 6.2.1(D)(2)(a).

9. *General Information*
   a. Alachua County Environmental Protection Department (EPD) requires the review and approval of all new commercial automatic irrigation systems. It is recommended that the applicant contact Alachua County EPD to coordinate this review and approval. Additional information is available at: https://alachuacounty.us/Depts/epd/WaterResources/WaterConservation/Pages/Irrigation-Efficiency-Design-and-Maintenance-Code.aspx
   b. Alachua County Environmental Protection Department (EPD) requires the review and approval of stormwater management systems, unless otherwise exempt. It is recommended that the applicant contact Alachua County EPD to coordinate this review and approval. Additional information is available at: https://library.municode.com/fl/alachua_county/codes/code_of_ordinances?nodeId=PTIIADCO_TIT7HESA_CH77WAQUSTMAPR_ARTIIISTTRCO

If you have any questions regarding the information above, please contact me at 386-418-6100 x 1602 or via e-mail at jtabor@cityofalachua.com. We look forward to receiving your revised application.

Sincerely,

Justin Tabor, AICP
Principal Planner

Attachment: Memorandum from Tom Ridgik, P.E., Engineering Supervisor, dated February 16, 2021
Memorandum from A.J. "Jay" Brown, P.E., of JBrown Professional Group, Inc., dated February 10, 2021

c: Adam Boukari, City Manager *(by electronic mail)*
Mike DaRoza, Assistant City Manager *(by electronic mail)*
Kamal Latham, Assistant City Manager *(by electronic mail)*
Kathy Winburn, AICP, Planning & Community Development Director *(by electronic mail)*
Adam Hall, AICP, Planner *(by electronic mail)*
Project File

DEVELOPMENT REVIEW TEAM SUMMARY

PROJECT NAME: Alchem Building 4

APPLICATION TYPE: Site Plan

PROPERTY OWNER: Alchem Laboratories Corporation

APPLICANT/AGENT: Claudia Vega, P.E., EDA Consultants, Inc.

DRT MEETING DATE: February 17, 2021

DRT MEETING TYPE: Staff

FLUM DESIGNATION: Industrial

ZONING: Light & Warehouse Industrial (ILW)

OVERLAY: N/A

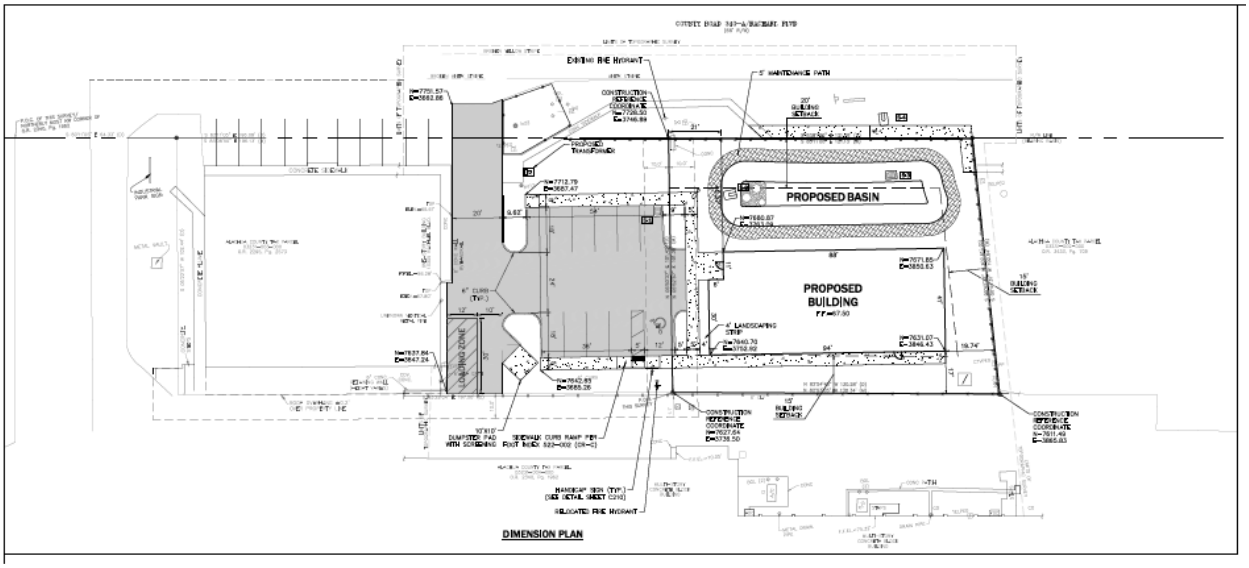
ACREAGE: ±0.76 acres

PARCEL: 03231-003-000 and a portion of 03231-004-000

PROJECT SUMMARY: Site Plan proposing the construction of a ±3,788 square foot building and associated site improvements

RESUBMISSION DUE DATE: All data, plans, and documentation addressing the insufficiencies identified below must be received by the Planning Department on or before **5:00 PM** on **Tuesday, March 9, 2020.**

TENTATIVE PZB DATE: If all comments are addressed by the resubmission date above, the application may be scheduled for the **April 20, 2021 PZB Meeting.**



Deficiencies to be Addressed

** Unless otherwise noted, references to code sections are to the City of Alachua Land Development Regulations. **

1. *Section 6.1, Off-Street Parking and Loading*
   a. Section 8.7.2 states expansions occurring over any continuous 5-year period that result in a greater than 50% increase of the gross square footage shall require the entire parcel of land to meet all parking and landscaping standards required by the LDRs. The proposed building addition represents an increase in building area of 52.46% (3,788 square feet proposed / 7,220 square feet existing), therefore full compliance with the parking standards of Section 6.1 are applicable. Several existing parking spaces are located partially within the ROW of CR 2054.
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City of Alachua

ADAM BOUKARI
CITY MANAGER

RODOLFO VALLADARES, P.E.
PUBLIC SERVICES DIRECTOR

INTER-OFFICE COMMUNICATION

DATE: **Feb 16, 2021**

TO: **Kathy Winburn, AICP**
Planning & Community Development Director

FROM: **Rodolfo Valladares, P.E.**
Public Services Director
Tom Ridgik, P.E.
Engineering Supervisor

TR
2/16/2021

RE: **Alchem Bldg 4 Site Plan**

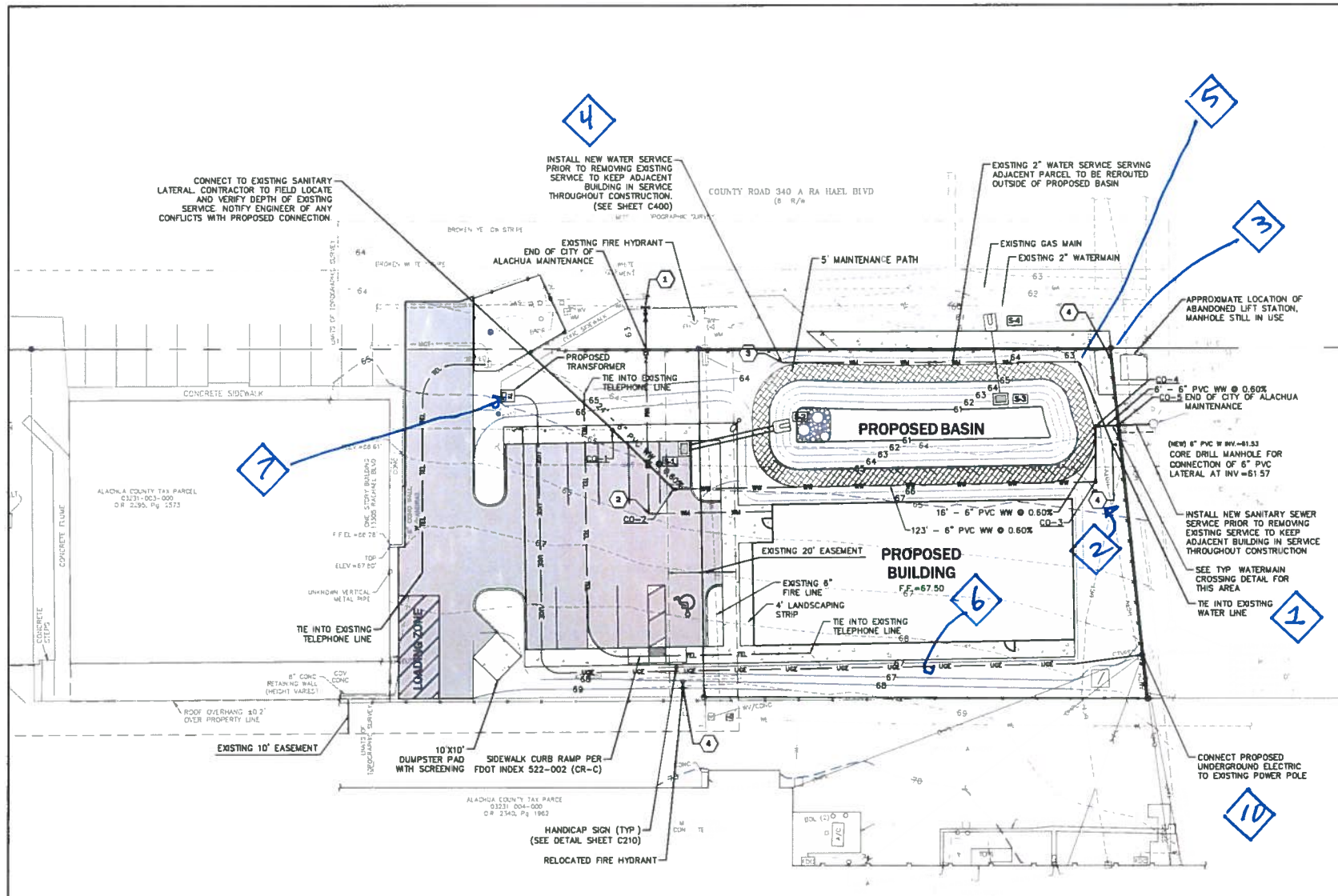
Public Services has reviewed the subject project (Jan 28 Submittal) and offer the following comments. Review was specific to the Public Services Utilities.

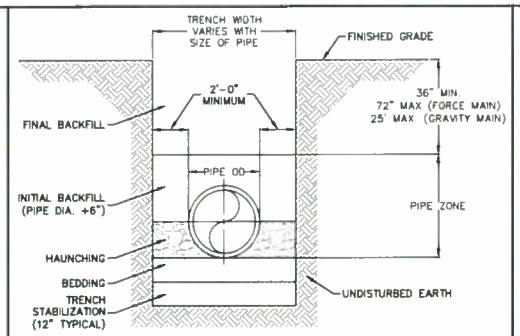
NO.	COMMENTS
1.	**Comment on Jan 28 Submittal** **General** The attached markup of C400 is provided as a convenience in addressing the submittal comments, which are listed below. **FYI - APPROVED**
2.	**Comment on Jan 28 Submittal** **Sheet C400** To the east of the proposed basin, there is a note "TIE INTO EXISTING WATER LINE". There does not appear to an existing water line in the area. Note that the subject rerouted water line appears to connect at a point northwest of the proposed basin.
3	**Comment on Jan 28 Submittal** **Sheet C400**

NO.	COMMENTS
	Keyed note 4 southeast of the proposed basin. Keyed note 4 refers to a fire hydrant assembly. The fire hydrant is being supplied by a small 2-inch line, which is undersized to supply a fire hydrant. Please evaluate.
4	**Comment on Jan 28 Submittal** **Sheet C400** There is also keyed note 4 northeast the proposed basin. Please confirm that the design intent is to install a fire hydrant at this location. If so, what is its water supply?
5	**Comment on Jan 28 Submittal** **Sheet C400** Northeast of the proposed basin, there is a fitting (45-degree elbow) in the new, rerouted 2-inch water main. Suggest calling out this fitting.
6	**Comment on Jan 28 Submittal** **Sheet C400** Please call out a PUE for the underground electric utilities and the transformer.
7	**Comment on Jan 28 Submittal** **Sheet C400** Identify the required voltage type (460-volt, 3-phase, etc.) for the load side of the transformer. Provide electrical load calculations to assist CoA Electrical Department in sizing the transformer.
8	**Comment on Jan 28 Submittal** **Sheet C400** The UGE power circuit is shown connecting to an existing power pole. Please contact CoA Electrical Department to confirm that this pole exists, or whether there is another preferred point of connection. CoA Electrical Department contact information is (1) Andy Lindsey, work cell – (352) 258-8915 and (2) Billy Shiskin, work cell – (352) 316-8182.
9	**Comment on Jan 28 Submittal** **Sheet C400** Water Fitting Schedule, Keyed Note 1: This is to confirm that CoA will perform the tap, but site conditions will dictate whether the tap is wet or dry. Thus, please change part of the note to: "1 – 2" X 1" TAP, WET OR DRY, DEPENDING ON SITE CONDITIONS"

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12	
13	
14	
15	
END OF COMMENTS	

Please advise if you have any questions or require additional information.
cc: Justin Tabor – AICP Principal Planner
Adam Hall – AICP Planner
Harry Dillard – Lead Engineering Technician





NOTES:

1. TRENCH STABILIZATION SHALL BE PROVIDED TO A DEPTH OF 12-INCHES. THE MATERIAL SHALL BE NO. 57 STONE
2. BEDDING SHALL BE A MINIMUM OF 6-INCHES AND COMPOSED OF IMPORTED GRANULAR FILL. COMPACT TO 95% STANDARD PROCTOR, AASHTO T-99.
3. HUNCHING PORTION OF THE PIPE ZONE SHALL BE PLACED TO THE SPRINGLINE OF THE PIPE AND COMPOSED OF IMPORTED GRANULAR FILL. COMPACT TO 95% STANDARD PROCTOR, AASHTO T-99.
4. INITIAL BACKFILL: COMMON FILL COMPACTED TO 95% (98% UNDER PAVEMENT) OF THE MAXIMUM DENSITY AS PER AASHTO T-180. FROM THE SPRINGLINE OF THE PIPE TO TWO (2) FEET ABOVE THE PIPE THE SOIL SHALL BE CAREFULLY BACKFILLED IN 6-INCH LIFTS AND THE SOIL CONSOLIDATED WITH THE HAND OPERATED TAMPING MACHINE (OR AS REQUIRED BY THE CITY, COUNTY OR STATE INSPECTORS).
5. FINAL BACKFILL: COMMON FILL COMPACTED TO 95% (95%) UNDER PAVEMENT OF THE MAXIMUM DENSITY AS PER AASHTO T-180. AFTER PLACEMENT AND COMPACTION OF THE INITIAL BACKFILL, THE BALANCE OF THE BACKFILL MATERIAL MAY BE MACHINE PLACED OR AS REQUIRED BY THE INSPECTOR AND SHALL NOT CONTAIN ANY ROCKS OR DEBRIS.
6. WATER SHALL NOT BE PERMITTED IN THE TRENCH DURING CONSTRUCTION.
7. ALL PIPE TO BE INSTALLED WITH BELL FACING UPSTREAM TO THE DIRECTION OF THE FLOW.
8. DEPTH FOR REMOVAL OF UNSUITABLE MATERIAL SHALL GOVERN DEPTH OF BEDDING ROCK BELOW THE PIPE. UTILITIES SHALL DETERMINE IN THE FIELD REQUIRED REMOVAL OF UNSUITABLE MATERIAL TO REACH SUITABLE FOUNDATION. THE BOTTOM OF THE TRENCH SHALL NOT BE EXCAVATED BELOW THE SPECIFIED GRADE. IF UNDERCUTTING OCCURS, THE BOTTOM OF THE TRENCH SHALL BE BROUGHT UP TO THE ORIGINAL GRADE WITH APPROVED MATERIAL AND THOROUGHLY COMPACTED TO A MINIMUM OF 95% MODIFIED PROCTOR.
9. FINAL RESTORATION IN IMPROVED AREAS SHALL BE IN COMPLIANCE WITH ALL APPLICABLE REGULATIONS OF GOVERNING AGENCIES.

DETAIL No. 405 - DETAIL BACKFILL REQUIREMENTS

NTS

WATER FITTING SCHEDULE

KEY	ASSEMBLY
1	CONNECT TO EXISTING 2" PVC WATER MAIN 1 – 2"X1" WET TAP (TAP IS BY CITY OF ALACHUA AND MOT, EXCAVATION, BACKFILL AND RESTORATION ARE BY CONTRACTOR. NOTIFY CITY OF ALACHUA UTILITY INSPECTOR 2 DAYS IN ADVANCE) 1 – 1" GATE VALVE AND BOX 1 – 3/4" WATER METER AT PROPERTY LINE (END OF CITY OF ALACHUA MAINTENANCE) 1 – 1" REDUCED PRESSURE ZONE BACKFLOW PREVENTER
2	1 – 1" PVC 90° BEND
3	1 – 2" PVC 22.5°BEND
4	1 – PERPENDICULAR FIRE HYDRANT ASSEMBLY (PER CITY OF ALACHUA)

WASTEWATER CLEANOUT SCHEDULE

CLEANOUT	CLEANOUT DATA
CO-1	TOP EL.=67.35 INV.=62.62
CO-2	TOP EL.=65.76 INV.=62.47 INV.=62.47
CO-3	TOP EL.=64.58 INV.=61.73 INV.=61.73
CO-4	TOP EL.=64.67 INV.=61.63 INV.=61.63
CO-5	TOP EL.=63.80 INV.=61.59 INV.=61.59

GENERAL UTILITY NOTES

1. ALL CONSTRUCTION WORK SHALL BE IN ACCORDANCE WITH THE LATEST CITY OF ALACHUA DETAILS & THE F.D.O.T "STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION" ADOPTED IN 1991 BY THE STATE OF FLORIDA AND ALL SUPPLEMENTS THERETO, AND THE "ROADWAY TRAFFIC DESIGN STANDARDS", WHICHEVER IS MORE STRINGENT.
2. THE LOCATIONS OF EXISTING UTILITIES SUCH AS WATER MAINS, SEWER, GAS LINES, ETC., AS SHOWN ON THE PLANS HAVE BEEN DETERMINED FROM THE BEST AVAILABLE INFORMATION, AND ARE PROVIDED FOR THE CONVENIENCE OF THE CONTRACTOR. HOWEVER, THE ENGINEER AND OWNER DO NOT ASSUME RESPONSIBILITY FOR THE SIZES AND LOCATIONS SHOWN. THE CONTRACTOR SHALL BE RESPONSIBLE FOR FIELD VERIFICATION OF THE SIZE AND LOCATION OF ALL EXISTING UTILITIES SHOWN AS WELL AS NOT SHOWN ON THESE PLANS AND PROTECTION OF SAME DURING CONSTRUCTION. ANY EXISTING UTILITY TO BE MODIFIED SHALL BE COORDINATED WITH THE APPROPRIATE UTILITY PROVIDER.
3. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO SECURE ANY AND ALL NEEDED LICENSES AND PERMITS REQUIRED FOR CONSTRUCTION PURPOSES.
4. ELECTRIC SERVICE TO BE COORDINATED WITH CITY OF ALACHUA PUBLIC SERVICE DEPARTMENT. SEE COA ELECTRICAL SCOPE OF WORK NOTES ON THIS SHEET.
5. NO GAS SERVICE IS REQUIRED FOR THIS DEVELOPMENT.
6. CONTRACTOR IS RESPONSIBLE FOR VERIFYING ALL SETBACKS AND EASEMENTS BEFORE BEGINNING CONSTRUCTION.
7. BENCHMARKS AND OTHER REFERENCE POINTS SHALL BE CAREFULLY MAINTAINED THROUGHOUT THE CONSTRUCTION PERIOD. IF DISTURBED OR DESTROYED, THESE POINTS SHALL BE REPLACED BY A FLORIDA P.L.S. AT CONTRACTOR'S EXPENSE.
8. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO PROVIDE ANY REQUIRED TRAFFIC CONTROL PLANS. ALL WORK SHALL BE EXECUTED UNDER THE ESTABLISHED PLAN AND FLORIDA DEPT. OF TRANSPORTATION AND CITY OF ALACHUA APPROVED PROCEDURES.
9. THE CONTRACTOR SHALL RESTORE OFF SITE CONSTRUCTION AREAS TO EQUAL OR BETTER CONDITION THAN EXISTED PRIOR TO COMMENCEMENT OF CONSTRUCTION.
10. CONTRACTOR IS RESPONSIBLE FOR PROTECTION AND VERIFICATION OF EXISTING UTILITIES. ANY EXISTING UTILITY TO BE MODIFIED SHALL BE COORDINATED WITH THE APPROPRIATE UTILITY PROVIDER.
11. WATER METER ASSEMBLIES SHALL BE INSTALLED BY CONTRACTOR.
12. UTILITY INSTALLATION SHALL NOT TAKE PLACE WITHIN TREE BARRICADE AREA AS SHOWN ON THE PAVING, GRADING, AND DRAINAGE PLAN.
13. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DISPOSAL OF UNSUITABLE MATERIALS OFF-SITE AND FURNISH APPROVED MATERIAL PER CITY OF ALACHUA FOR SANITARY SEWER LINES BACKFILL AT NO ADDITIONAL COST TO THE OWNER. THE CONTRACTOR MAY DISPOSE OF UNSUITABLE MATERIAL ON-SITE BY APPROVAL OF THE OWNER.
14. NO PERMANENT STRUCTURE SHALL BE CONSTRUCTED WITHIN 10 FEET OF ANY WATER LINE OR 15 FEET OF ANY SEWER LINE.
15. CONTRACTOR SHALL PROTECT LANDSCAPE AND ANY DISTURBED AREAS SHALL BE RESTORED AS EXISTING.
16. CONTRACTOR SHALL FURNISH AND INSTALL 2" PLUG VALVE WITH VALVE BOX AND CONCRETE VALVE COLLAR. INSTALL SIMILAR TO CITY OF ALACHUA DETAIL 11B (SEE DETAIL SHEET C420)

CITY OF ALACHUA ELECTRICAL SCOPE OF WORK

1.1 SCOPE OF COA ELECTRICAL WORK: AT NO COST TO THE CITY, THE COA WILL PERFORM CERTAIN WORK ELEMENTS TO ASSIST THE CONTRACTOR IN PROVIDING COMPLETE AND OPERATING ELECTRICAL SYSTEMS AS SHOWN ON THE DRAWINGS AND SPECIFIED HEREIN.

1.2 THE WORK TO EXTEND THIS EXISTING PRIMARY SERVICE AND TO PROVIDE THE TRANSFORMER SHALL BE A COMBINED EFFORT BETWEEN THE CONTRACTOR AND COA.

1.3 THE CONTRACTOR SHALL BE FULLY RESPONSIBLE FOR ALL ELECTRICAL WORK REQUIRED TO PROVIDE SECONDARY (LOAD-SIDE) SERVICE FROM THE TRANSFORMER

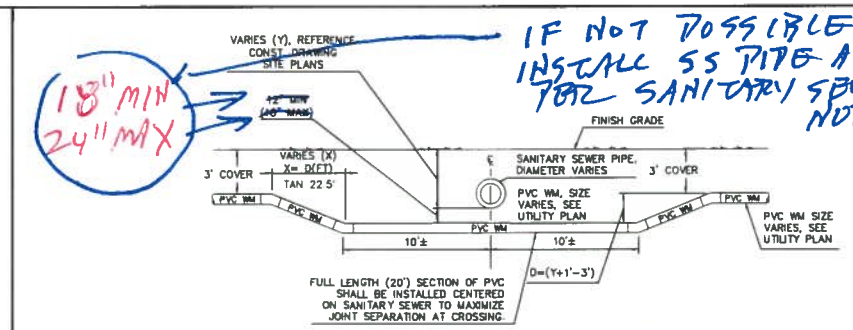
1.4 COA WILL ORDER THE FOLLOWING PRODUCTS, WHICH SHALL BE PAID FOR BY THE CONTRACTOR: ALL PRIMARY CABLES, ALL TRANSFORMERS AND ALL MAKE-UP MATERIALS. ONCE ON SITE, THESE PRODUCTS WILL BE INSTALLED BY THE CITY OF ALACHUA. ALL PRIMARY MAKEUP, CONNECTIONS AND TESTING WILL BE COMPLETED BY THE CITY OF ALACHUA. ALL WORK PERFORMED BY THE COA WILL BE BACKCHARGED TO THE CONTRACTOR.

1.5 THE CONTRACTOR SHALL FURNISH AND INSTALL ALL OTHER MATERIALS, INCLUDING, BUT NOT LIMITED TO ALL CONDUIT, ENCLOSURES, POLES, LUMINAIRES, TRANSFORMER PADS, AND SWITCHGEAR BASES.

1.6 THE CONTRACTOR SHALL SCHEDULE AN ELECTRICAL COORDINATION MEETING AT START OF PROJECT. REQUIRED ATTENDEES INCLUDE THE CONTRACTOR, THE ELECTRICAL SUB-CONTRACTOR, THE ENGINEER OF RECORD, THE COA PROJECT MANAGER AND COA ELECTRICAL STAFF.

SANITARY SEWER NOTES

1. ROOF DRAINS, FOUNDATION DRAINS OR OTHER STORM WATER CONNECTIONS TO THE SANITARY SEWER ARE PROHIBITED.
2. ALL SANITARY SEWER PIPE SHALL BE PVC SEWER PIPE CONFORMING TO ASTM D-3034, SDR 35 PVC, UNLESS OTHERWISE NOTED.
3. ALL MANHOLES SHALL BE 4 FT. INSIDE DIAMETER UNLESS OTHERWISE NOTED.
4. WHEN MINIMUM SEPARATION CANNOT BE MAINTAINED BETWEEN WATER AND GRAVITY SEWER, THE SEWER SHALL BE UPGRADED TO C-900, DR18 AND PRESSURE TESTED TO 150 PSI.
5. FOR HORIZONTAL AND VERTICAL CLEARANCES BETWEEN SANITARY SEWER MAINS/LATERALS AND WATER MAINS/LATERALS, SEE WATER NOTES 3 & 4.
6. WHERE SOIL REPORT INDICATES THE POSSIBILITY OF UNSUITABLE MATERIAL IN THE VICINITY OF SANITARY LINES, THE CONTRACTOR SHALL DETERMINE THE NATURE AND EXTENT OF THIS UNSUITABLE MATERIAL, PREPARE TRENCH AND INSTALL THE SEWER LINE IN ACCORDANCE WITH ASTM D-2321. IT WILL BE THE CONTRACTOR'S RESPONSIBILITY TO NOTIFY THE ENGINEER OF THIS UNSUITABLE MATERIAL.
7. THE CONTRACTOR SHALL BE REQUIRED TO PERFORM INFILTRATION OR EXFILTRATION TEST, WHICHEVER IS APPLICABLE. THE CONTRACTOR SHALL MEET ALL REQUIREMENTS, CODES AND ORDINANCES OF THE CITY OF ALACHUA UTILITIES INCLUDING ADVANCE NOTIFICATION PRIOR TO CONSTRUCTION AND TESTING.
8. THE CONSTRUCTION OF ALL UTILITIES CONNECTING TO THE CITY OF ALACHUA'S SYSTEMS SHALL CONFORM TO THE CITY OF ALACHUA DESIGN AND CONSTRUCTION MANUAL, LATEST EDITION.
9. THE CONTRACTOR SHALL NOTIFY THE CITY 48 HOURS PRIOR TO ANY UTILITY CONSTRUCTION.
10. THE CONTRACTOR SHALL PROVIDE TEST ASSEMBLIES AND TRACER WIRE TO BOTH METALLIC AND NON-METALLIC PIPE AS PER THE CITY OF ALACHUA DESIGN STANDARDS AND DETAILS.
11. FOR FORCE MAIN, PROVIDE TRACER WIRE AND APPURTENANCES FROM THE LIFT STATION TO THE CONNECTING COA MANHOLE.



TYPICAL WATERMAIN CROSSING SECTION DETAIL

NTS

1. DO NOT AN EXISTING WATER LINE IN THIS AREA PLEASE EVALUATE.
2. PLEASE CONFIRM ~~THIS~~ IS A FIRE HYDRANT. IF SO, IT IS BEING FED FROM A 2-INCH LINE (UNDERSIZED).
3. PLEASE CONFIRM THIS IS A FIRE HYDRANT. IF SO, DID NOT SEE A WATER LINE
4. SUGGEST CALLING OUT SIZE. IS IT 2-INCHES?
5. CALL OUT FITTING
6. CALL OUT PUE FOR UGE. TRANSFOTMER
7. WHAT IS REQUIRED VOLTAGE TYPE ON LOAD SIDE OF TRANSFORMER? WHAT IS THE TRANSFOTMER CAPA?
8. ~~THE 2" TEE WILL BE INSTALLED UNDER "DRY" NOT "WET" FAULT CONDITIONS~~ SEE SUBMITTAL COMMENTS
9. MODIFY NOTE 1 AS PER SUBMITTAL COMMENTS
10. CONFIRM THIS POWER POLE STILL EXISTS.

IF NOT POSSIBLE, INSTALL SS PIPE AS PER SANITARY SEWER NOTE 4.

eda consultants inc.
EB 2589
720 S.W. 2nd Ave, South Tower, Suite 300
GAINESVILLE, FLORIDA 32601
TEL: (352) 373-3541
www.eda-fl.com mail@eda-fl.com

NORTH
SCALE: 1" = 20'
0 10 20 40
GRAPHIC SCALE

No.	Date	Comment

Professional Engineer of Record:

Claudia S. Vega, P.E. 51532
Engineer Certificate No.

Project No: 20-103

Project phase: CITY SUBMITTAL

Project title: ALCHEM LABORATORIES: BUILDING 4 SITE PLAN CITY OF ALACHUA, FLORIDA

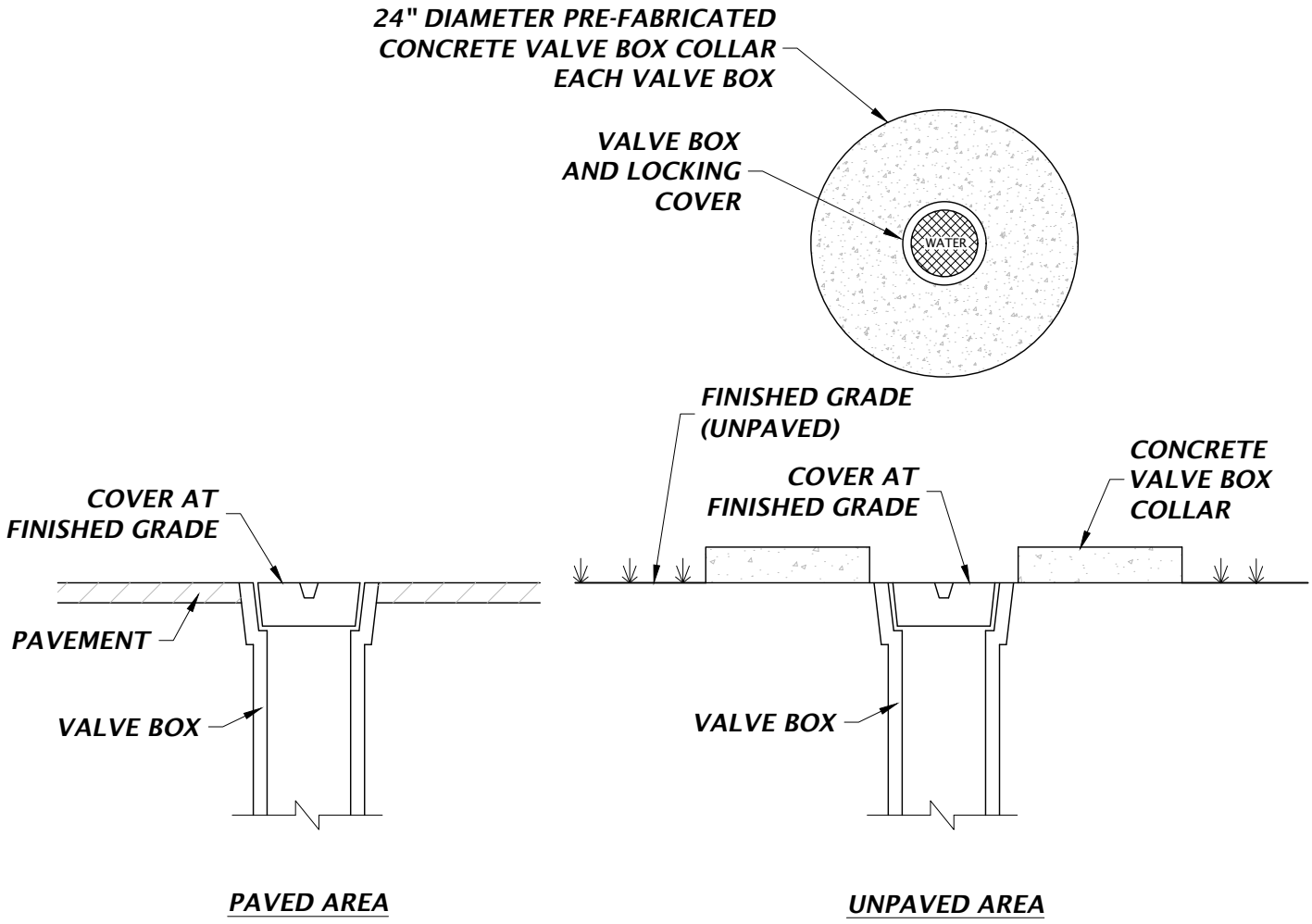
Sheet title: UTILITY PLAN

Designed: CV
Drawn: JB
Checked: CV
Date: 01/28/21

Sheet No.: C400

J:\Projects\Alchem - Bldg 4\Plans\Current DWG\A201031.dwg, C400-UTIL, 1/27/2021 3:43:32 PM

① ② ③



PAVED AREA

UNPAVED AREA

KEYED NOTE(S):

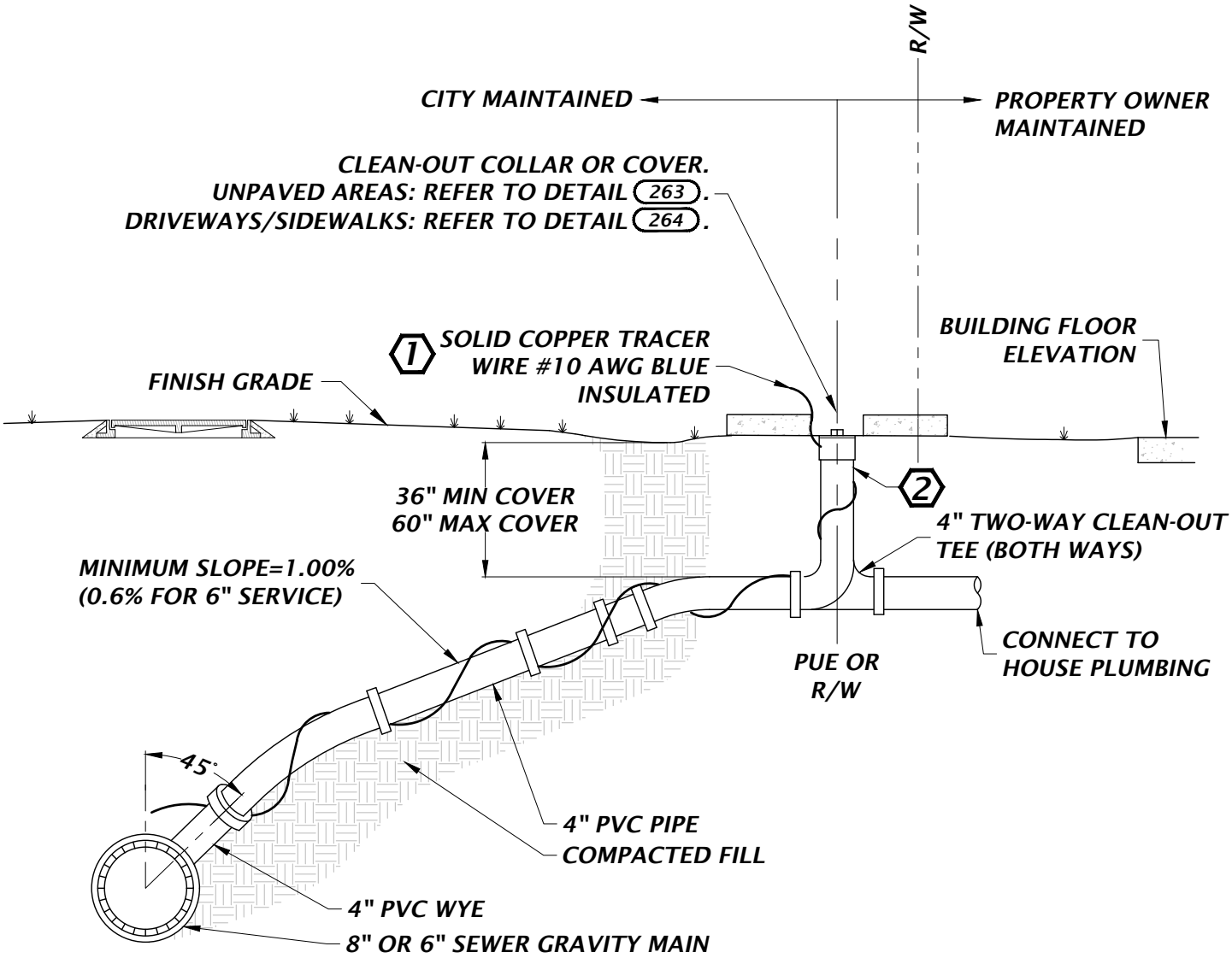
① *VALVE BOX COLLAR FEATURES:*
- *24" DIAMETER, 11" INNER OPENING*
- *4" THICK*
- *TWO RINGS OF #3 REBAR*
- *4000 PSI TEST CONCRETE*

② *SUPPLIER AND PRODUCT:*
- *MASONRY SUPPLY, INC., SELMA, NC OR EQUAL*
- *ITEM: 24" CONCRETE VALVE BOX COLLAR OR EQUAL*
- *PART: MSVB-24 OR EQUAL*

③ *PAINT VALVE BOX COVER AND COLLAR AS PER* **307**

VALVE BOX COVER

NTS



GENERAL NOTE(S):

A. SOLVENT WELD ALL PVC FITTINGS.

KEYED NOTE(S):

1. *EXTEND TRACER WIRE 8-INCHES BEYOND TOP OF VALVE BOX COLLAR.*
2. *DO NOT CONNECT HOUSE PLUMBING TO STUB RISER.*

200

WASTEWATER SERVICE LATERAL

NTS

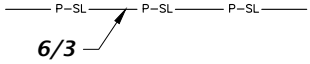
ELECTRICAL UTILITIES LEGEND AND ABBREVIATIONS



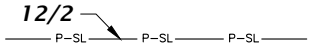
TRANSFORMER ON CONCRETE PAD, 50 KVA PHASE b, 120/240 VOLT SINGLE PHASE SECONDARY.

1/0 AL PRIMARY CABLE, IN 2" PVC AT 48" MIN. COVER.

4/0 AL TPX SECONDARY CABLE, IN 2.5" PVC AT 36" MIN. COVER.



6/3 AL TPX SECONDARY STREETLIGHT CABLE, IN 1.5" PVC AT 36" MIN. COVER.



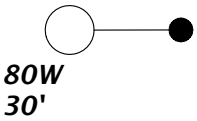
12/2 WITH GROUND, IN 1.5" PVC AT 36" MIN. COVER.



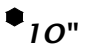
JUNCTION BOX



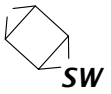
SERVICE ENCLOSURE, BURIED, APPROXIMATELY 33" X 20" X 15" DEEP.



80 WATT LED CUTOFF LUMINAIRE ON 30' FIBERGLASS POLE (25' MOUNTING HT) WITH 4' ARM.



ROUND BURIED STREETLIGHT WIRING ENCLOSURE APPROXIMATELY 10" DIAMETER X 12" DEEP HIGH DENSITY POLYETHYLENE.



600 AMP PAD MOUNTED SWITCHGEAR. DOORS INDICATE SOURCE AND LOAD SIDES.

PME	*PAD MOUNTED EQUIPMENT*
PUE	*PUBLIC UTILITY EASEMENT*
PVC	*POLYVINYL CHLORIDE*
CT	*CURRENT TRANSFORMER*
R/W	*RIGHT OF WAY*

JBrown Professional Group
CIVIL ENGINEERING • LAND SURVEYING • PLANNING
3530 NW 43rd Street • Gainesville, FL 32606 • 352.375.8999 • JBProGroup.com

February 10, 2021

Mr. Justin Tabor, AICP
Planner
City of Alachua
Office of Planning & Community Development
P.O. Box 9
Alachua, FL 32616-0009

Re: Alchem Laboratories Building 4

Dear Mr. Tabor:

As you requested, we have reviewed the Alchem Laboratories Building 4 submittal drawings and other materials provided to us for the above referenced project. The drawings reviewed were created by EDA and dated 01/28/2021. Our review generated a few minor comments and recommendations that are outlined below.

C000 – Cover Sheet

1. The parking calculations table show that there is one bike rack provided in the new development. This does not appear to be shown on the plans.

C120 – Demo and Erosion Control Plan

1. The 17" Palm just north of the parking lot is called out to be removed on the landscape plans. Please show this to be removed if this is the intention.
2. Please specify a measure of sediment control on the existing 24" RCP inlet, such as silt fence around the invert.
3. Please revise the "unknown" linetype to show that it is a sanitary line.
4. Please show inlet control measures for all proposed storm structures.

C200 – Dimension Plan

1. Concerns regarding accessibility to dumpster pad as currently oriented. Truck would need to pull in at the proposed angle and then back out towards 8" concrete wall. Suggest placing dumpster pad adjacent to the loading zone. This would allow dump truck easier access. Additionally, sidewalk can be extended to the front of the dumpster pad.
2. Consider widening dumpster pad to 20'. This will be beneficial if client needs additional recycling containers.
3. Please show the fencing around the dumpster pad. Per detail, this fencing will be 2' offset of the concrete pad. This would likely put the fence on the nearby sidewalk.
4. Will a bike rack be provided on site as mentioned on cover sheet?
5. Consider placing a stop bar and stop sign and crosswalk at the entrance to the site.

6. Please show the radii on curves on this plan.
7. There is an unlabeled line from the southwest corner of the south parking bay to near the northwest corner of the north parking bay. Please provide label or remove.

C200 – Paving Grading & Drainage Plan

1. Please provide an additional spot elevation at the northwest corner of the north parking bay.
2. Please provide additional spot elevations at the pavement markings in the handicap space to demonstrate ADA compliance.
3. Suggest providing additional spot elevations near the curb at the entrance of the parking lot. Concerns of steep slopes from the entry drive to the parking area.
4. Suggest providing additional spot elevations in the center of the parking lot drive aisle.
5. Please provide concrete energy dissipation pad at S-4 outfall.
6. Suggest revising invert of S-2 to the bottom of the pond to prevent additional erosion. Please ensure that this structure comes in perpendicular to basin side slopes.
7. Suggest grading a swale from the south of the building around the east of the building into the basin. Concerns of offsite drainage coming into the building.
8. Please revise grading at southeast corner of site. The 66.5 spot elevation is a low point, and water will pond here, potentially getting into the building.
9. Please provide spot grades at the door on the south side of the building.
10. Suggest lower 67.40 spot on the sidewalk on the west side of the building. Concerns of flooding here.
11. Looking at the street view, it appears that there may be a small stormwater pipe in the north side of the swale across from the 24” RCP inlet. Please verify that, if this pipe is there, there is no conflict with any portions of the development.

C210 – PGD Details

1. Please revise Sidewalk Ramp Detail to show 5’ wide handicap ramp as shown in plan view.

C220 – Basin Detail Plan

1. Please show cross section in plan view used for basin typical section.
2. Please show max slopes of maintenance path and whether it will be sloped up or down.
3. Consider using flatter back slope than 2:1 on the tiebacks from the proposed basin to prevent additional erosion.
4. Please show a section of the basin which shows the grading back up to the sidewalk on the west side of the basin.
5. Please label the orifice on both portions of the S-3 detail.
6. Suggest using a 6” wide slot weir rather than a 6”x3” weir. If the 100yr flood plain is 63.98, not having the top on the weir should not have an effect on this. This will prevent clogging of the smaller opening.

C300 – SWPPP

1. Please revise Project Name / Owner line in the inspection form.

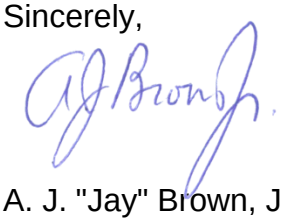
C400 – Utility Plan

1. Please confirm that the City of Alachua will allow a relocated fire hydrant rather than a new one.
2. On Water Fitting Schedule #4, please specify that this is a relocated fire hydrant if this is the intention.
3. Suggest eliminating WWCO-4 and going directly from WWCO-5 to WWCO-3.
4. The max length for a 6" wastewater line is 100'. Please add an additional cleanout in between WWCO-2 and WWCO-3.
5. Please revise water fitting schedule callouts on the west side of the property. They appear to be mislabeled.

Stormwater Report

1. The permeability's used in the ICPR model submitted do not match those in the geotechnical report. Please revise or show how these permeability's were obtained.
2. Please provide pipe calculations to show that a 12" pvc pipe will be large enough for the max flow out of the basin.

Sincerely,



A. J. "Jay" Brown, Jr., PE
President, JBrown Professional Group Inc.

Cc: Claudia Vega

February 8, 2021

Justin Tabor, AICP
Principal Planner
City of Alachua
PO Box 9
Alachua, FL 32616

Re: Completeness Review: Alchem Building 4 Site Plan Application

Dear Mr. Tabor:

The applicant's responses to the completeness review comments issued on February 2, 2021 are below.

1. **Mailing Labels:** The list of persons and organizations on file with the City to receive public notices was revised on January 11, 2021. Please ensure the updated list is used to create mailing labels for future submittals. The current list can be accessed via the City's web site.
RESPONSE: eda has replaced the list in our files and will use this list going forward- thank you for the notification.

2. **Proof of Ownership:** Provide a ***recorded*** copy of the Special Warranty Deed dated December 21, 2020 between Dual Enterprises, LLC and Alchem Laboratories Corporation. Revise parcel number as needed throughout all application materials to reflect the new parcel number assigned to this property.
RESPONSE: Recorded deed is included with this resubmittal (see 7 Proof of Ownership). All plans and application materials have been revised to reflect the new parcel number.

3. **Proof of Payment of Taxes:** Provide updated property tax record for Tax Parcel 03231-004-000.
RESPONSE: Updated property tax record is included with this resubmittal (see 8 Proof of Taxes Paid).

4. **Access Management Permit:** Please submit documentation which shows that a permit application for proposed roadway modifications to County Road 2054 have been submitted to Alachua County Public Works.
RESPONSE: The permit application and a printout of the e-mail submission to Alachua County Public Works has been included with this resubmittal (See 9 ACPW Permitting).

City of Alachua

ADAM BOUKARI
CITY MANAGER

PLANNING & COMMUNITY DEVELOPMENT
DIRECTOR KATHY WINBURN, AICP

February 2, 2021

Sent by electronic mail to cvega@edafl.com

Claudia Vega, P.E.
EDA Consultants, Inc.
720 SW 2nd Avenue
South Tower, Suite 300
Gainesville, FL 32601

RE: Completeness Review: Alchem Building 4 Site Plan Application

Dear Ms. Vega:

On January 28, 2021, the City of Alachua received your application for a Site Plan submitted on behalf of Alchem Laboratories Corporation. The application proposes the construction of a ±3,788 square foot building and associated site improvements on a ±0.76 acre property, located at 13305 Rachael Boulevard and consisting of Tax Parcel Number 03231-003-000 and a portion of 03231-004-000.

According to Section 2.2.6 of the Land Development Regulations (LDRs), upon receipt of an application, a completeness review shall be conducted to determine that the application contains all the necessary information and materials, is in proper form and of sufficient detail, and is accompanied by the appropriate fee. The Planning Department has reviewed the aforementioned application for completeness and finds that the application is complete, contingent upon receiving the information as noted below. Please provide materials addressing the comments below by **5:00 PM on Tuesday, February 9, 2021**.

The comments below are based solely on a preliminary review of your application for completeness. An in-depth review of the content of the application will be performed, and the findings of the in-depth review will be discussed at a Development Review Team (DRT) Meeting, which will be scheduled under separate cover.

Please address the following:

1. **Mailing Labels:** The list of persons and organizations on file with the City to receive public notices was revised on January 11, 2021. Please ensure the updated list is used to create mailing labels for future submittals. The current list can be accessed via the City's web site.

2. **Proof of Ownership:** Provide a ***recorded*** copy of the Special Warranty Deed dated December 21, 2020 between Dual Enterprises, LLC and Alchem Laboratories Corporation. Revise parcel number as needed throughout all application materials to reflect the new parcel number assigned to this property.

3. **Proof of Payment of Taxes:** Provide updated property tax record for Tax Parcel 03231-004-000.

4. **Access Management Permit:** Please submit documentation which shows that a permit application for proposed roadway modifications to County Road 2054 have been submitted to Alachua County Public Works.

If you have any questions regarding the information above, please contact me at 386-418-6100 x 107 or via e-mail at jtabor@cityofalachua.com.

Sincerely,

Justin Tabor, AICP
Principal Planner

c: Adam Boukari, City Manager *(by electronic mail)*
Mike DaRoza, Assistant City Manager *(by electronic mail)*
Kamal Latham, Assistant City Manager *(by electronic mail)*
Kathy Winburn, AICP, Planning & Community Development Director *(by electronic mail)*
Adam Hall, AICP, Planner *(by electronic mail)*
Project File

eda consultants • inc.

January 28, 2021

Justin Tabor, Principal Planner
City of Alachua Planning and Community Development
15100 NW 142nd Terrace
Alachua, FL 32616

Re: Alchem Building 4
Development Plan Application

The proposed project is the construction of a 3,788 square foot building, with associated parking, utility, and stormwater improvements. The project is located at 13305 Rachael Boulevard on Tax Parcel No. 03231-003-000 and a portion of 03231-004-000. The project site comprises approximately 0.76 (+/-) acres. There are two existing light manufacturing buildings already on the property. The 700 SF portable building is proposed to be removed as a part of this site plan.

Included with this letter is all supporting information required for a development plan and civil plans showing the proposed facilities.

If you have any questions, please feel free to contact our office at any time.

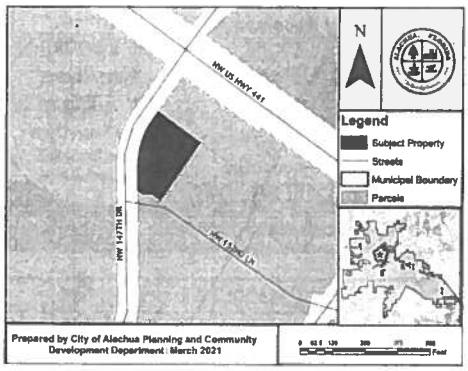
Sincerely,

Claudia Vega, P.E.
Director of Engineering

City of ALACHUA

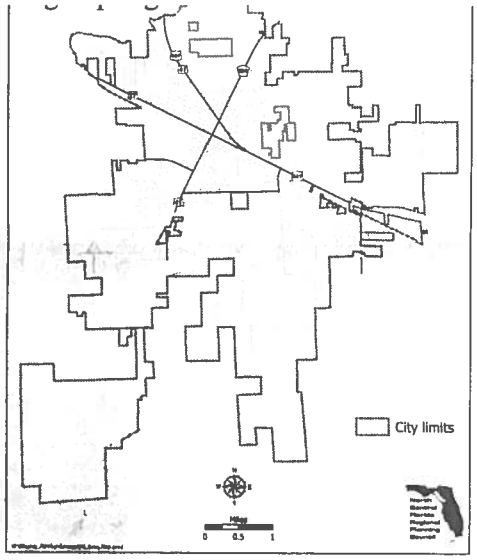
NOTICE OF PUBLIC HEARING BEFORE THE PLANNING AND ZONING BOARD OF THE CITY OF ALACHUA, FLORIDA

Notice is hereby given that the Planning & Zoning Board of the City of Alachua will hold a public hearing on April 20, 2021, at 6:00 p.m. The hearing will be held in the James A. Lewis Commission Chambers in City Hall, located at 15100 NW 142nd Terrace, Alachua, Florida, to consider the following: A request by Christopher Potts, P.E., of JBrown Professional Group, Inc., applicant and agent for Shakti Management, LLC, property owner, for consideration of a Site-Specific Amendment to the Official Zoning Atlas (Rezoning) to amend the Official Zoning Atlas from Central Business District (CBD) to Commercial Intensive (CI) on a ±1.09 acre subject property. The property subject to the proposed amendment is located south of US HWY 441, to the east of NW 147th Drive, and north of the Hitchcock's Plaza; Tax Parcel Number 03869-004-000; Existing Future Land Use Map (FLUM) Designation: Commercial. Existing Zoning: Central Business District (CBD).



At the public hearing, all interested parties may appear and be heard with respect to the application. Copies of the application are available for public inspection at the Planning and Community Development Department, 15100 NW 142nd Terrace, Alachua, Florida, on any regular business day between the hours of 7:30 a.m. to 6:00 p.m. Written comments on the application may be sent to the following address: City of Alachua, Planning and Community Development, P.O. Box 9, Alachua, FL 32616. Notice is given pursuant to Section 286.0105, Florida Statutes, that, in order to appeal any decision made at the public hearing, you will need a record of the proceedings, and that, for such purpose, you may need to ensure that a verbatim record of the proceedings is made, which includes the testimony and evidence upon which the appeal is to be based. In accordance with the Americans with Disabilities Act, any persons with a disability requiring reasonable accommodation in order to participate in this meeting should call the City Clerk at (386) 418-6100 x 1501 at least 48 hours prior to the public hearing.

(Published: Alachua County Today - April 08, 2021)



The first of two public hearings on the proposed amendment and the ordinance adopting said amendment will be held April 22, 2021 at 6:30 p.m., or as soon thereafter as the matter can be heard, in the City Commission Chambers, City Hall located at 23718 West U.S. Highway 27, High Springs, Florida. The title of said ordinance shall read, as follows:

ORDINANCE NO. LDC 21-02

AN ORDINANCE OF THE CITY OF HIGH SPRINGS, FLORIDA, AMENDING THE CITY OF HIGH SPRINGS LAND DEVELOPMENT CODE, APPENDIX B, LAND DEVELOPMENT CODE; ARTICLE XI ADMINISTRATION; PART 11.00.00 GENERALLY; TABLE 11.00.01 TYPE OF PROCEDURE BY DEVELOPMENT OPTION; PART 11.01.00 PROCEDURE FOR OBTAINING PERMITS; SECTION 11.01.07.01 DETERMINATION OF REQUIRED REVIEW; PART 11.02.00 PROCEDURES FOR OBTAINING DEVELOPMENT ORDERS; SECTION 11.02.02.01 LEVEL ONE, ACTION BY SITE PLAN REVIEW COMMITTEE; PART 11.04.00 SITE PLAN REVIEW; SECTION 11.04.04 PRELIMINARY SITE PLAN REQUIRED; SECTION 11.04.05 FINAL SITE PLAN REQUIRED; STRIKING PART 11.01.00 PROCEDURE FOR OBTAINING PERMITS; SECTION 11.01.07.03 PLATTED SUBDIVISION WITH NO IMPROVEMENT FACILTIIES, PROVIDING FOR SEVERABILTY; REPEALING ALL ORDINANCES IN CONFLICT AND PROVIDING AN EFFECTIVE DATE.

The public is advised to visit the website: https://www.highsprings.us for up-to-date information concerning the public hearing. If the public hearing is also held via communications media technology, all pertinent information will be made available via the City website https://www.highsprings.us or may be obtained by calling the Office of City Clerk at 386.454.1416.

The public hearing may be continued to one or more future dates. Any interested party shall be advised that the date, time and place of any continuation of the public hearing shall be announced during the public hearing and that no further notice concerning the matter will be published, unless said continuation exceeds six calendar weeks from the date of the above referenced public hearing.

At the aforementioned public hearing, all interested parties may appear to be heard with respect to the amendment.

Copies of the amendment and the ordinance adopting said amendment are available for public inspection at the Office of the City Clerk, 23718 West U.S. Highway 27, High Springs, Florida, during regular business hours.

All persons are advised that if they decide to appeal any decision made at the above referenced public hearing, they will need a record of the proceedings, and that for such purpose they may need to ensure that a verbatim record of the proceedings is made, which record includes the testimony and evidence upon which the appeal is to be based.

In accordance with the American with Disabilities Act, any person with disabilities needing any special accommodation to participate in City meetings should contact the Office of the City Clerk, 23718 West U.S. Highway 27, High Springs, Florida, 32643 or via telephone: 386.454.1416, ext. 7237, at least 48 hours prior to the date of the public hearing.

(Published: Alachua County Today - April 08, 2021)

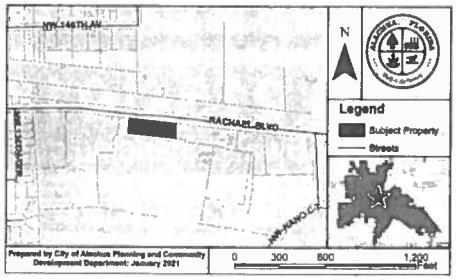
(Published: Alachua County Today - April 08, 2021)

Alachua County Today
is your best source for local news - 386.462.3355

City of ALACHUA

NOTICE OF PUBLIC HEARING BEFORE THE PLANNING AND ZONING BOARD OF THE CITY OF ALACHUA, FLORIDA

Notice is hereby given that the Planning and Zoning Board of the City of Alachua will hold a public hearing on April 20, 2021, at 6:00 p.m. The hearing will be held in the James A. Lewis Commission Chambers in City Hall, located at 15100 NW 142nd Terrace, Alachua, Florida, to consider the following: A request by Claudia Vega, P.E., of EDA Consultants, Inc., applicant and agent for Alchem Laboratories Corporation, property owner, for consideration of a Site Plan which proposes the construction of a ±3,788 square foot building and associated site improvements on a ±0.76 acre property, located at 13305 Rachael Boulevard; Tax Parcel Nos. 03231-003-000 and 03231-004-001; FLUM: Industrial; Zoning: Light & Warehouse Industrial (ILW).



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(Published: Alachua County Today - April 08, 2021)

AFFIDAVIT FOR POSTED LAND USE SIGN

I MELISSA WATSON, POSTED THE LAND USE
(name)
SIGN ON 4/1/2021 FOR THE Alchem Building 4 Site Plan
(date) (state type of action and project name)
LAND USE ACTION.

AS PER ARTICLE 2.2.9 D OF THE LAND DEVELOPMENT REGULATIONS.

THIS WILL BE INCLUDED IN THE STAFF REPORT.

Melissa Watson
(signature)

2
(number of signs)

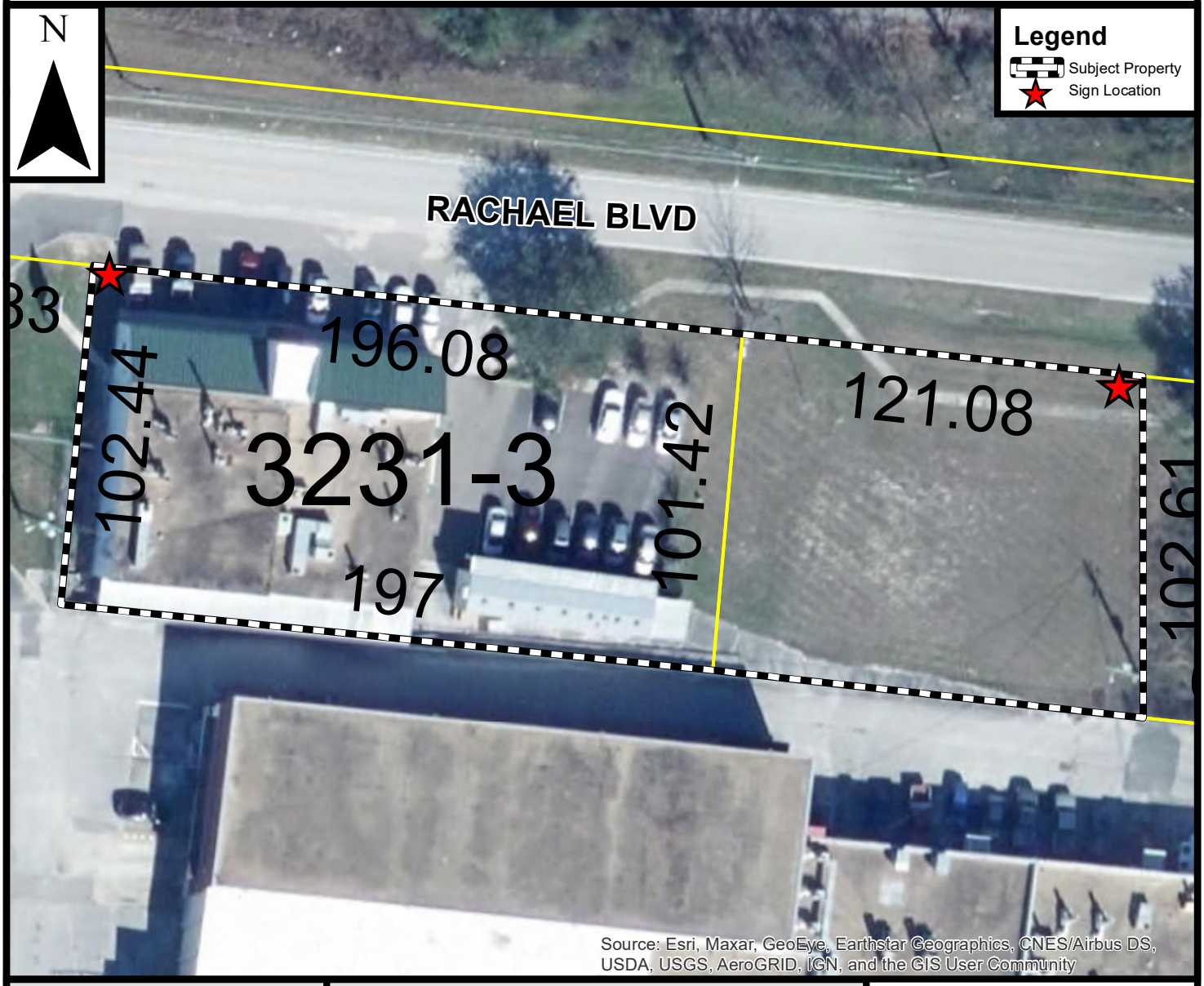
IMPORTANT INFORMATION REGARDING POSTED NOTICE SIGNS

Pursuant to Section 2.2.9(D) of the City's Land Development Regulations, posted notice signs must be placed on the land that is the subject of the application, along each street which is adjacent to or runs through the land in a manner that makes them clearly visible. Signs shall be posted at intervals of not more than 400 feet when the land subject to the application has less than 1,500 feet of road frontage. When the land subject to the application has 1,500 feet or more of road frontage, signs shall be posted at intervals of not more than 1,320 feet.

Signs shall be inspected by the City subsequent to their posting. The applicant shall be responsible for ensuring that the posted notice is maintained on the land subject to the application until the completion of the final public hearing on the application.

Signs must be removed by the applicant and returned to the City within ten days after the final decision on the application.

Alchem Building 4
Site Plan



Prepared by the City of Alachua
Planning & Community Development Department

Prepared February 2021

DISCLAIMER: Data is provided "as is" without warranty of any representation of accuracy, timeliness, or completeness. The burden of determining accuracy, timeliness, or completeness for use rests solely on the requestor. The City makes no warranties, expressed or implied, as to the use of the data. The requestor acknowledges and accepts the limitations of the data, including the fact that the data is dynamic and in a constant state of maintenance, correction, and update.

0 15 30 60 Feet

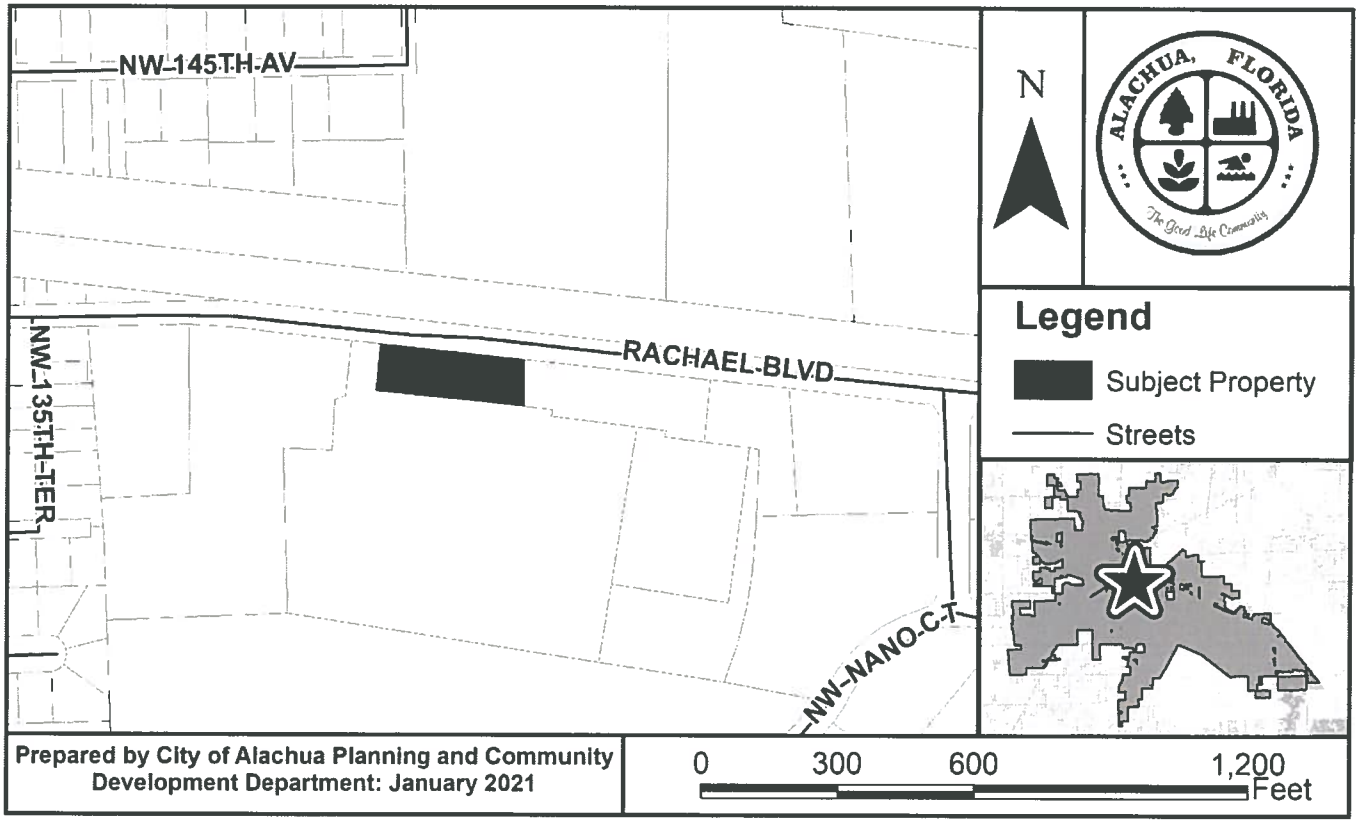
Mailed 4/5/21 KO

City of ALACHUA

THE GOOD LIFE COMMUNITY

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P.O. Box 9 ♦ Alachua, Florida 32616-0009
Phone: (386) 418-6121 ♦ Fax: (386) 418-6130

03231-002-000
BLUMBERG GLENN
11421 NW 120TH TER
ALACHUA, FL 32615

03934-001-020
ALLEN & ALLEN & ALLEN & FREENEY & JENKINS
12008 NW 148TH PL
ALACHUA, FL 32615

03231-004-000
DUAL ENTERPRISES LLC
132 NW 76TH DR
GAINESVILLE, FL 32607

03231-003-000
ALCHEM LABORATORIES CORP
13305 RACHEL BLVD
ALACHUA, FL 32615-6687

03230-001-031
WHITE JOANNA
13417 NW 145TH AVE
ALACHUA, FL 32615

03934-001-019
JENKINS & MCDANIEL-JENKINS H/W
13510 NW 141ST PL
ALACHUA, FL 32615

03934-001-001
TOUCHE NOLA L
13515 NW 142ND AVE
ALACHUA, FL 32615

03927-012-000
SNH MEDICAL OFFICE PROPERTIES TRUST
255 WASHINGTON ST STE 300
NEWTON, MA 02458

03230-001-034
JOHNSON KENNON KEITH
269 CROSSGATE DR
ELMORE, AL 36025-1039

03231-000-000
CONTEXT DEVELOPMENT CO
3020 CORAL WAY
MIAMI, FL 33145

03384-200-000
CSX TRANSPORTATION INC
500 WATER ST TAX DEPARTMENT J-910
JACKSONVILLE, FL 32202-4423

92060-517-900
CSX TRANSPORTATION INC
500 WATER ST TAX DEPARTMENT J-910
JACKSONVILLE, FL 32202-4423

03229-000-000
MARLOW ALACHUA HOLDINGS LLC
5212 SNEAD ISLAND RD
PALMETTO, FL 34221

03229-003-000
PHOENIX COMMERCIAL PARK
PO BOX 1000
ALACHUA, FL 32616

03231-005-001
ADC DEVELOPMENT & INVESTMENT
PO BOX 238
LAKE BUTLER, FL 32054

03230-002-000
ADC DEVELOPMENT & INVESTMENT GROUP LLC
PO BOX 238
LAKE BUTLER, FL 32054

03231-005-000
ADC DEVELOPMENT & INVESTMENT GROUP LLC
PO BOX 238
LAKE BUTLER, FL 32054

03927-000-000
ADC DEVELOPMENT & INVESTMENT GROUP LLC
PO BOX 238
LAKE BUTLER, FL 32054

03231-001-000
BARBER LUMBER SALES OF ALACHUA INC
PO BOX 263
ALACHUA, FL 32616-0263

03927-006-000
BARBER LUMBER SALES OF ALACHUA INC
PO BOX 263
ALACHUA, FL 32616-0263

03231-005-003
COPELAND INDUSTRIAL PARK OWNERS ASSOCIATION INC
PO BOX 365
LAKE BUTLER, FL 32054

03518-001-000
BRYANT FLORA M
PO BOX 556
ALACHUA, FL 32616-0556

Antoinette Endelicato
5562 NW 93rd Avenue
Gainesville, FL 32653

Dan Rhine
288 Turkey Creek
Alachua, FL 32615

Tom Gorman
9210 NW 59th Street
Alachua, FL 32653

Richard Gorman
5716 NW 93rd Avenue
Alachua, FL 32653

Peggy Arnold
410 Turkey Creek
Alachua, FL 32615

David Forest
23 Turkey Creek
Alachua, FL 32615

President of TCMOA
1000 Turkey Creek
Alachua, FL 32615

Linda Dixon, AICP
Assistant Director of Planning
PO Box 115050
Gainesville, FL 32611

Craig Parenteau
FL Dept. of Environmental Protection
4801 Camp Ranch Road
Gainesville, FL 32641

Jeannette Hinsdale
P.O. Box 1156
Alachua, FL 32616

Lynn Coullias
7406 NW 126th Ave
Alachua, FL 32615

Lynda Coon
7216 NW 126 Avenue
Alachua, FL 32615

Tamara Robbins
PO Box 2317
Alachua, FL 32616

Michele L. Lieberman
County Manager
12 SE 1st Street
Gainesville, FL 32601

Bonnie Flynn
16801 NW 166th Drive
Alachua, FL 32615

Hugh & Jean Calderwood
P.O. Box 2307
Alachua, FL 32616

Lisia Jenkins
PO Box 1071
Alachua, FL 32616

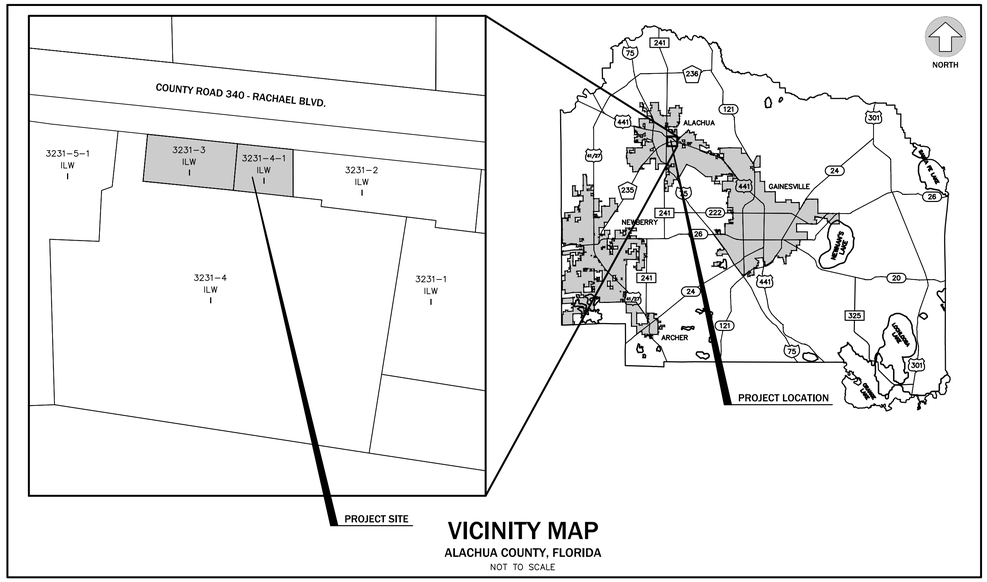
ALCHEM LABORATORIES: BUILDING 4
SITE PLAN

CITY OF ALACHUA, FLORIDA

DEVELOPMENT INFORMATION		
1.	PROJECT OWNER:	ALCHEM LABORATORIES
2.	NAME OF PROJECT:	ALCHEM LABORATORIES: BUILDING 4 SITE PLAN
3.	PROJECT DESCRIPTION:	CONSTRUCTION OF A 3,788 S.F. BUILDING WITH ASSOCIATED UTILITIES AND PARKING
4.	PROJECT ADDRESS:	13305 RACHAEL BLVD. ALACHUA, FL 32615
5.	TAX PARCEL NUMBER:	3231-3 AND 3231-4-1
6.	SECTION/TOWNSHIP/RANGE:	SECTION 23, TOWNSHIP 8 SOUTH, RANGE 18 EAST
7.	ZONING:	LIGHT & WAREHOUSE INDUSTRIAL (ILW)
8.	FUTURE LAND USE DESIGNATION:	INDUSTRIAL (I)
9.	THE PROJECT SITE WILL MEET ALL NPDES CRITERIA DURING AND AFTER CONSTRUCTION.	
10.	THE PROPOSED BUILDING SHALL COMPLY WITH THE CURRENT EDITION OF THE FLORIDA BUILDING CODE.	

DEVELOPMENT DATA			
#	CRITERIA	REQUIRED	PROVIDED
1.	FRONT BUILDING SETBACK	20'	20'
2.	SIDE/REAR BUILDING SETBACK	15'	15'
3.	MAXIMUM HEIGHT	65'	15.5'

IMPERVIOUS AREA CALCULATIONS				
#	DESCRIPTION	SQUARE FOOTAGE (S.F.)	ACREAGE (AC.)	PERCENTAGE (%)
1.	TOTAL SITE AREA:	32,698	0.76	100%
2.	EXISTING BUILDING COVERAGE:	7,191	0.16	22.00%
3.	EXISTING PAVEMENT AND SIDEWALK AREA:	10,406	0.24	31.82%
4.	EXISTING BUILDING TO BE REMOVED:	700	0.02	2.14%
5.	EXISTING PAVEMENT AND SIDEWALK TO BE REMOVED:	6,045	0.14	18.34%
6.	PROPOSED BUILDING COVERAGE:	3,788	0.08	11.26%
7.	PROPOSED PAVEMENT AND SIDEWALK AREA:	6,337	0.15	19.70%
8.	TOTAL PROPOSED IMPERVIOUS AREA:	10,017	0.23	31.00%
9.	TOTAL IMPERVIOUS AREA:	20,869	0.48	64.15%
10.	OPEN AREA:	11,829	0.27	35.84%



VICINITY MAP
ALACHUA COUNTY, FLORIDA
NOT TO SCALE

LEGEND:

ZONING:
ILW – LIGHT AND WAREHOUSE INDUSTRIAL

FUTURE LAND USE:
I – INDUSTRIAL

LEGAL DESCRIPTION

PARCEL 3231-004-001:
A PORTION OF THE PARCEL OF LAND DESCRIBED IN THE SPECIAL WARRANTY DEED RECORDED IN OFFICIAL RECORDS BOOK 2340, PAGE 1962 OF THE PUBLIC RECORDS OF ALACHUA COUNTY, FLORIDA, LYING IN SECTION 14, TOWNSHIP 8 SOUTH, RANGE 18 EAST, ALACHUA COUNTY, FLORIDA, BEING MORE PARTICULARLY DESCRIBED AS FOLLOWS:

COMMENCING AT THE NORTHERLY MOST NORTHWEST CORNER OF SAID PARCEL OF LAND DESCRIBED IN OFFICIAL RECORDS BOOK 2340, PAGE 1962, ALSO BEING ON THE SOUTHERLY RIGHT-OF-WAY LINE OF RACHAEL BOULEVARD/COUNTY ROAD 340-A (66 FOOT RIGHT-OF-WAY); THENCE ON THE NORTHERLY LINE OF SAID PARCEL OF LAND THE FOLLOWNG 6 (SIX) COURSES AND DISTANCES:
1. SOUTH 83 DEGREES 11 MINUTES 05 SECONDS EAST ON SAID RIGHT-OF-WAY LINE, A DISTANCE OF 64.33 FEET TO A STEEL ROD AND CAP;
2. THENCE DEPARTING SAID RIGHT-OF-WAY LINE SOUTH 06 DEGREES 22 MINUTES 07 SECONDS WEST, A DISTANCE OF 102.44 FEET TO A NAIL AND CAP;
3. THENCE SOUTH 83 DEGREES 29 MINUTES 04 SECONDS EAST, A DISTANCE OF 197.00 FEET TO A NAIL AND CAP AND THE POINT OF BEGINNING OF THE FOLLOWING DESCRIBED PARCEL OF LAND;
4. THENCE NORTH 05 DEGREES 50 MINUTES 37 SECONDS EAST, A DISTANCE OF 101.42 FEET TO A STEEL ROD AND CAP TO A POINT ON SAID RIGHT-OF-WAY LINE;
5. THENCE SOUTH 83 DEGREES 11 MINUTES 05 SECONDS EAST ON SAID RIGHT-OF-WAY LINE, A DISTANCE OF 121.08 FEET TO A STEEL ROD AND CAP;
6. THENCE DEPARTING SAID RIGHT-OF-WAY LINE SOUTH 00 DEGREES 42 MINUTES 22 SECONDS WEST, A DISTANCE OF 102.61 FEET TO A NAIL AND CAP;

THENCE DEPARTING SAID NORTHERLY LINE NORTH 82 DEGREES 54 MINUTES 40 SECONDS WEST, A DISTANCE OF 130.28 FEET TO A NAIL AND CAP AND THE POINT OF BEGINNING.

SAID LANDS LYING AND BEING IN SECTION 14, TOWNSHIP 8 SOUTH, RANGE 18 EAST, CITY OF ALACHUA, ALACHUA COUNTY, FLORIDA AND CONTAINING A TOTAL NET AREA OF 0.293 ACRES MORE OR LESS (12,782 SQUARE FEET MORE OR LESS).

PARCEL 3231-003-000:
A TRACT OF LAND SITUATED IN SECTION 14, TOWNSHIP 8 SOUTH, RANGE 18 EAST, ALACHUA COUNTY, FLORIDA, SAID TRACT OF LAND BEING MORE PARTICULARLY DESCRIBED AS FOLLOWS:

COMMENCE AT A CONCRETE MONUMENT AT THE SOUTHWEST CORNER OF THE SOUTHEAST QUARTER OF THE SOUTHWEST QUARTER OF THE AFOREMENTIONED SECTION 14 FOR A POINT OF REFERENCE AND THENCE RUN S.89 DEGREES 13'54" W., ALONG THE SOUTH LINE OF SAID SECTION 14, A DISTANCE OF 29.24 FEET TO A CONCRETE MONUMENT; THENCE RUN N.01 DEGREES 23'30" W., A DISTANCE OF 448.94 FEET TO AN IRON PIPE ON THE SOUTH RIGHT OF WAY LINE OF COUNTY ROAD 340-A (60 FOOT RIGHT OF WAY); THENCE RUN N. 88 DEGREES 37'00" E., ALONG SAID RIGHT OF WAY LINE , A DISTANCE OF 228.17 FEET TO A STEEL ROD AND CAP AT THE BEGINNING OF A CURVE CONCAVE SOUTHERLY, SAID CURVE HAVING A RADIUS OF 1723.55 FEET; THENCE RUN SOUTHEASTERLY, ALONG SAID RIGHT OF WAY LINE AND WITH SAID CURVE, THROUGH AN ARC ANGLE OF 07 DEGREES 09'49", AN ARC DISTANCE OF 215.49 FEET (CHORD BEARING AND DISTANCE OF S.86 DEGREES 47'30"E., 215.35 FEET RESPECTIVELY) TO A STEEL ROAD AND CAP; THENCE RUN S.83 DEGREES 11'05" E., ALONG SAID RIGHT OF WAY LINE, A DISTANCE OF 210.00 FEET TO A STEEL ROD AND CAP AND THE TRUE POINT OF BEGINNING; THENCE CONTINUE S.83 DEGREES 11'05"E., ALONG SAID RIGHT OF WAY LINE, A DISTANCE OF 196.08 FEET TO A STEEL ROD AND CAP; THENCE RUN S.05 DEGREES 50'37"W., A DISTANCE OF 101.42 FEET TO A NAIL AND CAP; THENCE RUN N.83 DEGREES 29'04" W., A DISTANCE OF 197.00 FEET TO A NAIL AND CAP; THENCE RUN N.06 DEGREES 22'07" E., A DISTANCE OF 102.44 FEET TO THE TRUE POINT OF BEGINNING.

TOGETHER WITH A WATERLINE EASEMENT OVER, UNDER AND ACROSS THE FOLLOWING DESCRIBED 10.00 FOOT WIDE STRIP OF LAND:

COMMENCE AT A CONCRETE MONUMENT AT THE SOUTHWEST CORNER OF THE SOUTHEAST QUARTER OF THE SOUTHWEST QUARTER OF THE AFOREMENTIONED SECTION 14 FOR A POINT OF REFERENCE AND THENCE RUN S.89 DEGREES 13'54" W., ALONG THE SOUTH LINE OF SAID SECTION 14, A DISTANCE OF 29.24 FEET TO A CONCRETE MONUMENT; THENCE RUN N.01 DEGREES 23'30"W., A DISTANCE OF 448.94 FEET TO AN IRON PIPE ON THE SOUTH RIGHT OF WAY LINE OF COUNTY ROAD 340-A (60 FOOT RIGHT OF WAY); THENCE RUN N.88 DEGREES 37'00"E., ALONG SAID RIGHT OF WAY LINE, A DISTANCE OF 228.17 FEET TO A STEEL ROD AND CAP AT THE BEGINNING OF A CURVE CONCAVE SOUTHERLY, SAID CURVE HAVING A RADIUS OF 1723.55 FEET; THENCE RUN SOUTHEASTERLY, ALONG SAID RIGHT OF WAY LINE AND WITH SAID CURVE, THROUGH AN ARC ANGLE OF 07 DEGREES 09'49", AN ARC DISTANCE OF 215.49 FEET (CHORD BEARING AND DISTANCE OF S.86 DEGREES 47'30"E., 215.35 FEET RESPECTIVELY) TO A STEEL ROD AND CAP; THENCE RUN S.83 DEGREES 11'05"E., ALONG SAID RIGHT OF WAY LINE, A DISTANCE OF 210.00 FEET TO A STEEL ROD AND CAP; THENCE CONTINUE S.83 DEGREES 11'05" E., ALONG SAID RIGHT OF WAY LINE, A DISTANCE OF 186.08 FEET TO A STEEL ROD AND CAP AND THE TRUE POINT OF BEGINNING; THENCE RUN S.05 DEGREES 50'37"W., A DISTANCE OF 101.47 FEET TO A NAIL AND CAP; THENCE RUN N.83 DEGREES 29'04"W., A DISTANCE OF 187.00 FEET TO A NAIL AND CAP; THENCE RUN N.06 DEGREES 22'07"E., A DISTANCE OF 10.46 FEET, THENCE RUN N.83 DEGREES 37'53"W., A DISTANCE OF 10.00 FEET; THENCE RUN S.06 DEGREES 22'07"W., A DISTANCE OF 20.44 FEET; THENCE RUN S.83 DEGREES 29'04"E., A DISTANCE OF 217.10 FEET, THENCE RUN N.05 DEGREES 50'37"E., A DISTANCE OF 111.37 FEET TO THE AFOREMENTIONED RIGHT OF WAY LINE, THENCE RUN N.83 DEGREES 11'05"W. ALONG SAID RIGHT OF WAY LINE, A DISTANCE OF 20.00 FEET TO THE TRUE POINT OF BEGINNING.

PARKING CALCULATIONS					
#	DESCRIPTION	CRITERIA	EXISTING BUILDING MINIMUM REQUIRED *(1)	PROPOSED BUILDING MINIMUM REQUIRED *(1)	PROVIDED
1.	VEHICULAR PARKING	MANUFACTURING, LIGHT FOR OFFICES, LABS AND AREAS SUPPORTING OFFICES. 1 SPACE PER 350 SF OF FLOOR AREA	1 SPACE PER 350 S.F. X 3,436 SF OFFICES = 10 SPACES X 48% REDEVELOPMENT RATIO = 5 SPACES	1 SPACE PER 350 S.F. X 1,908 SF OFFICES = 5 SPACES	14 INCLUDING 1 HANDICAP SPACE
		MANUFACTURING, LIGHT FOR ALL OTHER AREAS. 1 SPACE PER 1,000 SF OF FLOOR AREA	1 SPACE PER 1,000 S.F. X 3,755 SF = 4 SPACES X 48% REDEVELOPMENT RATIO = 2 SPACES	1 SPACE PER 1,000 S.F. X 1,880 SF = 2 SPACES	
		TOTAL	MINIMUM PARKING SPACES REQUIRED = 14 SPACES X 48% REDEVELOPMENT RATIO = 7 SPACES	MINIMUM PARKING SPACES REQUIRED = 7 SPACES	
		REQUIRED HANDICAP SPACES	N/A	N/A	UP TO 25 TOTAL SPACES = 1 HANDICAP SPACE
3.	MOTORCYCLE PARKING	N/A	N/A	N/A	N/A

* (1) REDEVELOPMENT RATIO IS 48%, WHICH REPRESENTS THE PERCENTAGE INCREASE OF GROSS SQUARE FOOTAGE ASSOCIATED WITH THIS PROJECT. PER LDR SEC. 8.7.2 (a), THE PROPOSED PARKING IS IN COMPLIANCE WITH (AND EXCEEDS) THIS CRITERIA, SPECIFICALLY 48% OF THE REQUIRED PARKING FOR THE EXISTING BUILDING IS PROVIDED AND 100% OF THE REQUIRED PARKING FOR THE PROPOSED BUILDING IS PROVIDED.

BUILDING INFORMATION	
SPRINKLERED:	NO
OCCUPANCY CLASS:	B
CONSTRUCTION TYPE:	III(200)
NUMBER OF STORIES:	1
FLOOR AREA:	
1ST FLOOR	3,788 SF

UTILITIES TO BE PROVIDED AND PROVIDER		
1.	ELECTRIC:	CITY OF ALACHUA
2.	POTABLE WATER:	CITY OF ALACHUA
3.	WASTE WATER:	CITY OF ALACHUA
4.	FIRE SUPPRESSION WATER SUPPLY:	CITY OF ALACHUA

TRIP GENERATION

ITE LAND USE: 110 GENERAL LIGHT INDUSTRIAL
PROPOSED: 3,788 SF GFA OF LAB SPACE

				TRIP DISTRIBUTION		PROJECTED TRIPS	
PERIOD	RATE	PER UNIT	TRIPS	ENTER	EXIT	IN	OUT
AM	0.70	3.79	2.65	88%	12%	2.33	0.32
PM	0.63	3.79	2.39	13%	87%	0.31	2.08
AVG	4.96	3.79	18.79	50%	50%	9.39	9.39

SOURCE: ITE TRIP GENERATION, 10TH EDITION

	AM	PM	AVG				
EXISTING BUILDING	5	5	36				
PROPOSED LAB	3	2	19				
NET	8	7	55				

DRAWING INDEX

DRAWING INDEX	SHEET TITLE
C100	COVER SHEET
C110	GENERAL NOTES AND LEGENDS
C120	DEMOLITION AND EROSION CONTROL PLAN
C200	DIMENSION, PAVING, GRADING, AND DRAINAGE PLAN
C210	PAVING, GRADING, AND DRAINAGE DETAILS
C220	BASIN DETAIL PLAN
C300	STORMWATER POLLUTION PREVENTION PLAN
C400	UTILITY PLAN
C410	UTILITY DETAILS
L101	TREE MITIGATION
L200	LANDSCAPE NOTES AND DETAILS
L201	LANDSCAPE CALCULATIONS
L202	LANDSCAPE PLAN
V-001	BOUNDARY AND TOPOGRAPHIC SURVEY
A	ARCHITECTURAL PLANS
E-1	PHOTOMETRIC PLAN

eda consultants inc.
EB 2389
720 S.W. 2nd Ave, South Tower, Suite 300
GAINESVILLE, FLORIDA 32601
TEL. (352) 373-3541
www.edafl.com mail@edafl.com

No.	Date	Comment

Professional Engineer of Record:

Claudia S. Vega, P.E. 51532
Engineer Certificate No.

Project No: 20-103

Project phase: CITY SUBMITTAL

Project title: ALCHEM LABORATORIES: BUILDING 4 SITE PLAN CITY OF ALACHUA, FLORIDA

Sheet title: COVER SHEET

Designed: CV
Drawn: JB
Checked: CV
Date: 03/24/21

Sheet No.: C100

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EXISTING SYMBOLS & ABBREVIATIONS

C/L	=	CENTERLINE
cmp	=	CORRUGATED METAL PIPE
CONC	=	CONCRETE
COR	=	CORNER
D	=	DELTA (CENTRAL) ANGLE
R	=	RADIUS
A	=	ARC
T	=	TANGENT
CB	=	CHORD BEARING
CD	=	CHORD DISTANCE
dip	=	DUCTILE IRON PIPE
ELEV	=	ELEVATION
ecmp	=	ELLIPTICAL CORRUGATED METAL PIPE
ercp	=	ELLIPTICAL REINFORCED CONCRETE PIPE
ID.	=	IDENTIFICATION
Inv	=	INVERT
F.D.O.T.	=	FLORIDA DEPARTMENT OF TRANSPORTATION
F.F.	=	FINISHED FLOOR
M.E.S.	=	MITERED END SECTION
No.	=	NUMBER
O.R.	=	OFFICIAL RECORDS BOOK
PGS.	=	PAGES
PVC	=	POLY-VINYL CHLORIDE
RCP	=	REINFORCED CONCRETE PIPE
R/W	=	RIGHT OF WAY
SEC 28-9-19	=	SECTION 28, TOWNSHIP 9 SOUTH, RANGE 19 EAST
TRAN	=	ELECTRICAL TRANSFORMER PAD
w/	=	WITH
ANCH	=	GUY ANCHOR
WBO	=	WATER BLOW OFF
SCO	=	SANITARY CLEAN-OUT
■	=	FOUND CONCRETE MONUMENT (size, ID)
■	=	FOUND DRILL HOLE
○	=	FOUND IRON PIPE (size, ID)
❀	=	FOUND NAIL & DISK (ID)
●	=	FOUND REBAR & CAP (size, ID)
▲	=	FOUND SPIKE
GM	=	GAS METER
GV	=	GAS VALVE
GP	=	GUY POLE
EH	=	ELECTRICAL HANDHOLE
FH	=	FIRE HYDRANT
IRRIG VLV	=	IRRIGATION VALVE
LT PL	=	LIGHT POLE
MBOX	=	MAIL BOX
◯	=	MANHOLE
PP	=	POWER POLE
○	=	SET 5/8" REBAR & CAP (LB 2389)
□	=	SET CONCRETE MONUMENT (PRM LB 2389)
❀	=	SET NAIL & DISC (PRM LB 2389)
△	=	SET SPIKE
◯	=	SHRUB
	=	SIGN
TELBX	=	TELEPHONE RISER
TVBOX	=	CABLE TELEVISION RISER
WM	=	WATER METER
WV	=	WATER VALVE
80	=	ELEVATION CONTOUR LINE
x x	=	FENCE LINE
OHGW	=	OVERHEAD GUY WIRE
OHPL	=	OVERHEAD POWER LINE
OHTL	=	OVERHEAD TELEPHONE LINE
OHTV	=	OVERHEAD TELEVISION LINE
SS	=	SANITARY SEWER LINE
UGE	=	UNDERGROUND ELECTRIC LINE
GAS	=	UNDERGROUND GAS LINE
UGT	=	UNDERGROUND TELEPHONE LINE
UGTV	=	UNDERGROUND CABLE TELEVISION LINE
WL	=	UNDERGROUND WATER LINE
88.50 x	=	SPOT ELEVATION

TREE LEGEND

TREE LEGEND:

ONLY TREES WITH A DIAMETER 8" AND GREATER WERE LOCATED FOR THIS SURVEY. (DIAMETER MEASURED 4.5' ABOVE GRADE)

LOCATION · SPECIES · DIAMETER AT GRADE

- to22 = SIZE/COMMON NAME *Genus species*
- pa17 = 22" LIVE OAK *Quercus virginiana*
- = 17" PALM *Sabal palmetto*

LEGEND

- TREE BARRICADE
- SILT FENCE
- (TBR) — EXISTING FEATURES TO BE REMOVED (TBR)
- LIMITS OF EXISTING ASPHALT PAVEMENT, CONCRETE AND/OR BUILDING TO BE REMOVED
- LIMITS OF ASPHALT PAVEMENT
- LIMITS OF CONCRETE PAVEMENT AND SIDEWALKS
- PEDESTRIAN WALKWAY (SEE SHEET C2.10 FOR DETAILS, MATERIAL AND INSTALLATION)
- pi20 — EXISTING TREE TO REMAIN
- pi20 — EXISTING TREE TO BE REMOVED
- 180.00 — EXISTING SPOT ELEVATION
- 180.00 — PROPOSED SPOT ELEVATION
- 183.25 — PROPOSED CONTOUR ELEVATION
- 183 — EXISTING CONTOUR ELEVATION
- PROPOSED EASEMENT

PROPOSED UTILITY SYMBOLS

- WM — WATER MAIN, PROPOSED
- WM — WATER MAIN, EXISTING
- WW — WASTEWATER GRAVITY MAIN, PROPOSED
- PLUG END AND CAP
- BLOW-OFF ASSEMBLY
- GATE VALVE & BOX
- BUTTERFLY VALVE
- PLUG VALVE
- SWING CHECK VALVE
- BACK FLOW PREVENTER
- RP — REDUCED PRESSURE ZONE BFP
- DC — DOUBLE CHECK BFP
- DUAL CHECK VALVE
- ARV — AIR RELEASE VALVE
- REDUCER FITTING
- R — PRESSURE REGULATOR
- POST INDICATOR VALVE
- CLEAN OUT OR ROOF DRAIN LATERAL
- FITTING W/ THRUST BLOCK
- FIRE HYDRANT W/ THRUST BLOCK
- FIRE HYDRANT ASSEMBLY
- POTABLE WATER METER
- SINGLE SERVICE WATER METER
- DUAL SERVICE WATER METER
- GANG WATER METER ASSEMBLY
- TX — ELECTRIC TRANSFORMER
- RECLAIMED WATER METER ASSEMBLY
- RCW — RECLAIMED WATER MAIN, PROPOSED
- RCW — PLUG END AND CAP
- RCW — BLOW-OFF ASSEMBLY
- RCW — GATE VALVE & BOX
- RCW — REDUCER FITTING
- FM — WASTEWATER (PRESSURE) MAIN, PROPOSED
- SS — SANITARY SEWER MAIN, PROPOSED
- SS — SANITARY SEWER MAIN, EXISTING
- SS — SANITARY SEWER MANHOLE, PROPOSED
- SS — SANITARY SEWER MANHOLE, EXISTING
- SANITARY SEWER PLUG
- PLUG VALVE
- SEWER LATERAL, SINGLE SERVICE
- GATE VALVE
- CHECK VALVE
- A. R. — AIR RELEASE VALVE

GENERAL NOTES

1. LOCATIONS, ELEVATIONS, AND DIMENSIONS OF EXISTING UTILITIES, STRUCTURES AND OTHER FEATURES ARE SHOWN ACCORDING TO THE BEST INFORMATION AVAILABLE AT THE TIME OF PREPARATION OF THESE PLANS. THE CONTRACTOR SHALL VERIFY THE LOCATIONS, ELEVATIONS, AND DIMENSIONS OF ALL EXISTING UTILITIES, STRUCTURES AND OTHER FEATURES, AFFECTING THIS WORK, PRIOR TO CONSTRUCTION.
2. PRIOR TO THE INITIATION OF SITE CONSTRUCTION, THE CONTRACTOR SHALL VERIFY ANY EXISTING UTILITIES INCLUDING GAS, WATER, ELECTRIC, CABLE TV, COMMUNICATIONS, SANITARY SEWERS AND STORM DRAINAGE SYSTEMS, ON AND/OR ADJACENT TO THE SITE. REMOVE OR CAP AS NECESSARY. CONTACT ENGINEER OF RECORD IMMEDIATELY WITH ANY DISCREPANCIES.
3. THE CONTRACTOR IS RESPONSIBLE FOR REPAIRING ANY DAMAGE TO EXISTING FACILITIES, ABOVE OR BELOW GROUND, THAT MAY OCCUR AS A RESULT OF THE WORK PERFORMED, BY THE CONTRACTOR OR SUB-CONTRACTORS, AS CALLED FOR IN THESE CONTRACT DOCUMENTS.
4. CONTRACTOR TO COORDINATE WITH UTILITY COMPANIES FOR THE RELOCATION OF EXISTING UTILITIES. UTILITIES NEEDING TO BE REMOVED OR RELOCATED SHALL BE COORDINATED WITH THE AFFECTED UTILITY COMPANY. ADEQUATE TIME SHALL BE PROVIDED FOR RELOCATION, AND CLOSE COORDINATION WITH THE UTILITY COMPANY IS NECESSARY TO PROVIDE A SMOOTH TRANSITION IN UTILITY SERVICE. THE CONTRACTOR SHALL COORDINATE WITH THE UTILITY COMPANY CONCERNING PORTIONS OF THE WORK WHICH MAY BE PERFORMED BY THE UTILITY COMPANY'S FORCES AND ANY FEES WHICH ARE TO BE PAID TO THE UTILITY COMPANY FOR THEIR SERVICES. THE CONTRACTOR IS RESPONSIBLE FOR PAYING ALL FEES AND CHARGES.
5. IT IS THE CONTRACTOR'S RESPONSIBILITY TO BECOME FAMILIAR WITH THE PERMIT AND INSPECTION REQUIREMENTS SPECIFIED BY THE VARIOUS GOVERNMENTAL AGENCIES AND THE ENGINEER. THE CONTRACTOR SHALL OBTAIN ALL NECESSARY PERMITS PRIOR TO CONSTRUCTION, AND SCHEDULE INSPECTIONS ACCORDING TO AGENCY INSTRUCTION/REQUIREMENTS.
6. THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS, ON ALL PRECAST AND MANUFACTURED ITEMS, TO THE OWNER'S ENGINEER FOR REVIEW. FAILURE TO OBTAIN APPROVAL BEFORE INSTALLATION MAY RESULT IN REMOVAL AND REPLACEMENT AT THE CONTRACTOR'S EXPENSE.
7. SAFETY:
   - A. DURING THE CONSTRUCTION AND/OR MAINTENANCE OF THIS PROJECT, ALL SAFETY REGULATIONS ARE TO BE ENFORCED. THE CONTRACTOR OR HIS REPRESENTATIVE SHALL BE RESPONSIBLE FOR THE CONTROL AND SAFETY OF THE TRAVELING PUBLIC AND THE SAFETY OF HIS/HER PERSONNEL.
   - B. LABOR SAFETY REGULATIONS SHALL CONFORM TO THE PROVISIONS SET FORTH BY OSHA.
   - C. THE MINIMUM STANDARDS AS SET FORTH IN THE CURRENT EDITION OF "THE STATE OF FLORIDA, MANUAL ON TRAFFIC CONTROL AND SAFE PRACTICES FOR STREET AND HIGHWAY CONSTRUCTION, MAINTENANCE AND UTILITY OPERATIONS" SHALL BE FOLLOWED IN THE DESIGN, APPLICATION, INSTALLATION, MAINTENANCE AND REMOVAL OF ALL TRAFFIC CONTROL DEVICES, WARNING DEVICES AND BARRIERS NECESSARY TO PROTECT THE PUBLIC AND CONSTRUCTION PERSONNEL FROM HAZARDS WITHIN THE PROJECT LIMITS.
   - D. ALL TRAFFIC CONTROL MARKINGS AND DEVICES SHALL CONFORM TO THE PROVISIONS SET FORTH IN THE MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES PREPARED BY THE U.S. DEPARTMENT OF TRANSPORTATION FEDERAL HIGHWAY ADMINISTRATION.
   - E. ALL SUBSURFACE CONSTRUCTION SHALL COMPLY WITH THE "TRENCH SAFETY ACT". THE CONTRACTOR SHALL INSURE THAT THE METHOD OF TRENCH PROTECTION AND CONSTRUCTION IS IN COMPLIANCE WITH THE OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION (OSHA) REGULATIONS.
   - F. IT SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR TO COMPLY AND ENFORCE ALL APPLICABLE SAFETY REGULATIONS. THE ABOVE INFORMATION HAS BEEN PROVIDED FOR THE CONTRACTOR'S INFORMATION ONLY AND DOES NOT IMPLY THAT THE OWNER OR ENGINEER WILL INSPECT AND/OR ENFORCE SAFETY REGULATIONS.
8. ALL UNDERGROUND UTILITIES MUST BE IN-PLACE, TESTED AND INSPECTED PRIOR TO BASE AND SURFACE CONSTRUCTION.
9. CONTRACTOR IS REQUIRED TO SECURE A FLORIDA DEPARTMENT OF ENVIRONMENTAL PROTECTION (FDEP) "NATIONAL POLLUTANT DISCHARGE ELIMINATION SYSTEM" (NPDES) PERMIT BEFORE BEGINNING CONSTRUCTION.
10. A COMPLETE SET OF PERMITTED DRAWINGS AND SPECIFICATIONS MUST BE MAINTAINED ON SITE AT ALL TIMES THAT THE CONTRACTOR IS PERFORMING WORK. THESE DRAWINGS SHALL BE MADE AVAILABLE UPON REQUEST.
11. ALL EROSION AND SEDIMENT CONTROL DEVICES SHALL BE CONSTRUCTED SIMULTANEOUSLY WITH THE DISTURBANCE OF THE LAND AND SHALL REMAIN FUNCTIONAL UNTIL THE CONTRIBUTING DISTURBED AREAS ARE STABILIZED. SILT BARRIERS WILL BE INSTALLED AS NECESSARY TO PREVENT EXCESSIVE SEDIMENTATION OF DOWNSTREAM AREAS.
12. CITY:
    ALL WORK WITHIN OR ON CITY OWNED AND MAINTAINED FACILITIES, ROW OR EASEMENTS WILL REQUIRE AS-BUILT PLANS. AS-BUILT PLANS SHOULD SHOW THE CONSTRUCTED CONDITIONS OF THE CITY OWNED OR MAINTAINED AREA AND BE PERFORMED BY A FLORIDA LICENSED PROFESSIONAL SURVEYOR AND MAPPER. THE COORDINATE SYSTEM SHALL BE FLORIDA STATE PLANE COORDINATES, NAD 83 ZONE NORTH US SURVEY FEET. VERTICAL COORDINATES SHALL BE REFERENCED TO THE NAVD 88 DATUM WITH ELEVATIONS GIVEN IN US SURVEY FEET.

    AS-BUILT PLANS SHALL BE SUBMITTED TO THE PUBLIC WORKS DEPARTMENT AS SIGNED AND SEALED PLANS AND AN ELECTRONIC DRAWING FILE. GUIDANCE ON PREPARATION OF AS-BUILT PLANS CAN BE FOUND IN CHAPTER 4 OF THE FDOT OFFICE OF CONSTRUCTION, PREPARATION AND DOCUMENTATION MANUAL. AS-BUILT PLANS FOR DRIVEWAY PERMITS MAY BE REQUIRED ON A CASE BY CASE BASIS.

    THE ENGINEER OF RECORD SHALL FILE AS-BUILT PLANS OR RECORD DRAWINGS WITH THE PUBLIC WORKS DEPARTMENT UPON COMPLETION OF ANY IMPROVEMENTS FOR WHICH CHANGES HAVE BEEN APPROVED DURING THE CONSTRUCTION PROCESS. THE REQUIREMENT FOR AN AS-BUILT PLAN OR RECORD DRAWING WILL BE DETERMINED ON A CASE BY CASE BASIS, DEPENDING ON THE SCOPE OF THE CHANGES.

    RECORD DRAWINGS SHALL BE LEGIBLY MARKED TO DOCUMENT ACTUAL CONSTRUCTION. ALL CHANGES TO FINAL UTILITY LOCATIONS INCLUDING HORIZONTAL AND VERTICAL LOCATIONS SHALL BE CLEARLY SHOWN AND REFERENCED TO PERMANENT SURFACE IMPROVEMENTS AND INCLUDE ALL MANHOLES, HYDRANTS, VALVES, VALVE BOXES, AND STRUCTURES. RECORD DRAWINGS SHALL ALSO DOCUMENT ACTUAL INSTALLED PIPE MATERIALS. RECORD DRAWINGS SHALL CLEARLY SHOW AND DOCUMENT ALL FIELD CHANGES OF DIMENSION AND DETAIL. RECORD DRAWINGS SHALL INCLUDE ALL DETAILS NOT ON THE ORIGINAL DRAWINGS, BUT CONSTRUCTED.
13. THE CONTRACTOR SHALL CONSTRUCT GRAVITY SEWER LATERALS, MANHOLES GRAVITY SEWER LINES AND DOMESTIC WATER AND FIRE PROTECTION SYSTEM AS SHOWN ON THESE PLANS. THE CONTRACTOR SHALL FURNISH ALL NECESSARY MATERIALS, EQUIPMENT, MACHINERY, TOOLS, MEANS OF TRANSPORTATION AND LABOR NECESSARY TO COMPLETE THE WORK IN FULL AND COMPLETE ACCORDANCE WITH THE SHOWN, DESCRIBED AND REASONABLY INTENDED REQUIREMENTS OF THE CONTRACT DOCUMENTS AND JURISDICTIONAL AGENCY REQUIREMENTS. IN THE EVENT THAT THE CONTRACT DOCUMENTS AND THE JURISDICTIONAL AGENCY REQUIREMENTS ARE NOT IN AGREEMENT, THE MOST STRINGENT SHALL GOVERN.

NOTE:
PLEASE REFER TO DEMOLITION PLAN FOR DEMOLITION NOTES.

CONSTRUCTION NOTES

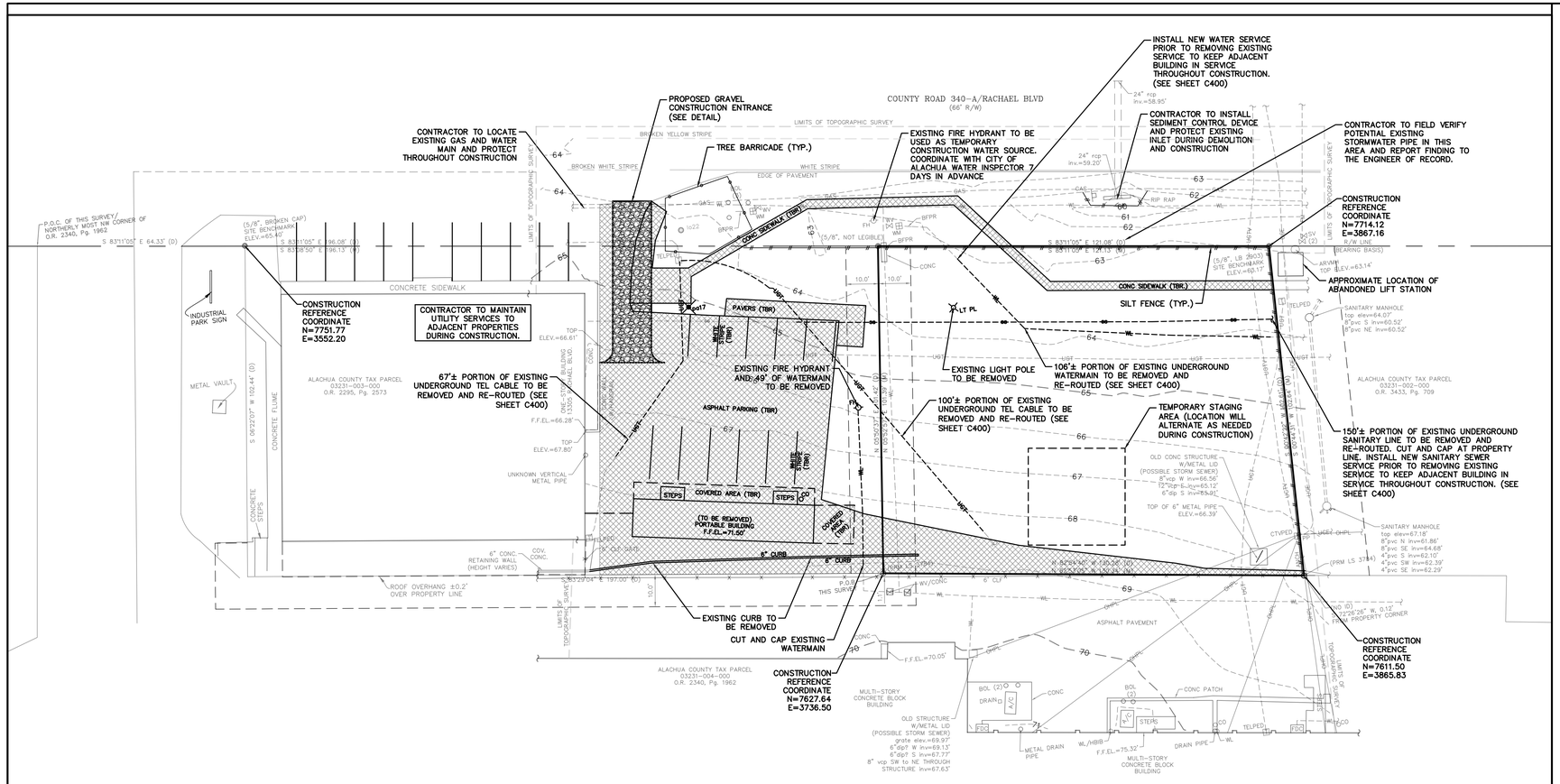
1. SIGNS AND BARRICADES SHALL BE IN ACCORDANCE WITH THE U.S. DEPARTMENT OF TRANSPORTATION'S 'MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES', AND THE FLORIDA DEPARTMENT OF TRANSPORTATION'S 'ROADWAY AND TRAFFIC DESIGN STANDARDS' INDEXES 600 THROUGH 685 (LATEST EDITIONS).
2. SAFE PEDESTRIAN TRAFFIC IS TO BE MAINTAINED AT ALL TIMES.
3. ANY SIDEWALK WHICH BECOMES UNDERMINED MUST BE REMOVED AND REPLACED. SIDEWALKS ARE TO BE RECONSTRUCTED WITHIN THREE (3) DAYS AFTER REMOVAL. WHEN EXISTING SIDEWALK IS REMOVED, IT IS TO BE REMOVED TO THE NEAREST JOINT.
4. DISTURBED AREA WITHIN THE R-O-W WILL BE COMPACTED TO 98% OF MAXIMUM DENSITY AND SODDED.
5. STOCKPILING OF MATERIAL IS NOT ALLOWED ON ROADWAYS OR SIDEWALKS. ALL DIRT AND DEBRIS WILL BE REMOVED FROM JOB SITE DAILY. ROADS AND SIDEWALKS ARE TO BE SWEPT DAILY AS PART OF DAILY CLEANUP.
6. ANY PORTION OF ROADWAYS OR SIDEWALKS THAT SUSTAIN EXCESSIVE CONSTRUCTION RELATED DAMAGE, IN THE OPINION OF APPLICABLE AGENCIES, SHALL BE REPAIRED AT CONTRACTOR EXPENSE IN A MANNER SPECIFIED BY THAT PARTICULAR AGENCY.
7. CONSTRUCTION MUST BE PER THE APPROVED SITE PLAN OF THE PZB (OR EQUIVALENT). DEVIATIONS IN ROADWAY, UTILITY OR DRAINAGE CONSTRUCTION WILL REQUIRE PRIOR WRITTEN APPROVAL OF THE CITY ENGINEER OR THE DIRECTOR OF PUBLIC SERVICES (OR EQUIVALENT). NOTE: SIGNIFICANT CHANGES FROM THE PZB APPROVED PLAN MAY REQUIRE THE OWNER/DEVELOPER TO SUBMIT A REVISED SITE PLAN FOR REVIEW THROUGH THE PZB AND MAY CAUSE PROJECT DELAYS.
8. OFF-SITE OR ROADWAY R-O-W CONSTRUCTION SHALL BE IN ACCORDANCE WITH ALL CONDITIONS OF THE APPROVED R-O-W PERMIT(S). A COPY OF THE APPROVED R-O-W PERMIT MUST BE KEPT ON-SITE AND READILY AVAILABLE DURING ALL CONSTRUCTION ACTIVITIES WITHIN THE R-O-W.
9. DURING THE CONSTRUCTION AND/OR MAINTENANCE OF THIS PROJECT, ALL SAFETY REGULATIONS ARE TO BE ENFORCED. THE CONTRACTOR OR HIS REPRESENTATIVE SHALL BE RESPONSIBLE FOR THE CONTROL AND SAFETY OF THE TRAVELING PUBLIC AND THE SAFETY OF HIS/HER PERSONNEL.
10. ALL CONSTRUCTION, MATERIALS AND WORKMANSHIP ARE TO BE IN ACCORDANCE WITH FLORIDA DEPARTMENT OF TRANSPORTATION SPECIFICATIONS AND STANDARDS.
11. CONTRACTOR IS RESPONSIBLE FOR CHECKING ACTUAL SITE CONDITIONS BEFORE STARTING CONSTRUCTION.
12. ANY DISCREPANCIES ON THE DRAWINGS SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER BEFORE COMMENCING WORK.
13. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL REQUIRED CONSTRUCTION PERMITS AND BONDS IF REQUIRED PRIOR TO CONSTRUCTION.
14. THE CONTRACTOR SHALL BE RESPONSIBLE FOR SUBMITTING TO THE ENGINEER A CERTIFIED RECORD SURVEY SIGNED AND SEALED BY A PROFESSIONAL LAND SURVEYOR REGISTERED IN THE STATE OF FLORIDA DEPICTING THE ACTUAL FIELD LOCATION OF ALL CONSTRUCTED IMPROVEMENTS THAT ARE REQUIRED BY THE JURISDICTIONAL AGENCIES FOR THE CERTIFICATION PROCESS. ALL SURVEY COSTS WILL BE THE CONTRACTORS RESPONSIBILITY. BOTH PAPER AND AUTOCAD FILES SHALL BE PROVIDED.
15. THE CONTRACTOR SHALL BE RESPONSIBLE FOR DOCUMENTING AND MAINTAINING AS-BUILT INFORMATION WHICH SHALL BE RECORDED AS CONSTRUCTION PROGRESSES OR AT THE COMPLETION OF APPROPRIATE CONSTRUCTION INTERVALS AND SHALL BE RESPONSIBLE FOR PROVIDING AS-BUILT DRAWINGS TO THE OWNER AND ENGINEER OF RECORD FOR THE PURPOSE OF CERTIFICATION TO JURISDICTIONAL AGENCIES AS REQUIRED. BOTH PAPER AND AUTOCAD SHALL BE PROVIDED.

EROSION CONTROL NOTES

1. THE STORM WATER POLLUTION PREVENTION PLAN (SWPPP) IS COMPRISED OF THE EROSION CONTROL PLAN, THE STANDARD DETAILS, THE PLAN NARRATIVE, ATTACHMENTS REFERENCED BY THE SWPPP, PLUS THE PERMIT AND ALL SUBSEQUENT REPORTS AND RELATED DOCUMENTS.
2. ALL CONTRACTORS AND SUBCONTRACTORS INVOLVED WITH STORM WATER POLLUTION PREVENTION SHALL OBTAIN A COPY OF THE STORM WATER POLLUTION PREVENTION PLAN AND THE STATE OF FLORIDA NATIONAL POLLUTANT DISCHARGE ELIMINATION SYSTEM GENERAL PERMIT (NPDES PERMIT) AND BECOME FAMILIAR WITH THEIR CONTENTS.
3. THE CONTRACTOR SHALL IMPLEMENT BEST MANAGEMENT PRACTICES AS REQUIRED BY THE SWPPP. ADDITIONAL BEST MANAGEMENT PRACTICES SHALL BE IMPLEMENTED AS DICTATED BY CONDITIONS AT NO ADDITIONAL COST TO THE OWNER THROUGHOUT ALL PHASES OF CONSTRUCTION.
4. PERMITS FOR ANY CONSTRUCTION ACTIVITY IMPACTING STATE WATERS OR REGULATED WETLANDS MUST BE MAINTAINED ON SITE AT ALL TIMES.
5. ALL WASH WATER (CONCRETE TRUCKS, VEHICLE CLEANING, EQUIPMENT CLEANING, ETC.) SHALL BE DETAINED AND PROPERLY TREATED OR DISPOSED.
6. SUFFICIENT OIL AND GREASE ABSORBING MATERIALS AND FLOTATION BOOMS SHALL BE MAINTAINED ON SITE OR READILY AVAILABLE TO CONTAIN AND CLEAN-UP FUEL OR CHEMICAL SPILLS AND LEAKS.
7. THE CONTRACTOR SHALL BE RESPONSIBLE FOR DUST CONTROL ON SITE. THE USE OF MOTOR OILS AND OTHER PETROLEUM BASED OR TOXIC LIQUIDS FOR DUST SUPPRESSION OPERATIONS IS PROHIBITED.
8. RUBBISH, TRASH, GARBAGE, LITTER, OR OTHER SUCH MATERIALS SHALL BE DEPOSITED INTO SEALED CONTAINERS. MATERIALS SHALL BE PREVENTED FROM LEAVING THE PREMISES THROUGH THE ACTION OF WIND OR STORM WATER RUNOFF.
9. ALL STORM WATER POLLUTION PREVENTION MEASURES PRESENTED ON THE PLAN SHALL BE INITIATED AS SOON AS PRACTICABLE.
10. DISTURBED PORTIONS OF THE SITE WHERE CONSTRUCTION ACTIVITY HAS PERMANENTLY STOPPED SHALL BE PERMANENTLY SEEDED. THESE AREAS SHALL BE SEEDED NO LATER THAN 7 DAYS AFTER THE LAST CONSTRUCTION ACTIVITY OCCURRED IN THESE AREAS.
11. THE CONTRACTOR SHALL BE RESPONSIBLE FOR REMOVING SEDIMENT IN THE DETENTION POND AND ANY SEDIMENT THAT MAY HAVE COLLECTED IN THE STORM DRAINS IN CONJUNCTION WITH THE STABILIZATION OF THE SITE.
12. ON-SITE & OFF SITE SOIL STOCKPILE AND BORROW AREAS SHALL BE PROTECTED FROM EROSION AND SEDIMENTATION THROUGH IMPLEMENTATION OF BEST MANAGEMENT PRACTICES. STOCKPILE AND BORROW AREA LOCATIONS SHALL BE NOTED ON THE EROSION CONTROL PLAN BY THE CONTRACTOR AND PERMITTED IN ACCORDANCE WITH GENERAL PERMIT REQUIREMENTS.
13. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ADJUSTING THE EROSION CONTROL MEASURES (SILT FENCES, STABILIZATION, SEDIMENT BASINS, ETC.) AS NEEDED FOR EACH STAGE OF SITE WORK / GRADING.
14. NO GRADING, CUTTING, OR FILLING SHALL COMMENCE UNTIL SUCH TIME AS APPROPRIATE EROSION AND SEDIMENTATION CONTROL DEVICES HAVE BEEN INSTALLED BETWEEN ALL DISTURBED AREAS AND WATER BODIES, WATERCOURSES OR WETLANDS AND ANY CONVEYANCES SUCH AS DRAINAGE DITCHES, STORM DRAINS, AND INLETS.
15. ALL EROSION AND SEDIMENT CONTROL DEVICES SHALL BE INSTALLED PRECEEDING ANY DISTURBANCE OF THE LAND AND SHALL REMAIN FUNCTIONAL UNTIL THE CONTRIBUTING DISTURBED AREAS ARE STABILIZED. THE CONTRACTOR SHALL INSTALL ALL EROSION AND PREVENTION STRUCTURES SHOWN ON THE PLANS AT A MINIMUM AND IN FULL CONFORMANCE WITH ALL APPLICABLE WATER MANAGEMENT DISTRICT PERMITS AND REGULATIONS.
16. ALL CONTROL STRUCTURE AND OUTFALL CULVERT INSTALLATIONS SHALL BE PROTECTED WITH SEDIMENT BARRIERS AT A MINIMUM.
17. ALL DISTURBED AREAS (ABOVE NORMAL WATER LEVELS) UNLESS OTHERWISE NOTED HEREIN, SHALL BE SEEDED AND MULCHED WITHIN 7 DAYS OF FINAL GRADING. UNLESS OTHERWISE NOTED, ALL SLOPES STEEPER THAN 4:1 SHALL BE SODDED.

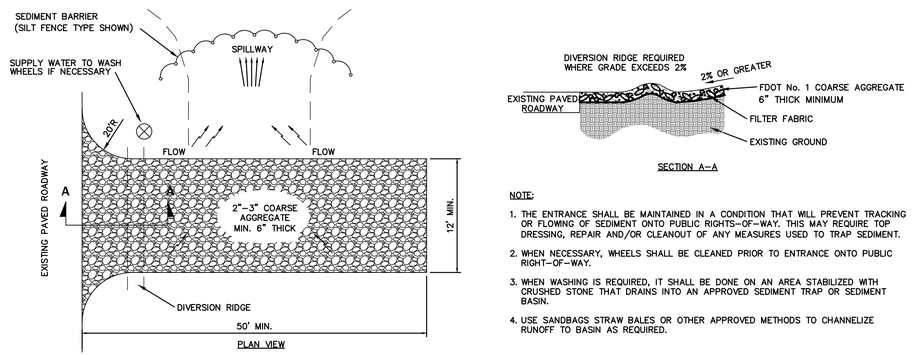
PAVING, GRADING, AND DRAINAGE NOTES

1. TRAFFIC CONTROL ON ALL FDOT, LOCAL AND COUNTY RIGHTS-OF-WAY SHALL MEET THE REQUIREMENTS OF THE MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES (U.S. DOT/FHA) AND THE REQUIREMENTS OF THE STATE AND ANY LOCAL AGENCY HAVING JURISDICTION. IN THE EVENT THAT THE CONTRACT DOCUMENTS AND THE JURISDICTIONAL AGENCY REQUIREMENTS ARE NOT IN AGREEMENT, THE MOST STRINGENT SHALL GOVERN.
2. ALL OPEN AREAS WITHIN THE PROJECT SITE SHALL BE SODDED UNLESS INDICATED OTHERWISE ON THE LANDSCAPE PLAN.
3. THE CONTRACTOR SHALL INSTALL FILTER FABRIC OVER ALL DRAINAGE STRUCTURES FOR THE DURATION OF CONSTRUCTION AND UNTIL ACCEPTANCE OF THE PROJECT BY THE OWNER. ALL DRAINAGE STRUCTURES SHALL BE CLEANED OF DEBRIS AS REQUIRED DURING AND AT THE END OF CONSTRUCTION TO PROVIDE POSITIVE DRAINAGE FLOWS.
4. IF DEWATERING IS REQUIRED, THE CONTRACTOR SHALL OBTAIN ANY APPLICABLE REQUIRED PERMITS. THE CONTRACTOR IS TO COORDINATE WITH THE OWNER AND THE DESIGN ENGINEER PRIOR TO ANY EXCAVATION.
5. THE CONTRACTOR MUST REVIEW AND MAINTAIN A COPY OF THE ENVIRONMENTAL RESOURCE PERMIT COMPLETE WITH ALL CONDITIONS, ATTACHMENTS, EXHIBITS, AND PERMIT MODIFICATIONS IN GOOD CONDITION AT THE CONSTRUCTION SITE. THE COMPLETE PERMIT MUST BE AVAILABLE FOR REVIEW UPON REQUEST BY WATER MANAGEMENT DISTRICT REPRESENTATIVES.
6. THE CONTRACTOR SHALL ENSURE THAT ISLAND PLANTING AREAS AND OTHER PLANTING AREAS ARE NOT COMPACTED AND DO NOT CONTAIN ROAD BASE MATERIALS. THE CONTRACTOR SHALL ALSO EXCAVATE AND REMOVE ALL UNDESIRABLE MATERIAL FROM ALL AREAS ON THE SITE TO BE PLANTED AND PROPERLY DISPOSED OF IN A LEGAL MANNER.
7. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROTECTING EXCAVATIONS AGAINST COLLAPSE AND WILL PROVIDE BRACING, SHEETING OR SHORING AS NECESSARY. DE-WATERING METHODS SHALL BE USED AS REQUIRED TO KEEP TRENCHES DRY WHILE PIPE AND APPURTENANCES ARE BEING PLACED.
8. THE CONTRACTOR WILL STABILIZE BY SEED AND MULCH, SOD, OR OTHER APPROVED MATERIALS ANY DISTURBED AREAS WITHIN ONE WEEK FOLLOWING CONSTRUCTION OF THE UTILITY SYSTEMS AND PAVEMENT AREAS. CONTRACTOR SHALL MAINTAIN SUCH AREAS UNTIL FINAL ACCEPTANCE BY OWNER. CONTRACTOR TO COORDINATE WITH OWNER REGARDING TYPE OF MATERIAL, LANDSCAPING AND IRRIGATION IMPROVEMENTS TO FOLLOW.
9. SITE GRADING, PAVING AND DRAINAGE MATERIALS AND CONSTRUCTION SHALL CONFORM TO FDOT STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION.
10. IMMEDIATELY AT ONSET OF CONSTRUCTION, CONTRACTOR SHALL FIELD VERIFY HORIZONTAL AND VERTICAL LOCATIONS OF ALL EXISTING UTILITIES CRITICAL TO COMPLETING THE PROJECT (INCLUDING WATER, SEWER, POWER, TELEPHONE, GAS, AND CABLE TV) AND SHALL EVALUATE POTENTIAL CONFLICTS. ALL SUCH CONFLICTS SHALL BE REPORTED TO ENGINEER/OWNER IMMEDIATELY UPON DISCOVERY.
11. PRIOR TO CONSTRUCTION, CONTRACTOR SHALL FIELD STAKE AND ROPE OFF CONSERVATION AREA LINES. OWNER RESERVES THE RIGHTS TO CHECK THE STAKING AND ROPING AND REQUIRE IT TO BE RELOCATED IF NECESSARY. IT SHALL REMAIN IN PLACE UNTIL ADJACENT CONSTRUCTION IS COMPLETE.
12. CONTRACTOR SHALL BE EXTREMELY CAUTIOUS WHEN WORKING NEAR TREES WHICH ARE TO BE SAVED, WHETHER SHOWN IN THE PLANS OR DESIGNATED IN THE FIELD.
13. ALL SIGNAGE, PAVEMENT MARKING AND TRAFFIC CONTROL DEVICES SHALL BE IN ACCORDANCE WITH FDOT 'ROADWAY AND TRAFFIC DESIGN STANDARDS' AND FHWA 'MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES'.
14. REGULATORY SIGNS (STOP, ETC.) SHALL BE PAID FOR BY THE CONTRACTOR AND IN PLACE PRIOR TO FINAL INSPECTION OF PAVING AND DRAINAGE IMPROVEMENTS.
15. BLUE REFLECTIVE PAVEMENT MARKERS SHALL BE PLACED OPPOSITE FIRE HYDRANTS IN THE CENTER OF THE NEAREST TRAVELED LANE TO MARK THEIR LOCATIONS.
16. CONTRACTOR IS RESPONSIBLE FOR GRADING ALL PAVEMENTS TO DRAIN POSITIVELY. INTERSECTIONS SHALL BE TRANSITIONED TO PROVIDE SMOOTH DRIVING SURFACE WHILE MAINTAINING POSITIVE DRAINAGE. SHOULD AREAS OF POOR DRAINAGE BE OBSERVED, CONTRACTOR SHALL NOTIFY THE ENGINEER PRIOR TO PLACEMENT OF CURBS OR PAVEMENT COURSES, SO THAT RECOMMENDATIONS FOR CORRECTION MAY BE MADE.
17. PROPOSED AND EXISTING SIDEWALKS SHALL BE RAMPED FLUSH WITH PAVEMENT. RAMPS SHALL NOT EXCEED SLOPES OF 12 HORIZONTAL TO 1 VERTICAL.
18. FINISHED FLOOR ELEVATIONS ARE MINIMUM ELEVATIONS REQUIRED TO SATISFY DRAINAGE AND/OR 100-FLOODPLAIN REQUIREMENTS. PAD ELEVATIONS, IMMEDIATELY OUTSIDE OF BUILDING WALLS, SHALL BE NO MORE THAN 8 INCHES BELOW THE FINISHED FLOOR ELEVATIONS SHOWN.
19. CONTRACTOR IS RESPONSIBLE FOR IDENTIFYING AND DISPOSING ALL WASTE MATERIALS CONSISTENT WITH ALL RULES AND REGULATIONS APPLICABLE TO THE SPECIFIC MATERIAL FOUND. ALL DELETERIOUS SUBSURFACE MATERIAL (I.E. MUCK, PEAT, BURIED DEBRIS, ETC.) IS TO BE EXCAVATED AND REPLACED WITH SUITABLE/COMPACTED SOILS, AS DIRECTED BY THE OWNER, THE OWNERS ENGINEERS, OR OWNERS SOILS TESTING COMPANY. DELETERIOUS MATERIAL IS TO BE STOCKPILED OR REMOVED FROM THE SITE AS DIRECTED BY THE OWNER. EXCAVATED AREAS ARE TO BE BACKFILLED WITH APPROVED MATERIALS AND COMPACTED AS SHOWN ON THESE PLANS. CONTRACTOR IS RESPONSIBLE FOR ACQUIRING ANY PERMITS THAT ARE NECESSARY FOR REMOVING DELETERIOUS MATERIAL FROM THE SITE.
20. ALL NECESSARY FILL AND EMBANKMENT THAT IS PLACED DURING CONSTRUCTION SHALL CONSIST OF MATERIAL SPECIFIED BY THE OWNER'S SOILS TESTING COMPANY OR ENGINEER AND BE PLACED AND COMPACTED ACCORDING TO THESE PLANS.
21. PROPOSED SPOT ELEVATIONS REPRESENT FINISHED PAVEMENT, SIDEWALK, OR GROUND SURFACE GRADES, UNLESS OTHERWISE NOTED.
22. CURBING SHALL BE PLACED AT THE EDGES OF ALL PAVEMENT, UNLESS OTHERWISE NOTED. REFER TO THE LATEST EDITION OF F.D.O.T. "ROADWAY AND TRAFFIC DESIGN STANDARDS" FOR DETAILS AND SPECIFICATIONS OF ALL FDOT TYPE CURB AND GUTTERS CALLED FOR IN THESE PLANS.
23. THE CONTRACTOR SHALL RESTORE OFF-SITE CONSTRUCTION AREAS TO EQUAL AND/OR BETTER CONDITION THAN EXISTING P



DEMOLITION NOTES

1. ALL MATERIAL REMOVED FROM THIS SITE BY THE CONTRACTOR SHALL BE DISPOSED OF BY THE CONTRACTOR IN A LEGAL MANNER.
2. REFER TO THE TOPOGRAPHIC SURVEY FOR ADDITIONAL DETAILS OF EXISTING STRUCTURES, ETC., LOCATED WITHIN THE PROJECT SITE. UNLESS OTHERWISE NOTED, ALL EXISTING BUILDINGS, STRUCTURES, SLABS, CONCRETE, ASPHALT, DEBRIS PILES, SIGNS, AND ALL APPURTENANCES ARE TO BE REMOVED FROM THE SITE BY THE CONTRACTOR AND PROPERLY DISPOSED OF IN A LEGAL MANNER AS PART OF THIS CONTRACT. SOME ITEMS TO BE REMOVED MAY NOT BE DEPICTED ON THE TOPOGRAPHIC SURVEY. IT IS THE CONTRACTOR'S RESPONSIBILITY TO VISIT THE SITE AND DETERMINE THE FULL EXTENT OF ITEMS TO BE REMOVED. IF ANY ITEMS ARE IN QUESTION, THE CONTRACTOR SHALL CONTACT THE OWNER PRIOR TO REMOVAL OF SAID ITEMS.
3. THE CONTRACTOR SHALL REFER TO THE DEMOLITION PLAN FOR DEMOLITION / PRESERVATION OF EXISTING TREES. ALL TREES NOT SPECIFICALLY SHOWN TO BE PRESERVED OR RELOCATED SHALL BE REMOVED AS A PART OF THIS CONTRACT. TREE PROTECTION FENCING SHALL BE INSTALLED PRIOR TO ANY DEMOLITION.
4. CONTRACTOR SHALL SUBMIT DEMOLITION SCHEDULE TO OWNER PRIOR TO PROCEEDING WITH DEMOLITION ACTIVITIES.
5. CONDUCT SITE DEMOLITION OPERATIONS TO ENSURE MINIMUM INTERFERENCE WITH ROADS, STREETS, WALKS, AND OTHER ADJACENT OCCUPIED OR USED FACILITIES. DO NOT CLOSE OR OBSTRUCT STREETS, WALKS, OR OTHER OCCUPIED OR USED FACILITIES WITHOUT PERMISSION FROM AUTHORITIES HAVING JURISDICTION. CLOSURE OF THESE FACILITIES MAY REQUIRE A MAINTENANCE OF TRAFFIC PLAN PREPARED BY A REGISTERED PROFESSIONAL AT THE CONTRACTORS EXPENSE.
6. PROVIDE PROTECTION AS NECESSARY TO PREVENT DAMAGE TO EXISTING IMPROVEMENTS SHOWN IN THE PLANS TO REMAIN.
7. LOCATE EXISTING ABOVE GROUND AND UNDERGROUND UTILITIES IN AREAS OF WORK. IF UTILITIES ARE TO REMAIN IN PLACE, PROVIDE ADEQUATE MEANS OF SUPPORT AND PROTECTION DURING DEMOLITION AND CONSTRUCTION OPERATIONS.
8. CONTRACTOR SHALL COORDINATE WITH APPLICABLE UTILITY COMPANIES FOR THE TERMINATION, CAPPING-OFF AND REMOVAL OF ALL EXISTING UNDERGROUND AND ABOVE GROUND UTILITY SERVICES UNLESS DIRECTED TO OTHERWISE.
9. EROSION CONTROLS ARE TO BE INSTALLED AND INSPECTED PRIOR TO BEGINNING DEMOLITION WORK.
10. IN AREAS WHERE PROPOSED CURBING IS TO BE PLACED, THE ASPHALT PAVEMENT SHALL BE REMOVED TO THE FACE OF CURB TO ALLOW PLACEMENT OF 6" STANDARD CONCRETE CURBING.
11. IN AREAS OF PROPOSED LANDSCAPE AREA THE PAVEMENT, LIMEROCK BASE, AND 12" OF THE STABILIZED SUBGRADE SHALL BE REMOVED. COORDINATE WITH LANDSCAPE ARCHITECT TO DETERMINE IF ADDITIONAL REMOVAL IS REQUIRED TO MAKE AREA SUITABLE FOR LANDSCAPE PURPOSES. THE REMOVED LIMEROCK BASE SHALL NOT BE USED IN THE BASE FOR THE NEW PAVEMENT (PER FDOT SECTION 200), BUT THE REMOVED BASE CAN BE USED IN THE STABILIZATION OF SUBGRADE. SEE GRADING PLAN FOR BACKFILL REQUIREMENTS.
12. ALL CONSTRUCTION DEBRIS, LIMEROCK, EXCESS OF BUILDER'S SAND, CONCRETE AND MORTAR DEBRIS, EXISTING WEEDS AND GRASSES, ALL FOREIGN MATERIALS IN THE PLANTING BED AND SOD AREAS SHALL BE REMOVED AND 36" OF CLEAN FILL OF PH 5.5 – 6.5 SHALL BE INSTALLED PRIOR TO ANY INSTALLATION OF PLANTS OR TREES.
13. SEE DIMENSION PLAN FOR DIMENSIONING OF PROPOSED LANDSCAPE AREAS.
14. CONTRACTOR IS RESPONSIBLE FOR PROTECTION OF EXISTING UTILITIES TO REMAIN AS SHOWN IN THE UTILITY PLAN. ANY EXISTING UTILITIES TO BE REMOVED SHALL BE COORDINATED WITH THE ASSOCIATED UTILITY COMPANY, AND PERFORMED IN ACCORDANCE WITH ALL APPLICABLE REGULATIONS.
15. SEE ELECTRICAL AND IRRIGATION PLANS FOR COORDINATION OF PAVEMENT CUTS FOR ASSOCIATED CONDUITS.
16. PRIOR TO BEGINNING ANY UTILITY DEMOLITION WORK THE CONTRACTOR SHALL SUBMIT A DEMOLITION REQUEST WITH A COPY OF THE DEMOLITION PLAN TO CITY OF ALACHUA TO COORDINATE ANY DEMOLITION ACTIVITIES AND TEMPORARY SERVICE INTERRUPTIONS. COORDINATE WITH THE APPROPRIATE UTILITY INSPECTOR THROUGH CITY OF ALACHUA 48 HOURS IN ADVANCE OF PERFORMING ANY UTILITY DEMOLITION WORK.
17. CALL 811 BEFORE DIGGING AND LOCATE AND PROTECT GAS MAIN, MAINTAIN 36" MIN. COVER OVER EXISTING GAS MAIN AND FUTURE GAS MAIN AND SERVICES DURING ALL PHASES OF DEMOLITION AND CONSTRUCTION.

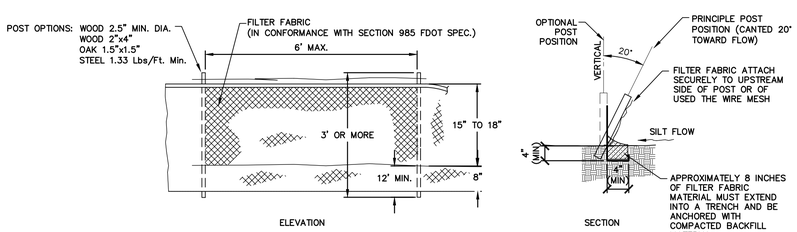


NOTE:

1. THE ENTRANCE SHALL BE MAINTAINED IN A CONDITION THAT WILL PREVENT TRACKING OR FLOWING OF SEDIMENT ONTO PUBLIC RIGHTS-OF-WAY. THIS MAY REQUIRE TOP DRESSING, REPAIR AND/OR CLEANOUT OF ANY MEASURES USED TO TRAP SEDIMENT.
2. WHEN NECESSARY, WHEELS SHALL BE CLEANED PRIOR TO ENTRANCE ONTO PUBLIC RIGHT-OF-WAY.
3. WHEN WASHING IS REQUIRED, IT SHALL BE DONE ON AN AREA STABILIZED WITH CRUSHED STONE THAT DRAINS INTO AN APPROVED SEDIMENT TRAP OR SEDIMENT BASIN.
4. USE SANDBAGS STRAW BALES OR OTHER APPROVED METHODS TO CHANNELIZE RUNOFF TO BASIN AS REQUIRED.

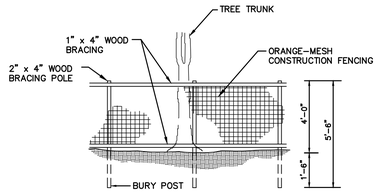
TYPICAL GRAVEL CONSTRUCTION ENTRANCE

N.T.S.



TYPE III SILT FENCE DETAIL

N.T.S.



NOTES:

1. TREE BARRICADES WILL BE BUILT BEFORE ANY SITE WORK IS UNDERTAKEN AND WILL REMAIN IN PLACE UNTIL THE LANDSCAPING IS PLANTED.
2. EACH BARRICADE MUST BE AT LEAST 4 FEET TALL, WITH CORNER POSTS OF 2" X 4" WOOD INSERTED AT LEAST ONE AND A HALF (1-1/2) FEET DEEP. THE TWO ROWS OF SIDE SLATS MUST BE 1" X 4" AND BE MARKED WITH PLASTIC RIBBONS OR MESH FENCING FOR VISIBILITY.
3. NO GRADING WITHIN FENCING. ANY ROOTS GREATER THAN 1" IN DIAMETER THAT ARE DAMAGED OR EXPOSED SHALL BE CLEANLY CUT AND COVERED OVER WITH SOIL.
4. NO CONSTRUCTION MATERIALS OR EQUIPMENT SHALL BE PERMITTED WITHIN CONSTRUCTION FENCING OR BEYOND THE CONSTRUCTION LIMITS.
5. THE AREA ENCLOSED MUST BE AT OR OUTSIDE THE DRIPLINE FOR ALL HERITAGE AND CHAMPION TREES AND ALL REGULATED PINE AND PALM TREES, OR EQUAL TO 2/3 OF THE DRIPLINE OF THE TREE CANOPY FOR ALL OTHER REGULATED SPECIES, OR AT THE TREE ROOT PLATE WHERE ALLOWED BY CONSTRUCTION LIMITS.

TREE BARRICADE FENCING DETAIL

N.T.S.

LEGEND

	SILT FENCE
	TREE BARRICADE
(TBR)	EXISTING FEATURES TO BE REMOVED (TBR)
	LIMITS OF EXISITING ASPHALT PAVEMENT, CONCRETE AND/OR BUILDING TO BE REMOVED
pi20	EXISTING TREE TO BE REMOVED

eda consultants inc.
EB 2389
720 S.W. 2nd Ave, South Tower, Suite 300
GAINESVILLE, FLORIDA 32601
TEL. (352) 373-3541
www.edafl.com mail@edafl.com

NORTH
SCALE: 1" = 20'
0 10 20 40
GRAPHIC SCALE

No.	Date	Comment

Professional Engineer of Record:

Claudia S. Vega, P.E. — Engineer — 51532 — Certificate No.

Project No: 20-103

Project phase: CITY SUBMITTAL

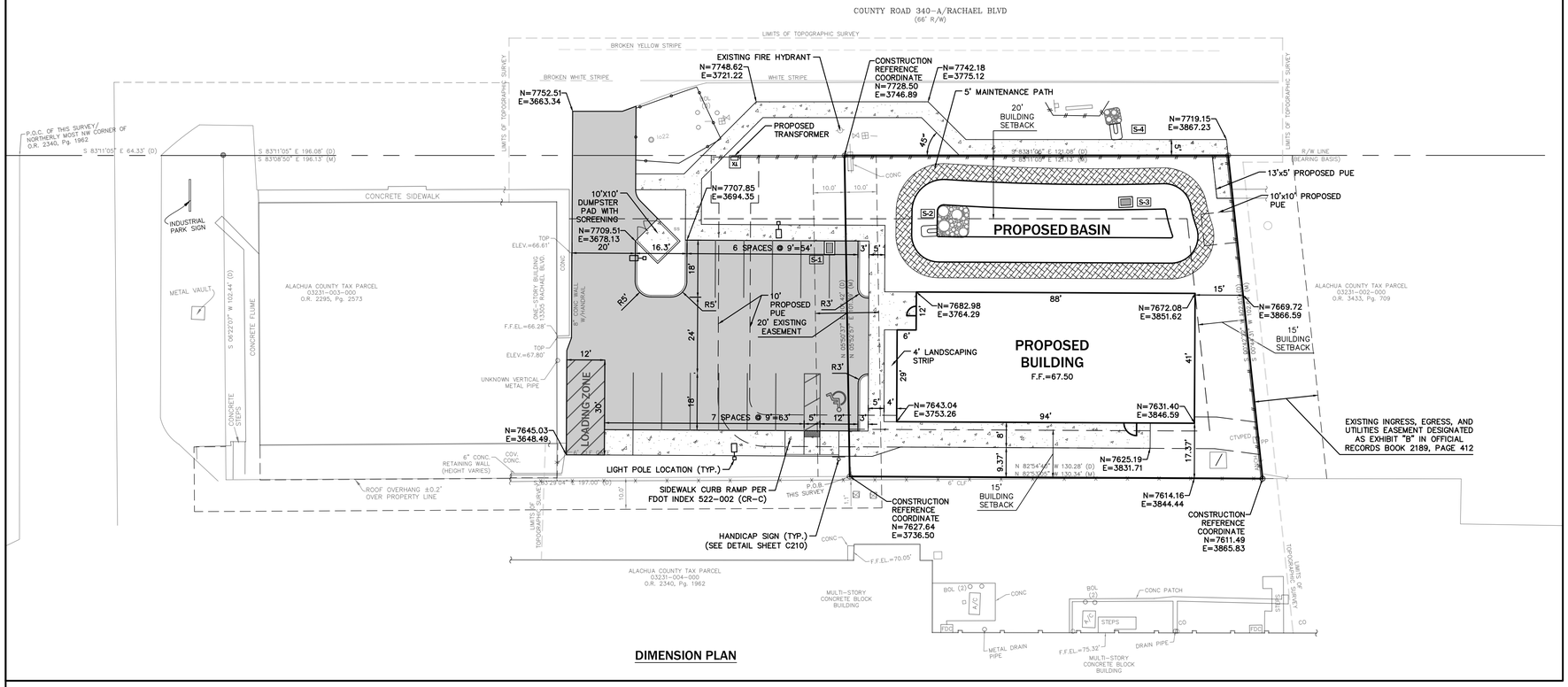
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ALCHEM LABORATORIES: BUILDING 4 SITE PLAN
CITY OF ALACHUA, FLORIDA

Sheet title:
DEMOLITION AND EROSION CONTROL PLAN

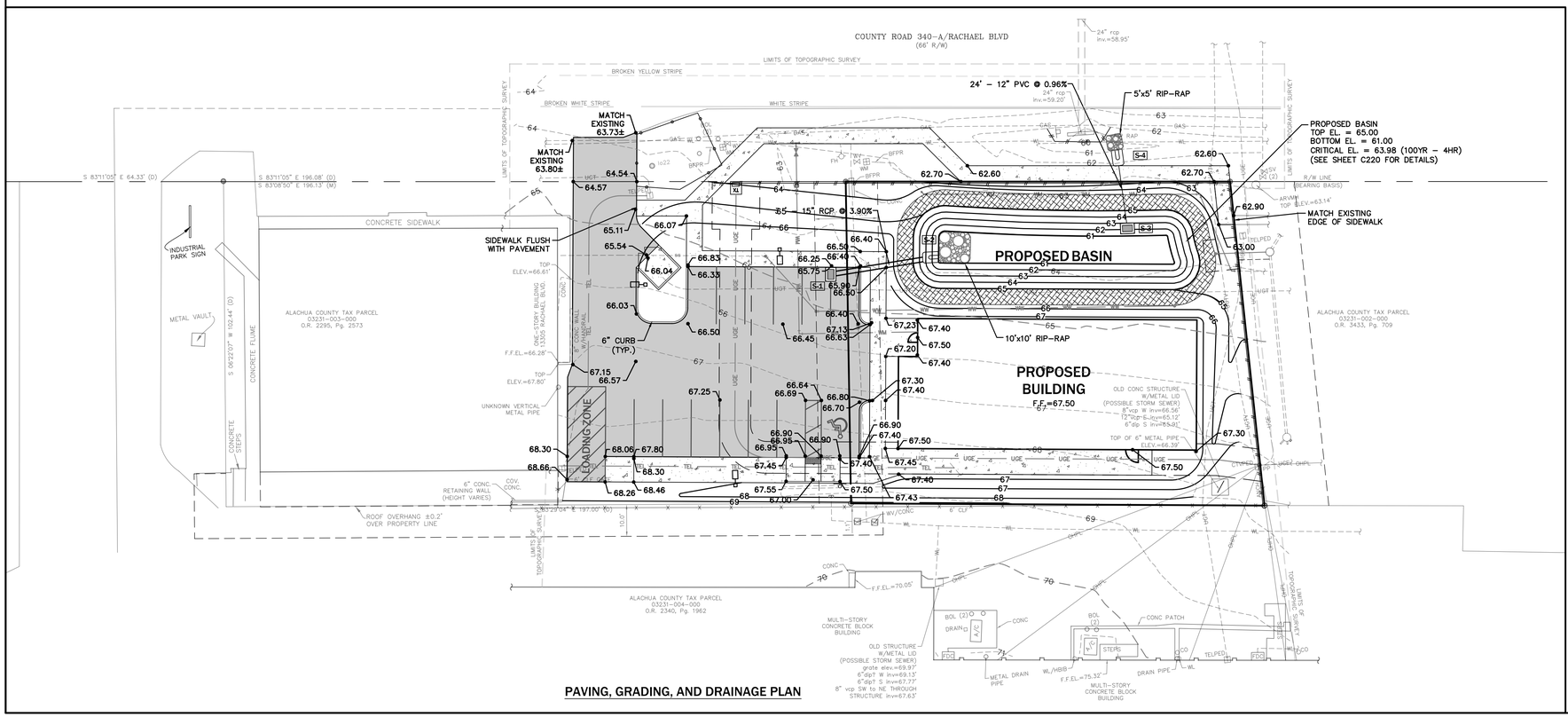
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Drawn:	JB
Checked:	CV
Date:	03/24/21

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DIMENSION PLAN



PAVING, GRADING, AND DRAINAGE PLAN

STORMWATER STRUCTURE SCHEDULE		
STRUCTURE	STRUCTURE DATA	FDOT INDEX
S-1 TYPE-C INLET	TOP EL.=65.75 E 15" INV.=62.34	232
S-3 TYPE-C INLET	TOP EL.=64.75 N 12" INV.=61.24	232

MITERED END SECTION SCHEDULE		
MES	PIPE INVERT	FDOT INDEX
S-2	15" INV.=61.00	272
S-4	12" INV.=61.00	272

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EB 2389
720 S.W. 2nd Ave, South Tower, Suite 300
GAINESVILLE, FLORIDA 32601
TEL. (352) 373-3541
www.edafl.com mail@edafl.com

NORTH
SCALE: 1" = 20'
0 10 20 40
GRAPHIC SCALE

No.	Date	Comment

Professional Engineer of Record:

Claudia S. Vega, P.E. 51532
Engineer Certificate No.

Project No: 20-103

Project phase: CITY SUBMITTAL

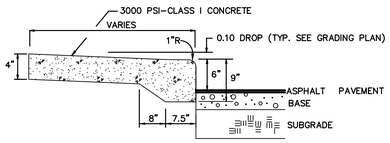
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Sheet title: DIMENSION, PAVING, GRADING, AND DRAINAGE PLAN

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Drawn: JB
Checked: CV
Date: 03/24/21

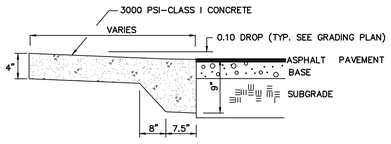
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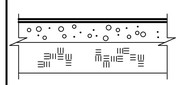
TYPICAL STEPUP SIDEWALK

N.T.S.



TYPICAL FLUSH SIDEWALK DETAIL

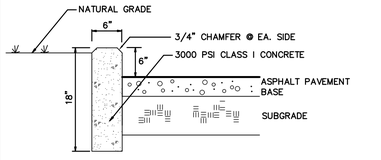
N.T.S.



1-1/2" TYPE SP-9.5, ASPHALTIC CONCRETE
6" LIMEROCK BASE, COMPACTED TO 98% OF MODIFIED PROCTOR DENSITY
12" SUBGRADE, LBR 40, COMP. TO 98% MODIFIED PROCTOR DENSITY

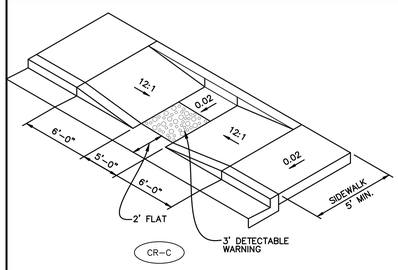
TYPICAL ASPHALT PAVEMENT DETAIL

N.T.S.



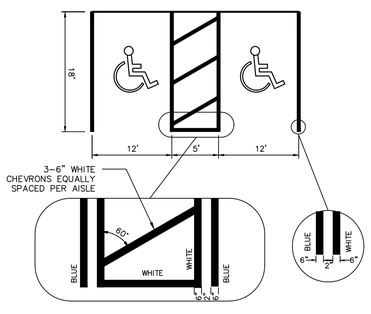
6" STANDARD CURB DETAIL

N.T.S.



SIDEWALK RAMP DETAIL-FDOT INDEX 522-002

N.T.S.

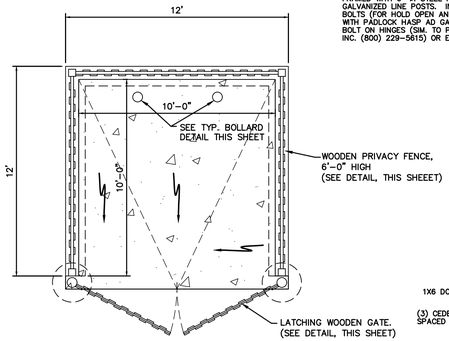


HANDICAP STRIPING DETAIL

N.T.S.

SOLID WASTE STORAGE FACILITY NOTES

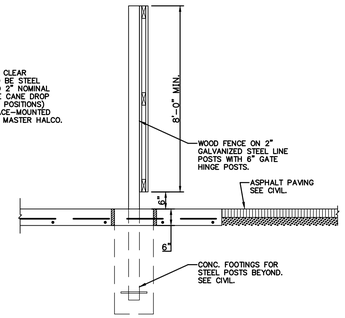
1. THE SOLID WASTE STORAGE FACILITY SHALL BE COMPLETELY SCREENED FROM VIEW BY AN OPAQUE ENCLOSURE OF WOOD.
2. ALL SCREENING SHALL BE 6 FEET HEIGHT.

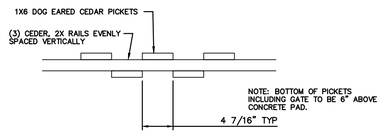


PLAN VIEW
N.T.S.

WOOD FENCE ENCLOSURE AND GATES:

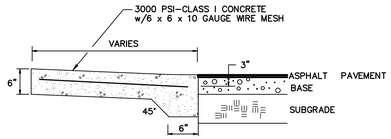
CEDAR WOOD BOARD ON BOARD FENCE WITH CLEAR PROTECTIVE FINISH. CEDAR WOOD GATES TO BE STEEL FRAMED WITH 6" ø. STEEL HINGE POSTS AND 2" NOMINAL GALVANIZED LINE POSTS. INDUSTRIAL GRADE CANE DROP BOLTS (FOR HOLD OPEN AND SECURE CLOSE POSITIONS) WITH PADLOCK HASP AD GALVANIZED, SURFACE-MOUNTED BOLT ON HINGES (SIM. TO PART #17202) BY MASTER HALCO. INC. (800) 229-5615) OR EQUAL.





FENCE DETAIL FOR DUMPSTER SCREENING

N.T.S.



DUMPSTER AND CONCRETE PAD DETAIL

N.T.S.

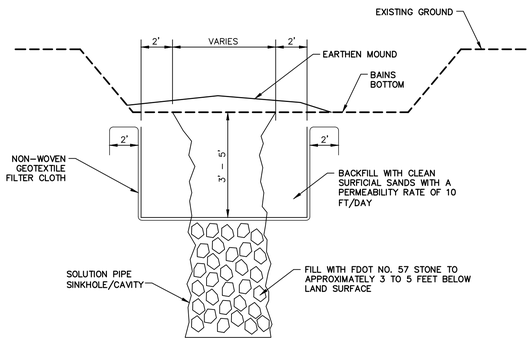


SIGN NOTES:

1. SIGN TO BE MADE OF 0.063" STEEL PAINTED WITH TRAFFIC PAINT
2. HANDICAP SIGN TO BE AS SHOWN ABOVE OR AS REQUIRED BY LOCAL CODE PAINT SIGN BLUE AND WHITE

HANDICAP PARKING SIGN DETAIL

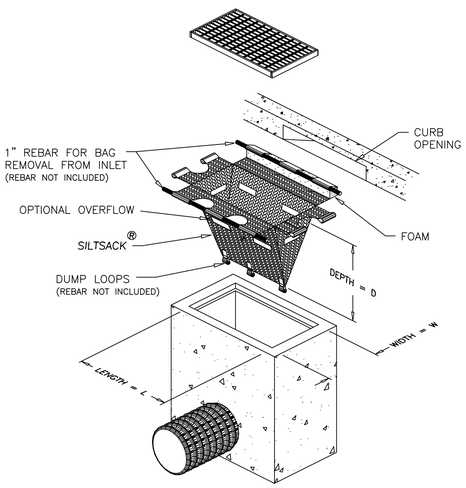
N.T.S.

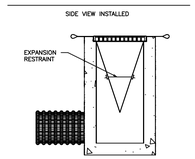


IF A SOLUTION PIPE SINKHOLE DOES FORM IN THE STORMWATER BASIN THEN THE SINKHOLE SHALL BE REPAIRED BY BACKFILLING WITH NO. 57 STONE TO APPROXIMATELY 3 TO 5 FEET BELOW LAND SURFACE. AN AREA AT LEAST 6' BEYOND THE EDGES OF THE CHIMNEY SHOULD BE EXCAVATED TO THE TOP OF THE GRAVEL. A NON-WOVEN GEOTEXTILE FILTER CLOTH SHOULD BE PLACED AT THE BOTTOM OF THE EXCAVATION AND THE EXCAVATION SHOULD BE BACKFILLED WITH CLEAN SAND HAVING A VERTICAL COEFFICIENT OF PERMEABILITY OF 10 FEET PER DAY. THE MATERIAL SHALL BE COMPACTED AND THE SINKHOLE REPAIR SHOULD BRING THE SURFACE BACK TO AN ELEVATION WHICH IS SLIGHTLY ABOVE THE ORIGINAL BOTTOM, CREATING A SMALL MOUND.

SOLUTION PIPE/CHIMNEY REPAIR DETAIL

N.T.S.





INSTALLATION DETAIL

SILTSACK®

SPECIFICATIONS

NOTE: THE SILTSACK® WILL BE MANUFACTURED FROM A WOVEN POLYPROPYLENE FABRIC THAT MEETS OR EXCEEDS THE FOLLOWING SPECIFICATIONS.

REGULAR FLOW SILTSACK®

(FOR AREAS OF LOW TO MODERATE PRECIPITATION AND RUN-OFF)

PROPERTIES	TEST METHOD	UNITS
GRAB TENSILE STRENGTH	ASTM D-4632	300 LBS
GRAB TENSILE ELONGATION	ASTM D-4632	20 %
PUNCTURE	ASTM D-4833	120 LBS
MULLEN BURST	ASTM D-3786	800 PSI
TRAPEZOID TEAR	ASTM D-4533	120 LBS
UV RESISTANCE	ASTM D-4355	80 %
APPARENT OPENING SIZE	ASTM D-4751	40 US SIEVE
FLOW RATE	ASTM D-4491	40 GAL/MIN/SQ FT
PERMITTIVITY	ASTM D-4491	0.55 SEC -1

HI-FLOW SILTSACK®

(FOR AREAS OF MODERATE TO HEAVY PRECIPITATION AND RUN-OFF)

PROPERTIES	TEST METHOD	UNITS
GRAB TENSILE STRENGTH	ASTM D-4632	265 LBS
GRAB TENSILE ELONGATION	ASTM D-4632	20 %
PUNCTURE	ASTM D-4833	135 LBS
MULLEN BURST	ASTM D-3786	420 PSI
TRAPEZOID TEAR	ASTM D-4533	45 LBS
UV RESISTANCE	ASTM D-4355	90 %
APPARENT OPENING SIZE	ASTM D-4751	20 US SIEVE
FLOW RATE	ASTM D-4491	200 GAL/MIN/SQ FT
PERMITTIVITY	ASTM D-4491	1.5 SEC -1

OIL-ABSORBANT SILTSACK®

(FOR AREAS WHERE THERE IS A CONCERN FOR OIL RUN-OFF OR SPILLS)

DEPENDING ON YOUR PARTICULAR APPLICATION, THE SILTSACK CAN BE MADE FROM EITHER ONE OF THE ABOVE FABRICS WITH AN OIL-ABSORBANT PILLOW INSERT OR, MADE COMPLETELY FROM AN OIL-ABSORBANT SILTSACK®, WITH A WOVEN PILLOW INSERT.

DETAIL OF INLET SEDIMENT CONTROL DEVICE WITH CURB DEFLECTOR

N.T.S.

ALL PROPOSED STORMWATER INLETS SHALL HAVE A TEMPORARY SILT SACK INSTALLED PER THE DETAIL ABOVE AND REMOVE UPON COMPLETION OF CONSTRUCTION.

PAVING, GRADING AND DRAINAGE SPECIFICATIONS

1. GENERAL: ALL ROADWAY AND DRAINAGE CONSTRUCTION, INCLUDING MATERIALS, CONSTRUCTION TECHNIQUES, AND TECHNICAL STANDARDS, SHALL BE IN ACCORDANCE WITH THE LATEST F.D.O.T. STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION, AND THE LATEST F.D.O.T. ROADWAY AND TRAFFIC DESIGN STANDARDS.
2. ALL AREAS OF NEW CONSTRUCTION SHALL BE PREPARED AFTER SITE DEMOLITION. TOP SOIL REMAINING ONSITE MAY BE STOCKPILED FOR FINE GRADING IN LANDSCAPED AREAS, IF SUITABLE. THE CONTRACTOR SHALL FURNISH ALL FILL REQUIRED AND DISPOSE OF ALL EXCESS OR UNSUITABLE MATERIAL OFFSITE IN ACCORDANCE WITH ALL REGULATORY REQUIREMENTS.
3. ALL NEW ASPHALT PAVEMENT CONSTRUCTION SHALL CONFORM TO THE FOLLOWING SPECIFICATIONS:
   A. EARTHWORK: FILL MATERIALS SHALL CONFORM TO AASHTO SOIL GROUPS A-1, A-3, OR A-2-4 AND SHALL BE PLACED IN 6"-12" LOOSE LIFTS AND COMPACTED TO 98% DENSITY USING MODIFIED PROCTOR METHOD (AASHTO T-180).
   B. SUBSOIL EXCAVATION: WHERE SUBSOIL EXCAVATION IS REQUIRED, UNSUITABLE MATERIALS SHALL BE REMOVED TO A DEPTH OF 18" BELOW THE LIMEROCK BASE AND BACKFILLED WITH CLEAN FILL.
   C. STABILIZED SUBGRADE: ALL STABILIZED MATERIAL SHALL BE TYPE 'B' CONFORMING TO SECTION 914-3 AND PLACED ACCORDING TO SECTION 160 IN ONE 12" MINIMUM COMPACTED LIFT. SUBGRADE SHALL BE STABILIZED TO A MINIMUM LBR VALUES AND DENSITIES AS SHOWN IN THE TYPICAL SECTIONS.
   D. BASE COURSE: ALL MATERIAL SHALL BE LIMEROCK CONFORMING TO SECTION 911 AND PLACED ACCORDING TO SECTION 200 IN ONE 6" MINIMUM COMPACTED LIFT OR DOUBLE COMPACTED LIFT. ALL BASE MATERIAL SHALL BE COMPACTED TO 98% DENSITY BY MODIFIED PROCTOR METHOD (AASHTO T-180). THE PRIME COAT SHALL CONFORM TO SECTION 300.
   E. ASPHALTIC CONCRETE: ALL ASPHALTIC CONCRETE MATERIAL SHALL BE AS PER DESIGN SECTIONS AND SHALL CONFORM TO SECTION 334. ALL ASPHALTIC CONCRETE CONSTRUCTION SHALL CONFORM TO SECTION 330. ASPHALT PAVEMENT SHALL BE SUPERPAVE SP-PG 67-22 ASPHALT BINDER.
4. ALL CONCRETE USED FOR CONSTRUCTION OF DRAINAGE STRUCTURES, SIDEWALKS, AND CURBING SHALL BE CLASS I CONFORMING TO SECTION 346.
5. REINFORCED CONCRETE PIPE SHALL CONFORM TO SECTION 430.
6. ALL PAVEMENT MARKINGS REQUIRED IN THE R/W SHALL BE THERMOPLASTIC AND INCLUDE RAISED PAVEMENT MARKERS, WHERE REQUIRED CONFORMING TO SECTION 711.
7. ALL PAVEMENT MARKING, SYMBOLS AND STRIPING WITHIN THE SITE SHALL MEET THE LATEST FLORIDA HANDICAP ACCESSIBILITY CODE. PAVEMENT MARKING SHALL BE 4" BLUE/WHITE (HANDICAP) OR WHITE (REGULAR) AND SHALL CONFORM TO THE LATEST F.D.O.T. AND M.U.T.C.D. STANDARDS.
8. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING A GEOTECHNICAL CONSULTANT TO PROVIDE A FIELD INVESTIGATION REPORT DELINEATING RECOMMENDATIONS FOR UNDERCUTTING AND/OR UNDERDRAINS. A COPY OF THIS REPORT SHALL BE FORWARDED TO THE OWNER AND ENGINEER FOR REVIEW AND APPROVAL PRIOR TO UNDERCUTTING OR INSTALLING UNDERDRAIN. UNIT PRICES SHALL BE PROVIDED FOR UNDERDRAIN AND UNDERCUTTING. THE CONTRACTOR SHALL NOT CONSTRUCT BASE COURSE UNTIL THE REPORT IS REVIEWED AND APPROVED BY THE OWNER AND THE ENGINEER.
9. SOIL TESTING RESULTS SHALL BE PROVIDED FOR THE PAVEMENT CONSTRUCTION. TESTING RESULTS SHALL BE SUBMITTED FOR THE SUBGRADE AND BASE COURSE, IN ACCORDANCE WITH THE DESIGN SECTION. A MINIMUM OF 5 TEST LOCATIONS SHALL BE PROVIDED ONSITE. THE TESTING REPORT SHALL DENOTE THE TEST LOCATIONS. THE CONTRACTOR SHALL NOT PROCEED TO THE SUBSEQUENT PAVEMENT SECTION UNTIL TESTING -RESULTS ARE APPROVED FOR PREVIOUS SECTION. ALL TESTING REQUIRED WITHIN THE COUNTY R/W SHALL BE COORDINATED WITH THE COUNTY INSPECTOR. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL TESTING COST.
10. LANDSCAPING: FINAL GRADING IN OPEN AREAS AND LANDSCAPE ISLANDS SHALL BE COORDINATED WITH THE LANDSCAPE CONTRACTOR AND THE OWNER. THE CONTRACTOR SHALL ALSO COORDINATE THE PLACEMENT OF ANY IRRIGATION AND ELECTRICAL CONDUIT SLEEVES DURING CONSTRUCTION.
11. ROOF RUNOFF WILL BE DIRECTED TO THE STORMWATER SYSTEM IF A GUTTER COLLECTION SYSTEM IS NOT SHOWN IN THE PLANS. RUNOFF COLLECTION SHALL BE COORDINATED WITH THE ARCHITECT.
12. REMOVAL OF ALL CONSTRUCTION DEBRIS, LIMEROCK, EXCESS OF BUILDERS SAND, CONCRETE AND MORTAR DEBRIS, EXISTING WEEDS AND GRASSES, AND ALL FOREIGN MATERIALS IN THE PLANTING BED AND SOD AREAS IS THE RESPONSIBILITY OF THE SITE WORK CONTRACTOR. SOIL IN AREAS TO BE LANDSCAPED SHALL BE UNCOMPACTED, SUITABLE FOR ROOT GROWTH WITH APPROPRIATE AMOUNTS OF ORGANIC MATTER, AND OF pH 5.5-6.5.
13. TREE BARRICADES MUST REMAIN IN PLACE AND IN THE DIMENSIONS SHOWN ON THE PLAN UNTIL LANDSCAPING BEGINS. SEE NOTE ON SHEET C0.20 AND THE DETAIL ON THE LANDSCAPE PLAN.
14. CITY RIGHT-OF-WAY:
   A. THE METHOD AND MANNER OF PERFORMING THE WORK AND THE QUALITIES OF MATERIAL FOR CONSTRUCTION WITHIN THE ROW SHALL CONFORM TO THE REQUIREMENT SPECIFIED BY THE CITY OF ALACHUA PUBLIC WORKS DEPARTMENT.
   B. NO WORK SHALL BE DONE NOR MATERIALS USED IN THE ROW, WITHOUT INSPECTION BY THE PUBLIC WORKS DEPARTMENT (334-5070), AND THE CONTRACTOR/DEVELOPER SHALL FURNISH THE DEPARTMENT WITH EVERY REASONABLE FACILITY FOR ASCERTAINING WHETHER THE WORK PERFORMED AND MATERIALS USED ARE IN ACCORDANCE WITH THE REQUIREMENTS AND INTENT OF THE PLANS AND SPECIFICATIONS.
   C. THE PUBLIC WORKS DEPARTMENT RESERVES THE RIGHT TO MODIFY THE PROPOSED WORK WITHIN THE ROW TO ENSURE COMPATIBILITY WITH EXISTING IMPROVEMENTS. SUCH MODIFICATION COSTS SHALL BE BORNE BY THE DEVELOPER.
15. COUNTY RIGHT-OF-WAY
   A. THE METHOD AND MANNER OF PERFORMING THE WORK AND THE QUALITIES OF MATERIAL FOR CONSTRUCTION WITHIN THE COUNTY ROW SHALL CONFORM TO THE REQUIREMENTS SPECIFIED BY THE ALACHUA COUNTY PUBLIC WORKS DEPARTMENT.
   B. NO WORK SHALL BE DONE NOR MATERIALS USED IN THE ROW, WITHOUT INSPECTION BY THE ALACHUA COUNTY PUBLIC WORKS DEPARTMENT (462-2147), AND THE CONTRACTOR/DEVELOPER SHALL FURNISH THE DEPARTMENT WITH EVERY REASONABLE FACILITY FOR ASCERTAINING WHETHER THE WORK PERFORMED AND MATERIALS USED ARE IN ACCORDANCE WITH THE REQUIREMENTS AND INTENT OF THE PLANS AND SPECIFICATIONS.

eda consultants inc.
EB 2389
720 S.W. 2nd Ave, South Tower, Suite 300
GAINESVILLE, FLORIDA 32601
TEL. (352) 373-3541
www.edafl.com mail@edafl.com

SCALES:
AS SHOWN

No.	Date	Comment

Professional Engineer of Record:

Claudia S. Vega, P.E. 51532
Engineer Certificate No.

Project No: 20-103

Project phase: CITY SUBMITTAL

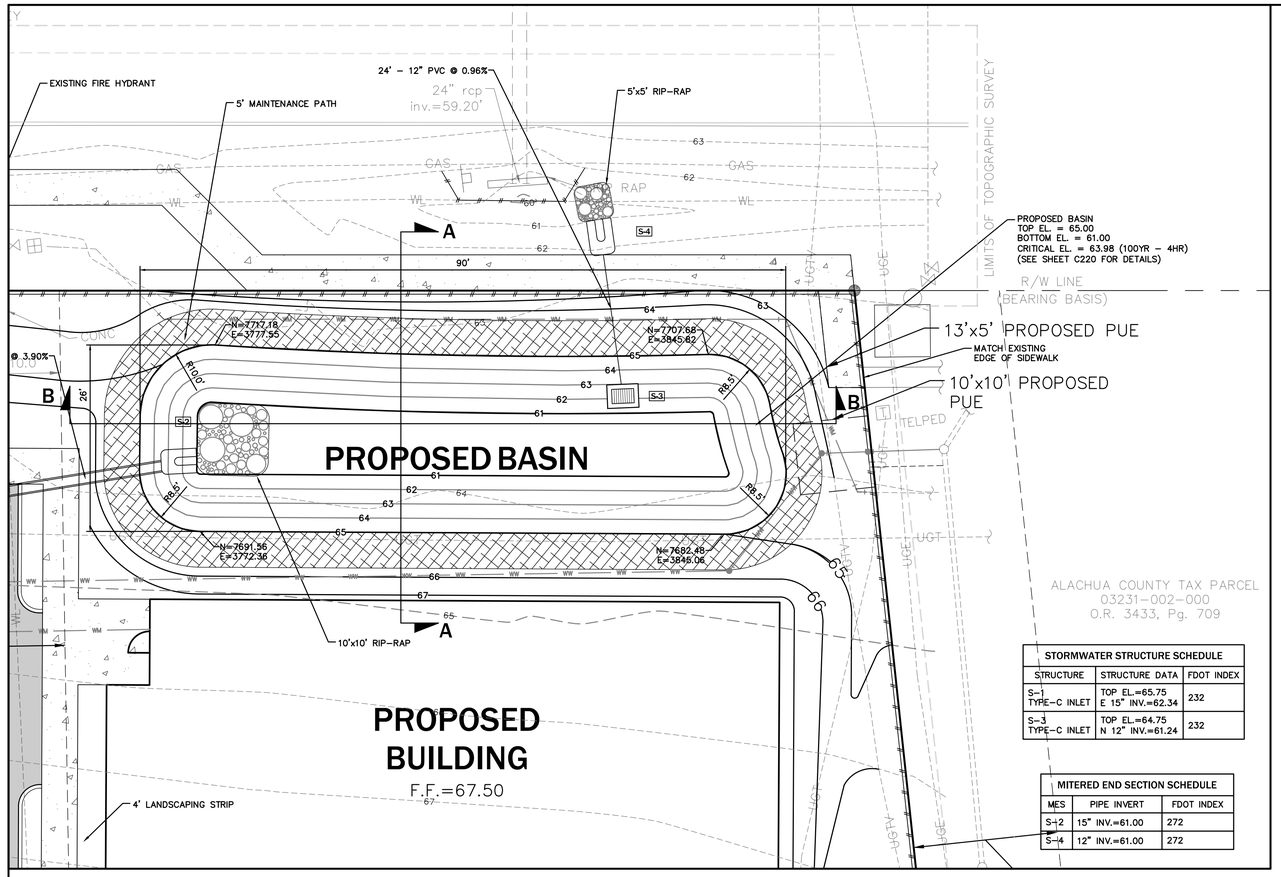
Project title: ALCHEM LABORATORIES: BUILDING 4 SITE PLAN CITY OF ALACHUA, FLORIDA

Sheet title: PAVING, GRADING, AND DRAINAGE DETAILS

Designed: CV
Drawn: JB
Checked: CV
Date: 03/24/21

Sheet No.: C210

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STORMWATER STRUCTURE SCHEDULE		
STRUCTURE	STRUCTURE DATA	FDOT INDEX
S-1 TYPE-C INLET	TOP EL.=65.75 E 15" INV.=62.34	232
S-3 TYPE-C INLET	TOP EL.=64.75 N 12" INV.=61.24	232

MITERED END SECTION SCHEDULE		
MES	PIPE INVERT	FDOT INDEX
S-2	15" INV.=61.00	272
S-4	12" INV.=61.00	272

MAINTENANCE NOTES

1. MAINTENANCE RESPONSIBILITY:

   ALCHEM LABORATORIES WILL BE THE ENTITY RESPONSIBLE FOR THE MAINTENANCE OF THE EXISTING STORMWATER MANAGEMENT SYSTEM.

2. MAINTENANCE PLAN:

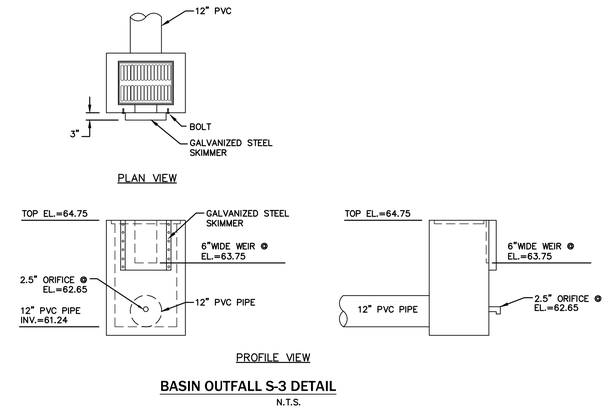
   A. BASINS SHALL BE MOWED REGULARLY TO AVOID EXCESSIVE VEGETATIVE GROWTH. MOWING SCHEDULE SHOULD BE MONTHLY DURING WINTER MONTHS AND MORE FREQUENTLY (BIWEEKLY) DURING SUMMER MONTHS.

   B. BASIN SHALL BE CLEANED OUT ANNUALLY OF ANY ACCUMULATED SEDIMENTATION BUILDUP. IF THE BASINS ARE SHOWING EXCESSIVE SEDIMENTATION ACCUMULATION AT THE BASIN BOTTOM, THE BASIN BOTTOM SHALL BE SCRAPED CLEAN MORE OFTEN AS THE CONDITION DICTATES.

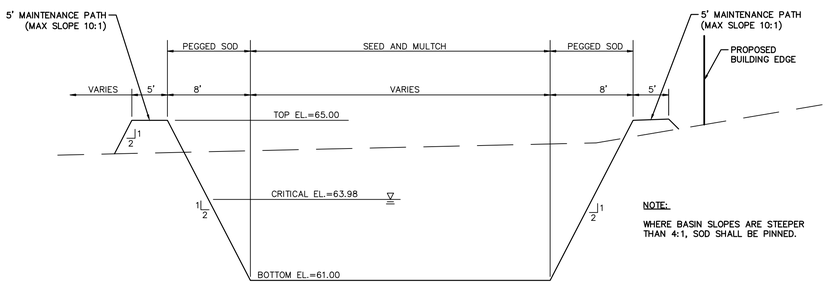
   C. BASIN SIDE SLOPES SHALL BE MAINTAINED WITH A GOOD STAND OF GRASS. SEASONAL GRASSES SHALL BE PLANTED TO AVOID EROSION (WINTER RYE, SUMMER MILLET).

   D. BASIN THAT DO NOT DRAWDOWN PROPERLY AND MAINTAIN STANDING WATER FOR AN EXTENDED PERIOD OF TIME MAY REQUIRE REMEDIAL ACTION. THE ENGINEER SHALL BE NOTIFIED TO HELP COORDINATE REMEDIAL ACTION IN THE EVENT THIS OCCURS.

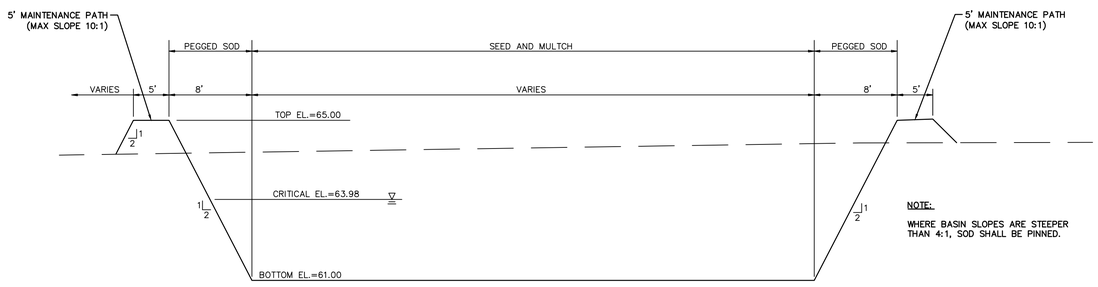
   E. THE REQUIRED LANDSCAPING SHALL BE MAINTAINED IN A SOUND CONDITION AT ALL TIMES. ANY DEAD LANDSCAPING MATERIALS SHALL BE REPLACED IMMEDIATELY TO ASSURE PUBLIC SAFETY.



BASIN OUTFALL S-3 DETAIL
N.T.S.



BASIN TYPICAL SECTION A-A
N.T.S.



BASIN TYPICAL SECTION B-B
N.T.S.

eda consultants inc.
EB 2389
720 S.W. 2nd Ave. South Tower, Suite 300
GAINESVILLE, FLORIDA 32601
TEL. (352) 373-3541
www.edafl.com mail@edafl.com

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GRAPHIC SCALE

No.	Date	Comment

Professional Engineer of Record:

Claudia S. Vega, P.E. 51532
Engineer Certificate No.

Project No: 20-103

Project phase: CITY SUBMITTAL

Project title: ALCHEM LABORATORIES: BUILDING 4 SITE PLAN CITY OF ALACHUA, FLORIDA

Sheet title: BASIN DETAIL PLAN

Designed: CV
Drawn: JB
Checked: CV
Date: 03/24/21

Sheet No.: C220

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THE STORMWATER POLLUTION PREVENTION PLAN (SWPPP) MAY BE MODIFIED AND UPDATED DURING CONSTRUCTION AS A RESULT OF WEATHER, UNPREDICTABLE EVENTS AND SITE INSPECTIONS.

THIS DOCUMENT WAS PREPARED IN ORDER TO BE IN COMPLIANCE WITH CHAPTER 62-621.300 (4) OF THE FLORIDA ADMINISTRATIVE CODE, WHICH PERTAINS TO THE GENERIC PERMIT FOR STORMWATER DISCHARGE FROM LARGE AND SMALL CONSTRUCTION ACTIVITIES. THE ADMINISTRATIVE CODE GRANTS THE FLORIDA DEPARTMENT OF ENVIRONMENTAL PROTECTION (FDEP) THE AUTHORITY TO REGULATE POINT SOURCE DISCHARGES OF STORM-WATER FROM CONSTRUCTION SITES. THIS DOCUMENT ESTABLISHES A STORMWATER POLLUTION PREVENTION PLAN FOR THE SITE AND IS ORGANIZED TO CORRESPOND TO PART V OF DEP DOCUMENT No. 62-621.300 (4) (A) FDEP FORM 62-261.300 (4) (B) IS TO BE SUBMITTED IN CONJUNCTION WITH THIS DOCUMENT.

I. PROJECT INFORMATION:

PROJECT: ALCHEM LABORATORIES: BUILDING 4 SITE PLAN
COUNTY: ALACHUA
SECTION/TOWNSHIP/RANGE: S 23, T 8 SOUTH, R 18 EAST
COUNTY PARCEL NO.: 3231-3 AND A PORTION OF 3231-4-1
STREET ADDRESS: 13305 RACHAEL BLVD. ALACHUA, FLORIDA 32615
PROJECT AREA: 0.76 AC
APPROXIMATE AREA TO BE DISTURBED BY CONSTRUCTION: 0.54 AC.

II. SITE DESCRIPTION:

1. THE PROPOSED DEVELOPMENT WILL CONSIST OF THE CONSTRUCTION OF A 3,788 SF BUILDING WITH ASSOCIATED PAVING, DRAINAGE AND UTILITY IMPROVEMENTS.

   THE PROPOSED STORMWATER SYSTEM IS A DRY STORMWATER BASIN LOCATED ON THE NORTHEASTERN SIDE OF THE SITE THE SYSTEM IS AN OPEN SYSTEM WHICH PROVIDES WATER QUALITY, AND FLOOD CONTROL FOR THE PROPOSED IMPROVEMENTS.

2. THE SOIL CONDITIONS WERE INVESTIGATED AND SUMMARIZED IN THE SOILS REPORT PREPARED BY UNIVERSAL ENGINEERING SCIENCES. THE PROPOSED DRAINAGE PLAN WILL CONSIST OF (1) DRAINAGE AREA. AREA (1): 0.31 ACRES WILL BE SELF CONTAINED IN BASIN No. 1.

3. EXISTING AND FUTURE DRAINAGE PATTERNS ARE SHOWN ON THE DRAINAGE PLAN FOR PRE-DEVELOPMENT CONDITIONS AND POST-DEVELOPMENT CONDITIONS. OUTFALLS, AND STORMWATER BASINS ARE SHOWN IN THE DRAINAGE PLAN AND THE DETAIL PLAN.

4. SEQUENCE OF CONSTRUCTION:

   A. PRIOR TO CONSTRUCTION, SILT FENCING AND TREE PROTECTION BARRICADES SHALL BE INSTALLED AND ALL EXISTING DRAINAGE STRUCTURES SHALL BE PROTECTED IN ACCORDANCE WITH THE FDOT FLORIDA EROSION AND SEDIMENTATION CONTROL MANUAL.

   B. THE CONSTRUCTION ENTRANCE(S) WILL BE STABILIZED TO MINIMIZE THE CREATION OF DUST AND OFF SITE TRACKING OF SEDIMENTS.

   C. THE SITE SHALL BE CLEARED AND GRUBBED OF UNDESIRABLE VEGETATION.

   D. THE UNDERGROUND UTILITIES AND STORMWATER PIPING WILL BE INSTALLED AND CONNECTED TO EXISTING STRUCTURES.

   E. THE SITE WILL BE ROUGHLY GRADED, IF SUITABLE, THE EXCAVATED MATERIAL MAY BE USED AS FILL FOR ON-SITE GRADING. THE ROADWAYS SHALL BE GRADED. (THE BASIN AREA SHALL BE STABILIZED AS SPECIFIED IN THE PLANS.)

   F. ROADWAYS AND PARKING LOTS WILL BE COMPACTED AND A LIMEROCK BASE WILL BE ESTABLISHED FOLLOWED BY AN OVERLAY OF ASPHALTIC CONCRETE. BUILDINGS SHALL BE CONSTRUCTED.

   G. UPON SIGNIFICANT COMPLETION OF CONSTRUCTION, THE STORMWATER SYSTEM SHALL BE FLUSHED OUT TO REMOVE ACCUMULATED DEBRIS AND SEDIMENT.

   H. (EXISTING) STORMWATER BASIN(S) WILL BE SCRAPED CLEAN OF ACCUMULATED SEDIMENT.

   I. ALL DISTURBED AREAS WITHIN THE CONSTRUCTION AREA SHALL BE COMPLETELY GRASSED AND/OR LANDSCAPED. EVIDENCE OF GROWTH MUST BE PRESENT PRIOR TO REMOVAL OF SILT FENCING AND OTHER EROSION CONTROL APPLICATIONS.

III. CONTROLS:

THE CONTROLS SHALL BE IMPLEMENTED AND MAINTAINED DURING THE ENTIRE CONSTRUCTION OF THE PROJECT. IF SITE CONDITIONS ARE SUCH THAT ADDITIONAL CONTROL MEASURES ARE REQUIRED THAN WHAT IS SPECIFIED IN THE EROSION AND SEDIMENTATION CONTROL PLAN, THEN THE CONTRACTOR SHALL IMPLEMENT ADDITIONAL BEST MANAGEMENT PRACTICES NECESSARY.

1. THE CONSTRUCTION ACCESS SHALL BE STABILIZED WITH GRAVEL AND TEMPORARY VEGETATION TO PREVENT SILT LEAVING THE SITE.
2. TREE BARRICADES SHALL BE IMPLEMENTED BEFORE CLEARING AND GRUBBING OF ANY OF THE WORK AREAS.
3. BEFORE CLEARING, SILT FENCES SHALL BE INSTALLED AROUND THE PERIMETER OF THE CONSTRUCTION AND AROUND THE WETLAND(S) AND/OR BASIN(S) AS SHOWN IN THE PLANS. ALL EXISTING STORM DRAINAGE SWALES AND INLETS SHALL BE PROTECTED PER THE FDOT FLORIDA EROSION AND SEDIMENTATION CONTROL MANUAL.
4. AFTER CLEARING BUT BEFORE EXCAVATION AND GRADING, TEMPORARY BERMS AND SWALES SHALL BE CONSTRUCTED AS REQUIRED TO DIVERT THE FLOW INTO THE CORRESPONDING STORMWATER BASIN.
5. THE BASIN (ALL BASIN) AREA(S) SHALL BE PROTECTED AS INDICATED ON THE PLANS.
6. THE STORMWATER BASIN(S) SHALL BE ROUGH GRADED TO WITHIN 6" OF THE DESIGNED BASIN BOTTOM. THE BASIN SIDE SLOPES SHALL BE STABILIZED AS SHOWN IN THE PLANS BY SEEDING, MULCHING AND/OR SODDING TO PREVENT EXCESSIVE EROSION.
7. DURING CONSTRUCTION OF PAVING AND BUILDINGS, EROSION AND SEDIMENTATION CONTROL MEASURES SHALL BE INSTALLED AS REQUIRED.
8. ALL DISTURBED AREAS WITHIN THE CONSTRUCTION SITE SHALL BE COMPLETELY LANDSCAPED AND/OR GRASSED. FINAL STABILIZATION (INCLUDING SEEDING, MULCHING, SODDING OR RIPRAP) SHALL BE INSTALLED AS REQUIRED. GRASS SEEDING RATES AND MIXTURES SHALL BE PER FDOT INDEX 104. EVIDENCE OF GROWTH MUST BE PRESENT PRIOR TO REMOVAL OF SILT FENCING AND OTHER EROSION CONTROL APPLICATIONS AND PRIOR TO FINAL RELEASE.

IV. EROSION AND SEDIMENTATION CONTROLS:

STABILIZATION PRACTICES

1. ALL ENTRANCES TO THE SITE SHALL BE STABILIZED BEFORE CONSTRUCTION AND FURTHER DISTURBANCE BEGINS. GRAVEL PAD SHALL PROVIDE STABILIZATION AND MINIMIZE THE AMOUNT OF SEDIMENT LEAVING THE SITE. MAINTENANCE OF THE ENTRANCE SHALL INCLUDE SWEEPING OF THE AREA ADJACENT TO THE ENTRANCE. STONE AND GRAVEL MIGHT NEED TO BE PERIODICALLY ADDED TO MAINTAIN THE EFFECTIVENESS OF THE ENTRANCE(S).
2. TREE BARRICADES SHALL BE INSTALLED AROUND THE TREES AS SHOWN IN THE DETAIL PLAN TO PROTECT THE EXISTING VEGETATION.
3. MULCH SHALL BE PLACED IN THE AREAS REQUIRED TO PREVENT EROSION FROM STORMWATER RUNOFF AND THE AREAS SHOWN ON THE PLANS. MULCH SHALL BE ANCHORED TO RESIST WIND DISPLACEMENT AND SHALL BE INSPECTED AFTER EVERY RAINSTORM TO IDENTIFY AREAS WHERE MULCH HAS BEEN WASHED OUT OR LOOSENED. THESE AREAS SHALL HAVE MULCH COVER REPLACEMENT.
4. SEEDING SHALL BE STARTED AFTER GRADING HAS BEEN FINISHED ON THE AREAS SHOWN IN THE PLANS. SEEDED AREAS SHOULD BE INSPECTED FOR FAILURE TO ESTABLISH, AND NECESSARY REPAIRS AND RESEEDING SHOULD BE MADE AS SOON AS POSSIBLE. ADDITIONAL SEEDING AND MULCH MAY BE REQUIRED AS NECESSARY TO PREVENT EROSION DURING OR AFTER CONSTRUCTION HAS FINISHED.
5. SOD SHALL BE INSTALLED IN THE AREAS SHOWN IN THE PLANS. SOD SHALL BE PEGGED IF INSTALLED ON SLOPES GREATER THAN 3:1. SODDED AREAS SHALL BE MAINTAINED AND INSPECTED TO ENSURE SUCCESSFUL ESTABLISHMENT.

SEDIMENTATION PRACTICES

1. SILT FENCES SHALL BE INSTALLED IN THE AREAS SHOWN IN THE PLANS AND AS REQUIRED TO PREVENT SEDIMENT FROM LEAVING THE CONSTRUCTION AREA. SILT FENCES SHALL BE INSPECTED AFTER EACH RAINFALL EVENT TO ENSURE THAT THERE ARE NO GAPS OR TEARS. IF GAPS OR TEARS ARE FOUND THE FABRIC SHOULD BE REPAIRED OR REPLACED. SEDIMENT REMOVAL SHALL BE PART OF THE REGULAR MAINTENANCE. SILT FENCES SHALL REMAIN IN PLACE UNTIL CONSTRUCTION HAS FINISHED AND DISTURBED AREAS ARE PERMANENTLY STABILIZED.
2. DIVERSION SWALES, IF REQUIRED, SHALL BE CONSTRUCTED BEFORE MAJOR LAND DISTURBANCE OF THE RECEIVING BASIN. DIVERSION SWALES SHALL BE STABILIZED AFTER CONSTRUCTION TO MAINTAIN ITS EFFICIENCY.
3. INLETS SHOULD BE TEMPORARILY PROTECTED TO PREVENT SEDIMENT ENTERING THE INLET. BARRIERS WILL CATCH SOIL, DEBRIS AND SEDIMENT AT THE ENTRANCE OF THE INLET.
4. OUTFALL STRUCTURES SHALL HAVE SILT FENCES TO PREVENT SILT FROM ENTERING THE STORMWATER BASINS AND SHALL BE STABILIZED AS REQUIRED TO PREVENT EROSION FROM WASHOUTS.

V. STORMWATER MANAGEMENT:

1. THE PROPOSED PROJECT OBTAINED AN ENVIRONMENTAL RESOURCE PERMIT FROM SUWANNEE RIVER WATER MANAGEMENT DISTRICT (SRWMD) FOR THE CONSTRUCTION AND OPERATION OF A STORMWATER TREATMENT SYSTEM AND CONTROLS. THE PROPOSED SYSTEM (AS SHOWN ON THE PLANS) INCLUDED THE USE OF THE BEST MANAGEMENT PRACTICES (BMP) CONSISTENT WITH THE APPLICABLE REQUIREMENTS OF RULE 40C-42 OF THE DISTRICT. THE OWNER AND/OR THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE OPERATION AND MAINTENANCE OF THE STORMWATER TREATMENT SYSTEM AND CONTROLS UNTIL CONSTRUCTION ACTIVITIES ARE COMPLETED AND FINAL STABILIZATION HAS BEEN ACCOMPLISHED. HOWEVER, THE OWNER AND/OR AN ENTITY SIMILAR TO A HOMEOWNERS ASSOCIATION SHALL BE RESPONSIBLE FOR THE OPERATION AND MAINTENANCE OF THE STORMWATER SYSTEM IN PERPETUITY, IN ACCORDANCE WITH THE REQUIREMENTS OF THE ENVIRONMENTAL RESOURCE PERMIT.
2. TO TREAT AND CONTROL THE STORMWATER PRODUCED BY THE PROPOSED DEVELOPMENT, THE PROJECT REQUIRES THE INSTALLATION AND CONSTRUCTION OF THE FOLLOWING BMP'S: (1) DRY RETENTION BASIN WITH ALL GRADING ASSOCIATED WITH THE CONSTRUCTION (DISCHARGE TO AN EXISTING MASTER STORMWATER SYSTEM). THE BASIN HAS BEEN DESIGNED TO CONTAIN AND ATTENUATE THE STORMS AND DISCHARGE AT PRE-DEVELOPMENT CONDITIONS, WHILE PROVIDING TREATMENT TO THE RUNOFF AS REQUIRED BY THE DISTRICT AND STATE RULES USING THE GUIDELINES CONTAINED IN THE SRWMD HANDBOOK.
3. AREA (1) WILL HAVE A DRY DETENTION BASIN THAT IS DISCHARGES TO AN EXISTING SWALE IN THE COUNTY RIGHT OF WAY.

VI. CONTROLS FOR OTHER POTENTIAL POLLUTANTS:

1. WASTE DISPOSAL: NO SOLID MATERIALS, INCLUDING CONSTRUCTION MATERIALS, SHALL BE DISCHARGED TO SURFACE WATERS AND ARE NOT AUTHORIZED UNDER THE ISSUED ENVIRONMENTAL RESOURCE PERMIT.
2. THE USE OF GRAVEL AND CONTINUING SWEEPING ACTIVITIES AT THE ENTRANCE OF THE SITE WILL CONTROL THE TRACKING OF SEDIMENT AND DUST LEAVING THE SITE.
3. THE PROPOSED DEVELOPMENT WILL PROVIDE WATER AND SEWER SYSTEM BY CONNECTING INTO THE CENTRAL MUNICIPAL SYSTEM OF CITY OF ALACHUA.
4. ANY APPLICATION OF FERTILIZERS AND PESTICIDES NECESSARY TO ESTABLISH AND MAINTENANCE OF VEGETATION DURING CONSTRUCTION AND THROUGH PERPETUITY MAINTENANCE SHALL FOLLOW THE MANUFACTURERS RECOMMENDATIONS AND THE APPLICABLE RULES OF THE STATE OF FLORIDA.
5. ANY TOXIC MATERIALS REQUIRED DURING CONSTRUCTION SHALL BE PROPERLY STORED, DISPOSED OF AND CONTRACTOR AND/OR OWNER SHALL PROVIDE THE APPROPRIATE PERMITS FROM THE LOCAL OR STATE AGENCIES.

VII. APPROVED STATE OR LOCAL PLANS:

1. ALL THE SEDIMENT AND EROSION CONTROLS THAT ARE LISTED IN THE SITE PLAN AS APPROVED BY THE SRWMD ARE INCLUDED IN THIS STORMWATER POLLUTION PREVENTION PLAN (SEE ITEM III AND IV).
2. THIS STORMWATER POLLUTION PREVENTION PLAN SHALL BE AMENDED IF REQUIRED BY ANY LOCAL OR STATE AGENCY OR AS REQUIRED BY UNFORESEEABLE CONDITIONS AND THE OWNER SHALL SUBMIT A RE-CERTIFICATION TO THE NPDES STATE OFFICE THAT THE PLAN HAS BEEN AMENDED TO ADDRESS THOSE CHANGES.

VIII. MAINTENANCE:

THE CONTRACTOR IS RESPONSIBLE FOR THE MAINTENANCE, INSPECTION SCHEDULE, AND REPAIRS OUTLINED IN THIS PLAN. MAINTENANCE SHALL CONTINUE THROUGHOUT THE PROJECT UNTIL WORK IS COMPLETE. THE CONTRACTOR IS RESPONSIBLE FOR REMOVING ALL TEMPORARY EROSION AND SEDIMENT CONTROL DEVICES AFTER CONSTRUCTION IS COMPLETE.

IN ADDITION TO THE ITEMS MENTIONED IN THE PREVIOUS SECTIONS, THE CONTRACTOR SHALL INITIATE ANY REPAIRS WITHIN 24 HOURS OF BEING REPORTED. IN THE EVENT THAT THE BASINS DO NOT PERFORM PROPERLY OR IF A SINKHOLE DEVELOPS, THE PROJECT ENGINEER SHALL BE NOTIFIED TO ASSIST IN COORDINATING REMEDIAL ACTION.

1. MAINTENANCE WOULD BE DIVIDED IN ROUTINE MAINTENANCE AND REPAIR MAINTENANCE. ALL STORMWATER BMP'S SHOULD BE INSPECTED FOR CONTINUED EFFECTIVENESS AND STRUCTURAL INTEGRITY ON A REGULAR BASIS. THE SYSTEMS SHOULD BE CHECKED AFTER EACH STORM EVENT IN ADDITION TO REGULARLY SCHEDULED INSPECTIONS.
2. ROUTINE MAINTENANCE REQUIREMENTS SHOULD BE INCLUDED IN THE INSPECTOR CHECKLIST TO AID THE INSPECTOR IN DETERMINING WHETHER A BMP'S MAINTENANCE IS ADEQUATE OR NEEDS A REVISION. INSPECTORS SHALL KEEP RECORD OF MAINTENANCE, ROUTINE OR REPAIR, TO PROVIDE EVIDENCE OF AN EFFICIENT INSPECTION AND MAINTENANCE.
3. SIDE ENTRANCES: MAINTENANCE SHALL INCLUDE REPLACEMENT OF GRAVEL AND CLEANING THE SOIL THAT IS TRACKED OFFSITE FOR PROPER DISPOSAL.
4. TREE BARRICADES: MAINTENANCE SHALL INCLUDE INSPECTION OF MESH AND POSTS AND REPAIR OR REPLACEMENT OF DAMAGED VEGETATION.
5. SILT FENCES: MAINTENANCE SHALL INCLUDE SEDIMENT REMOVAL AND INSPECTION TO ENSURE PROPER ANCHORING AND THAT NO TEARING OR GAPS HAVE OCCURRED. ACCUMULATED SEDIMENT SHALL BE REMOVED WHEN IT HAS REACHED ONE-THIRD THE HEIGHT OF SILT FENCE.
6. DIVERSION SWALES: MAINTENANCE SHALL INCLUDE INSPECTION AFTER EVERY RAINFALL EVENT AND ONCE EVERY TWO WEEKS BEFORE FINAL STABILIZATION. THEY SHOULD BE CLEARED OF SEDIMENT AND MAINTAIN VEGETATIVE COVER.
7. TEMPORARY BERMS: MAINTENANCE SHALL INCLUDE REMOVAL OF DEBRIS, TRASH SEDIMENT AND LEAVES. SIDES OF THE BERM SHALL BE INSPECTED FOR EROSION AFTER EACH STORM EVENT.
8. MULCHING: ROUTINE MAINTENANCE SHALL INCLUDE REPLACEMENT PERIODICALLY.
9. SEEDING: ROUTINE MAINTENANCE SHALL INCLUDE RESEEDING OF AREAS THAT FAILED TO ESTABLISH.
10. SODDING: ROUTINE MAINTENANCE SHALL INCLUDE WATERING AND MOWING. REPLACEMENT OF GRASS MAY BE NECESSARY IF COVER IS NOT FULLY ESTABLISHED.
11. INLETS: ROUTINE MAINTENANCE SHALL INCLUDE INSPECTION AFTER EVERY STORM EVENT AND MIGHT INCLUDE REMOVAL OF ACCUMULATED SEDIMENT.
12. OUTFALL STRUCTURES: ROUTINE MAINTENANCE SHALL INCLUDE INSPECTION AFTER EVERY STORM EVENT TO ASSURE NO EROSION OR SCOUR HAS OCCURRED.
13. DRY RETENTION BASINS: ROUTINE MAINTENANCE SHALL INCLUDE MONITORING FOR SEDIMENT ACCUMULATION, CLEAN AND REMOVE DEBRIS FROM INLETS AND OUTLETS, MOW SIDE SLOPES AND INSPECT FOR DAMAGE OF BERMS AND REPAIR UNDERCUT OR ERODED AREAS AS NECESSARY.
14. WET DETENTION BASINS: ROUTINE MAINTENANCE SHALL INCLUDE MONITORING FOR SEDIMENT ACCUMULATION, CLEAN AND REMOVE DEBRIS FROM INLETS AND OUTLETS, MOW SIDE SLOPES AND INSPECT FOR DAMAGE OF BERMS AND REPAIR UNDERCUT OR ERODED AREAS AS NECESSARY.

IX. INSPECTIONS:

1. THE OWNER AND /OR CONTRACTOR SHALL PROVIDE QUALIFIED PERSONNEL TO INSPECT ALL POINTS OF POTENTIAL DISCHARGE FROM THE PROJECT SITE FOR DISTURBED AREAS, THE EROSION AND SEDIMENTATION CONTROLS AND BMP'S AS LISTED IN THIS PLAN. THE INSPECTION SHALL BE PERFORMED DURING CONSTRUCTION AND BEFORE FINAL STABILIZATION, ONCE EVERY SEVEN-CALENDAR DAYS AND WITHIN 24 HOURS OF THE END OF A STORM THAT IS GREATER THAN 0.50 INCHES. AFTER FINAL STABILIZATION AND BEFORE FINISH OF CONSTRUCTION THE INSPECTION SHALL BE CONDUCTED ONCE EVERY MONTH.
2. THE CONTRACTOR SHALL INSTALL A RAIN GAUGE AT THE SITE TO MONITOR AND DOCUMENT RAINFALL EVENTS IN EXCESS OF 0.50 INCHES.
3. ALL DISTURBED AREAS AND AREAS USED FOR MATERIALS STORAGE SHALL BE INSPECTED FOR POLLUTANTS ENTERING THE STORMWATER SYSTEM. THE STORMWATER MANAGEMENT SYSTEM AND EROSION AND SEDIMENT CONTROL MEASURES IDENTIFIED IN THE PLAN SHALL BE INSPECTED TO ENSURE THEY ARE OPERATING CORRECTLY. LOCATIONS WHERE VEHICLES ENTER AND LEAVE THE SITE SHALL BE INSPECTED FOR EVIDENCE OF OFFSITE SEDIMENT TRACKING.
4. REPAIR OR MAINTENANCE NEEDED TO ASSURE PROPER OPERATION OF THE STORMWATER POLLUTION PREVENTION PLAN SHALL BE DONE IN A TIMELY MANNER BUT NO LATER THAN 7 CALENDAR DAYS FOLLOWING THE INSPECTION.
5. A REPORT SHALL BE KEPT OF EACH INSPECTION FOR THREE YEARS AFTER FINAL STABILIZATION AND SHALL INCLUDE THE DATES OF EACH INSPECTION, THE SCOPE OF THE INSPECTION, MAJOR OBSERVATIONS, ANY REPAIR AND/OR MAINTENANCE REQUIRED AND ANY INCIDENT OF NON-COMPLIANCE. IF THE REPORT DOES NOT CONTAIN ANY INCIDENTS OF NON-COMPLIANCE, THE REPORT SHALL CONTAIN A CERTIFICATION THAT THE FACILITY HAS BEEN IN COMPLIANCE WITH THE STORMWATER POLLUTION PREVENTION PLAN AND THE NPDES PERMIT. THE REPORT SHALL INCLUDE THE NAME AND QUALIFICATIONS OF THE INSPECTOR AND SHALL BE SIGNED IN ACCORDANCE TO FDEP RULE 62-621.300, PART VII.C. A COPY OF THE CONSTRUCTION INSPECTION FORM IS INCLUDED ON THIS STORMWATER POLLUTION PREVENTION PLAN SHEET. A COPY SHALL BE RETAINED AT THE CONSTRUCTION SITE FROM THE DATE OF PROJECT INITIATION TO THE DATE OF FINAL STABILIZATION.

X. NON-STORMWATER DISCHARGES:

1. THE FOLLOWNG NON-STORMWATER DISCHARGES MIGHT BE COMBINED WITH STORMWATER AND WOULD BE AUTHORIZED AS PART OF THIS PERMIT: FIRE HYDRANT FLUSHING, CONTROL OF DUST, POTABLE WATER FLUSHING AND IRRIGATION DRAINAGE. BECAUSE OF THE NATURE OF THESE DISCHARGES, THE EROSION, STABILIZATION AND TREATMENT SYSTEMS TO BE IMPLEMENTED, AS PART OF THIS PLAN WOULD BE APPROPRIATE TO PREVENT AND TREAT ANY POLLUTION RELATED TO THESE NON-STORMWATER DISCHARGES.
2. DISCHARGES FROM DEWATERING ACTIVITIES ASSOCIATED WITH SITE CONSTRUCTION ARE NOT AUTHORIZED AND REQUIRED CONSTRUCTION OF TEMPORARY SEDIMENTATION BASINS AND USE OF APPROPRIATE FLOCCULATING AGENTS TO ENHANCE PARTICLE SEGREGATION AND SPEED UP SETTLING OF PARTICLES.

XI. CONTRACTORS:

1. ALL CONTRACTORS AND/OR SUBCONTRACTORS RESPONSIBLE FOR IMPLEMENTING THE PLAN SHALL SIGN THE CERTIFICATION STATEMENT BEFORE STARTING CONSTRUCTION ACTIVITIES OF THE PROJECT. THE CERTIFICATION MUST INCLUDE THE NAME AND TITLE OF THE PERSON PROVIDING THE SIGNATURE, THE NAME, ADDRESS AND TELEPHONE NUMBER OF THE CONTRACTING FIRM, THE ADDRESS OF THE SITE AND THE DATE THE CERTIFICATION IS MADE. THE OWNER SHALL KEEP THESE CERTIFICATIONS AS PART OF THIS POLLUTION PLAN. MULTIPLE COPIES OF THE CERTIFICATION STATEMENT MAY BE NECESSARY DEPENDING UPON THE NUMBER OF SUBCONTRACTORS ASSOCIATED WITH THE PROJECT.

CERTIFICATION STATEMENT

"I CERTIFY UNDER PENALTY OF LAW THAT I UNDERSTAND AND SHALL COMPLY WITH THE TERMS AND CONDITIONS OF THE STATE OF FLORIDA GENERIC PERMIT FOR STORMWATER DISCHARGE FROM LARGE AND SMALL CONSTRUCTION ACTIVITIES AND THIS STORMWATER POLLUTION PREVENTION PLAN PREPARED THEREUNDER."

CONTRACTING FIRM: _____

ADDRESS: _____

CITY, STATE, ZIP CODE: _____

TELEPHONE: _____

FAX: _____

PROJECT NAME: ALCHEM LABORATORIES: BUILDING 4 SITE PLAN

PROJECT ADDRESS: _____

PROJECT ADDRESS: _____

CITY, STATE, ZIP CODE: _____, FLORIDA.

NAME: _____ SIGNATURE: _____

DATE: _____

STORMWATER POLLUTION PREVENTION PLAN INSPECTION REPORT FORM

Inspections must occur at least once a week and within 24 hours of the end of a storm event that is 0.50 inches or greater.

PROJECT NAME: **ALCHEM LABORATORIES: BUILDING 4 SITE PLAN** FDEP NPDES STORMWATER IDENTIFICATION NO.: FLR10 _____

OWNER: _____ CONTRACTOR: _____

CONSTRUCTION MANAGER: _____

Date of Inspection	Location	Rain data	Type of control (see below)	Date installed / modified	Current Condition (see below)	Observations or Corrective Action / Other Remarks	Inspected By

CONDITION CODE: G = Good; C = Needs to be cleaned; M = Marginal, needs maintenance or replacement soon; P = Poor, needs immediate maintenance or replacement; O = Other

CONTROL TYPE CODES

1. Silt Fence	10. Storm drain inlet protection	19. Reinforced soil retaining system	28. Tree protection
2. Earth dikes	11. Vegetative buffer strip	20. Gabion	29. Detention pond
3. Structural diversion	12. Vegetative preservation area	21. Sediment Basin	30. Retention pond
4. Swale	13. Retention Pond	22. Temporary seed / sod	31. Waste disposal / housekeeping
5. Sediment Trap	14. Construction entrance stabilization	23. Permanent seed / sod	32. Dam
6. Check dam	15. Perimeter ditch	24. Mulch	33. Sand Bag
7. Subsurface drain	16. Curb and gutter	25. Hay Bales	34. Other
8. Pipe slope drain	17. Paved road surface	26. Geotextile	
9. Level spreaders	18. Rock outlet protection	27. Rip-rap	

INSPECTOR INFORMATION:

Name _____ Qualification _____ Date _____

The above signature also shall certify that this facility is in compliance with the Stormwater Pollution Prevention Plan and the State of Florida Generic Permit for Stormwater Discharge from Large and Small Construction Activities if there are not any incidents of non compliance identified above.

* * * * * *

"I certify under penalty of law that this document and all attachments were prepared under my direction or supervision in accordance with a system designed to assure that qualified personnel properly gathered and evaluated the information submitted. Based on my inquiry of the person or persons who manage the system or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations.

eda consultants inc.
EB 2389
720 S.W. 2nd Ave, South Tower, Suite 300
GAINESVILLE, FLORIDA 32601
TEL. (352) 373-3541
www.edafl.com mail@edafl.com

No.	Date	Comment

Professional Engineer of Record:

Claudia S. Vega, P.E. 51532
Engineer Certificate No.

Project No: 20-103

Project phase: CITY SUBMITTAL

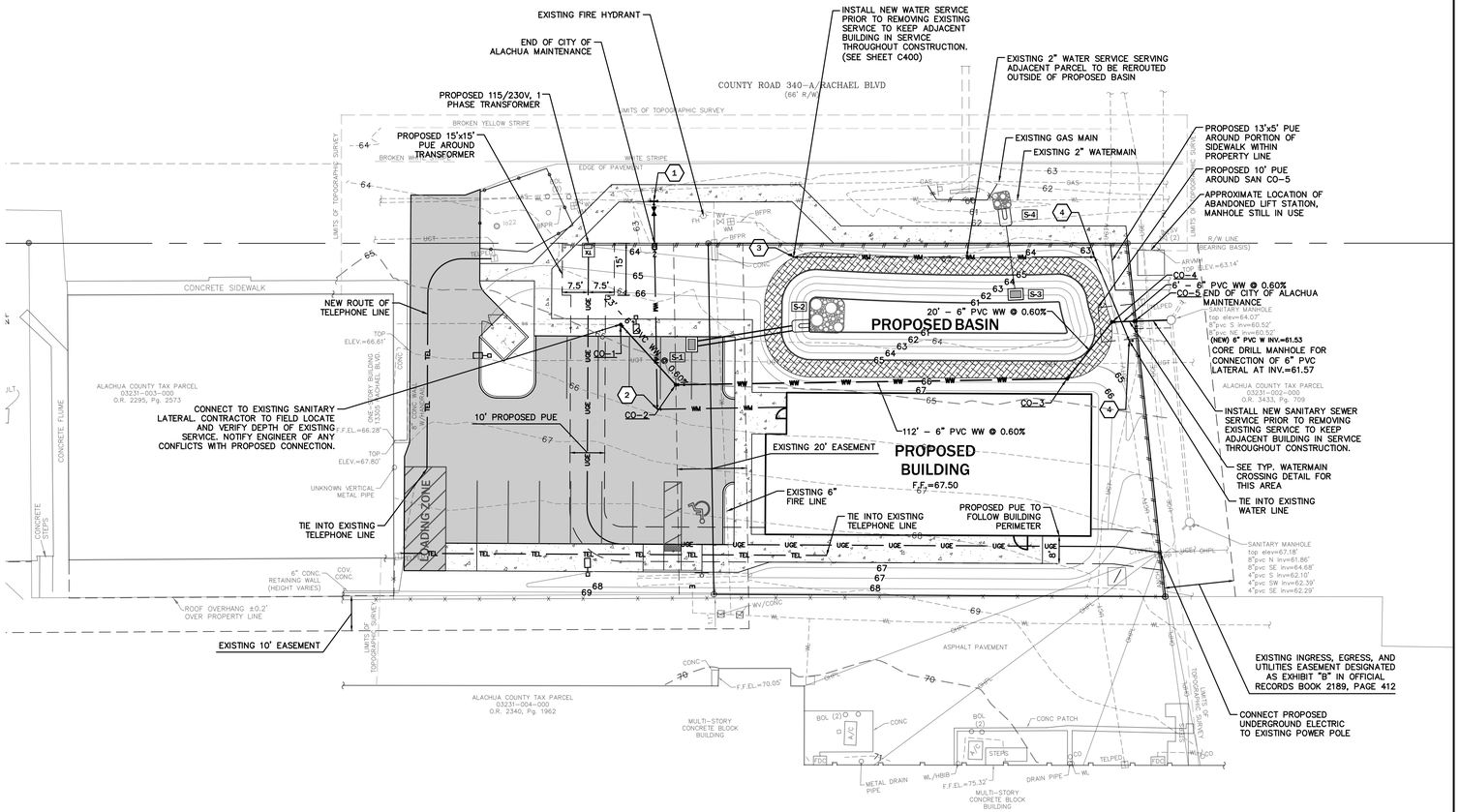
Project title: ALCHEM LABORATORIES: BUILDING 4 SITE PLAN CITY OF ALACHUA, FLORIDA

Sheet title: STORMWATER POLLUTION PREVENTION PLAN

Designed: CV
Drawn: JB
Checked: CV
Date: 03/24/21

Sheet No.: C300

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GENERAL UTILITY NOTES

1. ALL CONSTRUCTION WORK SHALL BE IN ACCORDANCE WITH THE LATEST EDITION CITY OF ALACHUA, PUBLIC SERVICE DESIGN AND CONSTRUCTION MANUAL, INCLUDING STANDARD DETAILS, AND THE F.D.O.T. "STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION" ADOPTED IN 1991 BY THE STATE OF FLORIDA, AND ALL SUPPLEMENTS THERETO, AND THE "ROADWAY TRAFFIC DESIGN STANDARDS", WHICHEVER IS MORE STRINGENT.
2. THE LOCATIONS OF EXISTING UTILITIES SUCH AS WATER MAINS, SEWER, GAS LINES, ETC., AS SHOWN ON THE PLANS HAVE BEEN DETERMINED FROM THE BEST AVAILABLE INFORMATION, AND ARE PROVIDED FOR THE CONVENIENCE OF THE CONTRACTOR. HOWEVER, THE ENGINEER AND OWNER DO NOT ASSUME RESPONSIBILITY FOR THE SIZES AND LOCATIONS SHOWN. THE CONTRACTOR SHALL BE RESPONSIBLE FOR FIELD VERIFICATION OF THE SIZE AND LOCATION OF ALL EXISTING UTILITIES SHOWN AS WELL AS NOT SHOWN ON THESE PLANS AND PROTECTION OF SAME DURING CONSTRUCTION. ANY EXISTING UTILITY TO BE MODIFIED SHALL BE COORDINATED WITH THE APPROPRIATE UTILITY PROVIDER.
3. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO SECURE ANY AND ALL NEEDED LICENSES AND PERMITS REQUIRED FOR CONSTRUCTION PURPOSES.
4. ELECTRIC SERVICE TO BE COORDINATED WITH CITY OF ALACHUA PUBLIC SERVICE DEPARTMENT. SEE COA ELECTRICAL SCOPE OF WORK NOTES ON THIS SHEET.
5. NO GAS SERVICE IS REQUIRED FOR THIS DEVELOPMENT.
6. CONTRACTOR IS RESPONSIBLE FOR VERIFYING ALL SETBACKS AND EASEMENTS BEFORE BEGINNING CONSTRUCTION.
7. BENCHMARKS AND OTHER REFERENCE POINTS SHALL BE CAREFULLY MAINTAINED THROUGHOUT THE CONSTRUCTION PERIOD. IF DISTURBED OR DESTROYED, THESE POINTS SHALL BE REPLACED BY A FLORIDA P.L.S. AT CONTRACTOR'S EXPENSE.
8. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO PROVIDE ANY REQUIRED TRAFFIC CONTROL PLANS. ALL WORK SHALL BE EXECUTED UNDER THE ESTABLISHED PLAN AND FLORIDA DEPT. OF TRANSPORTATION AND CITY OF ALACHUA APPROVED PROCEDURES.
9. THE CONTRACTOR SHALL RESTORE OFF SITE CONSTRUCTION AREAS TO EQUAL OR BETTER CONDITION THAN EXISTED PRIOR TO COMMENCEMENT OF CONSTRUCTION.
10. CONTRACTOR IS RESPONSIBLE FOR PROTECTION AND VERIFICATION OF EXISTING UTILITIES. ANY EXISTING UTILITY TO BE MODIFIED SHALL BE COORDINATED WITH THE APPROPRIATE UTILITY PROVIDER.
11. WATER METER ASSEMBLIES SHALL BE INSTALLED BY CONTRACTOR.
12. UTILITY INSTALLATION SHALL NOT TAKE PLACE WITHIN TREE BARRICADE AREA AS SHOWN ON THE PAVING, GRADING, AND DRAINAGE PLAN.
13. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DISPOSAL OF UNSUITABLE MATERIALS OFF-SITE AND FURNISH APPROVED MATERIAL PER CITY OF ALACHUA FOR SANITARY SEWER LINES BACKFILL AT NO ADDITIONAL COST TO THE OWNER. THE CONTRACTOR MAY DISPOSE OF UNSUITABLE MATERIAL ON-SITE BY APPROVAL OF THE OWNER.
14. NO PERMANENT STRUCTURE SHALL BE CONSTRUCTED WITHIN 10 FEET OF ANY WATER LINE OR 15 FEET OF ANY SEWER LINE.
15. CONTRACTOR SHALL PROTECT LANDSCAPE AND ANY DISTURBED AREAS SHALL BE RESTORED AS EXISTING.
16. CONTRACTOR SHALL FURNISH AND INSTALL 2" PLUG VALVE WITH VALVE BOX AND CONCRETE VALVE COLLAR. INSTALL SIMILAR TO CITY OF ALACHUA DETAIL 118 (SEE DETAIL SHEET C420)

CITY OF ALACHUA ELECTRICAL SCOPE OF WORK

1.1 SCOPE OF COA ELECTRICAL WORK: AT NO COST TO THE CITY, THE COA WILL PERFORM CERTAIN WORK ELEMENTS TO ASSIST THE CONTRACTOR IN PROVIDING COMPLETE AND OPERATING ELECTRICAL SYSTEMS AS SHOWN ON THE DRAWINGS AND SPECIFIED HEREIN.

1.2 THE WORK TO EXTEND THIS EXISTING PRIMARY SERVICE AND TO PROVIDE THE TRANSFORMER SHALL BE A COMBINED EFFORT BETWEEN THE CONTRACTOR AND COA.

1.3 THE CONTRACTOR SHALL BE FULLY RESPONSIBLE FOR ALL ELECTRICAL WORK REQUIRED TO PROVIDE SECONDARY (LOAD-SIDE) SERVICE FROM THE TRANSFORMER

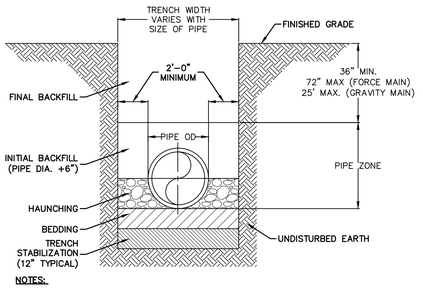
1.4 COA WILL ORDER THE FOLLOWING PRODUCTS, WHICH SHALL BE PAID FOR BY THE CONTRACTOR: ALL PRIMARY CABLES, ALL TRANSFORMERS AND ALL MAKE-UP MATERIALS. ONCE ON SITE, THESE PRODUCTS WILL BE INSTALLED BY THE CITY OF ALACHUA. ALL PRIMARY MAKEUP, CONNECTIONS AND TESTING WILL BE COMPLETED BY THE CITY OF ALACHUA. ALL WORK PERFORMED BY THE COA WILL BE BACKCHARGED TO THE CONTRACTOR.

1.5 THE CONTRACTOR SHALL FURNISH AND INSTALL ALL OTHER MATERIALS, INCLUDING, BUT NOT LIMITED TO ALL CONDUIT, ENCLOSURES, POLES, LUMINAIRES, TRANSFORMER PADS, AND SWITCHGEAR BASES.

1.6 THE CONTRACTOR SHALL SCHEDULE AN ELECTRICAL COORDINATION MEETING AT START OF PROJECT. REQUIRED ATTENDEES INCLUDE THE CONTRACTOR, THE ELECTRICAL SUB-CONTRACTOR, THE ENGINEER OF RECORD, THE COA PROJECT MANAGER AND COA ELECTRICAL STAFF.

SANITARY SEWER NOTES

1. ROOF DRAINS, FOUNDATION DRAINS OR OTHER STORM WATER CONNECTIONS TO THE SANITARY SEWER ARE PROHIBITED.
2. ALL SANITARY SEWER PIPE SHALL BE PVC SEWER PIPE CONFORMING TO ASTM D-3034, SDR 35 PVC, UNLESS OTHERWISE NOTED.
3. ALL MANHOLES SHALL BE 4 FT. INSIDE DIAMETER UNLESS OTHERWISE NOTED.
4. WHEN MINIMUM SEPARATION CANNOT BE MAINTAINED BETWEEN WATER AND GRAVITY SEWER, THE SEWER SHALL BE UPGRADED TO C-900, DR18 AND PRESSURE TESTED TO 150 PSI.
5. FOR HORIZONTAL AND VERTICAL CLEARANCES BETWEEN SANITARY SEWER MAINS/LATERALS AND WATER MAINS/LATERALS, SEE WATER NOTES 3 & 4.
6. WHERE SOIL REPORT INDICATES THE POSSIBILITY OF UNSUITABLE MATERIAL IN THE VICINITY OF SANITARY LINES, THE CONTRACTOR SHALL DETERMINE THE NATURE AND EXTENT OF THIS UNSUITABLE MATERIAL, PREPARE TRENCH AND INSTALL THE SEWER LINE IN ACCORDANCE WITH ASTM D-2321. IT WILL BE THE CONTRACTOR'S RESPONSIBILITY TO NOTIFY THE ENGINEER OF THIS UNSUITABLE MATERIAL.
7. THE CONTRACTOR SHALL BE REQUIRED TO PERFORM INFILTRATION OR EXFILTRATION TEST, WHICHEVER IS APPLICABLE. THE CONTRACTOR SHALL MEET ALL REQUIREMENTS, CODES AND ORDINANCES OF THE CITY OF ALACHUA UTILITIES INCLUDING ADVANCE NOTIFICATION PRIOR TO CONSTRUCTION AND TESTING.
8. THE CONSTRUCTION OF ALL UTILITIES CONNECTING TO THE CITY OF ALACHUA'S SYSTEMS SHALL CONFORM TO THE CITY OF ALACHUA DESIGN AND CONSTRUCTION MANUAL, LATEST EDITION.
9. THE CONTRACTOR SHALL NOTIFY THE CITY 48 HOURS PRIOR TO ANY UTILITY CONSTRUCTION.
10. THE CONTRACTOR SHALL PROVIDE TEST ASSEMBLIES AND TRACER WIRE TO BOTH METALLIC AND NON-METALLIC PIPE AS PER THE CITY OF ALACHUA DESIGN STANDARDS AND DETAILS.
11. FOR FORCE MAIN, PROVIDE TRACER WIRE AND APPURTENANCES FROM THE LIFT STATION TO THE CONNECTING COA MANHOLE.

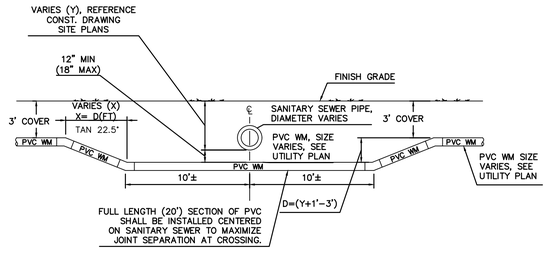


NOTES:

1. TRENCH STABILIZATION SHALL BE PROVIDED TO A DEPTH OF 12-INCHES. THE MATERIAL SHALL BE NO. 57 STONE.
2. BEDDING SHALL BE A MINIMUM OF 6-INCHES AND COMPOSED OF IMPORTED GRANULAR FILL. COMPACT TO 95% STANDARD PROCTOR, AASHTO T-99.
3. HUNCHING PORTION OF THE PIPE ZONE SHALL BE PLACED TO THE SPRINGLINE OF THE PIPE AND COMPOSED OF IMPORTED GRANULAR FILL. COMPACT TO 95% STANDARD PROCTOR, AASHTO T-99.
4. INITIAL BACKFILL: COMMON FILL COMPACTED TO 95% (98% UNDER PAVEMENT) OF THE MAXIMUM DENSITY AS PER AASHTO T-180. FROM THE SPRINGLINE OF THE PIPE TO TWO (2) FEET ABOVE THE PIPE THE SOIL SHALL BE CAREFULLY BACKFILLED IN 6-INCH LIFTS AND THE SOIL CONSOLIDATED WITH THE HAND OPERATED TAMPING MACHINE (OR AS REQUIRED BY THE CITY, COUNTY OR STATE INSPECTORS).
5. FINAL BACKFILL: COMMON FILL COMPACTED TO 95% (95%) UNDER PAVEMENT OF THE MAXIMUM DENSITY AS PER AASHTO T-180. AFTER PLACEMENT AND COMPACTION OF THE INITIAL BACKFILL, THE BALANCE OF THE BACKFILL MATERIAL MAY BE MACHINE PLACED OR AS REQUIRED BY THE INSPECTOR AND SHALL NOT CONTAIN ANY ROCKS OR DEBRIS.
6. WATER SHALL NOT BE PERMITTED IN THE TRENCH DURING CONSTRUCTION.
7. ALL PIPE TO BE INSTALLED WITH BELL FACING UPSTREAM TO THE DIRECTION OF THE FLOW.
8. DEPTH FOR REMOVAL OF UNSUITABLE MATERIAL SHALL GOVERN DEPTH OF BEDDING ROCK BELOW THE PIPE. UTILITIES SHALL DETERMINE IN THE FIELD REQUIRED REMOVAL OF UNSUITABLE MATERIAL TO REACH SUITABLE FOUNDATION. THE BOTTOM OF THE TRENCH SHALL NOT BE EXCAVATED BELOW THE SPECIFIED GRADE. IF UNDERCUTTING OCCURS, THE BOTTOM OF THE TRENCH SHALL BE BROUGHT UP TO THE ORIGINAL GRADE WITH APPROVED MATERIAL AND THOROUGHLY COMPACTED TO A MINIMUM OF 95% MODIFIED PROCTOR.
9. FINAL RESTORATION IN IMPROVED AREAS SHALL BE IN COMPLIANCE WITH ALL APPLICABLE REGULATIONS OF GOVERNING AGENCIES.

DETAIL No. 405 - DETAIL BACKFILL REQUIREMENTS

NTS



TYPICAL WATERMAIN CROSSING SECTION DETAIL

NTS

WATER FITTING SCHEDULE

KEY	ASSEMBLY
1	CONNECT TO EXISTING 2" PVC WATER MAIN 1 - 2"X1" TAP, WET OR DRY, DEPENDING ON SITE CONDITION (TAP IS BY CITY OF ALACHUA AND MOT, EXCAVATION, BACKFILL AND RESTORATION ARE BY CONTRACTOR. NOTIFY CITY OF ALACHUA UTILITY INSPECTOR 7 DAYS IN ADVANCE) 1 - 1" GATE VALVE AND BOX 1 - 3/4" WATER METER AT PROPERTY LINE (END OF CITY OF ALACHUA MAINTENANCE) 1 - 1" REDUCED PRESSURE ZONE BACKFLOW PREVENTER
2	1 - 1" PVC 90° BEND
3	1 - 2" PVC 22.5° BEND
4	1 - 2" PVC 45° BEND

WASTEWATER CLEANOUT SCHEDULE

CLEANOUT	CLEANOUT DATA
CO-1	TOP EL.=67.35 INV.=62.64
CO-2	TOP EL.=0.00 INV.=62.50 INV.=62.50
CO-3	TOP EL.=64.68 INV.=61.82 INV.=61.82
CO-4	TOP EL.=62.21 INV.=61.69 INV.=61.69
CO-5	TOP EL.=63.80 INV.=61.65 INV.=61.65

consultants inc.
EB 2389
720 S.W. 2nd Ave, South Tower, Suite 300
GAINESVILLE, FLORIDA 32601
TEL. (352) 373-3541
www.edafl.com mail@edafl.com

NORTH
SCALE: 1" = 20'
0 10 20 40
GRAPHIC SCALE

No.	Date	Comment

Professional Engineer of Record:

Claudia S. Vega, P.E. — Engineer
51532 — Cerlificate No.

Project No: 20-103

Project phase: CITY SUBMITTAL

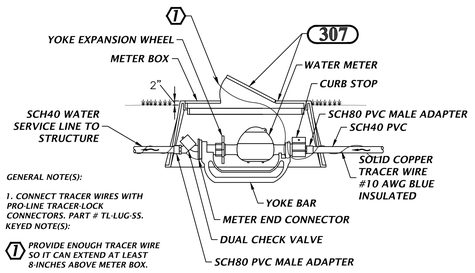
Project title: ALCHEM LABORATORIES: BUILDING 4 SITE PLAN CITY OF ALACHUA, FLORIDA

Sheet title: UTILITY PLAN

Designed:	CV
Drawn:	JB
Checked:	CV
Date:	03/24/21

Sheet No.: C400

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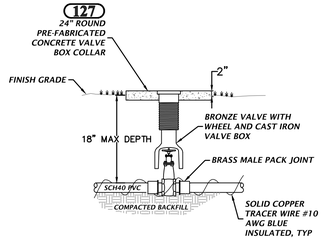
GENERAL NOTE(S):

1. CONNECT TRACER WIRES WITH PRO-LINE TRACER-LOCK CONNECTORS. PART # TL-LUG-SS.

KEYED NOTE(S):

(1) PROVIDE ENOUGH TRACER WIRE SO IT CAN EXTEND AT LEAST 8-INCHES ABOVE METER BOX.

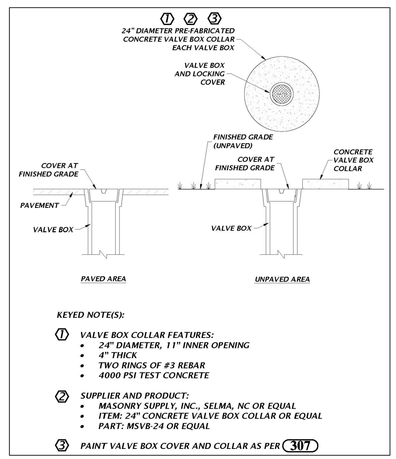
(115) 3/4" AND 1" WATER METER ASSEMBLY
NTS



GENERAL NOTE(S):

1. WATER VALVE BOX TOPS ARE TO BE CAST WITH THE WORD "WATER" ON TOP AND PAINTED BLUE.

(142) BRONZE GATE VALVE FOR WATER AND RECLAIMED WATER MAINS 2" OR SMALLER
NTS



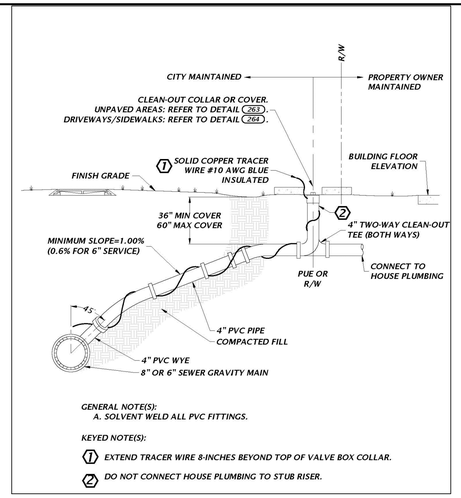
KEYED NOTE(S):

(1) VALVE BOX COLLAR FEATURES:
- 24" DIAMETER, 11" INNER OPENING
- 4" THICK
- TWO RINGS OF #3 REBAR
- 4000 PSI TEST CONCRETE

(2) SUPPLIER AND PRODUCT:
- MASONRY SUPPLY, INC., SELMA, NC OR EQUAL
- ITEM: 24" CONCRETE VALVE BOX COLLAR OR EQUAL
- PART: MSVB-24 OR EQUAL

(3) PAINT VALVE BOX COVER AND COLLAR AS PER (307)

(127) VALVE BOX COVER
NTS



GENERAL NOTE(S):
A. SOLVENT WELD ALL PVC FITTINGS.

KEYED NOTE(S):

(1) EXTEND TRACER WIRE 8-INCHES BEYOND TOP OF VALVE BOX COLLAR.

(2) DO NOT CONNECT HOUSE PLUMBING TO STUB RISER.

(200) WASTEWATER SERVICE LATERAL
NTS

PAINT SYSTEM NO. 5

SURFACE PREPARATION –PRESSSURE WASH SURFACE TO BE PAINTED

OPTIMAL APPLICATION CONDITIONS

AMBIENT TEMPERATURE BETWEEN 55 AND 75 DEGREES F.

HUMIDITY BELOW 60 PERCENT

IF OUTSIDE THESE LIMITS, REQUEST PERMISSION TO APPLY PAINT FROM COA INSPECTOR OR SERVICES DURING CONSTRUCTION ENGINEER

APPLY (SPRAY) FIRST COAT

APPLY (SPRAY) SECOND COAT EITHER WITHIN TWO HOURS OR MORE THAN 48 HOURS AFTER APPLYING FIRST COAT.

PRODUCT

KRYLON SPAY PAINT

FUSION ALL–IN–ONE

PAINT PLUS PRIMER

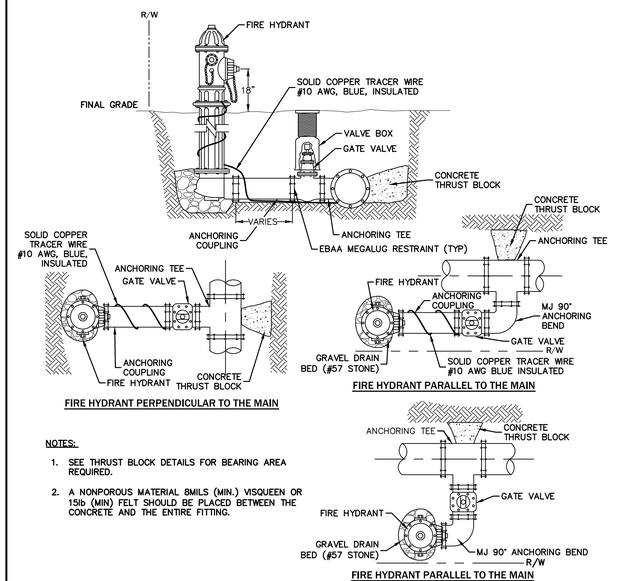
COLORS

POTABLE WATER – GLOSS PATRIOTIC BLUE

WASTEWATER – GLOSSY SPRING GRASS

RECLAIMED – GLOSSY ICY GRAPE

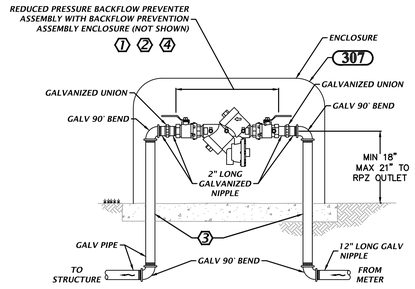
(307) PAINT SYSTEM NO. 5
NTS



NOTES:

1. SEE THRUST BLOCK DETAILS FOR BEARING AREA REQUIRED.
2. A NONPOROUS MATERIAL 8MILS (MIN.) VISQUEEN OR 15lb (MIN) FELT SHOULD BE PLACED BETWEEN THE CONCRETE AND THE ENTIRE FITTING.

DETAIL No. 210 - FIRE HYDRANT (PERPENDICULAR/PARALLEL TO MAIN)
N.T.S.



KEYED NOTE(S):

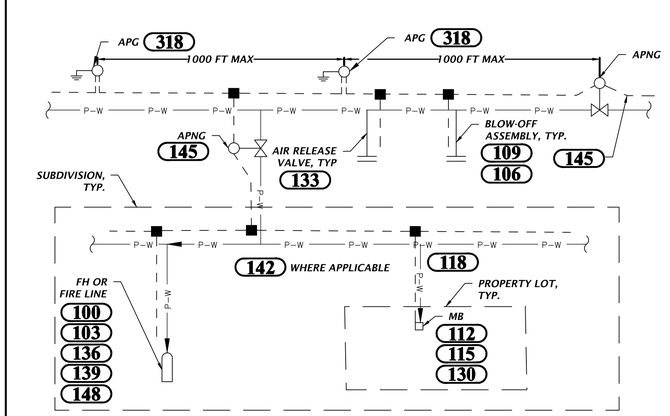
(1) PROVIDE BACKFLOW PREVENTION ASSEMBLY ENCLOSURE. COORDINATE WITH VENDOR TO SELECT ENCLOSURE SIZE SUITABLE FOR INSTALLED BACKFLOW PREVENTER ASSEMBLY.

(2) ENCLOSURE VENDOR AND FEATURES:
- MARINE GRADE ALUMINUM
- 1.5 INCH INSULATION, 9.0 R VALUE
- MEANS TO ACCESS ASSEMBLY
- LOCKABLE
- MOUNTING HARDWARE
- COLOR: TAN, UNLESS OTHERWISE NOTED.
- VENDORS: SAFE T COVER, NASHVILLE, TN; OR EQUAL

POUR CONCRETE PAD WITH DIMENSIONS AS PER VENDOR'S RECOMMENDATIONS. MINIMUM THICKNESS OF 4 INCHES.
- PROVIDE SLEEVES AROUND PIPING

BACKFLOW PREVENTER MAINTAINED BY PROPERTY OWNER, NOT COA.

(121) 3/4" TO 2" BACKFLOW PREVENTER AND ENCLOSURE
NTS

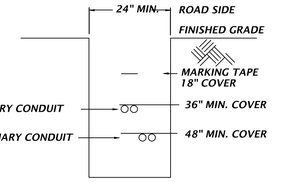


GENERAL NOTES: SEE (321A)

128 WHERE APPLICABLE
148 WHERE APPLICABLE

(321B) POTABLE (OR RECLAIMED) TRACER WIRE CONFIGURATION
NTS

WARNING TAPE SHALL BE RED WITH CONTINUOUS BLACK MARKINGS "CAUTION CAUTION - CITY OF ALACHUA - ELECTRIC LINE BURIED BELOW SHALL BE ALLENSYSTEM "MARKLINE" OR EQUAL REEFINDUSTRIES OR THORENTERPRISES.



NOTE:
CONTRACTOR SHALL CHECK FOR CONFLICTS AND MAKE ADJUSTMENTS AS NECESSARY. REFER TO THE OTHER UTILITY ALLOCATIONS AND LOCATIONS.

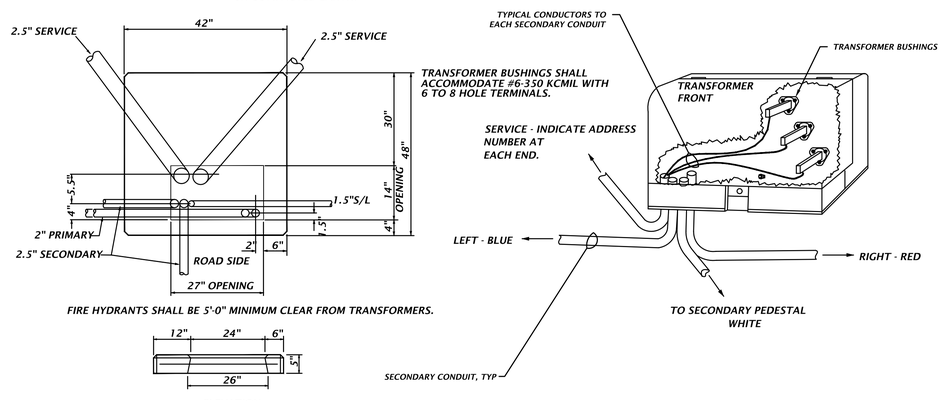
(506) TYPICAL CONDUIT TRENCH
NTS

ELECTRICAL UTILITIES LEGEND AND ABBREVIATIONS

50a	TRANSFORMER ON CONCRETE PAD, 50 KVA PHASE b, 120/240 VOLT SINGLE PHASE SECONDARY.
P–2a P–2a	1/0 AL PRIMARY CABLE, IN 2" PVC AT 48" MIN. COVER.
P–4/0 P–4/0	4/0 AL TPX SECONDARY CABLE, IN 2.5" PVC AT 36" MIN. COVER.
P–SL P–SL 6/3	6/3 AL TPX SECONDARY STREETLIGHT CABLE, IN 1.5" PVC AT 36" MIN. COVER.
12/2 P–SL P–SL	12/2 WITH GROUND, IN 1.5" PVC AT 36" MIN. COVER.
	JUNCTION BOX
SE	SERVICE ENCLOSURE, BURIED, APPROXIMATELY 33" X 20" X 15" DEEP.
80W 30'	80 WATT LED CUTOFF LUMINAIRE ON 30' FIBERGLASS POLE (25' MOUNTING HT) WITH 4' ARM.
10"	ROUND BURIED STREETLIGHT WIRING ENCLOSURE APPROXIMATELY 10" DIAMETER X 12" DEEP HIGH DENSITY POLYETHYLENE.
SW	600 AMP PAD MOUNTED SWITCHGEAR. DOORS INDICATE SOURCE AND LOAD SIDES.

PME	PAD MOUNTED EQUIPMENT
PUE	PUBLIC UTILITY EASEMENT
PVC	POLYVINYL CHLORIDE
CT	CURRENT TRANSFORMER
R/W	RIGHT OF WAY

TRANSFORMER PADS SHALL BE PLACED ON COMPACTED EARTH.



(500) SINGLE PHASE TRANSFORMER AND PAD
NTS

eda consultants inc.
EB 2389
720 S.W. 2nd Ave, South Tower, Suite 300
GAINESVILLE, FLORIDA 32601
TEL. (352) 373-3541
www.edafl.com mail@edafl.com

SCALES:
AS SHOWN

No.	Date	Comment

Professional Engineer of Record:

Claudia S. Vega, P.E. 51532
Engineer Certificate No.

Project No: 20-103

Project phase: CITY SUBMITTAL

Project title: ALCHEM LABORATORIES: BUILDING 4 SITE PLAN CITY OF ALACHUA, FLORIDA

Sheet title: UTILITY DETAILS

Designed: CV
Drawn: JB
Checked: CV
Date: 03/24/21

Sheet No.: C410

:\engprojects\Alchem - Bldg 4\Plans\Current DWG\A20103\1.dwg, C410-UTIL DETAILS, 3/24/2021 1:17:44 PM

BOUNDARY AND TOPOGRAPHIC SURVEY

IN

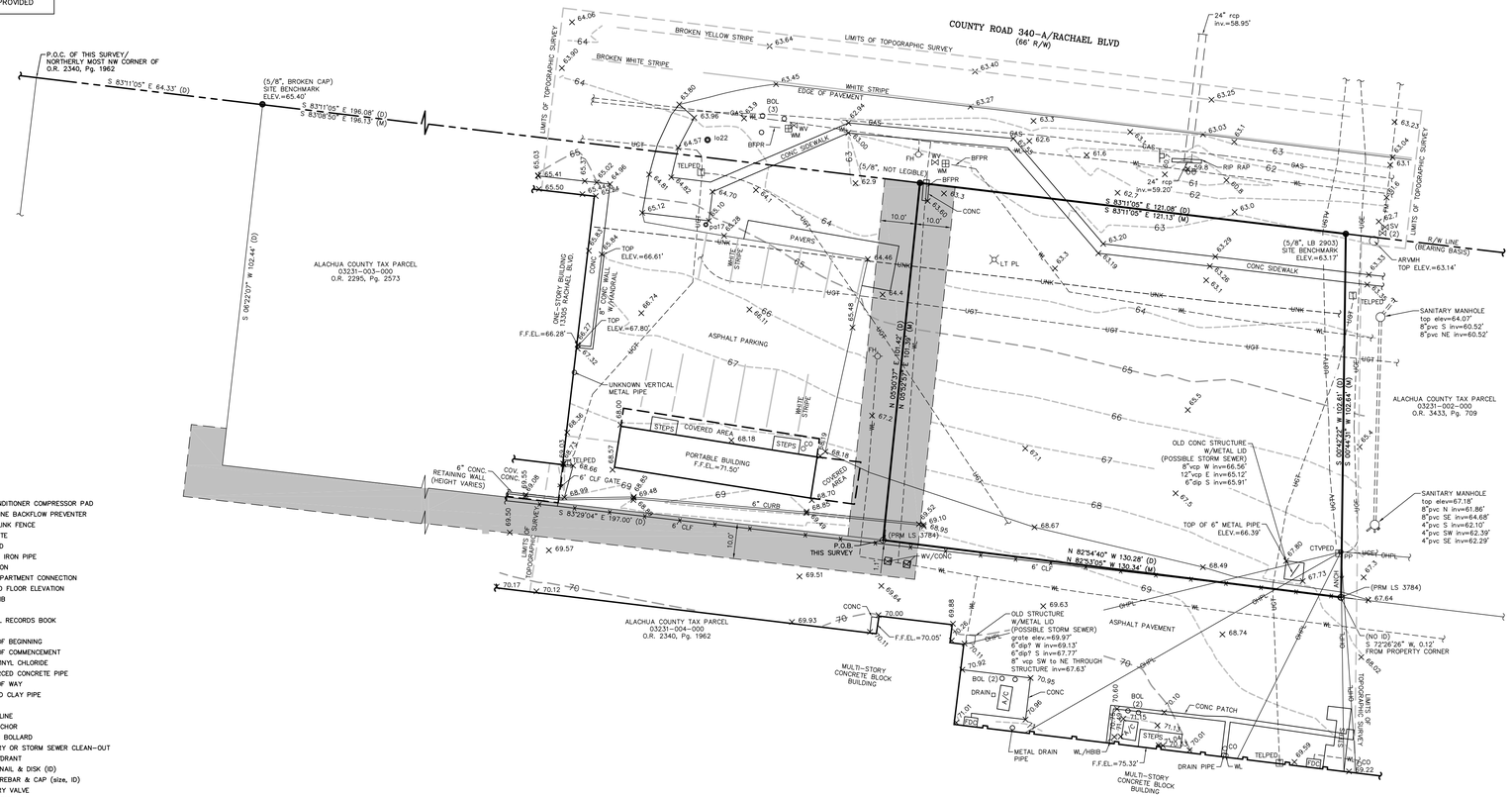
SECTION 14, TOWNSHIP 8 SOUTH, RANGE 18 EAST
CITY OF ALACHUA, ALACHUA COUNTY, FLORIDA

FOR

ALCHEM LABORATORIES CORPORATION

AMERICAN LAND TITLE ASSOCIATION COMMITMENT
FIRST AMERICAN TITLE INSURANCE COMPANY
COMMITMENT DATE: DECEMBER 1, 2020 AT 8:00 AM
FILE No: 2076-5074000
SCHEDULE B-II

ITEM NO.	DESCRIPTION	AFFECTS/ DOES NOT AFFECT	PLOTTED/ NOT PLOTTED
1-8	NOT SURVEY RELATED	N/A	NOT PLOTTED
9	DECLARATION OF COVENANTS, CONDITIONS, RESTRICTIONS, AND EASEMENTS (O.R. BOOK 2296, Pg. 1957)	AFFECTS ENTIRE PARCEL	NOT PLOTTED
	AMENDMENT TO DECLARATION OF COVENANTS, CONDITIONS, RESTRICTIONS, AND EASEMENTS (O.R. BOOK 2410, Pg. 876)	DOES NOT AFFECT	NOT PLOTTED
	SECOND AMENDMENT TO DECLARATION OF COVENANTS, CONDITIONS, RESTRICTIONS, AND EASEMENTS (O.R. BOOK 4700, Pg. 1153)	AFFECTS ENTIRE PARCEL	NOT PLOTTED
	ASSIGNMENT OF DECLARANT'S RIGHTS (O.R. BOOK 2410, Pg. 905)	DOES NOT AFFECT	NOT PLOTTED
	ASSIGNMENT OF DEVELOPMENT RIGHTS (O.R. BOOK 4232, Pg. 1797)	DOES NOT AFFECT	NOT PLOTTED
	PARTIAL WAIVER (O.R. BOOK 4234, Pg. 1585)	DOES NOT AFFECT	NOT PLOTTED
	RELEASE OF LAND FROM DECLARATION OF COVENANTS, CONDITIONS, RESTRICTIONS, AND EASEMENTS (O.R. BOOK 4398, Pg. 1796)	DOES NOT AFFECT	NOT PLOTTED
10	EASEMENT AGREEMENT BETWEEN UNIVERSITY OF FLORIDA FOUNDATION, INC. AND ALCHEM LABORATORIES CORP (O.R. BOOK 2295, PAGES 2573 AND 2576)	AFFECTS	PLOTTED
11	DRAINAGE EASEMENT (O.R. BOOK 2340, Pg. 1962)	AFFECTS ENTIRE PARCEL	NOT PLOTTED
	AMENDMENT TO DRAINAGE EASEMENT (O.R. BOOK 3561, Pg. 1401)	AFFECTS ENTIRE PARCEL	NOT PLOTTED
	SECOND AMENDMENT TO DRAINAGE EASEMENT (O.R. BOOK 4427, Pg. 1436)	AFFECTS ENTIRE PARCEL	NOT PLOTTED
12	OIL, GAS, AND MINERAL RESERVATIONS (DEED BOOK 259, Pg. 64). THE RIGHT OF ENTRY UNDER SAID RESERVATIONS HAS BEEN RELEASED OR IS BARRED BY MRTA.	AFFECTS ENTIRE PARCEL	NOT PLOTTED
13	TERMS AND CONDITIONS OF ANY EXISTING UNRECORDED LEASE(S), AND ALL RIGHTS OF LESSEE(S) AND ANY PARTIES CLAIMING THROUGH THE LESSEE(S) UNDER THE LEASE(S)	DOCUMENTS, IF ANY NOT PROVIDED	



0 20' 40'
1"=20'

LEGAL DESCRIPTION:

A PORTION OF THE PARCEL OF LAND DESCRIBED IN THE SPECIAL WARRANTY DEED RECORDED IN OFFICIAL RECORDS BOOK 2340, PAGE 1962 OF THE PUBLIC RECORDS OF ALACHUA COUNTY, FLORIDA, LYING IN SECTION 14, TOWNSHIP 8 SOUTH, RANGE 18 EAST, ALACHUA COUNTY, FLORIDA, BEING MORE PARTICULARLY DESCRIBED AS FOLLOWS:

COMMENCING AT THE NORTHERLY MOST NORTHWEST CORNER OF SAID PARCEL OF LAND DESCRIBED IN OFFICIAL RECORDS BOOK 2340, PAGE 1962, ALSO BEING ON THE SOUTHERLY RIGHT-OF-WAY LINE OF RACHAEL BOULEVARD/COUNTY ROAD 340-A (66 FOOT RIGHT-OF-WAY); THENCE ON THE NORTHERLY LINE OF SAID PARCEL OF LAND THE FOLLOWING 6 (SIX) COURSES AND DISTANCES:
1. SOUTH 83 DEGREES 11 MINUTES 05 SECONDS EAST ON SAID RIGHT-OF-WAY LINE, A DISTANCE OF 64.33 FEET TO A STEEL ROD AND CAP;
2. THENCE DEPARTING SAID RIGHT-OF-WAY LINE SOUTH 06 DEGREES 22 MINUTES 07 SECONDS WEST, A DISTANCE OF 102.44 FEET TO A NAIL AND CAP;
3. THENCE SOUTH 83 DEGREES 29 MINUTES 04 SECONDS EAST, A DISTANCE OF 197.00 FEET TO A NAIL AND CAP AND THE POINT OF BEGINNING OF THE FOLLOWING DESCRIBED PARCEL OF LAND;
4. THENCE NORTH 05 DEGREES 50 MINUTES 37 SECONDS EAST, A DISTANCE OF 101.42 FEET TO A STEEL ROD AND CAP TO A POINT ON SAID RIGHT-OF-WAY LINE;
5. THENCE SOUTH 83 DEGREES 11 MINUTES 05 SECONDS EAST ON SAID RIGHT-OF-WAY LINE, A DISTANCE OF 121.08 FEET TO A STEEL ROD AND CAP;
6. THENCE DEPARTING SAID RIGHT-OF-WAY LINE SOUTH 00 DEGREES 42 MINUTES 22 SECONDS WEST, A DISTANCE OF 102.61 FEET TO A NAIL AND CAP;
THENCE DEPARTING SAID NORTHERLY LINE NORTH 82 DEGREES 54 MINUTES 40 SECONDS WEST, A DISTANCE OF 130.28 FEET TO A NAIL AND CAP AND THE POINT OF BEGINNING.

SAID LANDS LYING AND BEING IN SECTION 14, TOWNSHIP 8 SOUTH, RANGE 18 EAST, CITY OF ALACHUA, ALACHUA COUNTY, FLORIDA AND CONTAINING A TOTAL NET AREA OF 0.293 ACRES MORE OR LESS (12,782 SQUARE FEET MORE OR LESS).

SURVEYOR'S NOTES:

1. BEARINGS AS SHOWN HEREON ARE BASED ON AN ASSUMED BEARING OF SOUTH 83 DEGREES 11 MINUTES 05 SECONDS EAST ON THE SOUTHERLY RIGHT-OF-WAY LINE OF COUNTY ROAD 340-A (RACHAEL ROAD).
2. NO SEARCH OF THE PUBLIC RECORDS WAS MADE BY THE SURVEYOR, THEREFORE, THERE MAY BE RESTRICTIONS OTHER THAN THOSE SHOWN HEREON WHICH MAY BE FOUND IN THE PUBLIC RECORDS OF ALACHUA COUNTY, FLORIDA. EASEMENTS AND BUILDING SETBACK REQUIREMENTS, OTHER THAN THOSE SHOWN, WERE NOT PROVIDED TO THIS SURVEYOR.
3. LEGAL DESCRIPTION SHOWN HEREON WAS WRITTEN BY THIS SURVEYOR.
4. THE PROPERTY DESCRIBED HEREON IS A PORTION OF ALACHUA COUNTY TAX PARCEL 03231-004-000 AS RECORDED IN OFFICIAL RECORDS BOOK 2340, PAGE 1962 OF THE PUBLIC RECORDS OF ALACHUA COUNTY, FLORIDA.
5. ELEVATIONS SHOWN HEREON ARE RELATIVE TO THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD 1988). BENCHMARK OF ORIGIN: NGS MONUMENT I75 F 1 (PID AR0284) ELEVATION=72.40'
6. ADDITIONS AND/OR DELETIONS MADE TO THE FACE SURVEY IS A VIOLATION OF 5J-17 F.A.C. AND MAKES THIS SURVEY NULL AND VOID.
7. THE SCIENTIFIC NAMES OF THE TREES SHOWN HEREON ARE TO THE BEST ABILITY OF THIS SURVEYOR, BUT THIS SURVEYOR DOES NOT PURPORT TO BE A CERTIFIED ARBORIST. THE LOCATION OF THE TREES SHOWN HEREON IS THE APPROXIMATE CENTER OF THE TREE AT GROUND LEVEL AND THE DIAMETER WAS MEASURED AT BREAST HEIGHT - THE EXTENT OF THE TREE CANOPY OR ROOT SYSTEM WAS NOT DETERMINED BY THIS SURVEYOR. IF THE LOCATION OF TREES IN RELATION TO NEARBY PROPERTY LINES IS CRITICAL A MORE DETAILED SURVEY MAY BE REQUIRED.

TREE LEGEND:

ONLY TREES WITH A DIAMETER 8" AND GREATER WERE LOCATED FOR THIS SURVEY.
(DIAMETER MEASURED 4.5' ABOVE GRADE)

LOCATION / SPECIES / DIAMETER AT BREAST HEIGHT

SIZE/COMMON NAME *Genus species*
lo22 = 22" LIVE OAK *Quercus virginiana*
pal7 = 17" PALM *Sabal palmetto*

LEGEND:

A/C	=	AIR CONDITIONER COMPRESSOR PAD
BFPR	=	WATERLINE BACKFLOW PREVENTER
CLF	=	CHAIN LINK FENCE
CONC.	=	CONCRETE
COV.	=	COVERED
DIP	=	DUCTILE IRON PIPE
ELEV	=	ELEVATION
FDC	=	FIRE DEPARTMENT CONNECTION
F.F.EL.	=	FINISHED FLOOR ELEVATION
HBIB	=	HOSE BIB
Inv	=	INVERT
O.R.	=	OFFICIAL RECORDS BOOK
Pg.	=	PAGE
P.O.B.	=	POINT OF BEGINNING
P.O.C.	=	POINT OF COMMENCEMENT
PVC	=	POLY-VINYL CHLORIDE
RCP	=	REINFORCED CONCRETE PIPE
R/W	=	RIGHT OF WAY
VCP	=	VITRIFIED CLAY PIPE
W/	=	WITH
WL	=	WATER LINE
ANCH	=	GUY ANCHOR
BOL	=	TRAFFIC BOLLARD
CO	=	SANITARY OR STORM SEWER CLEAN-OUT
FH	=	FIRE HYDRANT
	=	FOUND NAIL & DISK (ID)
	=	FOUND REBAR & CAP (size, ID)
SV	=	SANITARY VALVE
LT PL	=	LIGHT POLE
ARVMH	=	AIR RELEASE VALVE MANHOLE
PP	=	POWER POLE
	=	SIGN POST
TELPED	=	TELEPHONE PEDESTAL
CTVPED	=	CABLE TELEVISION PEDESTAL
WM	=	WATER METER
WV	=	WATER VALVE
69	=	ELEVATION CONTOUR LINE
X X	=	FENCE LINE
OHPL	=	OVERHEAD POWER LINE
FM	=	SANITARY SEWER FORCE MAIN
UGE	=	UNDERGROUND ELECTRIC LINE
GAS	=	UNDERGROUND GAS LINE
UGT	=	UNDERGROUND TELEPHONE LINE
UGTV	=	UNDERGROUND CABLE TELEVISION LINE
UNK	=	UNKNOWN UNDERGROUND UTILITY LINE
W	=	UNDERGROUND WATER LINE
69.00 ×	=	SPOT ELEVATION (HARD SURFACE)
69.0 ×	=	SPOT ELEVATION (SOFT SURFACE)
	=	WATERLINE EASEMENT (O.R. 2295, Pg. 2573)

UTILITY LOCATION NOTE:

THE LOCATIONS OF UNDERGROUND UTILITIES SHOWN WERE BASED ON ABOVE GROUND FIXTURES, UTILITY COMPANY MAPS AVAILABLE TO THE SURVEYOR, AND ELECTRONIC DETECTION METHODS. UNLESS NOTED OTHERWISE ON THE GRAPHIC PORTION OF THIS SURVEY, NO UTILITIES WERE UNCOVERED TO VERIFY THEIR LOCATION.

NO GUARANTEE IS EITHER EXPRESSED OR IMPLIED BY THE SURVEYOR THAT THE LOCATIONS SHOWN REPRESENT THE ACTUAL LOCATIONS OF THOSE UTILITIES, THE UTILITY TYPE, OR THAT NO OTHER UTILITIES EXIST ON THE SITE.

PRIOR TO THE DESIGN OF UTILITY CONNECTIONS, THE PROPOSED CONNECTION POINTS SHOULD BE EXCAVATED AS NECESSARY TO CONFIRM THEIR EXACT LOCATION, DEPTH AND CHARACTERISTICS.

IN ACCORDANCE WITH FLORIDA STATUTE CHAPTER 556, PRIOR TO ANY EXCAVATION, THE EXCAVATOR SHALL CONTACT SUNSHINE STATE ONE CALL OF FLORIDA AT 1-800-432-4770.

No.	Date	Comment
2	01/07/2021	ADDITIONAL TOPO AT SOUTHWESTERLY LIMITS
1	12/08/2020	REVIEWED TITLE COMMITMENT

FLOOD INSURANCE RATE MAP STATEMENT

THE REAL PROPERTY SHOWN HEREON LIES WITHIN ZONE(s) X (UNSHADED) AS DESIGNATED ON THE FLOOD INSURANCE RATE MAP NUMBER 12001C0145G, COMMUNITY NUMBER: 120884, PANEL: 0145G, EFFECTIVE DATE: 06/16/06, SAID MAP DESCRIBES ZONE(s) X (UNSHADED) AS BEING AREAS DETERMINED TO BE OUTSIDE THE 0.2% ANNUAL CHANCE FLOODPLAIN

eda consultants inc.
Civil Engineering Land Development Surveying
720 S.W. 2nd Ave. Suite 300
GAINESVILLE, FLORIDA 32601
(352) 373-3541
mail@eda-fl.com
www.eda-fl.com

eda consultants inc.
Corporate Authorization No. LB 2389
BY: ALEJANDRO PEREZ P.S.M. 6839

Project No. 2020-0103 S00
Drawn AP
Check AP

NOT VALID WITHOUT THE ORIGINAL SIGNATURE AND THE RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER. COPYRIGHT © 2020

PREPARED FOR: 1) ALCHEM LABORATORIES CORPORATION
2) DARRYL J. COMPANIS, P.A.
3) FIRST AMERICAN TITLE INSURANCE COMPANY
4)

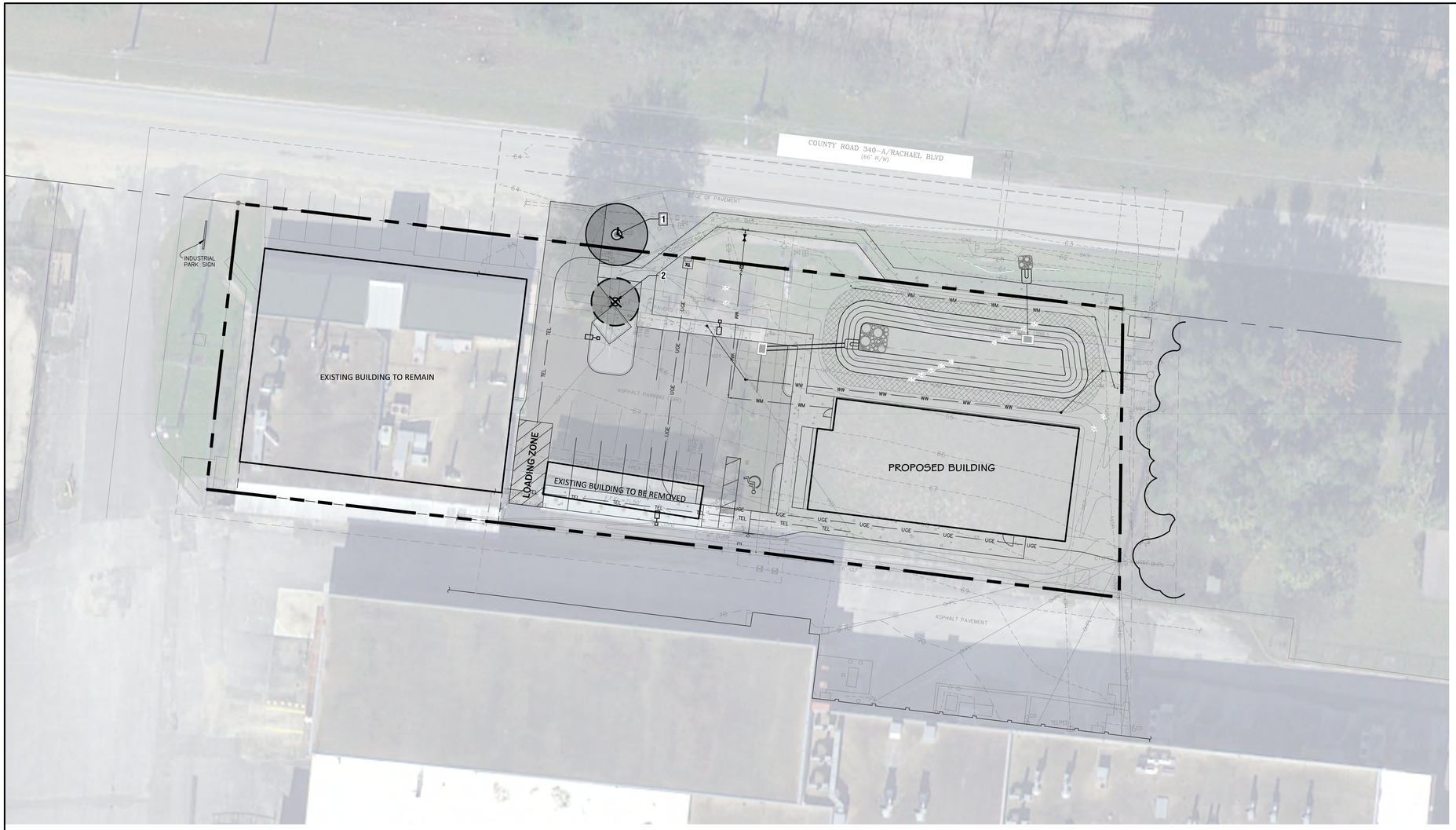
Fieldbook 1025 | Page 60-64 | Survey Date 11/17/2020 | Drawing Completed 11/17/2020 | Revised 01/07/2021

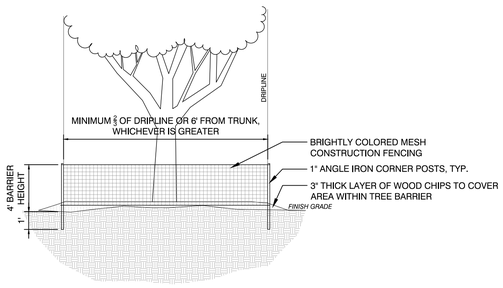
THIS SURVEY MEETS THE STANDARDS OF PRACTICE AS SET FORTH BY THE FLORIDA BOARD OF PROFESSIONAL SURVEYORS AND MAPPERS IN CHAPTER 5J-17, FLORIDA ADMINISTRATIVE CODE, PURSUANT TO SECTION 472.027, FLORIDA STATUTES. THIS SURVEY DEPICTS THE SITE CONDITIONS AS OF: 11/17/2020

Sheet No.:
V-001

Plotted: Jan 07, 2021 - 08:08:12 - dperez

\\server3\survey_Projects\2020\2020-0103 (Alchem Building No. 4)\2020-0103.dwg - Layout2





NOTES:

1. BARRICADES SHALL REMAIN IN PLACE UNTIL SITE CLEARING AND CONSTRUCTION ACTIVITIES ARE COMPLETE, EXCEPT WHERE LAND ALTERATION AND CONSTRUCTION ACTIVITIES ARE APPROVED WITHIN THE PROTECTED AREA. IF APPROVED WITHIN THE PROTECTED AREA, THEN THE PROTECTIVE BARRICADES SHALL ONLY BE REMOVED WHEN ACTIVITIES ARE OCCURRING AND SHALL BE REPLACED UPON COMPLETION OF THE ACTIVITIES.
2. A MINIMUM DISTANCE OF TEN FEET SHALL BE MAINTAINED FROM ALL RETAINED REGULATED TREES WHEN INSTALLING UNDERGROUND UTILITIES. IF THIS RESULTS IN UNREASONABLE HARDSHIP, A SOIL AUGER SHALL BE USED TO TUNNEL UNDER THE ROOT SYSTEMS.
3. NO ATTACHMENTS SHALL BE SECURED TO TREES DESIGNATED TO REMAIN ON SITE.
4. A THREE-INCH LAYER OF MULCH SHALL BE APPLIED OVER THE SURFACE OF ANY EXPOSED ROOTS OF RETAINED REGULATED TREES AND KEPT WET DURING THE SITE CLEARING AND CONSTRUCTION PHASES.
5. NO GRADE CHANGES MAY OCCUR IN THE TREE BARRIER AREA UNLESS OTHERWISE APPROVED BY THE LAND DEVELOPMENT REGULATIONS ADMINISTRATOR OR APPOINTEE.
6. DURING THE SITE CLEARING OR CONSTRUCTION PHASES, THE FOLLOWING ACTIVITIES SHALL BE PROHIBITED WITHIN THE PROTECTIVE AREA UNLESS APPROVED WITH THE APPROPRIATE PROTECTIVE STRATEGIES BY THE CITY DURING SITE PLAN OR CONSTRUCTION PLAN APPROVAL: (1)THE CLEARING OF VEGETATION EXCEPT BY HAND;(2)THE COMPACTION, FILLING, OR REMOVAL OF SOIL DEPOSITS;(3)THE PLACEMENT OF DEBRIS;(4)THE PLACEMENT OR DUMPING OF SOLVENTS OR OTHER CHEMICALS;(5)THE PLACEMENT OR STORAGE OF CONSTRUCTION MATERIALS, MACHINERY OR OTHER EQUIPMENT OF ANY KIND; AND(6)THE USE OF CONCRETE, ASPHALT, OR OTHER PAVING MATERIALS.
7. ANY RETAINED OR RELOCATED TREE SHALL BE REPLACED IN ACCORDANCE WITH THE REQUIREMENTS OF SUBSECTION 6.2.1 (D) (1), IF THE TREE DIES WITHIN ONE YEAR AFTER SITE CLEARING AND CONSTRUCTION.
8. ANY ROOT PRUNING AND/OR PRUNING OF RETAINED REGULATED TREES DURING THE SITE CLEARING OR CONSTRUCTION PHASES SHALL BE DONE IN ACCORDANCE WITH ARBORICULTURAL STANDARDS AND DIRECTLY OVERSEEN BY AN ISA-CERTIFIED ARBORIST.

1 REGULATED TREE PROTECTION

SCALE: NTS P-ED-COA-14

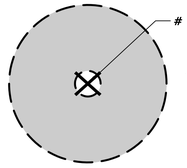
Tree Number	Botanical Name	Common Name	DBH	Remove/ Remain	Health	Credits	Mitigation
1	*Quercus virginiana*	Live Oak	22	Remain	Good	3	-
2	*Sabal palmetto*	Palm	17	Remove	Good	0	1 for 1
						3	1

MITIGATION REQUIRED: 1 TREE
MITIGATION PROVIDED: 1 TREE marked with "M" on sheet L202

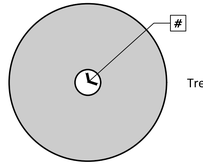

22" OAK TO BE PRESERVED


17" PALM TO BE REMOVED

LEGEND



Tree to be removed



Tree to be preserved

kd design
916 VASSAR STREET
ORLANDO, FL 32804
TEL | 321-277-0826
kaleymariedunlap@gmail.com

Contact: Marsha Swider
(352) 373-3541

REGISTERED LANDSCAPE ARCHITECT
KALEY M. DUNLAP
LA 6667256
STATE OF FLORIDA

NORTH
SCALE: 1" = 20'
0 10 20 40
GRAPHIC SCALE

No.	Date	Comment

Professional Landscape Architect of Record:

Kaley M. Dunlap, PLA — LA6667256
Landscape Architect — Certificate No.

Project No: 20-103

Project phase: CITY SUBMITTAL

Project title: ALCHEM LABORATORIES: BUILDING 4 SITE PLAN CITY OF ALACHUA, FLORIDA

Sheet title: TREE MITIGATION

Designed:	CV
Drawn:	JB
Checked:	CV
Date:	03/24/21

Sheet No.: L101

GENERAL LANDSCAPE NOTES

1. **INVASIVE SPECIES:** REMOVE INVASIVE, NONNATIVE PLANT SPECIES AS LISTED ON THE FLORIDA PROHIBITED AQUATIC PLANTS LIST OR THE FLORIDA NOXIOUS WEED LIST FROM THE SITE PRIOR TO ISSUANCE OF THE CERTIFICATION OF OCCUPANCY. ALL HERBICIDE APPLICATIONS TO CONTROL INVASIVE, NONNATIVE PLANTS IN WETLAND OR UPLAND SET-ASIDE AREAS, INCLUDING BUFFERS, SHALL BE APPLIED BY A CONTRACTOR LICENSED BY THE FLORIDA DEPARTMENT OF AGRICULTURE AND CONSUMER SERVICES, DIVISION OF AGRICULTURAL ENVIRONMENTAL SERVICES, WITH A CURRENT CERTIFICATION IN NATURAL AREAS WEED MANAGEMENT.

2. **ON-SITE LITTER REMOVAL:** REMOVE ALL EXISTING LITTER AND DEBRIS VISIBLE ON THE PROJECT SITE.

3. **PLANTING PREPARATION:** REMOVE ALL CONSTRUCTION DEBRIS, LIMEROCK, GRAVEL, ROAD BEDDING, LITTER, AND OTHER ITEMS POTENTIALLY DAMAGING TO PLANT GROWTH WITHIN PROPOSED LANDSCAPE AND TURF AREAS PRIOR TO PLANTING. MAINTAIN EXISTING GRADES UNLESS OTHERWISE APPROVED BY THE OWNERS REPRESENTATIVE. IF FILL IS REQUIRED, USE A CLEAN, SANDY LOAM WITH pH 5.5-6.5 AND EXCAVATED FROM LOCAL SOURCES AND DEEP PITS SUCH THAT IT IS FREE OF WEEDS, SEEDS, LITTER, TOXINS, AND OTHER ITEMS HARMFUL TO PLANTING.

4. **PRUNING:** IF TREE OR ROOT PRUNING IS REQUIRED, THESE ACTIVITIES SHALL BE PERFORMED BY AN ARBORIST CERTIFIED BY THE INTERNATIONAL SOCIETY OF ARBORICULTURAL (ISA). TREE PRUNING SHALL BE DONE IN ACCORDANCE WITH THE MOST CURRENT VERSION OF THE AMERICAN NATIONAL STANDARD FOR TREE CARE OPERATIONS "TREE, SHRUB AND OTHER WOODY PLANT MAINTENANCE" (ANSI 300) AND "PRUNING, TRIMMING, REPAIRING, MAINTAINING, AND REMOVING TREES, AND CUTTING BRUSH - SAFETY REQUIREMENTS" (ANSI Z133). NO MORE THAN 25 PERCENT OF THE CROWN SHOULD BE REMOVED AT ONE TIME. ON YOUNG TREES, LIMB REMOVAL SHALL LEAVE NO MORE THAN 33 PERCENT OF THE TRUNK BARE OF BRANCHES.

PLANT MATERIAL QUALITY: PLANT MATERIAL TO COMPLY WITH "FLORIDA #1" GRADE QUALITY STANDARD OR BETTER AS DEFINED AND SPECIFIED WITHIN THE LATEST EDITION OF THE "FLORIDA GRADES AND STANDARDS FOR NURSERY PLANTS" AS PUBLISHED BY THE FLORIDA DEPARTMENT OF AGRICULTURE AND CONSUMER SERVICES, DIVISION OF PLANT INDUSTRIES. PROVIDE HEALTHY, VIGOROUS STOCK GROWN IN A RECOGNIZED NURSERY IN ACCORDANCE WITH GOOD HORTICULTURAL PRACTICES AND FREE OF DISEASE, PESTS, AND DEFECTS.

5. **NEW TREES:** NEW TREES SHALL BE INSTALLED TO REPLACE HEALTHY REGULATED TREES REMOVED PURSUANT TO SECTION 6.2.1. REGULATED TREES SHALL BE REPLACED ON A ONE-FOR-ONE BASIS. HEALTHY HERITAGE AND CHAMPION TREES REMOVED AS PROVIDED HEREIN SHALL BE REPLACED ON AN INCH-FOR-INCH BASIS. REPLACEMENT TREES SHALL BE GRADED FLORIDA NO. 1 OR BETTER, AS OUTLINED IN THE MOST RECENT PUBLICATION OF THE STATE OF FLORIDA, DEPARTMENT OF AGRICULTURE AND CONSUMER SERVICES, DIVISION OF PLANT INDUSTRY, GRADES AND STANDARDS FOR NURSERY PLANTS, PART II, PALMS AND TREES. THE TERM "HEALTHY," AS STATED HEREIN, MEANS "GOOD" OR BETTER PER THE STANDARD DEFINITION FROM THE INTERNATIONAL SOCIETY OF ARBORICULTURE (ISA) STATING THE TREE HAS NO MAJOR STRUCTURAL PROBLEMS, NO SIGNIFICANT DAMAGE DUE TO DISEASE OR PESTS, NO SIGNIFICANT MECHANICAL DAMAGE, A FULL BALANCE CROWN, AND NORMAL TWIG CONDITION AND VIGOR FOR ITS SPECIES. PALM TREES MAY BE UTILIZED AS REPLACEMENT TREES BUT AT AN INCREASED RATIO OF 3:1 REPLACEMENT AND SHALL BE A MINIMUM OF EIGHT FEET TALL AT THE TIME OF PLANTING.

6. **DURING DEVELOPMENT:** DURING DEVELOPMENT OR SITE ALTERATION ACTIVITIES, THE FOLLOWING STANDARDS SHALL BE MET:
   - 6.1. PROTECTIVE BARRICADES SHALL BE PLACED TO DEFINE A PROTECTIVE AREA AROUND EXISTING TREES TO REMAIN. BARRIERS SHALL BE PLACED AROUND ALL REGULATED TREES AT A MINIMUM OF TWO-THIRDS OF THE AREA OF THE DRIPLINE OF THE TREE OR STAND OF TREES OR AT SIX FEET FROM THE TRUNK OF THE TREE, WHICHEVER IS GREATER. PROTECTIVE BARRICADES SHALL BE PLACED AT THE DRIPLINE OF ALL HERITAGE TREES, CHAMPION TREES, AND REGULATED PALM TREES. PROTECTIVE BARRICADES SHALL BE PLACED AROUND ALL TREES TO BE RETAINED ON THE SITE AND SHALL REMAIN IN PLACE UNTIL SITE CLEARING AND CONSTRUCTION ACTIVITIES ARE COMPLETE, EXCEPT WHERE LAND ALTERATION AND CONSTRUCTION ACTIVITIES ARE APPROVED WITHIN THE PROTECTED AREA. IF LAND ALTERATION AND CONSTRUCTION ACTIVITIES ARE APPROVED WITHIN THE PROTECTED AREA, THEN THE PROTECTIVE BARRICADES SHALL ONLY BE REMOVED WHEN ACTIVITIES ARE OCCURRING. PROTECTIVE BARRICADES SHALL BE REPLACED UPON COMPLETION OF THE ACTIVITIES WITHIN THE PROTECTED AREA. PROTECTIVE BARRICADES SHALL BE AT LEAST FOUR FEET HIGH AND CONSTRUCTED OF EITHER WOODEN CORNER POSTS AT LEAST FOUR INCHES IN WIDTH BY FOUR INCHES IN DEPTH BY FOUR FEET IN HEIGHT BURIED ONE FOOT DEEP WITH AT LEAST TWO COURSES OF WOODEN SIDE SLATS AT LEAST TWO INCHES IN WIDTH BY FOUR FEET WITH COLORED FLAGGING OR COLORED MESH CONSTRUCTION FENCING ATTACHED OR CONSTRUCTED OF ONE INCH ANGLE IRON CORNER POSTS WITH BRIGHTLY COLORED MESH CONSTRUCTION FENCING ATTACHED.
   - 6.2. A MINIMUM DISTANCE OF TEN FEET SHALL BE MAINTAINED FROM ALL RETAINED REGULATED, HERITAGE, AND CHAMPION TREES WHEN INSTALLING UNDERGROUND UTILITIES. IF THIS RESULTS IN UNREASONABLE HARDSHIP, A SOIL AUGER SHALL BE USED TO TUNNEL UNDER THE ROOT SYSTEMS.
   - 6.3. NO ATTACHMENTS SHALL BE SECURED TO TREES DESIGNATED TO REMAIN ON SITE.
   - 6.4. A THREE-INCH LAYER OF MULCH SHALL BE APPLIED OVER THE SURFACE OF ANY EXPOSED ROOTS OF RETAINED REGULATED, HERITAGE, AND CHAMPION TREES AND KEPT WET DURING THE SITE CLEARING AND CONSTRUCTION PHASES.
   - 6.5. RAISING OR LOWERING OF GRADE WITHIN THE DRIPLINE OF EXISTING TREES TO REMAIN SHALL NOT BE PERMITTED UNLESS OTHERWISE APPROVED BY THE LAND DEVELOPMENT REGULATIONS ADMINISTRATOR OR APPOINTEE.
   - 6.6. DURING THE SITE CLEARING OR CONSTRUCTION PHASES, THE FOLLOWING ACTIVITIES SHALL BE PROHIBITED WITHIN THE PROTECTIVE AREA UNLESS APPROVED WITH THE APPROPRIATE PROTECTIVE STRATEGIES BY THE CITY DURING SITE PLAN OR CONSTRUCTION PLAN APPROVAL:
     - 6.6.1. THE CLEARING OF VEGETATION EXCEPT BY HAND;
     - 6.6.2. THE COMPACTION, FILLING, OR REMOVAL OF SOIL DEPOSITS;
     - 6.6.3. THE PLACEMENT OF DEBRIS;
     - 6.6.4. THE PLACEMENT OR DUMPING OF SOLVENTS OR OTHER CHEMICALS;
     - 6.6.5. THE PLACEMENT OR STORAGE OF CONSTRUCTION MATERIALS, MACHINERY OR OTHER EQUIPMENT OF ANY KIND; AND
     - 6.6.6. THE USE OF CONCRETE, ASPHALT, OR OTHER PAVING MATERIALS.
   - 6.7. ANY RETAINED OR RELOCATED TREE SHALL BE REPLACED IN ACCORDANCE WITH THE REQUIREMENTS OF SUBSECTION 6.2.1(D)(1), IF THE TREE DIES WITHIN ONE YEAR AFTER SITE CLEARING AND CONSTRUCTION.
   - 6.8. ANY ROOT PRUNING AND/OR PRUNING OF RETAINED REGULATED, HERITAGE, AND CHAMPION TREES DURING THE SITE CLEARING OR CONSTRUCTION PHASES SHALL BE DONE IN ACCORDANCE WITH ARBORICULTURAL STANDARDS AND DIRECTLY OVERSEEN BY AN ISA-CERTIFIED ARBORIST.

7. **SOD:** ALL UNPAVED AND DISTURBED AREAS OUTSIDE OF PROPOSED TREE, SHRUB, AND GROUNDCOVER PLANTING AREAS SHALL BE SODDED. SEE SHEET L202.

8. **MULCHING:** THREE INCHES OF MULCH, SETTLED, SHOULD COVER TREE RING PLANTING AREAS AND ALL SHRUB AND GROUNDCOVER PLANTING AREAS. MULCH SHOULD BE NO DEEPER THAN ONE INCH OVER THE TOP OF TREE ROOTBALLS, USE PINESTRAW MULCH IN ALL AREAS. MULCH TO BE FREE OF DEBRIS, STICKS, AND CONES. NO PLASTIC OR NON-BIODEGRADABLE WEED CLOTH OR SURFACE COVERS TO BE USED.

9. **IRRIGATION:** THE IRRIGATION SYSTEM USED FOR LANDSCAPE SHALL BE DESIGNED TO CORRELATE TO THE WATER USE PLANT ZONES ESTABLISHED IN THE LANDSCAPE DESIGN. A PRELIMINARY IRRIGATION PLAN SHALL BE SUBMITTED AT TIME OF SITE PLAN OR CONSTRUCTION PLAN APPLICATION SUBMISSION WITH A NOTE THAT THE FOLLOWING STANDARDS FOR IRRIGATING THE SITE SHALL BE USED IN THE DESIGN OF THE SYSTEM:
   - IN THE HIGH WATER USE ZONE: ALL PORTIONS OF THE ZONE SHALL BE PROVIDED WITH AN AUTOMATIC IRRIGATION SYSTEM WITH LOW VOLUME HEADS THAT ARE TIMED TO PRODUCE NO MORE THAN TWO INCHES OF WATER PER WEEK IN ESTABLISHED LANDSCAPES.
   - IN THE MODERATE WATER USE ZONE: ALL PORTIONS OF THE ZONE SHALL BE PROVIDED WITH A READILY AVAILABLE WATER SUPPLY WITHIN 100 FEET, TO SUPPLEMENT NATURAL RAINFALL WHEN NEEDED, AND TO HELP ENSURE THAT WATERING WILL TAKE PLACE UNTIL THE PLANT MATERIAL IS ESTABLISHED.
   - IN LOW WATER USE ZONES: ALL PORTIONS OF THE ZONE SHALL BE PROVIDED WITH A READILY AVAILABLE WATER SUPPLY WITHIN 50 FEET, TO SUPPLEMENT NATURAL RAINFALL WHEN NEEDED, AND TO HELP ENSURE THAT WATERING WILL TAKE PLACE UNTIL THE PLANT MATERIAL IS ESTABLISHED.
   - RAIN SENSOR DEVICES SHALL BE REQUIRED ON ALL AUTOMATIC IRRIGATION SYSTEMS TO AVOID IRRIGATION DURING PERIODS OF SUFFICIENT RAINFALL.
   - THE IRRIGATION SYSTEM SHALL BE DESIGNED TO MINIMIZE IRRIGATION OVERTHROW ONTO IMPERVIOUS SURFACES AND TO NEGATE ANY PONDING EFFECTS.
   - . A FINAL IRRIGATION PLAN SHALL BE SUBMITTED AT THE TIME OF BUILDING PERMIT APPLICATION OR CONSTRUCTION PLAN APPLICATION AND SHALL BE REVIEWED TO ENSURE THAT THE DESIGN STANDARDS FOUND IN THIS SUBSECTION HAVE BEEN MET.

11. **RESPONSIBILITY:** THE OWNER OF THE PARCEL SHALL BE RESPONSIBLE FOR THE MAINTENANCE OF ALL PRESERVED, RELOCATED, OR REPLACEMENT TREES. ALL TREES WILL BE INSPECTED BY AN ARBORIST, FORESTER, OR REGISTERED LANDSCAPE ARCHITECT, HIRED BY THE OWNER, WITHIN SIX MONTHS AFTER PLANTING TO ENSURE THE TREES ARE SURVIVING IN A HEALTHY CONDITION. A CERTIFIED REPORT SHALL BE PROVIDED TO THE LAND DEVELOPMENT REGULATIONS ADMINISTRATOR DESCRIBING THE CONDITION OF TREES. TREES FOUND TO BE IN DECLINING CONDITION SHALL BE REPLACED BY THE OWNER OF THE PARCEL WITHIN 30 DAYS OF SUBMITTAL OF THE REPORT. IF REPLACEMENT IS NECESSARY, THERE SHALL BE A REINSPECTION REPORT SUBMITTED WITHIN SIX MONTHS AFTER THE REPLACEMENT REPLANTING.

12. **MAINTENANCE RECOMMENDATIONS:** PER CITY OF ALACHUA ULDR, ARTICLE 6., SEC. 6.2.2.D.7.B.vii. A MAINTENANCE SCHEDULE MUST BE SUBMITTED ADDRESSING THE FOLLOWING:

- **CHECKING:** CHECK DISTRIBUTION AND SPRINKLER FUNCTION OF THE IRRIGATION SYSTEM PERIODICALLY AND MONTHLY AT A MINIMUM. MAKE REPAIRS AS NEEDED. ADJUST SPRAY HEADS TO MINIMIZE OVERTHROW ONTO PAVEMENT.

- **RESETTING:** WATER AT A FREQUENCY AS ALLOWED BY THE LOCAL JURISDICTIONS INCLUDING CITY OF ALACHUA AND SUWANNEE RIVER WATER MANAGEMENT DISTRICT. WATER AT A DURATION AS INDICATED IN IRRIGATION DESIGN INDICATED FOR EACH ZONE. ADJUST FREQUENCY AND DURATION AS NEEDED AND AS DIRECTED BY REGULATING AGENCIES.

- **REMULCHING:** MAINTAIN THE DEPTH OF MULCH IN ALL PLANTING AREAS AS SPECIFIED IN PLANS. REPLENISH A MINIMUM OF TWICE A YEAR OR AS NEEDED. DEPTH SHALL BE MEASURED WHEN MULCH IS SETTLED. REMOVE STICKS, CONES, AND DEBRIS FROM MULCH PRIOR TO PLACING. UTILIZE MULCH TYPE AS SPECIFIED IN PLANS. REMOVE DEBRIS FROM MULCH AREAS PRIOR TO REMULCHING.

- **FERTILIZING:** APPLY A SLOW-RELEASE, GRANULAR FERTILIZER WITHIN ALL PLANTING AREAS. APPLY IN A MANNER AND AT A RATE AND FREQUENCY AS RECOMMENDED BY THE MANUFACTURER AND AS PER CITY OF ALACHUA REGULATIONS, IF APPLICABLE. DO NOT APPLY ON LEAVES OR BRANCHES. DO NOT ALLOW FERTILIZER TO FALL ON PAVEMENT OR NEAR DRAINS. FERTILIZER TO MEET SPECIFICATION FOR COMMERCIAL APPLICATIONS.

- **WEEDING:** KEEP PLANTING BEDS, TREE RINGS, AND MULCH AREAS FREE OF UNDESIRABLE VEGETATION. APPLY A PRE-EMERGENT HERBICIDE AS APPROVED BY THE OWNER TO PLANTING AREAS PRIOR TO THE PLANTING SEASON IN EARLY SPRING. MANUALLY REMOVE WEEDS AND OTHER UNDESIRABLE SPECIES OVER 12" IN HEIGHT. UTILIZE A POST-EMERGENT HERBICIDE AS APPROVED BY THE REGULATING AGENCIES FOR WEEDS LESS THAN 12". WEED MONTHLY, MINIMUM.

- **PRUNING:** MAINTAIN PLANTINGS TO MAINTAIN THE CLEAR SIGHT WINDOW AS DEFINED BY THE REGULATING AGENCIES OF THE ADJACENT ROADWAYS. PRUNING TO BE DONE IN ACCORDANCE WITH THE MOST CURRENT VERSION OF THE AMERICAN NATIONAL STANDARD "TREE, SHRUB, AND OTHER WOODY PLANT MAINTENANCE" (ANSI A300). PRUNE FLOWERING PLANTS FOLLOWING THEIR BLOOMING PERIOD. PRUNE OTHER PLANTS IN EARLY SPRING PRIOR TO THEIR FIRST FLUSH. REMOVE DEAD OR INJURED BRANCHES. HEAD BRANCHES IF NEEDED TO MAINTAIN HEIGHT AND TRIM AT DIFFERENT LENGTHS FOR A MORE NATURAL FORM, WHERE DESIRED BY THE OWNER. DO NOT PRUNE SHRUBS INTO BALLS OR OTHER SHAPES AND INSTEAD MAINTAIN IN THEIR NATURAL FORM, AS DIRECTED BY THE OWNER.

- **MOWING:** MOW TURF AT A FREQUENCY TO MAINTAIN TURF IN A VIGOROUS AND HEALTHY STATE WITHOUT APPEARING UNKEMPT. REMOVE UNWANTED VEGETATION. AND DEBRIS FROM TURF AREAS DURING MOWING ACTIVITIES.

13. **EXISTING UTILITIES:** LOCATE ALL UTILITIES PRIOR TO COMMENCEMENT OF WORK. CALL 811.

811 Know what's **below.** **Call** before you dig.

XERISCAPE CRITERIA

Per City of Alachua ULDR, Article 6, Sec. 6.2.2.D.7.b. Xeriscape Standards

Required: Preserve and maintain native vegetation
Provided: ***Site plan preserves existing tree buffer (east) and existing live oak at entrance driveway.***

Required: Group plant material into water use zones based on water needs.
Provided: ***Plant groupings have similarly low water needs and can thus be irrigated accordingly. Irrigation plan to be provided by others.***

Required: Ensure plant types are appropriate for soils.
Provided: ***All species provided are Florida Friendly species suited to the proposed site conditions.***

Required: Use of mulch.
Provided: ***Mulch is proposed for all planting areas, including trees outside of main landscape beds.***

Required: Use of low water use plants.
Provided: ***Plants with high drought tolerances and low water requirements are proposed.***

Required: Efficient irrigation.
Provided: ***Irrigation plan to be provided by others.***

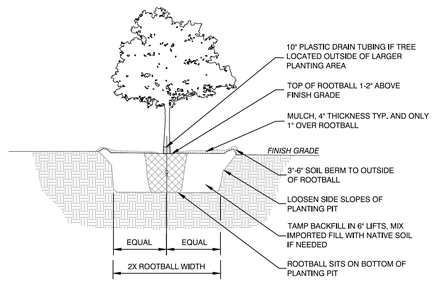
Required: Maintenance.
Provided: ***Maintenance is required as per general landscape notes 12.***

Required: Drip irrigation or emitters.
Provided: ***Irrigation Plan to be provided by others.***

FLORIDA FRIENDLY LANDSCAPING PRACTICES

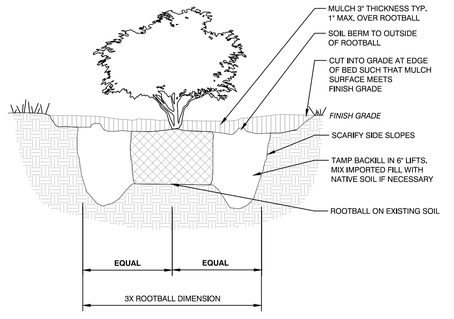
Per City of Alachua ULDR Article 6, Sec. 6.2.2.C.8 Florida Friendly Landscaping.

1. Right Plant Right Place. ***Proposed plants are native species or species that are well adapted to the site conditions. Plants are chosen and placed so as to avoid utility, vehicular and pedestrian conflicts. Preference has been given to species that are well-suited for the site microclimate and the industrial use of the site. Shrubs are varied to foster diversity, create visual interest and attract a variety of wildlife. Plants are a minimum of 2' from all building facades to reduce maintenance concerns.***
2. Water Efficiently. ***Irrigation Plan to be provided by others.***
3. Fertilize Appropriately. ***Fertilizer and application as required as per general landscape notes 12.***
4. Mulch. ***Mulch is to be provided in all planting areas as shown by the hatched bedlines.***
5. Attract Wildlife. ***Proposed plants include species that attract pollinators. All plants are well-adapted to the region.***
6. Manage Yard Pests Responsibly. ***Proposed pest management will be per best management practices.***
7. Recycle Yard Waste. ***Yard waste to be disposed of as required within Alachua County and the City of Alachua.***
8. Reduce Stormwater Runoff. ***Stormwater management meets regulating agency requirements. See civil plans.***
9. Protect the Waterfront. ***Wetland areas are protected as per Water Management Disctrict and City of Alachua requirements. See civil plans.***



NOTES:
1. ENSURE PLANTING PIT DRAINS WELL PRIOR TO INSTALLATION.
2. REMOVE TREE FROM CONTAINER OR REMOVE UPPER ⅓ OF BURLAP AND WIRE MESH FOR B&B TREES.
3. SLICE ROOTBALL VERTICALLY ON OUTSIDE EDGES OF AND AROUND ROOTBALL.
4. TREE TO BE PLUMB. STAKE TREE, SUBMIT TREE STAKING DETAILS AND LAYOUT FOR APPROVAL.

1 TREE PLANTING DETAIL
SCALE: N.T.S. P-ED-01



2 SHRUB PLANTING DETAIL
SCALE: N.T.S. P-ED-04

kd design
916 VASSAR STREET
ORLANDO, FL 32804
TEL | 321-277-0826
kaleymariedunlap@gmail.com

Contact: Marsha Swider
(352) 373-3541

REGISTERED LANDSCAPE ARCHITECT KALEY M. DUNLAP LA 6667256 STATE OF FLORIDA

NORTH
SCALE: 1" = 20'
0 10 20 40
GRAPHIC SCALE

No.	Date	Comment

Professional Landscape Architect of Record:

Kaley M. Dunlap, PLA — Landscape Architect
LA6667256 — Certificate No.

Project No: 20-103

Project phase: CITY SUBMITTAL

Project title: ALCHEM LABORATORIES: BUILDING 4 SITE PLAN CITY OF ALACHUA, FLORIDA

Sheet title: LANDSCAPE NOTES AND DETAILS

Designed:	CV
Drawn:	JB
Checked:	CV
Date:	03/24/21

Sheet No.: L200

CORRECTION OF OTHER NONCONFORMITIES

Per City of Alachua LDR, Article 8., Sec. 8.7.2 Additions and expansions. Additions to structures on nonconforming sites shall require correction of existing on-site nonconforming off-street parking and landscaping in accordance with this section.

A. Expansion of 50 percent or less of gross square footage over five years. Expansions in any continuous five-year period, that result in a 50 percent or less increase in the gross square footage of the existing structure (measured at the beginning of the five-year period), or the addition of new or increased areas for outdoor operations/storage/display which would result in a 50 percent or less increase of the gross square footage for such outdoor operations/storage/display, require that a corresponding percentage of the off-street parking and landscaping standards of these LDRs (see Section 6.1, Off-street parking and loading standards, and Section 6.2, Tree protection/landscape/xeriscape standards) be installed on the site, until the site achieves 100 percent compliance. (For example, if the addition is 25 percent of the area of the existing structure and the site contains only 50 percent of the required landscaping, 25 percent of the required landscaping for the entire site must be provided, thereby bringing the landscaping on the site to 75 percent of the total required.) Existing landscaping on the site shall be retained or replaced but shall not count toward the required percentage of new landscaping.

EXISTING BUILDING AREA	
DESCRIPTION	SQUARE FOOTAGE (S.F.)
BUILDING 1:	7,191 S.F.
BUILDING TO BE REMOVED:	700 S.F.
TOTAL:	7,891 S.F.

PROPOSED BUILDING AREA	
DESCRIPTION	SQUARE FOOTAGE (S.F.)
BUILDING 1 EXPANSION:	3,788 S.F.

3,788 S.F. / 7,891 S.F. = 48 %

REQUIRED: *48% of the required landscaping for the entire site*

PROVIDED: *Refer to the following Landscape Standards Calculations*

SITE LANDSCAPING REQUIREMENTS

Per City of Alachua, ULDR, Article 6., Sec. 6.2.2.(D)(1)(d) Business uses found in the Industrial Services, Manufacturing and Production, and Warehouse and Freight Movement Use Categories.

CANOPY REQUIREMENTS

CANOPY Per 6.2.2.D.(1)(d)(i)

Primary/Street facing side of the primary structure: one canopy tree per acre
Rear of the primary structure: 2 canopy trees per acre
Sides (East and West) of the primary structure: 2 canopy trees per acre

BUILDING FACADE Per 6.2.2.D.(1)(d)(i)

Primary structure: An additional canopy tree per 150 linear feet of facade. It is encouraged that the tree be located so that it may provide shade on the structure during the summer afternoon.

CANOPY REQUIREMENTS

LOCATION	REQUIREMENT	48% REQUIREMENT	PROVIDED
PRIMARY / STREET-FACING SIDE	1 canopy tree / acre *1 canopy trees x 0.75 acres =* ***1 canopy trees required***	*1 canopy tree x 48% = 0.47* *0 trees required*	**0 trees**
SIDE FACADE (EAST)	2 canopy trees / acre *2 trees x 0.75 acres = 1.5 trees =* ***2 canopy trees required***	*2 canopy trees x 48% = 0.96* *1 tree required*	**1 tree**
SIDE FACADE (WEST)	2 canopy trees / acre *2 trees x 0.75 acres = 1.5 trees =* ***2 canopy trees required***	*2 canopy trees x 48% = 0.96* *1 tree required*	**1 tree**
REAR FACADE	2 canopy trees / acre *2 trees x 0.75 acres = 1.5 trees =* ***2 canopy trees required***	*2 canopy trees x 48% = 0.96* *1 tree required*	**1 tree**
NORTH FACADE	1 canopy tree / 150 LF of facade *88 LF / 150 LF = .58 x 1 tree =* ***1 canopy tree required***	*1 canopy tree x 48% = 0.48* *0 trees required*	**0 trees**
EAST FACADE	1 canopy tree / 150 LF of facade *41 LF / 150 LF = .27 x 1 tree =* ***0 canopy trees required***	*0 canopy trees x 48% = .48* *0 trees required*	**0 trees**
SOUTH FACADE	1 canopy tree / 150 LF of facade *94 LF / 150 LF = .63 x 1 tree =* ***1 canopy tree required***	*1 canopy tree x 48% = 0.48* *0 trees required*	**0 trees**
WEST FACADE	1 canopy tree / 150 LF of facade *41 LF / 150 LF = .27 x 1 tree =* ***0 canopy trees required***	*0 canopy trees x 48% = .48* *0 trees required*	**0 trees**

ALTERNATIVE COMPLIANCE REQUEST
See attached Alternative Compliance Memo dated 3/25/2021.

UNDERSTORY REQUIREMENTS

UNDERSTORY Per 6.2.2.D.(1)(d)(ii)

Primary structure: Six ornamental/understory trees per acre, with 50 percent planted in front of the structure and 25 percent planted on each side.

UNDERSTORY REQUIREMENTS

LOCATION	REQUIREMENT	48% REQUIREMENT	PROVIDED
FRONT (50%)	6 understory trees / acre (50%) *6 trees x 0.75 acres = 4.5 trees* *4.5 trees x 50% =* ***2 understory trees required***	*2 trees x 48% = 0.96* *1 tree required*	**1 tree**
SIDE - EAST (25%)	6 understory trees / acre (25%) *6 trees x 0.75 acres = 4.5 trees* *4.5 trees x 25% =* ***1 understory trees required***	*1 tree x 48% = 0.48* *0 trees required*	**0 trees**
SIDE - WEST (25%)	6 understory trees / acre (25%) *6 trees x 0.75 acres = 4.5 trees* *4.5 trees x 25% =* ***1 understory trees required***	*1 tree x 48% = 0.48* *0 trees required*	**0 trees**

SHRUB REQUIREMENTS

Primary structure front and side facades Per 6.2.2.D.(1)(d)(iii): A row of shrubs planted along front and side facades of the primary structure, with consideration given to access to utility meters or mechanical equipment. Shrubs shall not be planted directly against the structure, but a minimum of two feet from the facade to facilitate adequate air circulation.

PROVIDED: A row of shrubs planted along front and side facades of the primary structure with minimum 2 foot separation from the front of the facade to facilitate adequate air circulation.

GRASSING REQUIREMENTS

Per 6.2.2.D.(1)(d)(iv)

A combination of solid sod, seeding and sprigs to cover 100 percent of the lot site disturbed by construction activities. Areas of native vegetation do not have to be sodded. An area within 20 feet of the front building facade shall be sodded with other disturbed areas to be sodded, seeded or sprigged. ***See sheet L300.***

PARKING LOT LANDSCAPING REQUIREMENTS

INTERIOR LANDSCAPING REQUIREMENTS

Per 6.2.2.D.(2)(a)(iii), Each interior planting area shall contain locally adapted trees and shrubs at the following rate:

a. Trees shall be required at the minimum rate of one canopy or ornamental/understory tree for every 1,800 square feet, or portion thereof, of the total parking lot area.
b. Shrubs shall be required at the minimum rate of ten shrubs per landscaped island.

PARKING LOT AREA	REQUIREMENT	48% REQUIREMENT	PROVIDED
5,929 SQ. FT **4 landscaped islands**	1 tree / 1,800 sq. ft = 5,929 *sq. ft. / 1,800 sq. ft* = ***3 trees required*** 10 shrubs / landscaped island = *10 shrubs x 3 islands* = ***30 shrubs required***	*3 trees x 48% = 1 trees required* *30 x 48% = 14 shrubs required*	***1 tree*** ***25 shrubs***

PARKING LOT BUFFER REQUIREMENTS

Per 6.2.2(D)(2)(b)(iii), The buffer for the parking lot shall be a minimum of five feet, and an average of seven feet in width along the entire length of the perimeter of the parking lot. The width of the buffer may vary to allow for design creativity, as long as a minimum width of five feet is maintained.

Per 6.2.2(D)(2)(b)(iv), The parking lot buffer shall have a minimum of:

a. Four canopy trees for every 100 lineal feet of buffer.
b. Two understory/ornamental trees per 100 lineal feet of buffer.
c. A continuous opaque screen of shrub material parallel to the parking lot area. The shrubs may be arranged in a linear or curvilinear pattern, as long as the screen does not have any visual breaks. The parking lot screen shall reach 36 inches in height within three years.
d. Other landscaping materials provided to meet any other requirement of this section may be considered a part of and count toward the requirement parking lot buffer, if such materials are located within 30 feet of the parking lot area.

Per 6.2.2(D)(2)(b)(v), Any area of the parking lot buffer that is not covered in trees or shrubs shall be planted in ground cover, turfgrass or mulch.

PARKING LOT PERIMETER	TREES & SHRUBS REQUIRED	48% REQUIREMENT	PROVIDED
107 LINEAL FEET	*134 lineal feet / 100 lineal feet = 1.34 lineal feet* *1.34 lineal feet x 4 canopy trees* ***= 5 canopy trees required*** *1.34 lineal feet x 2 understory trees* ***= 3 understory trees required*** ***Continuous opaque screen of shrub material required***	*5 trees x 48% = 2 canopy trees required* *2.14 x 48% = 1 understory tree required*	***2 canopy trees*** ***3 understory trees*** ***Continuous opaque screen of shrub material provided***

SITE PERIMETER LANDSCAPE BUFFER REQUIREMENTS

PERIMETER BUFFER REQUIREMENTS

Per 6.2.2.D.(3) Trees required for buffering shall be distributed evenly along each perimeter, except that up to 33% of the required number of trees may be clustered.

Where a developing parcel is adjacent to a vacant parcel, the developing parcel may provide one-half of the perimeter buffer required adjacent to the vacant land in accordance with Table 6.2-2, Buffer Class Application. Determination of the required buffer width and type shall be based upon the highest use classification available for the vacant property based upon its current zoning.

LOCATION AND BUFFER TYPE	REQUIREMENT	48% REQUIREMENT	PROVIDED
NORTH IG 5' BUFFER, TYPE A OPTION 1	One canopy tree every 60 linear feet, plus one understory or ornamental tree per 60 linear feet *317 lf /60 =* ***5 canopy, 5 understory trees required***	*5 trees x 48% = 2 canopy,* *2 understory trees required*	**2 canopy trees; 2 understory trees**
SOUTH ILW 5' BUFFER, TYPE A OPTION 1	One canopy tree every 60 linear feet, plus one understory or ornamental tree per 60 linear feet *327 lf /60 =* ***5 canopy, 5 understory trees required***	*5 trees x 48% = 2 canopy,* *2 understory trees required*	**2 canopy trees; 3 understory trees**
EAST ILW 5' BUFFER, TYPE A OPTION 1	One canopy tree every 60 linear feet, plus one understory or ornamental tree per 60 linear feet *103 lf /60 =* ***2 canopy, 2 understory trees required***	*2 trees x 48% = 1 canopy,* *1 understory tree required*	**Existing buffer to remain**
WEST ILW 5' BUFFER, TYPE A OPTION 1	One canopy tree every 60 linear feet, plus one understory or ornamental tree per 60 linear feet *103 lf /60 =* ***2 canopy, 2 understory trees required***	*2 trees x 48% = 1 canopy,* *1 understory tree required*	**1 canopy tree; 3 understory trees to remain**

ALTERNATIVE COMPLIANCE REQUEST
See attached Alternative Compliance Memo dated 3/25/2021.

kd design
916 VASSAR STREET
ORLANDO, FL 32804
TEL | 321-277-0826
kaleymariedunlap@gmail.com

Contact: Marsha Swider
(352) 373-3541

REGISTERED LANDSCAPE ARCHITECT
KALEY M. DUNLAP
LA 6667256
STATE OF FLORIDA

NORTH
SCALE: 1" = 20'
0 10 20 40
GRAPHIC SCALE

No.	Date	Comment

Professional Landscape Architect of Record:

Kaley M. Dunlap, PLA — LA6667256
Landscape Architect — Certificate No.

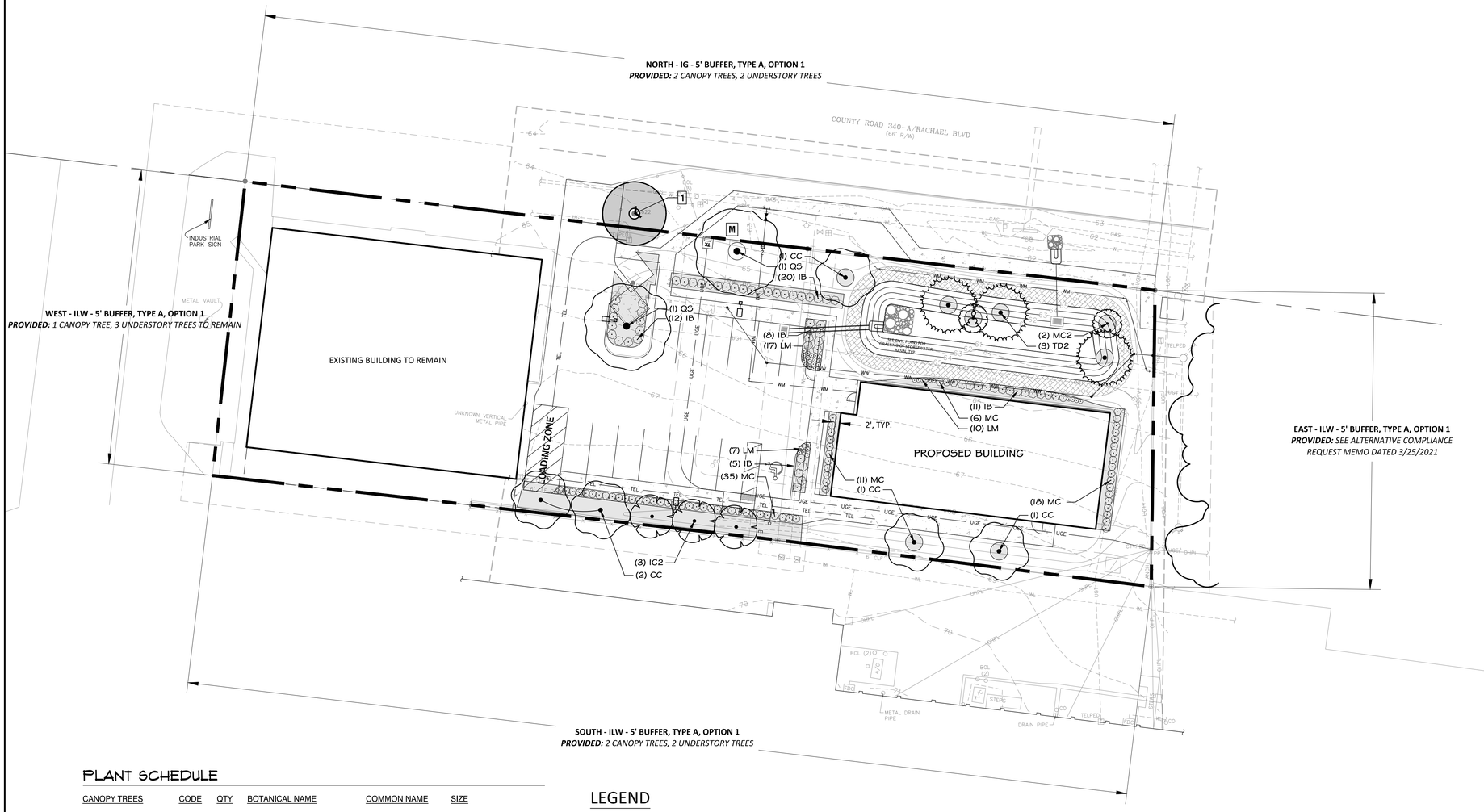
Project No: 20-103

Project phase: CITY SUBMITTAL

Project title: ALCHEM LABORATORIES: BUILDING 4 SITE PLAN CITY OF ALACHUA, FLORIDA

Sheet title: LANDSCAPE CALCULATIONS

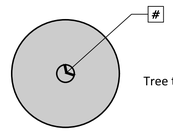
Designed: CV	Sheet No.: L201
Drawn: JB	
Checked: CV	
Date: 03/24/21	



PLANT SCHEDULE

CANOPY TREES	CODE	QTY	BOTANICAL NAME	COMMON NAME	SIZE
	IC2	3	Ilex cassine	Dahoon Holly	2" cal., min 8` ht.
	QS	2	Quercus shumardii	Shumard Red Oak	2" cal., min 8` ht.
	TD2	3	Taxodium distichum	Bald Cypress	2" cal., min 8` ht.
UNDERSTORY TREES	**CODE**	**QTY**	**BOTANICAL NAME**	**COMMON NAME**	**SIZE**
	CC	5	Cercis canadensis	Eastern Redbud	1.5" Cal.
	MC2	2	Myrica cerifera	Wax Myrtle	1.5" Cal.
SHRUBS	**CODE**	**QTY**	**BOTANICAL NAME**	**COMMON NAME**	**SIZE**
	IB	56	Ilex cornuta `Burfordii Nana`	Dwarf Burford Holly	3 gal.
	LM	34	Liriope muscari	Lilyturf	3 gal.
	MC	70	Muhlenbergia capillaris	Pink Muhly	3 gal.

LEGEND



Tree to be preserved

OPEN SPACE

Per 6.7.3. There shall be a minimum of 10% open space required.

Open space = 3,335 sf
Site area = 32,698 sf
3,335 / 32,698 = 10.20%

REQUIREMENT SATISFIED.

LANDSCAPING AREA

Per **Comprehensive Plan Future Land Use Element 2.4.a**, The minimum landscaped area shall be 10% of the development site, not inclusive of any designated open space areas.

Site area = 32,698 sf
Open space = 3,335 sf
Impervious = 20,977 sf
Landscape area = 32,698 - 3,335 - 20,977 = 8,386
8,386 / 32,698 = 25.67%

REQUIREMENT SATISFIED.

kd design
916 VASSAR STREET
ORLANDO, FL 32804
TEL | 321-277-0826
kaleymariedunlap@gmail.com

Contact: Marsha Swider
(352) 373-3541

REGISTERED LANDSCAPE ARCHITECT
KALEY M. DUNLAP
LA 6667256
STATE OF FLORIDA

NORTH
SCALE: 1" = 20'
0 10 20 40
GRAPHIC SCALE

No.	Date	Comment

Professional Landscape Architect of Record:

Kaley M. Dunlap, PLA — LA6667256
Landscape Architect — Certificate No.

Project No: 20-103

Project phase: CITY SUBMITTAL

Project title: ALCHEM LABORATORIES: BUILDING 4 SITE PLAN CITY OF ALACHUA, FLORIDA

Sheet title: LANDSCAPE PLAN

Designed: CV
Drawn: JB
Checked: CV
Date: 03/24/21

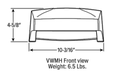
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TYPE XW

VWM LED Voltaire Mini Architectural Wall Pack

Williams





VWMH Front view
Weight: 6.5 Lbs.

CATALOG #:

TYPE:

PROJECT:

FEATURES

- Designed to illuminate sidewalks, entryways, perimeters or facades
- Intended for use in both uplight and downlight applications
- Savings of up to 80% energy compared to HID systems
- Blends seamlessly with a variety of architectural styles
- Made Right Here® in the USA

SPECIFICATIONS

- HOUSING – Die-cast aluminum enclosure.
- THERMAL MANAGEMENT – Integral die-cast aluminum heatsink and LED source provide passive thermal management. Rated ambient operating temperature -30°C to 50°C (L10, L17), -30°C to 45°C (L20).
- OPTICAL SYSTEM – General output provides full cutoff.
- LED SOURCE – ANSI 3000K, 4000K, or 5000K CCT, 70 or 80 CRI LEDs. L20: circuit board design. L10 & L17: Chip on board.
- LED DRIVER – 0-10V dimming.
- ELECTRICAL – 120-277 VAC input range; 50-60Hz; power factor > 90; THD <20% at full load. FCC Class A compliant. Quick-disconnect wiring provided. Calculated L70 >50,000 hours per IES TM-21.
- FINISH – Super durable polyester powder coat bonded to phosphate-free, multi-stage pretreated metal, meets and exceeds AAMA 2604 specifications for outdoor durability.
- MOUNTING – Surface mounts directly over a 4" maximum outlet box. Must be anchored to adequate structure that can support fixture weight (6.5 Lbs).
- LISTINGS
  - cULus certified as luminaire suitable for wet locations.
  - DesignLights Consortium qualified product. Not all versions of this product may be DLC qualified, see the DLC Qualified Products List at www.designlights.org/QPL.
  - IDA Fixture Seal approved (downlight applications only).
  - RoHS compliant.
  - IP65 rated.
  - Title 24 compliant with PC option.
- WARRANTY – 5-year limited warranty, see hew.com/warranty.

ORDERING EXAMPLE: VWM H - L20/740 - T3 - DBZ - SDGL - OPTIONS - DIM - UNV

ORDERING INFO

SERIES	TYPE [1]	LUMENS [2]	CRI/CCT	DISTRIBUTION [3]
VWM	H Horizontal V Vertical	L17 1,700lm L20 2,000lm L10 1,000lm L17 1,700lm	70 CRI 730 3000K 740 4000K 750 5000K 80 CRI 830 3000K 840 4000K	TL Lambertian distribution [4] T3 Type III [4]

FINISH OPTIONS [5]
BLK Black (RAL #9004)
DBZ Dark bronze
DBR Medium bronze
GRAY Standard gray
SLV Satin aluminum (RAL #9006)
WHT White (RAL #9003)

SHIELDING
SDGL Micro-prismatic tempered glass lens
CGL Clear tempered glass lens

OPTIONS
PC Button-style photocell [6]
SPD 10kA/10kV surge protection
TP Tamper-resistant Torx head screws [7]

CONTROL
DIM Dimming driver prewired for 0-10V low voltage applications

VOLTAGE
120 120V
208 208V
277 277V
UNV 120-277V

ACCESSORIES

TPTX-25 TOOL Tamper-resistant tool for Torx head screws. [7][8]

NOTES
1 See page 3 for FIXTURE DETAILS.
2 Lumen output based on 5000 CCT. Actual lumens may vary +/-5%, see page 2 for FIXTURE PERFORMANCE DATA.
3 See page 2 for DISTRIBUTION DETAILS.
4 Available with 750 CRI/CCT only.
5 L70 & L17 only.
6 Not available with photocontrol.
7 See page 3 for FINISH OPTIONS.
8 Factory-installed, 120V, 208V, or 277V only; must specify voltage. See page 3 for FIXTURE DETAILS. Left side when viewed from behind fixture.
9 Silver finish only. Safety screws providing added support to the lens are not tamper-resistant (lens is sealed to the fixture via silicone). Requires a tamper-resistant tool, see Accessories.
10 Separately-assembled hood must be ordered separately. Please specify quantity required per project.

TYPE XW

VWM LED Voltaire Mini Architectural Wall Pack

FIXTURE PERFORMANCE DATA

	LED PACKAGE	DISTRIBUTION	WATTAGE	CCT	CLEAR GLASS (CGL) DELIVERED LUMENS	CLEAR GLASS (CGL) EFFICACY(lm/W)	SOLITE GLASS (SDGL) DELIVERED LUMENS	SOLITE GLASS (SDGL) EFFICACY (lm/W)	BUG RATINGS
VWMH	L10	TL	13	3000	1188	91.3	1104	85.0	B1-U0-G0
				4000	1250	96.2	1163	89.4	
				5000	1349	103.8	1255	96.5	
	L17		16	3000	1644	102.8	1529	95.6	
				4000	1731	108.2	1610	100.6	
				5000	1840	115.0	1711	107.0	
	L20	T3	25	3000	2387	95.5	2197	87.9	B1-U0-G1
				4000	2390	95.6	2229	89.2	
				5000	2510	100.4	2341	93.6	
VWMV	L10	TL	13	3000	1006	77.4	936	72.0	B1-U0-G0
				4000	1059	81.5	985	75.8	
				5000	1201	92.4	1117	85.9	
	L17		16	3000	1520	95.0	1414	88.4	
				4000	1600	100.0	1488	93.0	
				5000	1704	106.5	1585	99.0	
	L20	T3	25	3000	2139	85.6	2022	80.9	B1-U0-G1
				4000	2290	91.6	2136	85.4	
				5000	2301	92.0	2148	85.8	

- Wattage shown is average for 120V through 277V input.
- Efficacy/lumen output shown is average based on voltage input of 120V through 277V.
- Photometrics tested in accordance with IESNA LM-79. Results shown are based on 25°C ambient temperature.

LUMEN MAINTENANCE

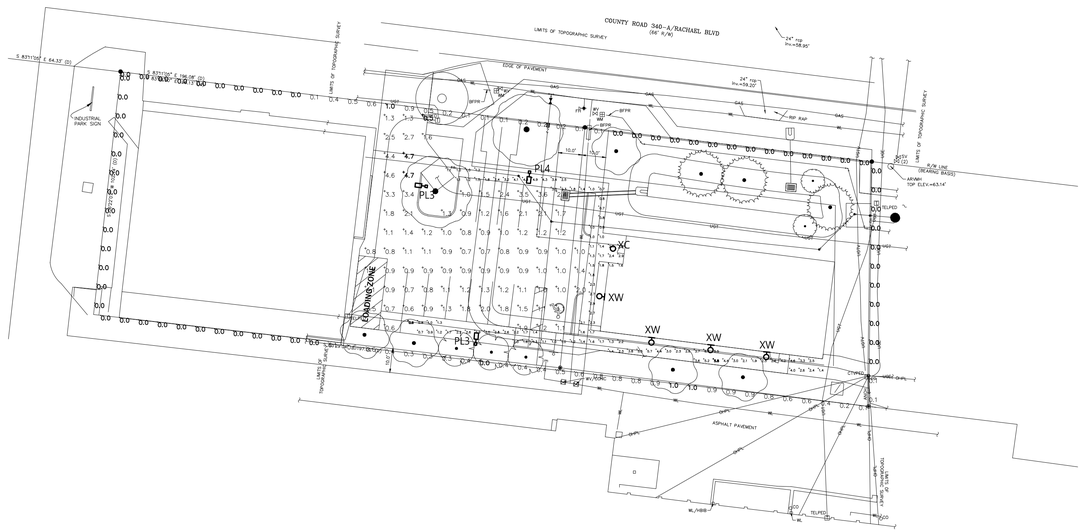
	AMBIENT TEMP.	REPORTED L70 HOURS (TM-21)
L25	25°C	>60,000
	35°C	>60,000
	45°C	>60,000
L10/L17	25°C	>55,000
	35°C	>55,000
	45°C	>55,000

- Predicted lumen maintenance calculated from LED manufacturer IES LM-80 data and in-situ temperature measurement.
- Reported L70 hours in accordance with IES TM-21

DISTRIBUTION DETAILS

VWMH TL | VWMV TL | VWMH/V T3 | VWMH/V T3

Clear Glass (CGL) | Solite Glass (SDGL)



SITE PHOTOMETRIC PLAN

SCALE: 1" = 30'-0"

NORTH

GRAPHIC SCALE
0 15 30 60

Photometric Statistics						
Description	Symbol	Avg	Max	Min	Max/Min	Avg/Min
BUILDING WALKWAY	+	2.3 fc	5.8 fc	0.6 fc	9.7:1	3.8:1
PARKING LOT	+	1.5 fc	4.7 fc	0.5 fc	9.4:1	3.0:1
PROPERTY LINE	+	0.2 fc	1.0 fc	0.0 fc	N/A	N/A

GENERAL NOTES

1. HIGHLIGHTED POINTS REPRESENT MAXIMUM/MINIMUM VALUE FOR EACH AREA.
2. FIXTURES WILL BE CONTROLLED WITH PHOTOCELL AND OPERATE DUSK-TO-DAWN.
3. POLES SHALL BE LOCATED MINIMUM 24" BEHIND CURB AND SHALL BE PROVIDED WITH CONCRETE POLE BASE.

Luminaire Schedule for Photometrics										
Symbol	Label	Quantity	Manufacturer	Catalog Number	Description	Lamp	Number Lamps	Filename	Lumens Per Lamp	Light Loss Factor	Mounting Height
▭	PL3	2	SIGNIFY GARDCO	ECF-S-32L-365-WW-G2-3	EcoForm Area LED ECF - Small, 32 LED's, 3000K CCT, TYPE 3 OPTIC, No Shield	(2) LEDGINE SLD LIGHT ARRAY(S) DRIVEN AT 365mA	1	ECF-S-32L-365-WW-G2-3.IES	5428	0.81	15
▭	PL4	1	SIGNIFY GARDCO	ECF-S-32L-365-WW-G2-4	EcoForm Area LED ECF - Small, 32 LED's, 3000K CCT, TYPE 4 OPTIC, No Shield	(2) LEDGINE SLD LIGHT ARRAY(S) DRIVEN AT 365mA	1	ecf-s-32l-365-ww-g2-4.ies	5637	0.81	15
◦	XW	4	WILLIAMS OUTDOOR	VWMH-L20-730-T3-SDGL-xxx-xxx	VOLTAIRE HORIZONTAL MINI ARCHITECTURAL WALL PACK WITH TYPE III GLASS LENS	(4) LUXEON M 3000K LEDS	1	VWMH-L20-730-T3-xxx-SDGL-xxx-xxx-xxx.ies	2197	0.81	12
○	XC	1	WILLIAMS	6DR-TL-L10-835-DIM-UNV--L-W-OF-WH-AD	Black formed aluminum housing, white reflector, frosted plastic lens enclosure	One white LED	1	6DR-TL-L10-835-DIM-UNV-L-W-OF--WH-AD.ies	650	0.81	CEILING

TYPES PL3/PL4

GARDCO by Signify

Site & Area
EcoForm
ECF-S small area light



Gardco EcoForm Gen-2 combines economy with performance in an LED area luminaire. Capable of delivering up to 27,800 lumens or more in a compact, low profile LED luminaire, EcoForm offers a new level of customer value. EcoForm features an innovative retrofit arm kit, simplifying site conversions to LED by eliminating the need to drill additional holes in most existing poles. Integral control systems available for further energy savings. Includes Service Tag, our innovative way to provide assistance throughout the life of the product.

Project:
Location:
Cat.No:
Type:
Lamps: Qty:
Notes:

Ordering guide

example: ECF-S-64L-900-NW-G2-AR-S-120-HIS-MGY

Prefix	Number of LEDs	Drive Current	LED Color / Generation	Mounting	Distribution	Voltage	Options: Dimming controls	Options: Motion sensing	Options: Photo-sensors	Options: Electrical	Luminaire	Finish
ECF-S EcoForm site and area, small	32L 32 LEDs (2 modules)	365 365mA 530 530mA 700 700mA 1A 1000mA 1.2A 1200mA	WW-G2 Warm White 3000K, 70CRI, Generation 2 NW-G2 Neutral White 4000K, 70CRI, Generation 2	AR Arm Mount (standard) [1] The following mounting kits must be ordered separately (See accessories): SF Slip Fitter Mount [2] (fits to 2 ⅛" O.D. tenon) WS Wall mount with surface conduit rear entry permitted RAM RetroFit arm mount kit [3]	Type 2 2 Type 2 2-90 Rotated left 90° 2-270 Rotated right 270° Type 3 3 Type 3 3-90 Rotated left 90° 3-270 Rotated right 270° Type 4 4 Type 4 4-90 Rotated left 90° 4-270 Rotated right 270° Type 5 5 Type 5W 5W Type 5W AFR Auto Front Row AFR-90 Auto Front Row, Rotated left 90° AFR-270 Auto Front Row, Rotated right 270°	120 120V 208 208V 240 240V 277 277V 347 347V 480 480V [4] UNV 120-277V (50/60Hz) HVU 347-480V (50/60Hz)	DD 0-10V External dimming (by others) DCC Dual Circuit Control [5] FAWD Field Adjustable Wattage Selector [6] SW Interface module for SiteWise [7] LLC Lutron Hi-lume wireless module [8] BL Bi-level functionality [9,10] DynaDimmer Automatic Profile Dimming CS50 Safety 50% Dimming, 7 hours [11] CM50 Median 50% Dimming, 8 hours [12] CS30 Safety 30% Dimming, 7 hours [11] CM30 Median 30% Dimming, 8 hours [12]	IMR3 Integral with #3 lens [13] IMR7 Integral with #7 lens [13]	PCB Photocontrol, Button [14] TLR05 Twist Lock Receptacle 5 Pin [15] TLR07 Twist Lock Receptacle 7 Pin [15] TLRPC Twist Lock Receptacle w/ Photocell [15]	Fusing #1 Single (120, 277, 347VAC)* [16] #2 Double (208, 240, 480VAC)* Pole Mount Fusing FP1 Single (120, 277, 347VAC)* [16] FP2 Double (208, 240, 480VAC)* FP3 Canadian Double Pull (208, 240, 480VAC)* Surge Protection (SPD) (OOA standard) SP2 Increased 20kA	Square Pole Adapter included in standard product TB Terminal Block* [17] RPA Round Pole Adapter (fits to 3"-3.5" O.D. pole)* [18] HIS Internal Housing Side Shield* [19]	Textured BK Black WH White BZ Bronze DGY Dark Gray MGY Medium Gray Customer specified RAL Specify optional color or RAL (ex: RAL 7024) CC Custom color (Must supply color chip for required factory quote)

1. BL-IMR(3/7 equipped with out-boarded sensor housing when voltage is HVU (347-480V).
2. Mounts to a 4" round pole with adapter included for square poles.
3. Limited to a maximum of 45 degrees aiming above horizontal.
4. Not available with other dimming control options.
5. Not available with motion sensor.
6. Not available with photocontrol.
7. Available only in 120 or 277V.
8. Not available in 347 or 480V.
9. Must specify input voltage.
10. Dimming will not be connected to NEMA receptacle if ordering with other control options.
11. Not available in 480V. Order photocell separately with TLR05/7.
12. Not available with DCC.
13. Not available with SF and WS. RPAs provided with black finish standard.
14. HIS not available with Type 5 and SW optics.
15. Not available with DD, DCC, and FAWD dimming control options.
16. Not available with DD, DCC, FAWD and LLC dimming control options.
17. Must specify a motion sensor lens.

TYPES PL3/PL4

ECF-S EcoForm small

Area luminaire

EcoForm Accessories (ordered separately, field installed)

Shielding Accessories

House Side shield
Standard optic orientation
HIS-32-N [1] Internal House Side Shield for 32 LEDs (2 modules)
HIS-48-N [1] Internal House Side Shield for 48 LEDs (3 modules)
HIS-64-N [1] Internal House Side Shield for 64 LEDs (4 modules)

Optic at 90 or 270 orientation
HIS-32-V [1] Internal House Side Shield for 32 LEDs (2 modules)
HIS-48-V [1] Internal House Side Shield for 48 LEDs (3 modules)
HIS-64-V [1] Internal House Side Shield for 64 LEDs (4 modules)

18. Not available with Type 5 or SW optics

Luminaire Accessories

ECF-BD-G2 Bird deterrent
ECF-RAM-G2-(F) RetroFit Arm mount kit
ECF-SF-G2-(F) Slip Fitter Mount (fits to 2 3/8" O.D. tenon)
ECF-WS-G2-(F) Wall mount with surface conduit rear entry permitted

EcoForm PTF2
(pole top fitter fits 2 3/8-2½" OD x 4" depth tenon)
PTF2-ECF-S/L-1-90-(F) 1 luminaire at 90°
PTF2-ECF-S/L-2-90-(F) 2 luminaires at 90°
PTF2-ECF-S/L-2-180-(F) 2 luminaires at 180°
PTF2-ECF-S/L-3-90-(F) 3 luminaires at 90°
PTF2-ECF-S/L-4-90-(F) 4 luminaires at 90°
PTF2-ECF-S/L-3-120-(F) 3 luminaires at 120°

EcoForm PTF3
(pole top fitter fits 3-3½" OD x 6" depth tenon)
PTF3-ECF-S/L-1-90-(F) 1 luminaire at 90°
PTF3-ECF-S/L-2-90-(F) 2 luminaires at 90°
PTF3-ECF-S/L-2-180-(F) 2 luminaires at 180°
PTF3-ECF-S/L-3-90-(F) 3 luminaires at 90°
PTF3-ECF-S/L-4-90-(F) 4 luminaires at 90°
PTF3-ECF-S/L-3-120-(F) 3 luminaires at 120°

EcoForm PTF4
(pole top fitter fits 3½-4" OD x 6" depth tenon)
PTF4-ECF-S/L-1-90-(F) 1 luminaire at 90°
PTF4-ECF-S/L-2-90-(F) 2 luminaires at 90°
PTF4-ECF-S/L-2-180-(F) 2 luminaires at 180°
PTF4-ECF-S/L-3-90-(F) 3 luminaires at 90°
PTF4-ECF-S/L-4-90-(F) 4 luminaires at 90°
PTF4-ECF-S/L-3-120-(F) 3 luminaires at 120°

(F) - Specify finish

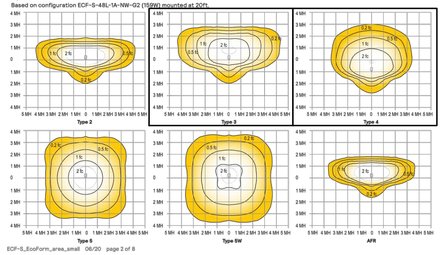
Predicted Lumen Depreciation Data

Predicted performance derived from LED manufacturer's data and engineering design estimates, based on IESNA LM-80 methodology.
Actual experience may vary due to field application conditions. L_{70} is the predicted time when LED performance depreciates to 70% of initial lumen output. Calculated per IESNA TM21-11. Published L_{70} hours limited to 6 times actual LED test hours

Ambient Temperature °C	Driver mA	Calculated L_{70} Hours	L_{70} per TM-21	Lumen Maintenance % at 60,000 hrs
25°C	up to 1200 mA	>100,000 hours	>60,000 hours	>88%

Optical distribution

Based on configuration ECF-S-48L-1A-NW-G2 (5190 lm) mounted at 20ft.



KENNETH W. HUNTER LICENSE No. 76961 STATE OF FLORIDA PROFESSIONAL ENGINEER

This item has been electronically signed and sealed by Kenneth W. Hunter, P.E. on 3/23/21 using a Digital Signature.

Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

HUNTER DESIGN AND CONSULTING, INC.
696 1ST AVE N. STE 200
ST. PETERSBURG, FL 33701
352-238-6366
FLORIDA CA #31946, PE #76961

ALCHEM BUILDING 4
SITE LIGHTING PLAN
ALACHUA, FLORIDA

PROJECT INFORMATION	
PROJECT NUMBER:	20085
DRAFTED:	K. NORTHCUTT
DESIGNED:	K. NORTHCUTT
REVIEWED:	K. HUNTER
ISSUE DATE:	3/9/21
REVISIONS	

SHEET NUMBER

E-1

January 28, 2021

Justin Tabor, Principal Planner
City of Alachua Planning and Community Development
15100 NW 142nd Terrace
Alachua, FL 32616

Re: Alchem Building 4
Development Plan Application

The proposed project is the construction of a 3,788 square foot building, with associated parking, utility, and stormwater improvements. The project is located at 13305 Rachael Boulevard on Tax Parcel No. 03231-003-000 and a portion of 03231-004-000. The project site comprises approximately 0.76 (+/-) acres. There are two existing light manufacturing buildings already on the property. The 700 SF portable building is proposed to be removed as a part of this site plan.

Included with this letter is all supporting information required for a development plan and civil plans showing the proposed facilities.

If you have any questions, please feel free to contact our office at any time.

Sincerely,

Claudia Vega, P.E.
Director of Engineering

City of ALACHUA

THE GOOD LIFE COMMUNITY

FOR PLANNING USE ONLY
Case #:
Application Fee: $
Filing Date:
Acceptance Date:
Review Type: P&Z

Site Plan Application

Reference City of Alachua Land Development Regulations Article 2.4.9

A. PROJECT

1. Project Name: Alchem - Building 4
2. Address of Subject Property: 13305 Rachael Blvd
3. Parcel ID Number(s): 03231-003-000 & a portion of 03231-004-000
4. Existing Use of Property: Light Industrial
5. Future Land Use Map Designation : Industrial
6. Zoning Designation: Industrial, Light and Warehousing (ILW)
7. Acreage: 0.76 +/-

B. APPLICANT

1. Applicant's Status ☐ Owner (title holder) ■ Agent
2. Name of Applicant(s) or Contact Person(s): Claudia Vega, P.E. Title: Director of Engineering
   Company (if applicable): eda consultants inc.
   Mailing address: 720 SW 2nd Ave, South Tower, Suite 300
   City: Gainesville State: FL ZIP: 32601
   Telephone: 352-373-3541 FAX: e-mail: cvega@edafl.com
3. If the applicant is agent for the property owner*:
   Name of Owner (title holder): Dr. James D. Talton
   Mailing Address: 13305 Rachael Blvd
   City: Alachua State: FL ZIP: 32615

* Must provide executed Property Owner Affidavit authorizing the agent to act on behalf of the property owner.

C. ADDITIONAL INFORMATION

1. Is there any additional contact for sale of, or options to purchase, the subject property? ☐ Yes ■ No
   If yes, list names of all parties involved:
   If yes, is the contract/option contingent or absolute? ☐ Contingent ☐ Absolute

D. ATTACHMENTS

1. Site Plan including but not limited to:
   a. Name, location, owner, and designer of the proposed development.
   b. Zoning of the subject property.
   c. Vicinity map - indicating general location of the site and all abutting streets and properties.
   d. Complete legal description.
   e. Statement of Proposed Uses.
   f. Location of the site in relation to adjacent properties, including the means of ingress and egress to such properties and any screening or buffers along adjacent properties.
   g. Date, north arrow, and graphic scale (not to exceed one (1) inch equal to fifty (50) feet.)
   h. Area and dimensions of site.
   i. Location of all property lines, existing right-of-way approaches, sidewalks, curbs, and gutters.
   j. Access and points of connection to utilities (electric, potable water, sanitary sewer, gas, etc.)
   k. Location and dimensions of all existing and proposed parking areas and loading areas.
   l. Location, size, and design of proposed landscaped areas (including existing trees and required landscaped buffer areas) with detail illustrating compliance with Section 6.2.2 of the Land Development Regulations.

m. Location and size of any lakes, ponds, canals, or other waters and waterways.
n. Structures and major features – fully dimensioned – including setbacks, distances between structures, floor area, width of driveways, parking spaces, property or lot lines, and floor area ratio.
o. Location of waste receptacles and detail of waste receptacle screening.
p. For development consisting of a nonresidential use, except for single tenant retail sales and services uses greater than or equal to 20,000 square feet in area and except for use types within the industrial services, manufacturing and production, warehouse freight and movement, waste-related services, and wholesale sales use categories:
    i. Architectural plans and dimension plans which demonstrate compliance with the design standards for business uses as provided in Section 6.8.2 of the LDRs, including:
        (a) Calculation of glazing of the front façade.
        (b) Calculation of the area of ground floor façades subject to glazing.
        (c) Detail on the architectural plans and dimension plans depicting façade massing and/or alternatives to required façade massing.
        (d) Sufficient plan detail and calculations of each material utilized in each façade.
q. For development consisting of a nonresidential use where a single tenant is greater than or equal to 20,000 square feet in area:
    i. Architectural plans and dimension plans which demonstrate compliance with the design standards for single tenant retail sales and service uses greater than or equal to 20,000 square feet in area as provided in Section 6.8.3 of the LDRs, including:
        (a) Calculation of glazing of the façades facing streets, residential uses, and vacant residential/agricultural land.
        (b) Calculation of the area of ground floor façades subject to glazing.
        (c) If glazing alternatives are used, calculation of area of alternative materials used.
        (d) Detail on the architectural plans and dimension plans depicting façade massing and/or alternatives to required façade massing.
        (e) Color architectural plans depicting the color of all materials used in the façade.
r. For development consisting of one or more of the following: Multi-family residential; Hotel; or Mobile Home Park:
    i. Tabulation of gross acreage.
    ii. Tabulation of density.
    iii. Number of dwelling units proposed.
    iv. Location and percent of total open space and recreation areas.
    v. Floor area of dwelling units.
    vi. Number of proposed parking spaces.
    vii. Street layout.
    viii. Layout of mobile home stands (for mobile home parks only).
    ix. City of Alachua Public School Student Generation Form.

Sheet Size: 24" X 36" with 3" left margin and ½" top, bottom, and right margins

2. Stormwater management plan - including the following:

    a. Existing contours at one (1) foot intervals based on U.S. Coastal and Geodetic Datum.
    b. Proposed finished floor elevation of each building site.
    c. Existing and proposed stormwater management facilities with size and grades.
    d. Proposed orderly disposal of surface water runoff.
    e. Centerline elevations along adjacent streets.
    f. Water Management District surfacewater management Statement of proposed uses on the site plan

3. Fire Department Access and Water Supply: The design criteria shall be Chapter 18 of the Florida Fire Prevention Code. Plans must be on separate sealed sheets and must be prepared by a professional Fire engineer licensed in the State of Florida. Fire flow calculations must be provided for each newly constructed building. When required, fire flow calculations shall be in accordance with the Guide for Determination of Required Fire Flow, latest edition, as published by the Insurance Service Office (ISO) and /or Chapter 18, Section 18.4 of the Florida Fire Prevention Code, whichever is greater. All calculations must be demonstrated and provided. All calculations and specifications must be on the plans and not on separate sheets. All fire protection plans are reviewed and approved by the Alachua County Fire Marshal.

4. Concurrency Impact Analysis showing the impact on public facilities, including potable water, sanitary sewer, transportation, solid waste, recreation, stormwater, and public schools in accordance with Article 2.4.14 of the Land Development Regulations.

5. Analysis of Consistency with the City of Alachua Comprehensive Plan (analysis must identify specific Goals, Objectives, and Policies and describe in detail how the application complies with the noted Goal, Objective, or Policy.)

For commercial project Applications:

a. In addition to submitting specific written information regarding your **commercial** development's compliance with the relevant Goals, Objectives, and Policies of the City of Alachua Comprehensive Plan, you must respond directly to the standards listed below. You should be specific in terms of how your commercial development will comply with these standards.

Policy 1.3.d Design and performance standards

The following criteria shall apply when evaluating commercial development proposals:

1. Integration of vehicular and non-vehicular access into the site and access management features of site in terms of driveway cuts and cross access between adjacent sites, including use of frontage roads and/or shared access;
2. Buffering from adjacent existing/potential uses;
3. Open space provisions and balance of proportion between gross floor area and site size;
4. Adequacy of pervious surface area in terms of drainage requirements;
5. Placement of signage;
6. Adequacy of site lighting and intrusiveness of lighting upon the surrounding area;
7. Safety of on-site circulation patterns (patron, employee and delivery vehicles), including parking layout and drive aisles, and points of conflict;
8. Landscaping, as it relates to the requirements of the Comprehensive Plan and Land Development Regulations;
9. Unique features and resources which may constrain site development, such as soils, existing vegetation and historic significance; and
10. Performance based zoning requirements, which may serve as a substitute for or accompany land development regulations in attaining acceptable site design.
11. Commercial uses shall be limited to an intensity of less than or equal to .50 floor area ratio for parcels 10 acres or greater, .50 floor area ratio for parcels less than 10 acres but 5 acres or greater, a .75 floor area ratio for parcels less than 5 acres but greater than 1 acre, and 1.0 floor area ratio to parcels 1 acre or less.

For industrial project Applications:

b. In addition to submitting specific written information regarding your **industrial** development's compliance with the relevant Goals, Objectives, and Policies of the City of Alachua Comprehensive Plan, you must respond directly to the standards listed below. You should be specific in terms of how your industrial development will comply with these standards.

Policy 1.5.d

The City shall develop performance standards for industrial uses in order to address the following:

1. Integration of vehicular and non-vehicular access into the site and access management features of site in terms of driveway cuts and cross access between adjacent sites, including use of frontage roads and/or shared access;
2. Buffering from adjacent existing/potential uses;
3. Open space provisions and balance of proportion between gross floor area and site size;
4. Adequacy of pervious surface area in terms of drainage requirements;
5. Placement of signage;
6. Adequacy of site lighting and intrusiveness of lighting upon the surrounding area;
7. Safety of on-site circulation patterns (patron, employee and delivery vehicles, trucks), including parking layout and drive aisles, and points of conflict;
8. Landscaping, as it relates to the requirements of the Comprehensive Plan and Land Development Regulations;
9. Unique features and resources which may constrain site development, such as soils, existing vegetation and historic significance; and
10. Performance based zoning requirements that may serve as a substitute for or accompany land development regulations in attaining acceptable site design.
11. Industrial uses shall be limited to an intensity of less than or equal to .50 floor area ratio for parcels 10 acres or greater, .50 floor area ratio for parcels less than 10 acres by 5 acres or greater, .75 floor area ratio for parcels less than 5 acres but greater than 1 acre, and 1.0 floor area ratio for parcels 1 acre or less.

6. *For Site Plans for Buildings Less than 80,000 Square Feet in Area:* One (1) set of labels for all property owners within 400 feet of the subject property boundaries – even if property within 400 feet falls outside of City limits (obtain from the Alachua County Property Appraiser's web site) – and all persons/organizations registered to receive notice of development applications.
   For Site Plans for Buildings Greater than or Equal to 80,000 Square Feet in Area: Two (2) sets of labels for all property owners within 400 feet of the subject property boundaries – even if property within 400 feet falls outside of City limits (obtain from the Alachua County Property Appraiser's web site) – and all persons/organizations registered to receive notice of development applications.

7. Neighborhood Meeting Materials, including:
   i. Copy of the required published notice (advertisement) – must be published a newspaper of general circulation, as defined in Article 10 of the City's Land Development Regulations
   ii. Copy of written notice (letter) sent to all property owners within 400 feet and to all persons/organizations registered with the City to receive notice, and mailing labels or list of those who received written notice
   iii. Written summary of meeting – must include (1) those in attendance; (2) a summary of the issues related to the development proposal discussed; (3) comments by those in attendance about the development proposal; and, (4) any other information deemed appropriate.

8. Legal description with tax parcel number, separate from all other documentation on 8.5" x 11" paper.

9. Proof of ownership (i.e., copy of deed.)

10. Proof of payment of taxes.

11. Environmental Resource Permit (or Letter of Exemption) from the Suwannee River Water Management District or Self-Certification for a Stormwater Management System in Uplands Serving Less than 10 Acres of Total Project Area and Less than 2 Acres of Impervious Surfaces from the Florida Department of Environmental Protection pursuant to Section 403.814(12), Florida Statutes.

12. If access is from a County Road, access management permit from Alachua County Public Works (or documentation providing evidence that a permit application has been submitted).

13. If access is from a State Road, access management permit from Florida Department of Transportation (or documentation providing evidence that a permit application has been submitted).

14. **Fee.** Please see fee schedule for fee determination. No application shall be accepted for processing until the required application fee is paid in full by the applicant. Any necessary technical review or additional reviews of the application beyond the initial engineering review fee will be billed to the applicant at the rate of the reviewing entity. The invoice shall be paid in full prior to any legislative and/or quasi-judicial action of any kind on the petition, appeal, or development application.

All 14 attachments are required for a complete application. A completeness review of the application will be conducted within five (5) business days of receipt. If the application is determined to be incomplete, the application will be returned to the applicant.

I/We certify and acknowledge that the information contained herein is true and correct to the best of my/our knowledge.

Claudia Vega

Signature of Applicant

Claudia Vega, P.E. - Director of Engineering

Typed or printed name and title of applicant

—

Signature of Co-applicant

—

Typed or printed name of co-applicant

State of Florida County of Alachua

The foregoing application is acknowledged before me this 28th day of January, 2021, by Claudia Vega, who is/are personally known to me, or who has/have produced _____ as identification.

NOTARY SEAL
ASHLEY SCANNELLA
MY COMMISSION # GG91185
EXPIRES: April 06, 2021

Ashley E Scannella

Signature of Notary Public, State of FL

City of ALACHUA

THE GOOD LIFE COMMUNITY

Authorized Agent Affidavit

A. PROPERTY INFORMATION

Address of Subject Property: 13200 block of Rachael Blvd

Parcel ID Number(s): 03231-003-000 and a portion of 03231-004-000

Acreage: 0.55 +/-

B. PERSON PROVIDING AGENT AUTHORIZATION

Name: Dr. James D. Talton Title: President & CEO

Company (if applicable): Alchem Laboratories Corporation

Mailing Address: 13305 Rachael Blvd

City: Alachua State: FL ZIP: 32615

Telephone: 386-418-1650 FAX: e-mail: jtalton@alchem.com

C. AUTHORIZED AGENT

Name: Sergio Reyes, PE Title: President

Company (if applicable): eda consultants inc

Mailing address: 720 SW 2nd Ave. South Tower, Suite 300

City: Gainesville State: FL ZIP: 32601

Telephone: 352-373-3541 FAX: e-mail: sreyes@edafl.com

D. REQUESTED ACTION:

Site Plan

I hereby certify that I am the property owner of record, or I have received authorization from the property owner of record to file an application for a development permit related to the property identified above. I authorize the agent listed above to act on my behalf for purposes of this application.

Signature of Applicant

JAMES TALTON

Typed or printed name and title of applicant

Signature of Co-applicant

Typed or printed name of co-applicant

State of Florida County of Alachua

The foregoing application is acknowledged before me this 16th day of December 2020 by James Talton who is/are personally known to me, or who has/have produced as identification.

NOTARY SEAL

Signature of Notary Public, State of FL

MERRIE SHAW
Notary Public - State of Florida
Commission # GG 320664
My Comm. Expires Apr 7, 2023
Bonded through National Notary Assn.

Hydrant Flow Test Report

Test Date 11/04/2020 **Test Time** 4:30 PM

Location

Alchem Labotories Building #4
03231-004-000
13301 RACHAEL BLVD
ALACHUA, FL 32615

Tested by

Gator Fire Equipment Co., Inc.
Tested By: R. Weaver & A. Miller
Witnessed By: City of Alachua

Notes

Reading Hydrant at Rachael Blvd and NW 129th Way
Flowing Hydrant at 13301 Rachael Blvd

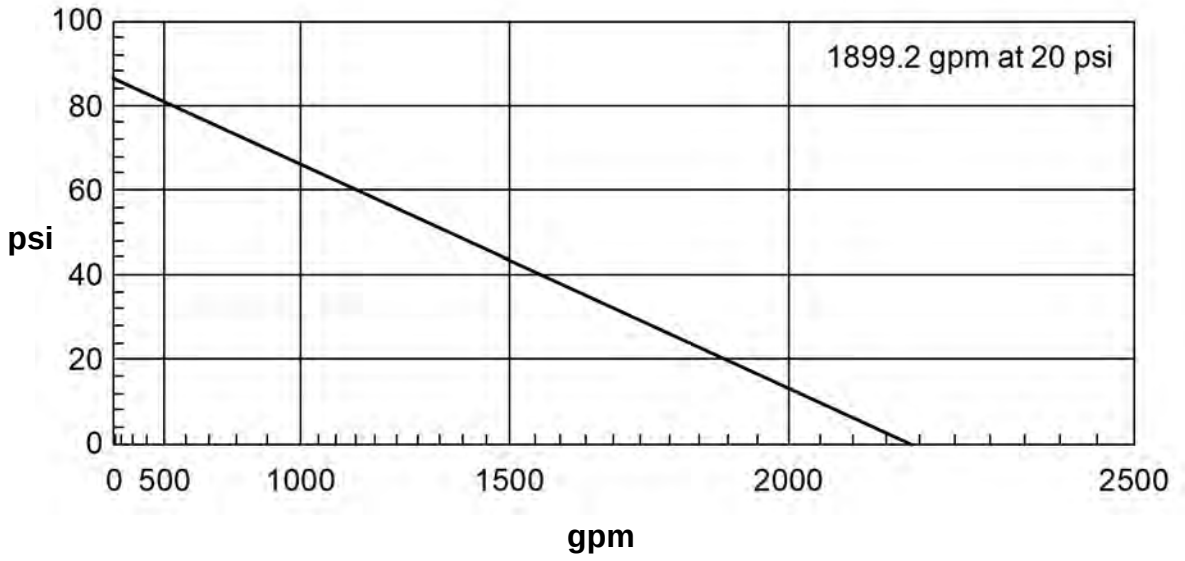
Read Hydrant

87 psi **static pressure**
60 psi **residual pressure**
0 ft **hydrant elevation**

Flow Hydrant(s)

Outlet	Elev	Size	C	Pitot Pressure	Flow
#1	0	2.5	.9	12	581 gpm
#2	0	2.5	.9	12	581 gpm
				Total	1162 gpm

Flow Graph



Created with the free hydrant flow test program from www.igneusinc.com

consultants inc.

Worksheet for Needed Fire Flow

Project Name :	Alchem Laboratories - Building 4
Customer / Builder Name :	Alchem Laboratories Corporation
Property Address :	13305 Rachael Blvd Alachua, FL 32615

NFPA Calculation

Fire Flow Area (SF)	=	3,788	
Occupancy Class	=	B	
Construction Type	=	III (200)	
Sprinkler System	=	No	
Sprinkler Reduction, per 18.4.5.2.1	=	0%	
NFPA Fire Flow	=	1,500	gpm
Required Fire Flow Per NFPA	=	1,500	gpm
Flow Duration (hours)	=	2	

(Resulting Fire Flow shall not be less than 1,000 gpm)

REQUIRED Needed Fire Flow	**1,500**	**gpm**

720 SW 2nd Avenue, STE 300 Gainesville, FL 32606 · Phone: (352)373-3541 · Fax: (352)373-7249 · www.edafl.com

ISO Needed Fire Flow (NFF) Worksheet

(Page references are to the appropriate sections in the ISO Guide for Determination of Needed Fire Flow)

Petition Number:		**Date:**	12/17/2020
Project:	Alchem Laboratories	**Engineer:**	GM
	Building 4	**Checked By:**	CMG
Location:	13305 Rachael Blvd		
	Alachua, FL		

Subject Building

Construction Class (p. 4): Joisted Masonry Construction **construction coefficient (F) (p. 2):** 1

Area of largest floor in the building (if modifications are made for division walls (p. 8), the division walls must be shown on the site plan.): 3788 **sq.ft.**

Total area of all other floors (if modifications are made for division walls (p. 8), the division walls must be shown on the site plan.): 0 **sq. ft.**

Effective Area (A_i) (p. 9) : 3,788 **sq. ft.** **(Show calculations below)**

Needed Fire Flow attributed to construction (C_i) (per formula (p. 2)): **1107.841144**

(Round to the nearest 250 gpm. See p. 10 for maximum and minimum values of C_i)

Type of Occupancy: Limited-combustible (C-2) **Occupancy Factor (O_i) (p. 11):** 0.85

Exposures (p. 16)

Front:

construction of facing wall of exposure building (p. 4): Joisted Masonry Construction

Distance (ft.) to the exposure building: 61 - 100 **Length of exposure wall:** 30

Number of stories of exposure wall: 1 **Length x number of stories:** 30

Opening Protection in exposure wall: Unprotected

Factor for exposure (X_i) from Table 330.A (p. 17): 0.06

Back:

construction of facing wall of exposure building (p. 4): Joisted Masonry Construction

Distance (ft.) to the exposure building: 61 - 100 **Length of exposure wall:** 39

Number of stories of exposure wall: 1 **Length x number of stories:** 39

Opening Protection in exposure wall: Unprotected

Factor for exposure (X_i) from Table 330.A (p. 17): 0.06

Left:

construction of facing wall of exposure building (p. 4): Joisted Masonry Construction

Distance (ft.) to the exposure building: 31 - 60 **Length of exposure wall:** 41

Number of stories of exposure wall: 1 **Length x number of stories:** 41

Opening Protection in exposure wall: Unprotected

Factor for exposure (X_i) from Table 330.A (p. 17): 0.1

Right:

construction of facing wall of exposure building (p. 4): Joisted Masonry Construction

Distance (ft.) to the exposure building: 61 - 100 **Length of exposure wall:** 41

Number of stories of exposure wall: 1 **Length x number of stories:** 41

Opening Protection in exposure wall: Unprotected

Factor for exposure (X_i) from Table 330.A (p. 17): 0.06

Communications (p. 18)

Passageway Opening Protection:

Construction class of communication (Table 330.B) :

Is communication open or enclosed?

Length of communication (in feet):

Factor for Communications (P_i) from Table 330.B on p.19): 0

Calculation of Needed Fire Flow (p. 1)

$NFF=(C_i)(O_i)[1.0+(X+P)_i]$ (substitute values as determined above. For exposures and communications use the single side with the highest charge.)

NFF= 1000 x 0.85 x [1 + (0.1 + 0)

NFF= 935 gpm

NFF= 1000 gpm (rounded to nearest 250 gpm per ISO requirements)

Note: ISO evaluates hydrant distribution by examining the number and type of hydrants within 1,000 feet of each representative building. They also look at the distance from each such hydrant to the subject building, measured as apparatus can lay hose.

Hydrants with at least one large pumper outlet may receive credit for up to 1,000 gpm. Hydrants with at least two hose outlets, but no pumper outlet, may receive credit for up to 750 gpm. And hydrants with only one hose outlet may receive credit for up to 500 gpm.

Hydrants within 300 feet of the subject building may receive credit for up to 1,000 gpm (but not more than the credit that would apply based on the number and type of outlets). Hydrants from 301 feet to 600 feet from the subject building may receive credit for up to 670 gpm (but not more than the credit that would apply based on the number and type of outlets). And hydrants from 601 feet to 1,000 feet from the subject building receive credit for 250 gpm. Under certain circumstances, when all fire department pumpers carry sufficient large-diameter hose, ISO may allow maximum credit for hydrants up to 1,000 feet from the subject building.

More than one fire hydrant may be required for proper distribution of water per ISO requirements.

Concurrency Impact Analysis
Alchem Laboratories – Building 4

The proposed building will provide an additional 3,788 SF of light manufacturing space for an existing business located on the property.

Stormwater:

A detailed stormwater management plan is included in this submittal. The proposed stormwater system shall be in compliance with City of Alachua and Suwannee River Water Management District requirements.

Potable Water:

Goal 4: Provide an adequate supply of high quality potable water to customers throughout the service area.
Objective 4.1 Achieve and maintain acceptable levels of service for potable water quality and quality.

Project Impact: For the proposed light manufacturing development, it is estimated that approximately 15 G.P.D. will be used per 100 square feet of building area (Ch. 64E-6, F.A.C.). The 3,788 square feet of office will generate approximately 568 G.P.D., based on this calculation (3,788 SF / 100 SF x 15 G.P.D. = 568 G.P.D.). As shown in the following table, there is adequate capacity available to support this development.

Table 3a. Potable Water Impacts - Final Development Orders

System Category	Gallons Per Day
Current Permitted Capacity[1]	2,300,000
Less Actual Potable Water Flows[1]	1,295,000
Alchem Building 4	568
Reserved Capacity[2]	152,524
Residual Capacity	**851,908**
Percentage of Permitted Design Capacity Utilized	**62.96%**

Sanitary Sewer:

Goal 1: Plan for and provide adequate, high quality and economical wastewater service while protecting the environment, especially groundwater resources.

Objective 1.2 Wastewater service will be made available to new development in a manner to promote compact urban growth, promoting development where wastewater service is available, and discouraging urban sprawl.

Project Impact: For the proposed office development, it is estimated that approximately 15 G.P.D. will be used per 100 square feet of building area (Ch. 64E-6, F.A.C.). The 3,788 square feet of office will generate approximately 568 G.P.D., based on this calculation (3,788 SF / 100 SF x 15 G.P.D. = 568 G.P.D.). As shown in the following table, there is adequate capacity available to support this development.

Table 4a. Sanitary Sewer Impacts - Final Development Orders

System Category	Gallons Per Day
Treatment Plant Current Permitted Capacity	1,500,000
Less Actual Treatment Plant Flows[1]	675,000
Alchem Building 4	568
Reserved Capacity[2]	145,298
Residual Capacity	**679,134**
Percentage of Permitted Design Capacity Utilized	**54.72%**

Solid Waste:

Goal 2: The City of Alachua will provide for solid waste disposal service in a sanitary, economic, and environmentally safe manner.

Project Impact: Commercial uses generate approximately 12 pounds per day of solid waste per 1,000 square feet (Environmental Engineering: A Design Approach, Cincero and Cincero, 1996). The proposed facility will generate approximately 144 pounds of solid waste will be generated per day 3,788 SF / 1,000 SF x 12 = 45.4 pounds per day). As indicated in the following table, the proposed solid waste generated as part of this project will not reduce the level of service in the City of Alachua.

Table 6a. Solid Waste Impacts - Final Development Orders

System Category	Lbs Per Day	Tons Per Year
Existing Demand[1]	41,192.00	7,517.54
Reserved Capacity[2]	9,953.23	1,816.46
New River Solid Waste Facility Capacity[3]	**50 years**	

Traffic:

The proposed use of the project site as a light industrial/manufacturing use will not create a traffic impact that will exceed the approved level of service standards for the impacted roadways.

TABLE 1 – PROJECT TRIP GENERATION

ITE LAND USE: 110 GENERAL LIGHT INDUSTRIAL PROPOSED: 3,788 SF							
				TRIP DISTRBUTION		PROJECTED TRIPS	
PERIOD	RATE	PER UNIT	TRIPS	ENTER	EXIT	IN	OUT
AM	0.70	3.79	2.65	88%	12%	2.33	0.32
PM	0.63	3.79	2.39	13%	87%	0.31	2.08
AVG	4.96	3.79	18.79	50%	50%	9.39	9.39
SOURCE: ITE TRIP GENERATION, 10TH EDITION							

TABLE 2: TRIP DISTRIBUTION

Segment ID	Distribution Share	Projected Trips
15 (CR 2054 from SR 235 to US Highway 441)	100%	19
8 (SR 235 from NW 143rd Place to US 441)	25%	5
11 (CR 235 from NW 143rd Place to South City Limit)	25%	5

No other road segments will be impacted more than 5% of their maximum service volume.

TABLE 3: ROADWAY LEVEL OF SERVICE (LOS) ANALYSIS

Segment ID:	Segment Limits:		Comp Plan	Existing	Res'vd	Projected Trips	Available
CR 2054 (15)	SR 235 to US Highway 441	AADT	15,120	3,007	2,138	19	9,956
		Peak Hour	1,359	354	156	3	846
SR 235 (8)	NW 143rd Place to US 441	AADT	14,400	9,972	152	5	4,451
		Peak Hour	1,290	930	40	1	319
CR 235 (11)	NW 143rd Place to South City Limit	AADT	15,120	3,007	2,138	5	9,970
		Peak Hour	1,359	354	156	1	848

consultants inc.

Worksheet for Trip Generation

Project Name : Alchem Laboratories - Building 4
Customer / Builder Name : Alchem Laboratories Corporation
Property Address : 13305 Rachael Blvd Alachua, FL 32615

PROPOSED

PROPOSED: 3,788 SF GFA OF LAB SPACE
ITE LAND USE: 110 GENERAL LIGHT INDUSTRIAL
SOURCE: ITE TRIP GENERATION, 10TH EDITION

3,788 SF GFA

				TRIP DISTRIBUTION		PROJECT TRIPS	
PERIOD	RATE	PER 1,000 SF	TRIPS	ENTER	EXIT	IN	OUT
AM	0.70	3.79	2.65	88%	12%	2.33	0.32
PM	0.63	3.79	2.39	13%	87%	0.31	2.08
AVG	4.96	3.79	18.79	50%	50%	9.39	9.39

Alchem Laboratories – Building 4

Statement of Proposed Uses

The proposed building will provide an additional 3,788 SF of light manufacturing space for an existing business located on the adjacent property.

Comprehensive Plan Consistency

Future Land Use Element:

Objective 1.5: Industrial
The City of Alachua shall establish one industrial district: Industrial. This district shall provide a broad range of clean industry, warehousing, research, and technology industries, to provide a variety of job opportunities to the citizens of Alachua and the North Central Florida Region.

Consistency: The proposed development will serve the intent of the Industrial future land use designation, as it will provide additional space for a growing company within Alachua. In addition, the site is consistent with the policies outlined in Future Land Use Policy 1.5.b and 1.5.d as indicated below:

Policy 1.5.b: The Industrial land use category may also include industrial service uses, office/business parks, biotechnology and other technologies, business incubators, self-storage facilities, a limited amount of retail sales and services, traditional neighborhood design planned developments, employment center planned developments, outdoor storage yard or lots, and construction industry uses either as allowed uses or with special exceptions.

Consistency: The proposed development will serve the intent of the Industrial future land use category. The site will include light industrial and office space for Alchem to expand their existing operation on the adjacent property.

Policy 1.5.d: The City shall develop performance standards for industrial uses in order to address the following:

1. *Integration of vehicular and non-vehicular access into the site and access management features of site in terms of driveway cuts and cross access between adjacent sites, including use of frontage roads and/or shared access;*

   Consistency: The development plan includes a sidewalks to serve the proposed building. Parking is proposed to the side of the existing and the proposed building on the site.

2. *Buffering from adjacent existing/potential uses;*

   Consistency: All required buffers and setbacks are shown on plans.

3. *Open space provisions and balance of proportion between gross floor area and site size;*

<u>Consistency</u>: The proposed site plan exceeds the 10% required open space and has less than the maximum 0.50 floor area ratio.

4. *Adequacy of pervious surface area in terms of drainage requirements;*

   <u>Consistency:</u> The proposed stormwater system will be approved through the Water Management District

5. *Placement of signage;*

   <u>Consistency:</u> The permitting of signs will occur under a separate process and those permits shall be prepared in compliance with the applicable criteria.

6. *Adequacy of site lighting and potential impacts of lighting upon the surrounding area. Lighting should be designed to minimize impacts and preserve the ambiance and quality of the nighttime sky by reducing light trespass and light pollution on adjacent properties by utilizing lighting at an appropriate intensity, direction and times to ensure light is not overused or impacting areas where it is not intended;*

   <u>Consistency:</u> The site plans include a photometric plan that complies with all elements of the Comprehensive Plan and Land Development Regulations.

7. *Safety of on-site circulation patterns (patron, employee and delivery vehicles), including parking layout and drive aisles, and points of conflict;*

   <u>Consistency:</u> The dumpster and loading zone are located at the side of the site to avoid conflicts with pedestrians and vehicles. Sidewalks are provided to serve the proposed building.

8. *Landscaping, as it relates to the requirements of the Comprehensive Plan and Land Development Regulations;*

   <u>Consistency</u>: The site plans include a landscape plan that complies with all elements of the Comprehensive Plan and Land Development Regulations.

9. *Unique features and resources which may constrain site development, such as soils, existing vegetation and historic significance; and*

   <u>Consistency:</u> There are no significant environmental resources on this site.

10. *Performance based zoning requirements, which may serve as a substitute for or accompany land development regulations in attaining acceptable site design.*

    <u>Consistency</u>: None are proposed.

11. *Industriall uses shall be limited to an intensity of less than or equal to .50 floor area ratio for parcels 10 acres or greater, .50 floor area ratio for parcels less than 10 acres but 5 acres or greater, a .75 floor area ratio for parcels less than 5 acres but greater than 1 acre, and 1.0 floor area ratio to parcels 1 acre or less.*

    <u>Consistency:</u> The industrial use on site has less than a 0.50 floor area ratio.

Barrett

Continued from Page 1A

Barrett plans to say she looks at every case "from the perspective of the losing party" and asks herself, "how I would view the decision if one of my children was the party I was ruling against."

The standard she sets in every case, she says, is: "Even though I would not like the result, would I understand that the decision was fairly reasoned and grounded in the law?"

She will address the legacy left by Justice Ruth Bader Ginsburg, who died in mid-September, kicking off a battle to fill the Supreme Court seat.

"I have been nominated to fill Justice Ginsburg's seat, but no one will ever take her place. I will be forever grateful for the path she marked and the life she led," Barrett plans to say.



"Courts are not designed to solve every problem or right every wrong in our public life," Judge Amy Coney Barrett says in her planned confirmation hearing remarks. DEMETRIUS FREEMAN/POOL FILE

Bader Ginsburg, who died in mid-September, kicking off a battle to fill the Supreme Court seat.

"I have been nominated to fill Justice Ginsburg's seat, but no one will ever take her place. I will be forever grateful for the path she marked and the life she led," Barrett plans to say.

Ballots

Continued from Page 3A

been made in vote-by-mail procedures, such as elections office members contacting voters through phone calls, email or mail to make them aware of any type of issue their ballot may have.

Ultimate rejection authority — particularly if there's a signature mistake on the ballot — lies with the county's canvassing board, which is made up of a county judge, county commissioner and Alachua's Supervisor of Elections Kim Barton.

Appeals for a rejected ballot can be submitted through an affidavit to the Supervisor of Elections Office until 5 p.m. on the Thursday after Election Day.

Pyche estimates the office has received 17,000 mail-in ballots so far. Early next week, they'll begin logic and accuracy testing to ensure voting equipment is working properly. Next Wednesday through Election Day, the counting begins.

To check the status of a mail-in ballot or voter registration, visit www.votealachua.com/My-Registration-Status.

PUBLIC NOTICE

A neighborhood workshop will be held to discuss a proposed development plan for an approx. 4,000 SF building on a portion of parcels 03231-004-000 & 03231-003-000 located in the 13200 block of Rachael Blvd in Alachua, FL. This is not a public hearing. The purpose of this meeting is to inform neighboring property owners of the proposed development and to seek their comments.

The meeting will be held digitally as a Zoom teleconference on Monday, October 26, 2020 at 5:30 pm. The teleconference can be accessed by the following information:

URL: https://us02web.zoom. us/j/5733319527
Meeting ID: 573 331 9527
Dial-in: (646) 558-8656

Following the teleconference, a recording of the workshop will be available at www.edafl.com/neighborhoodworkshops. Upon request, a link to the meeting can be requested by e-mailing the contact below. In addition, paper copies of all materials discussed at the workshop can be provided via US Mail. Comments on the development plan may also be submitted in writing, by e-mail, or over the phone by contacting the number below.

eda consultants, inc.
Contact: Sergio Reyes, P.E. of eda consultants inc.
E-mail: sreyes@edafl.com **Phone:** (352) 373-3541
GF-GH057375

PUBLIC NOTICE

A neighborhood workshop will be held to discuss a site plan for a proposed building addition and associated site improvements located on a tax parcel number 05920-001-002 at 12556 NW US Hwy 441, Alachua, FL.
This is not a public hearing. The purpose of this meeting is to inform neighboring property owners of the proposed development and to seek their comments.

The meeting will be held digitally as a Zoom teleconference on Monday, October 26, 2020 at 6:30 pm. The teleconference can be accessed by the following information:

URL: https://us02web.zoom.us/j/5733319527
Meeting ID: 573 331 9527
Dial-in: (646) 558-8656

Following the teleconference, a recording of the workshop will be available at www.edafl.com/neighborhoodworkshops. Upon receipt, a link to the meeting can be requested by e-mailing the contact below. In addition, paper copies of all materials discussed at the workshop can be provided via US Mail. Comments on the development plan may also be submitted in writing, by e-mail, or over the phone by contacting the number below.

eda consultants, inc.
Contact: Sergio Reyes, P.E. of eda consultants inc.
E-mail: sreyes@edafl.com **Phone:** (352) 373-3541
GF-GH057374

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GF-GH058502

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GF-GH059279

NEIGHBORHOOD WORKSHOP NOTICE

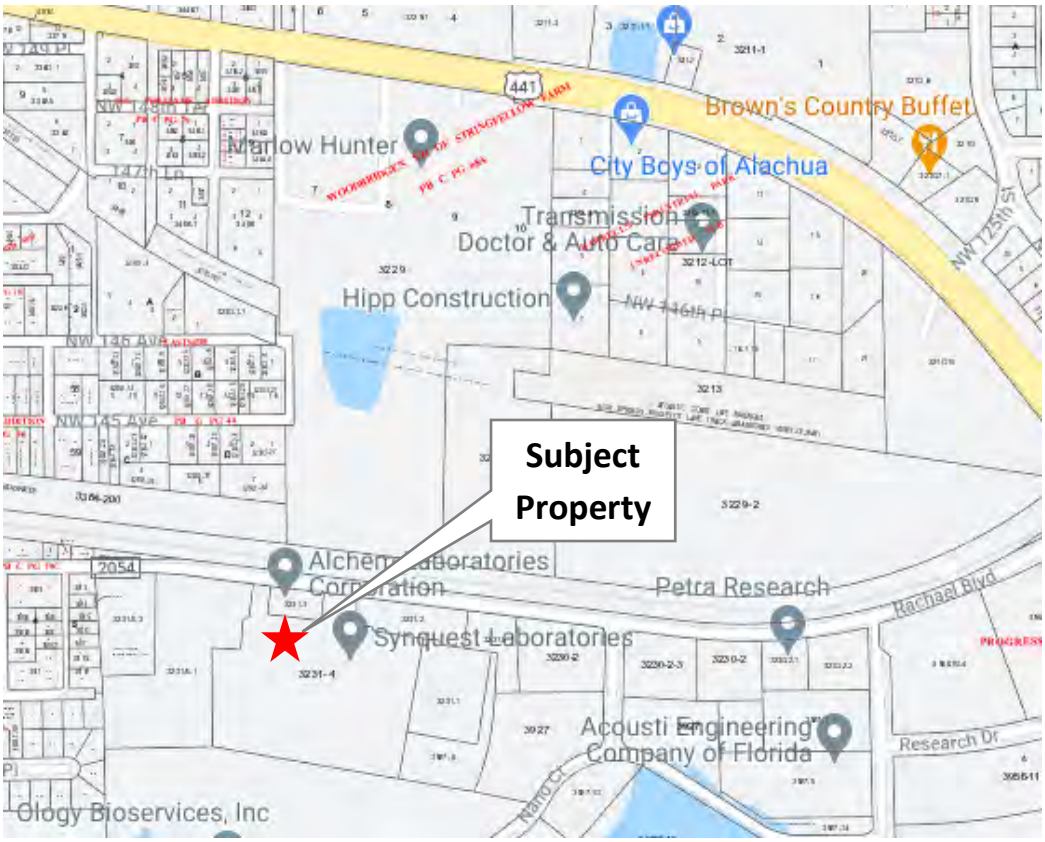
A neighborhood workshop will be held to discuss a proposed development plan for an approximately 4,000 square foot building on a portion of parcels 03231-004-000 and 03231-003-000 located in the 13200 block of Rachael Boulevard in Alachua, FL. This is not a public hearing. The purpose of this meeting is to inform neighboring property owners of the proposed development and to seek their comments. The meeting will be held digitally as a Zoom teleconference. The teleconference can be accessed by the following information:

Date:	Monday, October 26, 2020
Time:	5:30 pm
URL:	https://us02web.zoom.us/j/5733319527
Meeting ID:	573 331 9527
Dial-in Phone Number:	(646) 558-8656

Following the teleconference, a recording of the workshop will be available to view on **eda**'s website at http://edafl.com/neighborhoodworkshops/. Upon request, a link to the meeting can be requested by e-mailing the contact below. In addition, paper copies of all materials discussed at the workshop can be provided via US Mail. Comments on the development plan may also be submitted in writing to the address below or by phone by contacting the number below.

Contact:

Sergio Reyes, P.E. eda consultants, inc.
sreyes@edafl.com (352) 373-3541



03231-002-000
BLUMBERG GLENN
11421 NW 120TH TER
ALACHUA, FL 32615

03231-003-000
ALCHEM LABORATORIES CORP
13305 RACHEL BLVD
ALACHUA, FL 32615-6687

03934-001-001
TOUCHE NOLA L
13515 NW 142ND AVE
ALACHUA, FL 32615

03231-000-000
CONTEXT DEVELOPMENT CO
3020 CORAL WAY
MIAMI, FL 33145

03229-000-000
MARLOW ALACHUA HOLDINGS LLC
5212 SNEAD ISLAND RD
PALMETTO, FL 34221

03230-002-000
ADC DEVELOPMENT & INVESTMENT GROUP LLC
PO BOX 238
LAKE BUTLER, FL 32054

03231-001-000
BARBER LUMBER SALES OF ALACHUA INC
PO BOX 263
ALACHUA, FL 32616-0263

03518-001-000
BRYANT FLORA M
PO BOX 556
ALACHUA, FL 32616-0556

03934-001-020
ALLEN & ALLEN & ALLEN & FREENEY & JENKINS
12008 NW 148TH PL
ALACHUA, FL 32615

03230-001-031
WHITE JOANNA
13417 NW 145TH AVE
ALACHUA, FL 32615

03927-012-000
SNH MEDICAL OFFICE PROPERTIES TRUST
255 WASHINGTON ST STE 300
NEWTON, MA 02458

03384-200-000
CSX TRANSPORTATION INC
500 WATER ST TAX DEPARTMENT J-910
JACKSONVILLE, FL 32202-4423

03229-003-000
PHOENIX COMMERCIAL PARK
PO BOX 1000
ALACHUA, FL 32616

03231-005-000
ADC DEVELOPMENT & INVESTMENT GROUP LLC
PO BOX 238
LAKE BUTLER, FL 32054

03927-006-000
BARBER LUMBER SALES OF ALACHUA INC
PO BOX 263
ALACHUA, FL 32616-0263

03231-004-000
DUAL ENTERPRISES LLC
132 NW 76TH DR
GAINESVILLE, FL 32607

03934-001-019
JENKINS & MCDANIEL-JENKINS H/W
13510 NW 141ST PL
ALACHUA, FL 32615

03230-001-034
JOHNSON KENNON KEITH
269 CROSSGATE DR
ELMORE, AL 36025-1039

92060-517-900
CSX TRANSPORTATION INC
500 WATER ST TAX DEPARTMENT J-910
JACKSONVILLE, FL 32202-4423

03231-005-001
ADC DEVELOPMENT & INVESTMENT
PO BOX 238
LAKE BUTLER, FL 32054

03927-000-000
ADC DEVELOPMENT & INVESTMENT GROUP LLC
PO BOX 238
LAKE BUTLER, FL 32054

03231-005-003
COPELAND INDUSTRIAL PARK OWNERS ASSOCIATION INC
PO BOX 365
LAKE BUTLER, FL 32054

Antoinette Endelicato
5562 NW 93rd Avenue
Gainesville, FL 32653

Dan Rhine
288 Turkey Creek
Alachua, FL 32615

Tom Gorman
9210 NW 59th Street
Alachua, FL 32653

Richard Gorman
5716 NW 93rd Avenue
Alachua, FL 32653

Peggy Arnold
410 Turkey Creek
Alachua, FL 32615

David Forest
23 Turkey Creek
Alachua, FL 32615

President of TCMOA
1000 Turkey Creek
Alachua, FL 32615

Linda Dixon, AICP
Assistant Director of Planning
PO Box 115050
Gainesville, FL 32611

Craig Parenteau
FL Dept. of Environmental Protection
4801 Camp Ranch Road
Gainesville, FL 32641

Jeannette Hinsdale
P.O. Box 1156
Alachua, FL 32616

Lynn Coullias
7406 NW 126th Ave
Alachua, FL 32615

Lynda Coon
7216 NW 126 Avenue
Alachua, FL 32615

Tamara Robbins
PO Box 2317
Alachua, FL 32616

Michele L. Lieberman
County Manager
12 SE 1st Street
Gainesville, FL 32601

Bonnie Flynn
16801 NW 166th Drive
Alachua, FL 32615

Hugh & Jean Calderwood
P.O. Box 2307
Alachua, FL 32616

720 SW 2nd Ave.
South Tower, Suite 300
Gainesville, FL 32601



03231-000-000
CONTEXT DEVELOPMENT CO
3020 CORAL WAY
MIAMI, FL 33145

NSN

NIXIE 331 DE 1 0010/20/20
RETURN TO SENDER
NO SUCH NUMBER
UNABLE TO FORWARD
BC: 32601121275 *0638-00824-14-02

Notice of Online Neighborhood Workshop

Date: Monday, October 26, 2020 Time: 5:30 pm

<u>Property Address/Location of Project:</u>

Parcels 03231-004-000 & 03231-003-000

located in the 13200 block of Rachael Blvd

<u>Action Proposed:</u> Approx. 4,000 square foot building and associated site improvements

The Meeting will be held digitally on Zoom.

URL: https://us02web.zoom.us/j/5733319527

Meeting ID: 573 331 9527

Dial-in: (646) 558-8656



Please call **eda** at (352) 373-3541 with any questions or email sreyes@edafl.com for more information.

eda consultants · inc.

Neighborhood Meeting Minutes

Project: Proposed Development Plan

Meeting Date & Time: August 29, 2020 at 10:00 AM

Community Participants: 0

Project Representatives: Sergio Reyes, eda
Ashley Scannella, eda

Meeting Minutes:

No participants joined the virtual meeting, so no presentation was given.

Legal Description
Alachua County Tax Parcel 03231-003-000
City of Alachua, Alachua County, Florida

PARCEL 3:

A TRACT OF LAND SITUATED IN SECTION 14, TOWNSHIP 8 SOUTH, RANGE 18 EAST, ALACHUA COUNTY, FLORIDA, SAID TRACT OF LAND BEING MORE PARTICULARLY DESCRIBED AS FOLLOWS:

COMMENCE AT A CONCRETE MONUMENT AT THE SOUTHWEST CORNER OF THE SOUTHEAST QUARTER OF THE SOUTHWEST QUARTER OF THE AFOREMENTIONED SECTION 14 FOR A POINT OF REFERENCE AND THENCE RUN S.89 DEGREES 13'54" W., ALONG THE SOUTH LINE OF SAID SECTION 14, A DISTANCE OF 29.24 FEET TO A CONCRETE MONUMENT; THENCE RUN N.01 DEGREES 23'30" W., A DISTANCE OF 448.94 FEET TO AN IRON PIPE ON THE SOUTH RIGHT OF WAY LINE OF COUNTY ROAD 340-A (60 FOOT RIGHT OF WAY); THENCE RUN N. 88 DEGREES 37'00" E., ALONG SAID RIGHT OF WAY LINE , A DISTANCE OF 228.17 FEET TO A STEEL ROD AND CAP AT THE BEGINNING OF A CURVE CONCAVE SOUTHERLY, SAID CURVE HAVING A RADIUS OF 1723.55 FEET; THENCE RUN SOUTHEASTERLY, ALONG SAID RIGHT OF WAY LINE AND WITH SAID CURVE, THROUGH AN ARC ANGLE OF 07 DEGREES 09'49", AN ARC DISTANCE OF 215.49 FEET (CHORD BEARING AND DISTANCE OF S.86 DEGREES 47'30"E., 215.35 FEET RESPECTIVELY) TO A STEEL ROAD AND CAP; THENCE RUN S.83 DEGREES 11'05" E., ALONG SAID RIGHT OF WAY LINE, A DISTANCE OF 210.00 FEET TO A STEEL ROD AND CAP AND THE TRUE POINT OF BEGINNING; THENCE CONTINUE S.83 DEGREES 11'05"E., ALONG SAID RIGHT OF WAY LINE, A DISTANCE OF 196.08 FEET TO A STEEL ROD AND CAP; THENCE RUN S.05 DEGREES 50'37"W., A DISTANCE OF 101.42 FEET TO A NAIL AND CAP; THENCE RUN N.83 DEGREES 29'04" W., A DISTANCE OF 197.00 FEET TO A NAIL AND CAP; THENCE RUN N.06 DEGREES 22'07" E., A DISTANCE OF 102.44 FEET TO THE TRUE POINT OF BEGINNING.

TOGETHER WITH A WATERLINE EASEMENT OVER, UNDER AND ACROSS THE FOLLOWING DESCRIBED 10.00 FOOT WIDE STRIP OF LAND:

COMMENCE AT A CONCRETE MONUMENT AT THE SOUTHWEST CORNER OF THE SOUTHEAST QUARTER OF THE SOUTHWEST QUARTER OF THE AFOREMENTIONED SECTION 14 FOR A POINT OF REFERENCE AND THENCE RUN S.89 DEGREES 13'54" W., ALONG THE SOUTH LINE OF SAID SECTION 14, A DISTANCE OF 29.24 FEET TO A CONCRETE MONUMENT; THENCE RUN N.01 DEGREES 23'30"W., A DISTANCE OF 448.94 FEET TO AN IRON PIPE ON THE SOUTH RIGHT OF WAY LINE OF COUNTY ROAD 340-A (60 FOOT RIGHT OF WAY); THENCE RUN N.88 DEGREES 37'00"E., ALONG SAID RIGHT OF WAY LINE, A DISTANCE OF 228.17 FEET TO A STEEL ROD AND CAP AT THE BEGINNING OF A CURVE CONCAVE SOUTHERLY, SAID CURVE HAVING A RADIUS OF 1723.55 FEET; THENCE RUN SOUTHEASTERLY, ALONG SAID RIGHT OF WAY LINE AND WITH SAID CURVE, THROUGH AN ARC ANGLE OF 07 DEGREES 09'49", AN ARC DISTANCE OF 215.49 FEET (CHORD BEARING AND DISTANCE OF S.86 DEGREES 47'30"E., 215.35 FEET RESPECTIVELY) TO A STEEL ROD AND CAP; THENCE RUN S.83 DEGREES 11'05"E., ALONG SAID RIGHT OF WAY LINE, A DISTANCE OF 210.00 FEET TO A STEEL ROD AND CAP; THENCE CONTINUE S.83 DEGREES 11'05" E., ALONG SAID RIGHT OF WAY LINE, A DISTANCE OF 186.08 FEET TO A STEEL ROD AND CAP AND THE TRUE POINT OF BEGINNING; THENCE RUN S.05 DEGREES 50'37"W., A DISTANCE OF 101.47 FEET TO A NAIL AND CAP; THENCE RUN N.83 DEGREES 29'04"W., A DISTANCE OF 187.00 FEET TO A NAIL AND CAP; THENCE RUN N.06 DEGREES 22'07'E., A DISTANCE OF 10.46 FEET, THENCE RUN N.83 DEGREES 37'53"W., A DISTANCE OF 10.00 FEET; THENCE RUN S.06

DEGREES 22'07"W., A DISTANCE OF 20.44 FEET; THENCE RUN S.83 DEGREES 29'04"E., A DISTANCE OF 217.10 FEET, THENCE RUN N.05 DEGREES 50'37"E., A DISTANCE OF 111.37 FEET TO THE AFOREMENTIONED RIGHT OF WAY LINE, THENCE RUN N.83 DEGREES 11'05"W. ALONG SAID RIGHT OF WAY LINE, A DISTANCE OF 20.00 FEET TO THE TRUE POINT OF BEGINNING.

Legal Description
A Portion of Alachua County Tax Parcel 03231-004-000
City of Alachua, Alachua County, Florida

A portion of the parcel of land described in the Special Warranty Deed recorded in Official Records Book 2340, Page 1962 of the Public Records of Alachua County, Florida, lying in Section 14, Township 8 South, Range 18 East, Alachua County, Florida, being more particularly described as follows:

COMMENCING at the Northerly most Northwest corner of said parcel of land described in Official Records Book 2340, Page 1962, also being on the Southerly right-of-way line of Rachael Boulevard/County Road 340-A (66 foot right-of-way); thence on the Northerly line of said parcel of land the following 6 (six) courses and distances:

1. South 83 degrees 11 minutes 05 seconds East on said right-of-way line, a distance of 64.33 feet to a steel rod and cap;
2. Thence departing said right-of-way line South 06 degrees 22 minutes 07 seconds West, a distance of 102.44 feet to a nail and cap;
3. Thence South 83 degrees 29 minutes 04 seconds East, a distance of 197.00 feet to a nail and cap and the POINT OF BEGINNING of the following described parcel of land;
4. Thence North 05 degrees 50 minutes 37 seconds East, a distance of 101.42 feet to a steel rod and cap to a point on said right-of-way line;
5. Thence South 83 degrees 11 minutes 05 seconds East on said right-of-way line, a distance of 121.08 feet to a steel rod and cap;
6. Thence departing said right-of-way line South 00 degrees 42 minutes 22 seconds West, a distance of 102.61 feet to a nail and cap;

Thence departing said Northerly line North 82 degrees 54 minutes 40 seconds West, a distance of 130.28 feet to a nail and cap and the POINT OF BEGINNING.

Said lands lying and being in Section 14, Township 8 South, Range 18 East, City of Alachua, Alachua County, Florida and containing a total net area of 0.293 acres more or less (12,782 square feet more or less).

15.00
112.00
1142.00

*1177
Return to:
Abstract & Title Services, Inc.
3801 NW 40th Terrace, Suite B
Gainesville, Florida 32605

Prepared by and Return to:
Susan G. Goffman, Esquire
University of Florida Foundation, Inc.
Post Office Box 14425
Gainesville, FL 32604-2425

Tax parcel #: _____
Grantee TIN _____

RECORDED IN OFFICIAL RECORDS
INSTRUMENT # 1686755 3 PGS
2000 JUN 13 01:37 PM BK 2295 PG 2573
J. K. "BUDDY" IRBY
CLERK OF CIRCUIT COURT
ALACHUA COUNTY, FLORIDA
CLERK3 Receipt#018486
Doc Stamp-Deed: 1,127.00
By: _____ D.C.

SPECIAL WARRANTY DEED

Grantor, UNIVERSITY OF FLORIDA FOUNDATION, INC., a Florida corporation not for profit, in consideration of the sum of ten dollars and other valuable consideration received from Grantee, hereby grants and conveys to Grantee, ALCHEM LABORATORIES CORP., a Florida corporation, whose mailing address is 13305 Rachael Boulevard, Alachua, Florida 32615 the real property (Property) in Alachua County, Florida, described in Exhibit A hereto.

This conveyance is subject to covenants, conditions, restrictions, reservations, limitations, easements and other matters of record, and to real estate taxes for 2000 and subsequent years, and to a nonexclusive perpetual easement for a waterline over, across, under, and through the east ten feet of the Property, which is hereby reserved by Grantor.

As against all persons claiming by, through, or under Grantor, Grantor covenants that the Property is free of all encumbrances except as stated herein, that lawful seisin of and good right to convey the Property are vested in Grantor, and that Grantor fully warrants the title to the property and will defend the same against the lawful claims of all persons claiming by, through, or under Grantor.

Date this 25th day of May, 2000.

Witnesses:

Deborah C. Smith

Deborah C. Smith
{print 1st witness name}

P.A. Arcidiacono

P.A. ARCIDIACONO
{print 2nd witness name}

GRANTOR:
UNIVERSITY OF FLORIDA FOUNDATION, INC.

By: Leslie D. Bram

Leslie D. Bram
As its Associate Vice President for Administration
Post Office Box 14425
Gainesville, Florida 32604-2425

STATE OF FLORIDA

COUNTY OF ~~FLORIDA~~ Alachua

The foregoing instrument was executed, acknowledged, and delivered before me this 25th day of May, 2000, by Leslie D. Bram, as Associate Vice President for Administration for UNIVERSITY OF FLORIDA FOUNDATION, INC., a Florida corporation not for profit, on behalf of the corporation, who is personally known to me.

SUSAN G. GOFFMAN
My Comm Exp. 4/18/04
No. CC 903771
[ ] Personally Known [ ] Other I.D.

Susan G. Goffman
Notary Public, State and county aforesaid
Print Name: ~~Susan G. Goffman~~
Commission No:
My commission expires:

(Notarial Seal)

EXHIBIT "A"

A TRACT OF LAND SITUATED IN SECTION 14, TOWNSHIP 8 SOUTH, RANGE 18 EAST, ALACHUA COUNTY, FLORIDA, SAID TRACT OF LAND BEING MORE PARTICULARLY DESCRIBED AS FOLLOWS:

COMMENCE AT A CONCRETE MONUMENT AT THE SOUTHWEST CORNER OF THE SOUTHEAST QUARTER OF THE SOUTHWEST QUARTER OF THE AFOREMENTIONED SECTION 14 FOR A POINT OF REFERENCE AND THENCE RUN S.89 DEGREES 13' 54" W., ALONG THE SOUTH LINE OF SAID SECTION 14, A DISTANCE OF 29.24 FEET TO A CONCRETE MONUMENT; THENCE RUN N.01 DEGREES 23' 30" W., A DISTANCE OF 448.94 FEET TO AN IRON PIPE ON THE SOUTH RIGHT OF WAY LINE OF COUNTY ROAD 340-A (60 FOOT RIGHT OF WAY); THENCE RUN N. 88 DEGREES 37' 00" E., ALONG SAID RIGHT OF WAY LINE , A DISTANCE OF 228.17 FEET TO A STEEL ROD AND CAP AT THE BEGINNING OF A CURVE CONCAVE SOUTHERLY, SAID CURVE HAVING A RADIUS OF 1723.55 FEET; THENCE RUN SOUTHEASTERLY, ALONG SAID RIGHT OF WAY LINE AND WITH SAID CURVE, THROUGH AN ARC ANGLE OF 07 DEGREES 09' 49", AN ARC DISTANCE OF 215.49 FEET (CHORD BEARING AND DISTANCE OF S.86 DEGREES 47'30"E., 215.35 FEET RESPECTIVELY) TO A STEEL ROAD AND CAP; THENCE RUN S.83 DEGREES 11'05" E., ALONG SAID RIGHT OF WAY LINE, A DISTANCE OF 210.00 FEET TO A STEEL ROD AND CAP AND THE TRUE POINT OF BEGINNING; THENCE CONTINUE S.83 DEGREES 11' 05"E., ALONG SAID RIGHT OF WAY LINE, A DISTANCE OF 196.08 FEET TO A STEEL ROD AND CAP; THENCE RUN S.05 DEGREES 50'37"W., A DISTANCE OF 101.42 FEET TO A NAIL AND CAP; THENCE RUN N.83 DEGREES 29' 04" W., A DISTANCE OF 197.00 FEET TO A NAIL AND CAP; THENCE RUN N.06 DEGREES 22' 07" E., A DISTANCE OF 102.44 FEET TO THE TRUE POINT OF BEGINNING.

TOGETHER WITH A WATERLINE EASEMENT OVER, UNDER AND ACROSS THE FOLLOWING DESCRIBED 10.00 FOOT WIDE STRIP OF LAND:

COMMENCE AT A CONCRETE MONUMENT AT THE SOUTHWEST CORNER OF THE SOUTHEAST QUARTER OF THE SOUTHWEST QUARTER OF THE AFOREMENTIONED SECTION 14 FOR A POINT OF REFERENCE AND THENCE RUN S.89 DEGREES 13' 54" W., ALONG THE SOUTH LINE OF SAID SECTION 14, A DISTANCE OF 29.24 FEET TO A CONCRETE MONUMENT; THENCE RUN N.01 DEGREES 23'30"W., A DISTANCE OF 448.94 FEET TO AN IRON PIPE ON THE SOUTH RIGHT OF WAY LINE OF COUNTY ROAD 340-A (60 FOOT RIGHT OF WAY); THENCE RUN N.88 DEGREES 37'00"E., ALONG SAID RIGHT OF WAY LINE, A DISTANCE OF 228.17 FEET TO A STEEL ROD AND CAP AT THE BEGINNING OF A CURVE CONCAVE SOUTHERLY, SAID CURVE HAVING A RADIUS OF 1723.55 FEET; THENCE RUN SOUTHEASTERLY, ALONG SAID RIGHT OF WAY LINE AND WITH SAID CURVE, THROUGH AN ARC ANGLE OF 07 DEGREES 09'49", AN ARC DISTANCE OF 215.49 FEET (CHORD BEARING AND DISTANCE OF S.86 DEGREES 47'30"E., 215.35 FEET RESPECTIVELY) TO A STEEL ROD AND CAP; THENCE RUN S.83 DEGREES 11'05"E., ALONG SAID RIGHT OF WAY LINE, A DISTANCE OF 210.00 FEET TO A STEEL ROD AND CAP; THENCE CONTINUE S.83 DEGREES 11'05" E., ALONG SAID RIGHT OF WAY LINE, A DISTANCE OF 186.08 FEET TO A STEEL ROD AND CAP AND THE TRUE POINT OF BEGINNING; THENCE RUN S.05 DEGREES 50'37"W., A DISTANCE OF 101.47 FEET TO A NAIL AND CAP; THENCE RUN N.83 DEGREES 29'04"W., A DISTANCE OF 187.00 FEET TO A NAIL AND CAP; THENCE RUN N.06 DEGREES 22'07"E., A DISTANCE OF 10.46 FEET, THENCE RUN N.83 DEGREES 37'53"W., A DISTANCE OF 10.00 FEET; THENCE RUN S.06 DEGREES 22'07"W., A DISTANCE OF 20.44 FEET; THENCE RUN S.83 DEGREES 29'04"E., A DISTANCE OF 217.10 FEET, THENCE RUN N.05 DEGREES 50'37"E., A DISTANCE OF 111.37 FEET TO THE AFOREMENTIONED RIGHT OF WAY LINE, THENCE RUN N.83 DEGREES 11'05"W. ALONG SAID RIGHT OF WAY LINE, A DISTANCE OF 20.00 FEET TO THE TRUE POINT OF BEGINNING.

Prepared by and return to:
James D. Salter, Esq.
Attorney at Law
Salter Feiber, P.A.
3940 N.W. 16th Boulevard Bldg B
Gainesville, FL 32605
352-376-8201
File Number: 20-1284.2DE

[Space Above This Line For Recording Data]

Special Warranty Deed

This Special Warranty Deed made this 21st day of December, 2020, between **Dual Enterprises, L.L.C., a Florida limited liability company**, whose post office address is 3600 S. Ocean Shore Blvd., Unit 719, Flagler Beach, Florida, 32136, grantor, and **Alchem Laboratories Corporation, a Florida corporation** whose post office address is **13305 Rachael Blvd., Alachua, Florida, 32615**, grantee:

(Whenever used herein the terms grantor and grantee include all the parties to this instrument and the heirs, legal representatives, and assigns of individuals, and the successors and assigns of corporations, trusts and trustees)

Witnesseth, that said grantor, for and in consideration of the sum TEN AND NO/100 DOLLARS ($10.00) and other good and valuable considerations to said grantor in hand paid by said grantee, the receipt whereof is hereby acknowledged, has granted, bargained, and sold to the said grantee, and grantee's heirs and assigns forever, the following described land, situate, lying and being in **Alachua County, Florida**, to-wit:

See Exhibit "A" attached hereto and by reference made a part hereof.

Parcel Identification Number: 03231-004-000

Subject to covenants, conditions, restrictions, easements, reservations, and limitations of record, if any.

Together with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

To Have and to Hold, the same in fee simple forever.

And the grantor hereby covenants with said grantee that the grantor is lawfully seized of said land in fee simple; that the grantor has good right and lawful authority to sell and convey said land; that the grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons claiming by, through or under grantors, but against none other.

In Witness Whereof, grantor has hereunto set grantor's hand and seal the day and year first above written.

Signed, sealed and delivered in our presence:

Witness Name: RACHEL ALTY

Witness Name: James L. Miles

Dual Enterprises, L.L.C., a Florida limited Liability company

By: Adam C. Alty
Adam C. Alty, Manager

State of Florida
County of ~~Flagler~~ Volusia

The foregoing instrument was acknowledged before me by means of ☒ physical presence or ☐ online notarization, this 17 day of December, 2020, by Adam C. Alty as Manager of Dual Enterprises, L.L.C., a Florida limited liability company, who executed the same on behalf of the company, who is personally known ~~to me or has produced~~ FLDL as identification.

[Notary Seal]

James L. Miles
NOTARY PUBLIC
STATE OF FLORIDA
Comm# GG210264
Expires 4/23/2022

Notary Public
James L Miles
Name typed, printed or stamped
My Commission Expires: 4/23/2022

DoubleTime®

EXHIBIT "A"

The land referred to herein below is situated in the County of ALACHUA, State of Florida, and described as follows:

A portion of the parcel of land described in the Special Warranty Deed recorded in Official Records Book 2340, Page 1962 of the Public Records of Alachua County, Florida, lying in Section 14, Township 8 South, Range 18 East, Alachua County, Florida, being more particularly described as follows:

COMMENCING at the Northerly most Northwest corner of said parcel of land described in Official Records Book 2340, Page 1962, also being on the Southerly right-of-way line of Rachael Boulevard/County Road 340-A (66 foot right-of-way); thence on the Northerly line of said parcel of land the following 6 (six) courses and distances:
1. South 83 degrees 11 minutes 05 seconds East on said right-of-way line, a distance of 64.33 feet to a steel rod and cap;
2. Thence departing said right-of-way line South 06 degrees 22 minutes 07 seconds West, a distance of 102.44 feet to a nail and cap;
3. Thence South 83 degrees 29 minutes 04 seconds East, a distance of 197.00 feet to a nail and cap and the POINT OF BEGINNING of the following described parcel of land;
4. Thence North 05 degrees 50 minutes 37 seconds East, a distance of 101.42 feet to a steel rod and cap to a point on said right-of-way line;
5. Thence South 83 degrees 11 minutes 05 seconds East on said right-of-way line, a distance of 121.08 feet to a steel rod and cap;
6. Thence departing said right-of-way line South 00 degrees 42 minutes 22 seconds West, a distance of 102.61 feet to a nail and cap;
Thence departing said Northerly line North 82 degrees 54 minutes 40 seconds West, a distance of 130.28 feet to a nail and cap and the POINT OF BEGINNING.

Said lands lying and being in Section 14, Township 8 South, Range 18 East, City of Alachua, Alachua County, Florida

INCUMBENCY CONFIRMATION AND RESOLUTIONS
ON BEHALF OF DUAL ENTERPRISES, L.L.C.

We, **Adam C. Alty and Richard A. DuBoisson**, hereby certify that we are now, and at all times mentioned herein have been, the only Members and Managers of Dual Enterprises, L.L.C., a Florida limited liability company (the "LLC"), and we have access to the records of the LLC which records of the LLC reflect that:

1. Resolutions. Set forth below is a true and correct copy of resolutions which have been duly adopted by us on behalf of the LLC, none of such resolutions have been amended, modified, or repealed in any respect, and all of such resolutions are in full force and effect on the date hereof.

2. Incumbency. The following signatures set opposite our respective names are our true, authentic signatures.

Name	Office	Specimen Signature
Adam C. Alty	Manager/Member	Adam C. Alty
Richard A. DuBoisson	Manager/Member	RADB

We have duly executed this Certificate dated to be effective the _____ day of December, 2020.

Adam C. Alty

Richard A. DuBoisson

RESOLUTIONS

WHEREAS, it is proposed that the LLC, sell to Alchem Laboratories Incorporation, a Florida corporation, ("Alchem"), the real estate more particularly described in **Exhibit "A"**, attached hereto and made a part hereof for all purposes; and

WHEREAS, the proposed Warranty Deed and other papers evidencing, creating, governing or evidencing the sale, executed, or to be executed in connection therewith (the "Sale Documents"), have been submitted to, and reviewed by the Manager/Members in the LLC.

NOW, THEREFORE, RESOLVED, that the proposed sale and the proposed Sale Documents, be, and each hereby is, authorized and approved, and that either **Adam C. Alty or Richard A. DuBoisson** acting alone is hereby authorized, empowered and directed on behalf of the LLC to execute and deliver the Sale Documents for and on behalf and in the name of the LLC, with such changes in the terms and provisions thereof as the one executing the same shall, in his sole discretion, deem necessary or desirable and in the best interest of the LLC, his signature being conclusive evidence that he did so deem any such changes to be necessary or desirable and in the best interest of the LLC; and

FURTHER RESOLVED, that either **Adam C. Alty or Richard A. DuBoisson** acting alone is hereby, authorized, empowered and directed on behalf of the LLC to perform all acts and do all things which he may deem necessary or desirable to consummate the transactions contemplated by the Sale Documents, with such modifications, amendments or further deeds, assignments, bills of sale, certificates and other agreements, instruments or documents as he, in his sole discretion, may deem necessary or desirable and in the best interest of the LLC, his taking of any such action, for and on behalf and in the name of the LLC, and/or his execution and delivery, for and on behalf and in the name of the LLC, of any such agreement, instrument or document to be conclusive evidence that he did so deem the same to be necessary or desirable and in the best interest of the LLC; and

FURTHER RESOLVED, that the foregoing powers and authorization shall continue in full force and effect until written notice of revocation has been given to the LLC and its receipt obtained therefor.

This document may be executed in several counterparts, each of which shall be deemed an original and all such counterparts shall together constitute one and the same instrument.

EXHIBIT "A"

The land referred to herein below is situated in the County of ALACHUA, State of Florida, and described as follows:

A portion of the parcel of land described in the Special Warranty Deed recorded in Official Records Book 2340, Page 1962 of the Public Records of Alachua County, Florida, lying in Section 14, Township 8 South, Range 18 East, Alachua County, Florida, being more particularly described as follows:

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1. South 83 degrees 11 minutes 05 seconds East on said right-of-way line, a distance of 64.33 feet to a steel rod and cap;
2. Thence departing said right-of-way line South 06 degrees 22 minutes 07 seconds West, a distance of 102.44 feet to a nail and cap;
3. Thence South 83 degrees 29 minutes 04 seconds East, a distance of 197.00 feet to a nail and cap and the POINT OF BEGINNING of the following described parcel of land;
4. Thence North 05 degrees 50 minutes 37 seconds East, a distance of 101.42 feet to a steel rod and cap to a point on said right-of-way line;
5. Thence South 83 degrees 11 minutes 05 seconds East on said right-of-way line, a distance of 121.08 feet to a steel rod and cap;
6. Thence departing said right-of-way line South 00 degrees 42 minutes 22 seconds West, a distance of 102.61 feet to a nail and cap;
Thence departing said Northerly line North 82 degrees 54 minutes 40 seconds West, a distance of 130.28 feet to a nail and cap and the POINT OF BEGINNING.

Said lands lying and being in Section 14, Township 8 South, Range 18 East, City of Alachua, Alachua County, Florida

John Power TAX COLLECTOR
Serving Alachua County

Account Summary

Real Estate Account #03231 003 000

Owner:
ALCHEM LABORATORIES CORP

Situs:
13305 RACHAEL BLVD
ALACHUA 32615
Parcel details

Get bills by email

Amount Due

Your account is **paid in full**. There is nothing due at this time.
Your last payment was made on **02/04/2020** for **$8,571.03**. Print paid bill (PDF)

Account History

BILL	AMOUNT DUE
2019 Annual Bill	$0.00 Print (PDF)
2018 Annual Bill	$0.00 Print (PDF)
2017 Annual Bill	$0.00 Print (PDF)
2016 Annual Bill	$0.00 Print (PDF)
2015 Annual Bill	$0.00 Print (PDF)
2014 Annual Bill	$0.00 Print (PDF)
2013 Annual Bill	$0.00 Print (PDF)
2012 Annual Bill	$0.00 Print (PDF)
Total Amount Due	**$0.00**

BILL	AMOUNT DUE
2011 Annual Bill	$0.00 Print (PDF)
2010 Annual Bill	$0.00 Print (PDF)
2009 Annual Bill	$0.00 Print (PDF)
2008 Annual Bill	$0.00 Print (PDF)
2007 Annual Bill	$0.00 Print (PDF)
2006	
2006 Annual Bill	$0.00 Print (PDF)
Refund	
	Paid $9,784.72
2005 Annual Bill	$0.00 Print (PDF)
2004 Annual Bill	$0.00 Print (PDF)
2003 Annual Bill	$0.00 Print (PDF)
2002 Annual Bill	$0.00 Print (PDF)
Total Amount Due	**$0.00**

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John Power TAX COLLECTOR
Serving Alachua County

Account Summary

Real Estate Account #03231 004 000

Owner:
DUAL ENTERPRISES LLC

Situs:
13201 RACHAEL BLVD
ALACHUA 32615
ⓘ Parcel details

Get bills by email

Amount due

BILL	AMOUNT DUE
2020 Installment Bill #4	$6,897.19

Add to Cart

Print (PDF)

Account history

BILL	AMOUNT DUE
2020 ⓘ	
2020 Installment Bill #4 ⓘ	$6,897.19
Total Amount Due	**$6,897.19**

BILL	AMOUNT DUE
	Print (PDF)
2020 Installment Bill #3	$0.00
	Print (PDF)
2020 Installment Bill #2	$0.00
	Print (PDF)
2020 Installment Bill #1	$0.00
	Print (PDF)
	Paid $20,207.77
2019	
2019 Installment Bill #4	$0.00
	Print (PDF)
2019 Installment Bill #3	$0.00
	Print (PDF)
2019 Installment Bill #2	$0.00
	Print (PDF)
2019 Installment Bill #1	$0.00
	Print (PDF)
	Paid $27,604.40
2018	
2018 Installment Bill #4	$0.00
	Print (PDF)
2018 Installment Bill #3	$0.00
	Print (PDF)
2018 Installment Bill #2	$0.00
	Print (PDF)
2018 Installment Bill #1	$0.00
	Print (PDF)
Certificate #821	
	Paid $27,927.17
2017 Annual Bill	$0.00
	Print (PDF)
2016 Annual Bill	$0.00
	Print (PDF)
2015 Annual Bill	$0.00
	Print (PDF)
2014 Annual Bill	$0.00
	Print (PDF)
2013 Annual Bill	$0.00
Total Amount Due	$6,897.19

BILL	AMOUNT DUE
	Print (PDF)
2012 Annual Bill ⓘ	$0.00
	Print (PDF)
2011 Annual Bill ⓘ	$0.00
	Print (PDF)
2010 Annual Bill ⓘ	$0.00
	Print (PDF)
2009 Annual Bill ⓘ	$0.00
	Print (PDF)
2008 Annual Bill ⓘ	$0.00
	Print (PDF)
2007 Annual Bill ⓘ	$0.00
	Print (PDF)
2006 Annual Bill ⓘ	$0.00
	Print (PDF)
2005 Annual Bill ⓘ	$0.00
	Print (PDF)
2004 Annual Bill ⓘ	$0.00
	Print (PDF)
2003 Annual Bill ⓘ	$0.00
	Print (PDF)
2002 Annual Bill ⓘ	$0.00
	Print (PDF)
Total Amount Due	**$6,897.19**

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FLORIDA DEPARTMENT OF
Environmental Protection

Bob Martinez Center
2600 Blair Stone Road
Tallahassee, Florida 32399-2400

Ron DeSantis
Governor

Jeanette Nuñez
Lt. Governor

Noah Valenstein
Secretary

SELF-CERTIFICATION FOR A STORMWATER MANAGEMENT SYSTEM IN UPLANDS SERVING LESS THAN 10 ACRES OF TOTAL PROJECT AREA AND LESS THAN 2 ACRES OF IMPERVIOUS SURFACES

Owner(s)/Permittee(s):	Alchem Laboratories Corporation
File No:	0397918001EG
File Name:	ALCHEM BLDG ADDITION
Site Address:	13305 Rachael Blvd Alachua FL - 32615 6687
County:	Alachua
Latitude:	29° 47' 10.6417"
Longitude:	-82° 29' 12.3627"
Total Project Area:	0.76
Total Impervious Surface Area:	0.53
Approximate Date of Commencement of Construction:	
Registered Florida Professional:	CLAUDIA VEGA
License No.:	51532
Company:	eda consultants inc.

Date: January 28, 2021

Ashley Scannella certified through the Department's Enterprise Self-Service Application portal that the project described above was designed by the above-named Florida registered professional to meet the following requirements:

(a)The total project area involves less than 10 acres and less than 2 acres of impervious surface;
(b)Activities will not impact wetlands or other surface waters;
(c)Activities are not conducted in, on, or over wetlands or other surface waters;
(d)Drainage facilities will not include pipes having diameters greater than 24 inches, or the hydraulic equivalent, and will not use pumps in any manner;
(e)The project is not part of a larger common plan, development, or sale; and
(f)The project does not:
1.Cause adverse water quantity or flooding impacts to receiving water and adjacent lands;

2.Cause adverse impacts to existing surface water storage and conveyance capabilities;
3.Cause a violation of state water quality standards; or
4.Cause an adverse impact to the maintenance of surface or ground water levels or surface water flows established pursuant to s. 373.042 or a work of the district established pursuant to s. 373.086, F.S.

This certification was submitted before initiation of construction of the above project. The system is designed, and will be operated and maintained in accordance with applicable rules adopted pursuant to part IV of chapter 373, F.S. There is a rebuttable presumption that the discharge from such system will comply with state water quality standards.Therefore, construction, alteration, and maintenance of the stormwater management system serving this project is authorized in accordance with s.403.814(12), F.S.

In accordance with s. 373.416(2), F.S., if ownership of the property or the stormwater management system is sold or transferred to another party, continued operation of the system is authorized only if notice is provided to the Department within 30 days of the sale or transfer.This notice can be submitted to:

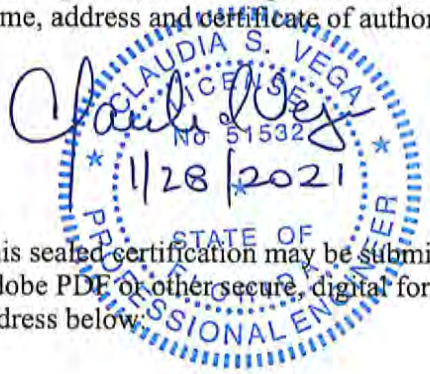
FDEP Northeast District
8800 Baymeadows Way West
Jacksonville, FL32256

This certification was submitted along with the following electronic documents:

File Description
Plans
Drainage Design Note
Deeds
Survey 1
Survey 2

If you have submitted this certification as a Florida Registered Professional, you may wish to sign and seal this certification, and return a copy to the Department, in accordance with your professional practice act requirements under Florida Statutes.

I, CLAUDIA VEGA, License No. 51532, do hereby certify that the above information is true and accurate, based upon my knowledge, information and belief. In the space below, affix signature, date, seal, company name, address and certificate of authorization (if applicable).



This sealed certification may be submitted to the Department, either electronically (as an attachment in Adobe PDF or other secure, digital format) at DEP_NED@dep.state.fl.us, or as a hardcopy, at the postal address below:

FDEP Northeast District
8800 Baymeadows Way West
Jacksonville, FL32256

To: Adam Hall, AICP, City Manager
Cc: Kathy Winburn, AICP, Planning & Community Development Director
Justin Tabor, AICP, Principal Planner
From: Marsha Swider, Project Manager
Date: 3/25/2021
RE: Development Review Team (DRT) Summary for: Alchem Building 4 Site Plan Application
Alternative Compliance Request

Dear Adam,

An alternative compliance is requested for two landscape requirements:

1. Site Landscaping Requirements, Canopy Tree Requirements Per 6.2.2.D.(1)(d)(i)
2. Site Perimeter Landscape Buffer Requirements Per 6.2.2.D.(3)

Per Section 6.2.2.(D)(10)(b)(i):
A reduction in the total number of required trees and/or alteration of the spacing requirements between trees when underground connections to public facilities or public utilities, or public easements or rights-of-way, are located upon or in close proximity to the parcel or whenever a fewer number of trees would be more desirable in terms of good landscape planning practice.

Due to the location of existing overhead and underground utilities along the eastern site of the site, neither landscape requirement can be met within the required area. To mitigate, the proposed canopy and understory trees within the proposed stormwater facility have been upsized to provide the total number of caliper inches that are required. This includes (3) canopy trees and (2) understory trees.

Additionally, per the City of Alachua Development Review Team request, photos of the existing vegetation located on the adjacent property have been provided for record. While the existing vegetation cannot mitigate the eastern perimeter buffer requirements for this development, photos have been included as part of the site evaluation. All of the trees photographed are within 10-20' of the adjacent property boundary.

These photos were captured by eda Project Manager, Marsha Swider, during a site visit conducted on Tuesday, February 23rd.

Quercus virginiana – Live Oak (1 @ 45”)





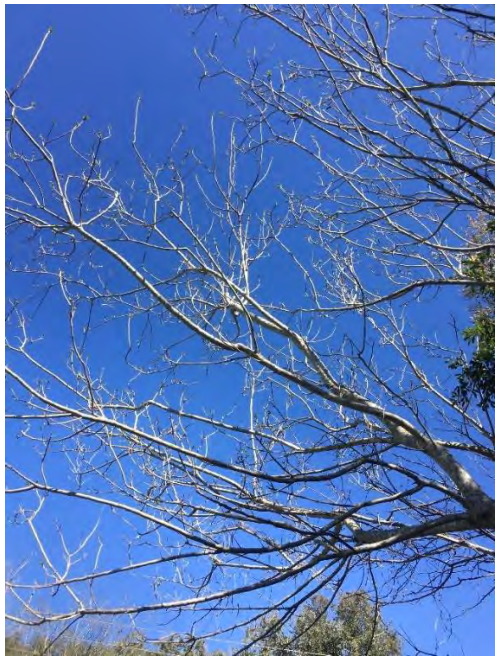
Prunus caroliniana – Carolina Cherry Laurel (1 @ 4”)



Additional trees were evaluated within the existing vegetation buffer as well, plant identification photographs are provided on the following pages. While these additional trees are not necessarily proposed to meet the eastern buffer requirement, they have been inventoried to provide a better representation of the density of the existing eastern buffer.

Albizia julibrissin – silk tree (1 @ 11.5”)







Sabal palmetto – Sabal Palm (6 @ 18-20”; lots of understory small)



Prunus caroliniana – Carolina Cherry Laurel (2.5”, 3”, 4”; 9 @ 2”-3”)






Ulmus americana – Florida Elm (1 @7”)





MEMORANDUM

ATTN:	**City of Alachua**	**DATE:**	**March 25, 2021**
FROM:	**eda consultants, inc.**	**EMAIL:**	**cvega@edafl.com**
SUBJECT:	**Alchem Building 4 Electrical Loads**		

Per the architect, the proposed Alchem Building 4 will require a 300-amp 115/230-volt single phase electrical service. A breakdown of the requirements is below. Please let us know if you have additional questions or comments.

NMR
208V 60 Hz / 20A single phase
110V for computer and controller

Mass Spec (2)
220V 20A

Everything else is 110

consultants inc.

Worksheet for Commercial Water Meter Sizing

Project Name : Alchem Laboratories - Building 4
Customer / Builder Name : Alchem Laboratories Corporation
Property Address : 13305 Rachael Blvd Alachua, FL 32615

Type	Requirement Per GRU/64E-6 (GPD)	Quantity	Unit	Total (GPD)
Office Building	15.00	38	per 100 SF of floor space	568
TOTAL DEMAND				568
Peaking Factor				2.5
Operating Period (hrs)				12
Peak Demand				**2 gpm**
TOTAL DEMAND				**2 gpm**
MINIMUM WATER METER SIZE		**3/4" Water Meter**		



720 SW 2nd Avenue, STE 300 Gainesville, FL 32606 · Phone: (352)373-3541 · Fax: (352)373-7249 · www.edafl.com

consultants inc.

Worksheet for Pressure Loss

Project Name : Alchem Laboratories - Bldg 4
Owner/Developer : Alchem Laboratories Corporation
Address : 13305 Rachael Blvd Alachua, FL 32615

I. DESIGN PARAMETERS

Required Fire Flow Rate (gpm)	1,500
Minimum Residual Pressure at Hydrant (psi)	46.20
Minimum Required Residual Pressure (psi)	20.00
Pressure at Fire Flow Test Location (ft)	138.60
Pressure at Fire Flow Test Location (psi)	60.00

- Fire Flow analysis results can be found in Attachment A.
- A diagram depicting the analyzed water line can be found in Attachment B.

A.	PIPE CHARACTERISTICS	EXISITING HYDRANT
	Diameter (in)	8
	Material	PVC
	Hazen-Williams Coefficient, C	150
	Velocity (ft/s)	**9.57**

B.	PIPE CHARACTERISTICS	PROPOSED RELOCATION
	Diameter (in)	8
	Material	PVC
	Hazen-Williams Coefficient, C	150
	Velocity (ft/s)	**9.57**

II. WATERMAIN CALCULATIONS

EXISTING FIRE HYDRANT

A. WATERMAIN HEAD LOSS

Type of Fittings and Pipe	Quantity	Length
Elbows - 90°	2	40.00
Elbows - 45°	0	0.00
Elbows - 22°	0	0.00
Gate Valve	0	0.00
Tee - Straight Flow	0	0.00
Tee - Turn Flow	1	40.00
Air Release Valve	0	0.00
Length of Pipe (ft)		126.50
Total Length (ft)		206.50
Head Loss (ft)		6.20
Head Loss (psi)		**2.68**

B. PRESSURE AT EXISTING HYDRANT

Upstream Pressure (psi)	60.00
Total Losses (psi)	2.68
Downstream Pressure (psi)	57.32
Minimum Required Residual Pressure (psi)	20.00
Excess Pressure (psi)	**40.00**

PROPOSED FIRE HYDRANT RELOCATION

A. HEAD LOSS FROM PROPOSED HYDRANT TO WATERMAIN

Type of Fittings and Pipe	Quantity	Length
Elbows - 90°	1	20.00
Elbows - 45°	0	0.00
Elbows - 22°	0	0.00
Elbows - 11°	0	0.00
Gate Valve	2	66.67
Tee - Straight Flow	0	0.00
Tee - Turn Flow	1	40.00
Air Release Valve		0.00
Length of Pipe (ft)		174.61
Total Length (ft)		301.27
Head Loss (ft)		9.04
Head Loss (psi)		**3.91**

B. PRESSURE AT PROPOSED HYDRANT

Pressure of Point of Connection	57.32
Total Losses (psi)	3.91
Destination Pressure (psi)	**53.40**
Minimum Residual Pressure (psi)	20.00
Excess Pressure (psi)	**33.40**

COUNTYWIDE STORMWATER TREATMENT CODE AFFIDAVIT OF COMPLIANCE

Instructions: Complete and submit this form for all activities that involve the construction of a stormwater management system that does not meet the exemptions identified in Chapter 77, Article III, Sec. 77.25 of the Alachua County Code. Please submit this form, along with the other required documents as listed in Sec. 77.28 of the Alachua County Code.

Part 1. Applicant Information	
Applicant/Owner: Claudia Vega, PE	Registered Professional: Claudia Vega, PE
Company: eda consultants, Inc.	Company: eda consultants, Inc.
Address: 720 SW 2nd Ave, South Tower, Suite 300, Gainesville, FL 32601	Address: 720 SW 2nd Ave, South Tower, Suite 300, Gainesville, FL 32601
Phone: 352-373-3541	Phone: 352-373-3541
Email: cvega@edafl.com	Email: cvega@edafl.com
Part 2. Project Information	
Project Name: Alchem Building 4	
Tax Parcel(s): 03231-003-000 and a portion of 03231-004-000	
☐ Unincorporated Alachua County ■ Municipality: City of Alachua	Note: For projects located in Municipalities submittal of this form is a self-certification of compliance. An acknowledgement of receipt will be sent once all required documents are provided. This acknowledgement is not a review of the submitted materials. Projects in Unincorporated Alachua County will be reviewed pursuant to Sec. 77.27 of the Alachua County Code.
Part 3. Stormwater Discharge Information Please select all that apply:	
Watershed Name:	Waterbody Identification (WBID) #:
■ Project discharges stormwater offsite as surface flow	☐ Project discharges stormwater to an Outstanding Florida Water (OFW)
☐ Project is in a WBID listed as impaired with nutrients as cause of impairment or where a nutrient TMDL has been established (net improvement criteria of Sec. 77.27 of the Alachua County Code applies).	
■ Project stormwater infiltrates to groundwater	☐ Project is located in a Sensitive Karst Area
Please briefly describe the Best Management Practices used: Proposed stormwater basin.	

Part 4. Exemption and Waiver Information
If applicable, Please list the exemption(s) this project qualifies for under Sec. 77.25 of the Alachua County Code. Supporting Documentation may be required. N/A
If applicable, Please describe the waiver you are requesting under Sec. 77.26 of the Alachua County Code. Supporting documentation is required. N/A

I authorize Alachua County, and its agents and contractors, to enter the property for the purpose of verifying compliance. If the property is sold and/or the entity responsible for operation and maintenance of the stormwater management system, the Property Owner will notify the Alachua County Environmental Protection Department within 30 calendar days of the sale or change in operation and maintenance entity. Failure to comply may result in enforcement action using the provisions of Alachua County Code Chapter 24 or any other remedy available by law or equity.

Applicant/Owner: Claudia S Vega (Printed Name) Signature: _____ Date: 8/31/2020

I hereby certify that the above referenced project meets, or is exempt from, the requirements of the Alachua County Code Chapter 77, Article III Stormwater Treatment Code. I further certify that the Operation and Maintenance requirements have been provided to the owner and entity responsible for operation and maintenance of the stormwater management system.

Registered Professional: Claudia S Vega (Printed Name) Florida Registration Number: 51532

Date: 12/21/2020 Signature: _____

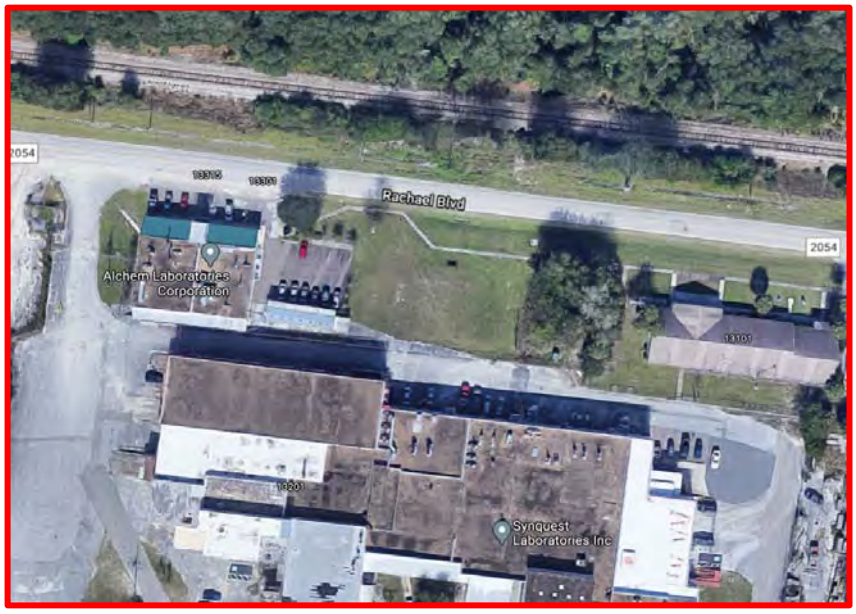
CLAUDIA S. VEGA LICENSE 51532 STATE OF FLORIDA PROFESSIONAL ENGINEER

eda consultants inc.

DRAINAGE DESIGN NOTES

for

Alchem - Building 4



Engineer of Record:
Claudia Vega
Cert. No. 51532

Project Designer:
Christina Griggs, EI

Submitted to:
City of Alachua
Suwannee River Water Management District

Submitted:
January 28, 2021

DRAINAGE DESIGN NOTES

Prepared for

Alchem - Building 4

Professional Engineer of Record:

Claudia Vega
Cert. No. 51532

TABLE OF CONTENTS

Contents

Executive Summary

Introduction

Design Criteria

Site Characteristics

Drainage Analysis

Attachments

Attachment A	Geotechnical Report by Universal Engineering Sciences
Attachment B	Aerial Photograph
Attachment C	Pre- and Post-Development Drainage Maps
Attachment D	Soils Map
Attachment E	Quad Map
Attachment F	FEMA Map
Attachment G	Pre- and Post-Development ICPR Model
Attachment H	Recovery Analysis
Attachment I	Pipe Calculations

EXECUTIVE SUMMARY

The Alchem Building 4 project proposes the demolition of an existing parkling lot and modular building and the construction of an office/laboratory building with associated drainage and utility improvements. A new dry retention pond is proposed to treat the proposed improvements.

The proposed dry retention pond, SMF, is located north side of the proposed office/laboratory building. The SMF will provide the required water quality treatment volume and attenuation of the 100-YR critical storm events including the 240-HR(10 day) such that the post-development discharge rate is less than the pre-development discharge rate, as required by Suwannee River Water Management District (SRWMD) and the City of Alachua (CoA).

The proposed SMF discharges through a discharge structure on the north side of the pond to the existing right-of-way where water flows by sheet flow today.

INTRODUCTION

The Alchem Building 4 project proposes the demolition of an existing parkling lot and modular building and the construction of an office/laboratory building with associated drainage and utility improvements. A new dry retention pond is proposed to treat the proposed improvements.

The proposed project is located in the City of Alachua on tax parcels 03231-000-003 and and a portion of 03231-000-004. The address of the project site is 10908 Rachael Blvd. Alachua, FL 32615 and the Section, Township, and Range of the project are S13 & S24, T8S, R18E. Figure 1 - Tax Parcel Map by Alachua County Property Appraiser shows the two tax parcels. Figure 2, on the next page, shows an Aerial of the project site. For FEMA information, please refer to Appendix G.

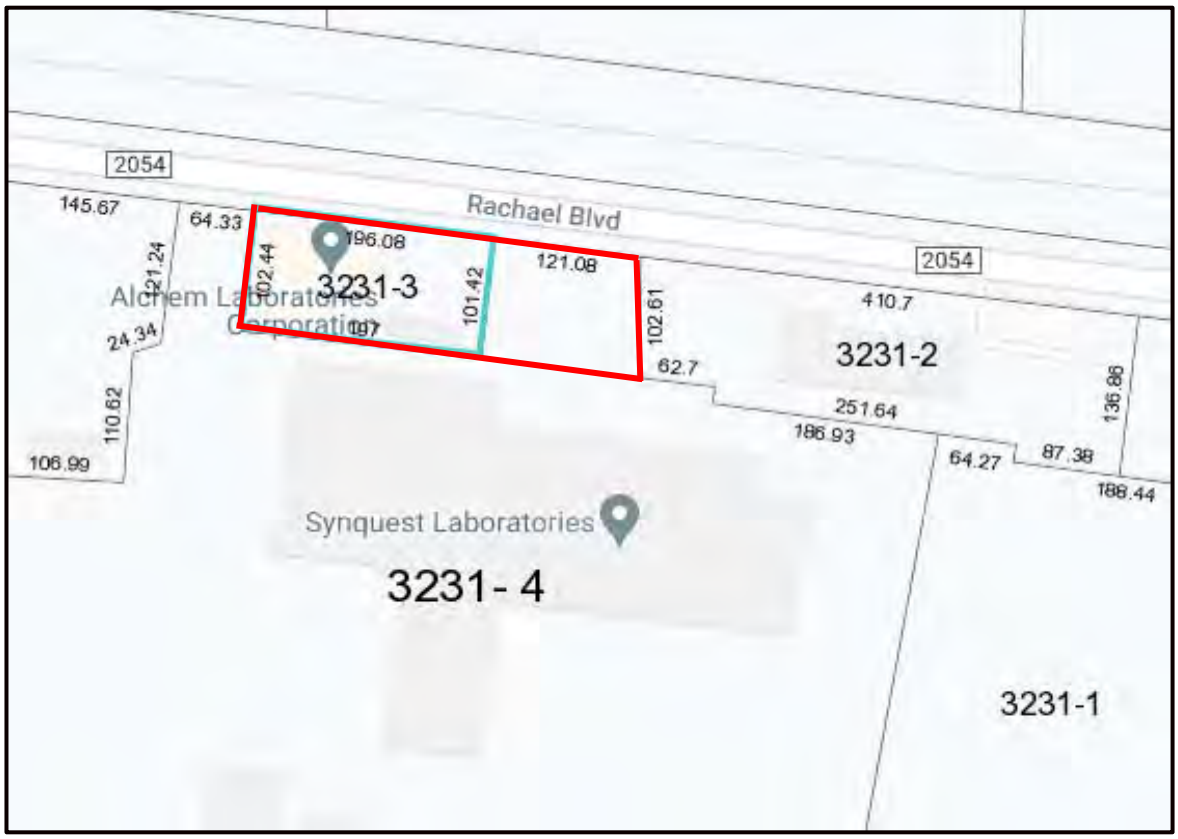


Figure 1 - Tax Parcel Map from Alachua County Property Appraiser



Figure 2 - Aerial of Project Site

The USGS quadrangle map can be found in Appendix F.

DRAINAGE DESIGN NOTES

A. PROJECT NAME Alchem - Building 4
Gainesville, Florida

B. PROJECT LOCATION

County: Alachua
Sections: 14 Township: 08 South Range: 18 East
General Location: 13305 Rachael Blvd
Alachua, FL 32615

C. GENERAL PROJECT INFORMATION

The proposed development consists of the construction of one office/laboratory building that is about 3,788 sf with associated parking, utility, and stormwater improvements.

D. DRAINAGE AND DESIGN CRITERIA

1. Suwannee River Water Management District (SRWMD)

2. City of Alachua, Florida

E. SITE SOILS INFORMATION

Universal Engineering Sciences, Inc. conducted a subsurface investigation on the site and summarized their findings in the report No. 1819436 dated November 20, 2020. A copy of the report is provided in Attachment A.

F. EXISTING SITE CONDITIONS

The existing site is partially developed. There is an existing laboratory building and modular building with a small parking lot and an open grassy area where the proposed building will be constructed.

G. DRAINAGE DESCRIPTION

1. Pre-development Conditions
   The site consists of an existing parking lot and modular building that will be demolished. There is not an existing stormwater facility on site to treat the current impervious area. Stormwater flows by sheet flow to the northern side of the site where it flows into the right-of-way to a road side swale.

2. Post-development Conditions
   The proposed stormwater conveyance system consists of one above ground dry retention basin that will provide water quality treatment , recovery, and attenuation as required by the Suwannee River Water Management District and the City of Alachua, Florida. Stormwater is conveyed to the dry detension system through an inlet and pipe where it discharges to the existing swale in the right-of-way as it does in pre-development conditions.

   Pre and Post-development conditions have been met as required by the Suwannee River Water Management District and the City of Alachua, Florida.

DRAINAGE DESIGN

1) PRE DEVELOPMENT DRAINAGE AREA

PRE DRAINAGE AREA	Area (sf)	Area (ac)	Curve CN
Existing Impervious	4,483	0.10	98.0
Basin	0	0.00	100.0
Open (Type C Soils)	9,018	0.21	39.0
TOTALS	**13,501**	**0.31**	**58.6**

2) POST DEVELOPMENT DRAINAGE AREAS

POST DRAINAGE AREA	Area (sf)	Area (ac)	Curve CN
Impervious	9,250	0.21	98.0
Open - Type A Soils	2,071	0.05	39.0
Basin	2,180	0.05	100.0
TOTALS	**13,501**	**0.31**	**89.3**

3) POST DEVELOPMENT BASIN STORAGE DATA

		SMF			
Stage (MSL)	Area (sf)	Area (ac)	Volume (cf)	Volume (ac-ft)	
61.00	663	0.015	0	0.00	
62.00	1,005	0.023	834	0.02	
62.65	1,240	0.028	1,564	0.04	*ORIFICE EL
63.00	1,545	0.035	2,051	0.05	
64.00	1,763	0.040	3,705	0.09	
65.00	2,180	0.050	5,677	0.13	

4) WATER QUALITY TREATMENT VOLUME

Basin 1 provides water quality treatment volume per SRWMD criteria for dry retention basins.

This criteria includes two thresholds, whichever of the two is greater plus an additional 0.50" over the total drainage area for online dry retention systems:

Volume V1 = 0.50 inches over the total area, or
Volume V2 = 1.25 inches over the impervious area
Volume V3 = 0.50 inches over the total area

Drainage Area	Volume V1 (cf)	Volume V2 (cf)	Volume V3 (cf)	Treat. Vol Required (cf)	Treat. Vol Provided (cf)
DA 1	563	964	563	1,526	**1,564**

Treatment Volume Required = [Greater of V1 or V2] + V3

6) SUBSURFACE INVESTIGATION INFORMATION

Based on the Soils Report No. 1819436, dated November 20, 2020, prepared by Universal Engineering Sciences, Inc. the recommendations of the soil characteristics are summarized below:

Soil Report No. 1819436	
Soil Boring	(B-1/B-2)
Average Ground El.	63.60
Depth Confined layer (ft)	5.75
Depth of groundwater (ft)	5.50
Veritcal (ft/d)	17.00
Safety factor	2.00
Veritcal Infiltration rate (ft/d)	8.50
Horizontal (ft/d)	22.00
Safety factor	2.00
Horizontal (ft/d)	11.00
Fillable porosity (%)	25.00

7) STRUCTURE INFORMATION

The stormwater management facilities will have a structures for discharge as follows:

CONTROL STRUCTURE	SMF
Orifice Elevation	62.65
Orifice Width	2.5"
Weir Elevation (MSL)	63.75
Weir Width (in)	6"

8) RECOVERY OF TREATMENT VOLUME FOR DRY RETENTION SYSTEM

The criteria for the recovery of the system is the recovery of the required water quality volume within 72 hours following the critical storm event.

WQTV Recovery	
Total WQTV (ac-ft):	0.04
Recovery Time (hrs):	7.2 hours

9) STORM ROUTING RESULTS

The computer program ICPR was used to route the critical design storms, the input data and results can be seen in Attachment C. The 'Runoff Area' is analyzed to compare the pre-existing runoff. Discharge rate and volume are reduced with the proposed design. 1' of minimum freeboard has been provided as required by SRWMD.

Storm	SMF		Rates		Volumes	
Event	Stage	Freeboard (ft)	Pre (cfs)	Post (cfs)	Pre (ac-ft)	Post (ac-ft)
100YR-001HR	63.57	1.43	0.60	0.16	0.02	0.02
100YR-002HR	63.70	1.30	0.55	0.17	0.04	0.03
100YR-004HR	63.98	1.02	0.48	0.36	0.06	0.06
100YR-008HR	63.93	1.07	0.58	0.31	0.08	0.08
100YR-024HR	63.35	1.65	0.21	0.14	0.14	0.12
100YR-072HR	63.34	1.66	0.18	0.14	0.20	0.13
100YR-168HR	63.16	1.84	0.13	0.12	0.25	0.12
100YR-240HR	63.42	1.58	0.17	0.14	0.30	0.13

Storm Event	Recovery (days)
100YR-001HR	0.86
100YR-002HR	1.14
100YR-004HR	1.64
100YR-008HR	1.89
100YR-024HR	3.25
100YR-072HR	6.35
100YR-168HR	11.70
100YR-240HR	14.87
WQTV	7.2 hours

Attachment A

Geotechnical Report by Universal Engineering Sciences

UNIVERSAL
ENGINEERING SCIENCES

REPORT OF GEOTECHNICAL CONSULTING SERVICES

Alchem Lab Building 4
Tax Parcel: 03231-004-000
Alachua, Alachua County, Florida

UES Project No. 0230.2000140
UES Report No. 1819436

Prepared for:

Alchem Laboratories Corp.
13305 Rachael Boulevard
Alachua, FL 32615

Prepared by:

Universal Engineering Sciences, LLC
4475 SW 35th Terrace
Gainesville, Florida 32608
(352) 372-3392

November 20, 2020

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- Pensacola
- Rockledge
- Sarasota
- Tampa
- West Palm Beach

November 20, 2020

Alchem Laboratories Corp.
13305 Rachael Boulevard
Alachua, FL 32615

Attention: Dr. James Talton, PhD
President & CEO

Reference: **Report of Geotechnical Consulting Services**
Alchem Laboratory Building 4
Tax Parcel: 03231-004-000
Alachua, Alachua County, Florida
UES Project No. 0230.2000140 UES Report No. 1819436

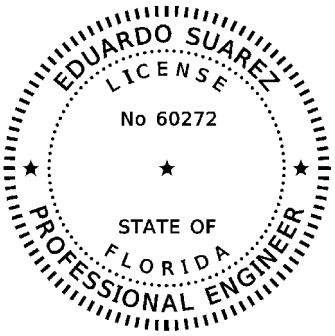
Dear Dr. Talton:

Universal Engineering Sciences, LLC (UES) has completed the geotechnical engineering services for the subject project in Alachua, Alachua County, Florida. This geotechnical Report is submitted in satisfaction of the contracted scope of services as summarized in UES Proposal No. 1811056, dated October 14, 2020.

This Report presents the results of our field subsurface exploration and laboratory soil testing programs, and recommendations for geotechnical site preparation, foundation design and construction, and stormwater management facilities.

We appreciate the opportunity to have assisted you on this project and look forward to a continued association. Please do not hesitate to contact our office if you should have any questions, or to assist your office with the remaining phases of project design and construction.

Respectfully submitted,
UNIVERSAL ENGINEERING SCIENCES, LLC
Certificate of Authorization 549



Timothy Kwiatkowski, P.E.
Project Geotechnical Engineer
Florida P.E. 86444

Eduardo Suarez, P.E.
Senior Geotechnical Engineer
Florida P.E. No. 60272

This item has been electronically signed and sealed by Eduardo Suarez, PE on the date adjacent to the seal using Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

TABLE OF CONTENTS

EXECUTIVE SUMMARY 1
1.0 INTRODUCTION 2
1.1 GENERAL 2
2.0 SCOPE OF SERVICES 2
2.1 PROJECT DESCRIPTION 2
2.2 PURPOSE 2
2.3 FIELD EXPLORATION 3
2.3.1 Standard Penetration Test (SPT) Borings 3
2.3.2 Auger Borings 3
2.4 LABORATORY TESTING 4
2.4.1 Visual Classification 4
2.4.2 Index Testing 4
3.0 FINDINGS 4
3.1 REGIONAL GEOLOGY 4
3.2 GENERAL AREA SOIL INFORMATION 4
3.3 SURFACE CONDITIONS 4
3.4 SUBSURFACE CONDITIONS 5
3.5 GROUNDWATER DEPTH 5
3.6 LABORATORY TESTING 5
3.6.1 Percent Passing No. 200 Sieve 5
3.6.2 Moisture Content 6
3.6.3 Atterberg Limits 6
3.6.4 Permeability 6
4.0 RECOMMENDATIONS 6
4.1 GENERAL 6
4.2 GEOTECHNICAL CONSIDERATIONS 7
4.3 GROUNDWATER CONSIDERATIONS 8
4.4 BUILDING FOUNDATION 8
4.4.1 Bearing Pressure 8
4.4.2 Foundation Size 8
4.4.3 Bearing Depth 9
4.4.4 Bearing Material 9
4.4.5 Settlement Estimates 9
4.4.6 Ground Floor Slab 9
4.5 SITE PREPARATION 10
4.6 STORMWATER MANAGEMENT SYSTEM 12
4.7 STORMWATER MANAGEMENT SYSTEM FILL SUITABILITY 12
4.8 CONSTRUCTION RELATED SERVICES 13
5.0 REPORT LIMITATIONS 13

APPENDIX A
Boring Location Plan A-1
Boring Logs ..... A-2 to A-9
Key to Boring Logs ..... A
APPENDIX B
Important Information About Your Geotechnical Engineering Report, Constraints and Restrictions, General Conditions ..... B
LIST OF TABLES
Table 1 – Laboratory Soil Test Results 6
Table 2 – Stormwater Management System Soil Design Parameters 12

EXECUTIVE SUMMARY

We have prepared this executive summary solely to provide a general overview. Do not rely on this executive summary for any purpose except that for which it was prepared. Rely on the full report for information about findings, recommendations, and other concerns.

Project Location and Description

The project parcel is located at Tax Parcel: 0321-004-000 in Alachua, Alachua County, Florida. Current site development plans include construction of a new laboratory building, and associated stormwater management facility. The new building will be 41 feet by 94 feet (3,854 square feet). Our office was provided with a Concept Plan, prepared by EDA, Inc.

Soil and Groundwater Conditions

The soil test borings generally encountered very loose to loose sand to sand with silt [SP/SP-SM] to depths of 4 to 12 feet below existing grades followed by silty-clayey sands to sandy clay [SM-SC/SC/CH] to maximum boring termination depths of 15 feet. Soil boring B-7 encountered somewhat shallow deleterious material (glass, debris) at a depth of 2.5 feet. The groundwater level was not generally encountered below existing site grades at the boring locations at the time of our exploration. As an exception, soil boring B-6 encountered a transient perched groundwater level at a depth of 7.5 feet. Fluctuations of groundwater level conditions on this project parcel should be expected to occur seasonally as a result of rainfall, surface runoff, and nearby construction activities. Based upon our review of regional hydrogeology and the Alachua County Soil Survey, we estimate the normal seasonal high groundwater level will occur more than 6 feet below the ground surface in the general area of the project site; however, stormwater can perch on the clayey sands when sandy soils are present at the surface. Isolated areas with a transient perched groundwater should be expected to occur. The perched groundwater will be a transient condition, directly related to rainfall, irrigation and site grading.

Site Preparation

Based on our exploration, the geotechnical considerations for the design and construction of the proposed structure is the presence of loose sand layers in the shallow subsurface soil profile,. We recommend normal, good practice site preparation procedures. These procedures include: stripping the site of existing vegetation and topsoil, compacting the subgrade and placing necessary fill or backfill to grade with engineered fill.

Foundation Design

A shallow foundation system may be used for support of the proposed construction on this project with the understanding that some aesthetic cracking and other minor architectural type nuisance issues may occur during the useful life of the structure. Following completion of the recommend geotechnical site preparation and building pad preparation activities, the proposed building may be supported on a shallow foundation system designed with a maximum average soil bearing pressure of 2,000 pounds per square foot (psf).

1.0 INTRODUCTION

1.1 GENERAL

In this report, we present the results of the subsurface exploration of the site for the proposed new development located in Alachua, Alachua County, Florida. We have divided this report into the following sections:

- SCOPE OF SERVICES - Defines what we did
- FINDINGS - Describes what we encountered
- RECOMMENDATIONS - Describes what we encourage you to do
- LIMITATIONS - Describes the restrictions inherent in this report
- APPENDICES - Presents support materials referenced in this report

2.0 SCOPE OF SERVICES

2.1 PROJECT DESCRIPTION

The project parcel is located at Tax Parcel: 0321-004-000 in Alachua, Alachua County, Florida. Current site development plans include construction of a new laboratory building, and associated stormwater management facility. The new building will be 41 feet by 94 feet (3,854 square feet). Our office was provided with a Concept Plan, prepared by EDA, Inc.

Our office was not provided with Foundation Plans or any other construction-related information other than that discussed herein. Considering the limitations stated above and based on prior experience with structures of this type, we assumed the following structural loading conditions: ground floor slab loads not exceeding 200 psf, a maximum of 3 kips per linear feet (klf) on wall footings, and a maximum load of 50 kips on individual footings. We understand the building construction will require nominal fill (2 feet or less) placement for leveling of the proposed building footprint and building pad construction.

We note that, our authorized scope of services and this Report do not address any other project elements, such as earth retaining walls, sidewalks, or slope stability issues that may be part of the overall project site plan. Since other site improvements could have detrimental effects on the performance of a foundation system at this site, UES, or other qualified geotechnical consultant, should be consulted to review the entire site development plan and conduct additional services as required to minimize any impact of associated improvements on foundation performance.

Our recommendations are based upon the above considerations. If any of this information is incorrect, or if you anticipate any changes, please inform Universal Engineering Sciences so that we may review our recommendations.

2.2 PURPOSE

The purposes of this exploration were:

- Perform a subsurface exploration within the suggested areas to gather information concerning the near-surface soil conditions,

- Conduct a laboratory soil testing program to aid in the classification of the prevailing site soils, and with the evaluation of relevant soil strength and engineering properties,

- Classify and stratify the various soil strata encountered in the soil test borings,
- Evaluate the groundwater level in the area of exploration and make appropriate recommendations,
- Prepare building foundation design and construction recommendations for the proposed building,
- Recommend appropriate subsurface soil design parameter values for design of the on-site stormwater management system.

This report presents an evaluation of site conditions on the basis of traditional geotechnical procedures for site characterization. The recovered samples were not examined, either visually or analytically, for chemical composition or environmental hazards. Universal Engineering Sciences would be pleased to perform these services, if you desire.

Our exploration was confined to the zone of soil likely to be stressed by the proposed construction. Our work did not address the potential for surface expression of deep geological conditions such as sinkholes. This evaluation requires a more extensive range of field services than performed in this study. We will be pleased to conduct an investigation to evaluate the probable effect of the regional geology upon the proposed construction, if you desire.

2.3 FIELD EXPLORATION

The field geotechnical testing activities were started and completed on November 4, 2020. Field tests for the geotechnical study included six (6) soil test borings to a depth of 15 feet below the ground surface within the limits of the proposed building area, and two (2) soil test borings to a depth of 15 feet within the proposed stormwater management area. The actual test locations shown are approximate and were staked in the field by UES personnel using existing landmarks and site features. All boreholes were backfilled upon field work completion. The soil test boring locations are shown in the attached Boring Location Plan drawing.

Representative portions of the subsurface soil samples recovered were transported to our Gainesville soils laboratory. The soil samples were visually classified by an experienced geotechnical engineer. It should be noted that soil conditions might vary between soil test boring locations, and between the subsurface soil strata interfaces which have been shown on the Boring Logs. The soil test boring data reflect information from the specific test locations only.

2.3.1 Standard Penetration Test (SPT) Borings

Penetration tests were performed in accordance with ASTM Procedure D-1586, Penetration Test and Split-Barrel Sampling of Soils. This test procedure generally involves driving a 1.4-inch I.D. split-tube sampler into the soil profile in six inch increments for a minimum distance of 18 inches using a 140-pound hammer free-falling 30 inches. The total number of blows required to drive the sampler the second and third 6-inch increments is designated as the N-value, and provides an indication of in-place soil strength, density and consistency.

2.3.2 Auger Borings

Auger borings were performed in accordance with ASTM Procedure D-1452, *Standard Practice for Soil Investigation and Sampling by Auger Borings*. This test procedure advances a solid stem auger into the soil in a manner which reduces soil disturbance. At the selected depth, the auger advance and rotation are stopped, and the auger flight retracted from the borehole. The

in-place soil profile is determined by visual inspection of the soils recovered from the auger flights.

2.4 LABORATORY TESTING

2.4.1 Visual Classification

The soil samples recovered from the soil test borings were returned to our laboratory where an engineer visually reviewed the field descriptions in accordance with ASTM D-2488. We then selected representative soil samples for laboratory testing. Using the results of the laboratory tests, our visual examination, and our review of the field boring logs we classified the soil borings in accordance with the current Unified Soil Classification System (USCS).

2.4.2 Index Testing

Laboratory testing was performed on selected samples of the soils encountered in the field exploration to better define soil composition and properties. Testing was performed in accordance to ASTM procedures and included Percent Passing No. 200 Sieve (ASTM D-1140), moisture content (ASTM D-2216), Atterberg Limits (ASTM D-4318), and permeability testing (ASTM D-2434). The test results have been presented on the attached Boring Logs.

3.0 FINDINGS

3.1 REGIONAL GEOLOGY

The general geology of central Alachua County is characterized by a surface veneer of Pleistocene and Pliocene sands and sandy clays overlying the Miocene-age Hawthorn Group. The Hawthorn Group includes a highly variable mixture of interbedded quartz sands, clays, carbonates, pebbles and grains occurring with thicknesses of up to 150 feet. In the general area of the subject project, it is anticipated that the Hawthorn Group is laterally discontinuous and perforated.

The general hydrogeology of Alachua County consists of three aquifer systems; a surficial aquifer, an intermediate aquifer, and the Floridan aquifer system. The surficial aquifer exists as an unconfined water table situated over the impermeable Hawthorn Group and is usually a subdued reflection of surface topography. The intermediate aquifer system includes all rocks that collectively retard the exchange of water between the overlying surficial aquifer system and the underlying Floridan aquifer system. Water in this system is contained under confined conditions. The Floridan aquifer system is a thick, carbonate sequence that functions regionally as a water-yielding hydraulic unit. Water exists under confined conditions.

3.2 GENERAL AREA SOIL INFORMATION

The United States Department of Agriculture (USDA) *Soil Survey of Alachua County*, *Florida* describes the near-surface soil profile in the project parcel as Urban Land. The soil classification for this site indicate that between 30 and 45 percent of the area is “urban land,” which is made up of sidewalks, driveways, and existing buildings. The soil may have been previously impacted by other construction work.

3.3 SURFACE CONDITIONS

UES personnel visited the project site prior to and during the performance of the field portion of this geotechnical study. Our on-site observations have been summarized as follows. At the time

of our exploration, the project parcel was undeveloped and clear. Exposed surface soils were observed to be sandy and dry. Surface organic soils, surface debris, or rock outcroppings were not observed on the project site.

Based on the Topographic Survey from the Alachua County Growth Management website, the site grades range from a high of about +68 feet NAVD, to a low of about +64 feet NAVD, sloping from the south to the north. We anticipate the building limits may be cut a few feet into grade at the south end, and will require structural fill placement operations (2 feet or less) for leveling of the proposed building footprint and building pad construction.

3.4 SUBSURFACE CONDITIONS

The soil test borings performed beneath the proposed were reviewed to evaluate the subsurface soil strata lateral continuity and uniformity, both parameters that would have an impact in foundation system selection and performance. Soil classifications and descriptions for this geotechnical study are based both on the results of the laboratory soil testing programs and on visual examinations of soil specimens by the Geotechnical Engineer. The subsurface soil conditions encountered in the soil test borings have been summarized in the attached Boring Logs and described below.

The soil test borings generally encountered very loose to loose sand to sand with silt [SP/SP-SM] to depths of 4 to 12 feet below existing grades followed by silty-clayey sands to sandy clay [SM-SC/SC/CH] to maximum boring termination depths of 15 feet. Soil boring B-7 encountered somewhat shallow deleterious material (glass, debris) at a depth of 2.5 feet.

3.5 GROUNDWATER DEPTH

The groundwater level was not generally encountered below existing site grades at the boring locations at the time of our exploration. As an exception, soil boring B-6 encountered a transient perched groundwater level at a depth of 7.5 feet. Fluctuations of groundwater level conditions on this project parcel should be expected to occur seasonally as a result of rainfall, surface runoff, and nearby construction activities.

3.6 LABORATORY TESTING

The soil samples recovered from the field exploration program were placed in containers and returned to our soils laboratory, where the Geotechnical Engineer visually examined and classified the samples. Laboratory soil tests are performed to aid in the classification of the soils, and to help in the evaluation of engineering characteristics of the soils. Representative soil samples were selected for Atterberg Limits, permeability testing, moisture content and percent fines determination. The test results have been presented on the attached Boring Logs and summarized in Table 1.

3.6.1 Percent Passing No. 200 Sieve

Certain recovered soil samples were selected to determine the percentage of fines. In these tests the soil sample was dried and washed over a U.S. No. 200 mesh sieve. The percent of soil by weight passing the sieve was the percentage of fines or portion of the sample in the silt and clay size range. This test was conducted in accordance with ASTM Procedure D-1140, *Standard Test Methods for Amount of Material in Soils Finer than the No. 200 Sieve.*

3.6.2 Moisture Content

Certain recovered soil samples were selected to determine their moisture content. The moisture content is the ratio expressed as a percentage of the weight of water in a given mass of soil to the weight of the solid particles. This test was conducted in accordance with ASTM Procedure D-2216, Standard Test Method for Laboratory Determination of Water (Moisture) Content of Soil and Rock.

3.6.3 Atterberg Limits

Certain recovered soil samples were selected for Atterberg Limits testing to evaluate the soil plasticity characteristics. The soil's Plasticity Index (PI) is the range of moisture content over which the soil deforms as a plastic material. It is bracketed by the Liquid Limit (LL) and the Plastic Limit (PL). The LL is the moisture content at which the soil will flow as a heavy viscous fluid. The PL is the lowest moisture content at which the soil is sufficiently plastic so as to be manually rolled into a 1/8-inch diameter thread. These tests were conducted in accordance with ASTM Procedure D-4318, *Standard Test Methods for LL, PL and Plasticity Index of Soils.*

3.6.4 Permeability

Representative soil samples were selected to determine the permeability rate of the soil. Constant head permeability tests were performed on remolded representative samples of the near surface soils from the proposed stormwater management area. These tests were conducted following the concepts outlined in ASTM D-2434, Standard Test Method for Permeability of Granular Soils (Constant Head and Falling Head).

Table 1 – Laboratory Soil Test Results

Soil Boring	Sample Depth	Type of Test	Results	Soil Description
B-1	3 feet	% Finer #200	7 %	Sand with silt
		Moisture Content	7 %	
		Permeability	15 feet/day	
B-2	4.5 feet	% Finer #200	6 %	Sand with silt
		Moisture Content	4 %	
		Permeability	20 feet/day	
B-3	3 feet	% Finer #200	4 %	Sand
		Moisture Content	3 %	
B-8	6.5 feet	% Finer #200	24 %	Clayey Sand
		Moisture Content	15 %	
		Atterberg Limits	LL = 31 %, PI = 15 %	

4.0 RECOMMENDATIONS

4.1 GENERAL

The following recommendations are made based upon a review of the attached soil test data, our understanding of the *client's willingness to accept nuisance and aesthetic movements of the structure*, and experience with similar projects and subsurface conditions. If plans change from

those discussed previously, we request the opportunity to review and possibly amend our recommendations with respect to those changes.

Additionally, if subsurface conditions are encountered during construction, which were not encountered in the borings, report those conditions immediately to us for observation and recommendations. In this section of the report, we present our detailed recommendations for groundwater control, building foundations, site preparation, and construction related services.

4.2 GEOTECHNICAL CONSIDERATIONS

Recommendations for foundation design are dependent, among other factors, on the amount of total settlement and more importantly differential settlement between various structural elements that can be safely tolerated by the structure. If the anticipated total and differential settlements estimated herein exceed the tolerable limits as set forth by the Structural Engineer, we should be so advised so that we may consider other foundation system alternatives.

It should further be noted that the estimated magnitudes of total and differential settlements are dependent on foundation loading conditions among other factors, and that we have made certain assumptions regarding those loading conditions in this Report. If unusually heavy foundation loading conditions are expected for some of the proposed project elements, or if our estimates vary significantly from actual anticipated conditions, we should be so advised so that we may revisit our engineering evaluations and foundation settlement estimates.

Based on our exploration, the geotechnical considerations for the design and construction of the proposed structure is the presence of loose sand layers in the shallow subsurface soil profile. Subsurface sandy soils will undergo immediate settlements upon application of surface loads, and this could be a result of both fill placement for building pad construction and building construction. In light of the above, it becomes important to compact the subgrade soils as much as possible prior to both building pad fill placement and foundation construction.

Another geotechnical consideration is the presence of somewhat shallow buried debris/deleterious material that will need to be removed. Soil boring B-7 encountered somewhat shallow deleterious material (glass, debris) at a depth of 2.5 feet. We recommend test pits are dug in the vicinity of the proposed structures to delineate and evaluate the soil conditions. All debris, deleterious material that may be identified should be removed in its entirety. Excavated zones will need to be replaced with compacted structural fill soils.

Our local experience has found that clay layers are often laterally discontinuous, which makes it more difficult to ascertain their presence on a given project parcel with a few soil test borings. If at the time of construction the contractor encounters or suspects that clay soils may be near the grade slab or foundation bearing elevations, UES should be contacted to prepare appropriate recommendations. If encountered, these shallow deposits of clay soils must be addressed through site grading, over-excavation and replacement, site drainage and stiffened foundation.

We recommend that we be provided the opportunity to review the project plans and specifications to confirm that our recommendations have been properly interpreted and implemented. If the structural loadings or the building location change significantly from those discussed previously, we request the opportunity to review and possibly amend our recommendations with respect to those changes. The discovery of any subsurface conditions during construction which deviate from those encountered in the borings should be reported to us immediately for observation, evaluation, and recommendations.

4.3 GROUNDWATER CONSIDERATIONS

The groundwater level will fluctuate seasonally depending upon local rainfall. The rainy seasons in North Florida are normally between June and September and December and February. Based upon our review of regional hydrogeology and the Alachua County Soil Survey, we estimate the normal seasonal high groundwater level will occur more than 6 feet below the ground surface in the general area of the project site; however, stormwater can perch on the clayey sands when sandy soils are present at the surface. Isolated areas with a transient perched groundwater should be expected to occur. The perched groundwater will be a transient condition, directly related to rainfall, irrigation and site grading.

It should be noted that the normal estimated seasonal high water levels do not provide any assurance that groundwater levels will not exceed these estimated levels during any given year in the future. Should the impediments to surface water drainage be present, or should rainfall intensity and duration, or total rainfall quantities, exceed the normally anticipated rainfall quantities, groundwater levels might once again exceed our seasonal high estimates. One potential case includes a perched water table above clayey sand or sandy clay layers. We recommend positive drainage be established and maintained on the site during construction. We further recommend permanent measures be constructed to maintain positive drainage from the site throughout the life of the project.

4.4 BUILDING FOUNDATION

Based on the results of our exploration, we consider the subsurface conditions at the site adaptable for support of the proposed building when constructed on a properly designed conventional shallow foundation system.

A conventional shallow foundation system may be used for support of the proposed building construction on this project with the understanding that some aesthetic cracking and other minor architectural type nuisance issues may occur during the useful life of the structure. Our recommendations provide for prudent geotechnical site preparation and foundation construction methods in light of the above conditions. Provided the site preparation and earthwork construction recommendations outlined in Section 4.5 of this report are performed, the following parameters may be used for foundation design.

4.4.1 Bearing Pressure

The net maximum allowable soil bearing pressure for use in shallow foundation design should not exceed 2,000 psf. Net bearing pressure is defined as the soil bearing pressure at the foundation bearing level in excess of the natural overburden pressure at that level. The foundations should be designed based on the maximum load which could be imposed by all loading conditions.

4.4.2 Foundation Size

The minimum widths recommended for any isolated column footings and continuous wall footings are 24 inches and 18 inches, respectively. Even though the maximum allowable soil bearing pressure may not be achieved, these width recommendations should control the minimum size of the foundations.

4.4.3 Bearing Depth

The exterior foundations should bear at a depth of at least 18 inches below the finished exterior grades and the interior foundations should bear at a depth of at least 12 inches below the finish floor elevation to provide confinement to the bearing level soils. It is recommended that stormwater be diverted away from the building exteriors to reduce the possibility of erosion beneath the exterior footings.

Foundations adjacent to a slope surface should be supported on compacted material with a setback of more than one third of the height of the slope. Set back is measured horizontally from edge of the footing to the edge of the slope or in steeper slopes (100 percent) from an imaginary plane projecting upward at 1:1 (horizontal to vertical) from the toe of the slope.

4.4.4 Bearing Material

The foundations may bear in either the compacted suitable existing soils or compacted structural fill. The bearing level soils, after compaction, should exhibit densities equivalent to at least 95 percent of the modified Proctor maximum dry density (AASHTO T-180) to a depth of at least **two feet** below the foundation bearing level. We recommend that the bottom of all footings be probed as to confirm the suitability of the bearing soils.

4.4.5 Settlement Estimates

Post-construction settlement of the structure will be influenced by several interrelated factors, such as (1) subsurface stratification and strength/compressibility characteristics; (2) footing size, bearing level, applied loads, and resulting bearing pressures beneath the foundations; and (3) site preparation and earthwork construction techniques used by the Contractor. Our settlement estimates for the structure are based on the use of site preparation/earthwork construction techniques as recommended in Section 4.5 of this report. Any deviation from these recommendations could result in an increase in the estimated post-construction settlement of the structure.

We expect the majority of settlement to occur in an elastic manner and fairly rapidly during construction. Using the recommended maximum bearing pressure, the assumed maximum structural loads and the field data which we have correlated to geotechnical strength and compressibility characteristics of the subsurface soils, we estimate that total settlements of the structures could be on the order of 1 inch or less.

Differential settlement results from differences in applied bearing pressures and variations in the compressibility characteristics of the subsurface soils. Because of the general uniformity of the subsurface conditions and the recommended site preparation and earthwork construction techniques outlined in Section 4.5, we anticipate that differential settlement of the structure should be within tolerable magnitudes (½ inch or less).

4.4.6 Ground Floor Slab

The floor slab can be constructed as a slab-on-grade provided the subgrade and subsequent lifts of structural backfill are compacted and tested in accordance with the recommendations included in this report. The floor slab can be designed using a modulus of subgrade reaction (K) of 100 pounds per cubic inch (pci). It is recommended the floor slab bearing soils be covered with an impervious membrane to reduce moisture entry and floor dampness. A 10-mil thick

plastic membrane is commonly used for this purpose. Care should be exercised not to tear large sections of the membrane during placement of reinforcing steel and concrete.

4.5 SITE PREPARATION

We recommend normal, good practice site preparation procedures. These procedures include: stripping the site of existing vegetation and topsoil, compacting the subgrade and placing necessary fill or backfill to grade with engineered fill. We recommend that the bottom of all footings be probed as to confirm the suitability of the bearing soils. A more detailed synopsis of this work is as follows:

1. Prior to construction, the location of any existing underground utility lines within the construction area should be established. Provisions should then be made to relocate interfering utilities to appropriate locations. It should be noted that if underground pipes are not properly removed or plugged, they may serve as conduits for subsurface erosion which may subsequently lead to excessive settlement of the overlying structure.

2. If required, perform remedial dewatering prior to any earthwork operations. Dewatering operations scheduled immediately adjacent to existing structure footings should be carefully evaluated for possible impacts to the existing foundation systems. Dewatering systems should not be decommissioned until the excavation is backfilled two feet above the groundwater level at the time of construction. Further, the site should always be graded to prohibit ponding of stormwater runoff.

3. As a prudent measure, we recommend that a pre-construction survey of the existing structures be performed to assess those buildings for current architectural and structural distress, as may be evidenced by exterior/interior vertical and horizontal surface cracking, unusual separation of building elements, and out-of-level conditions. The pre-construction survey results would provide a baseline for comparison and decision making during the building construction process.

4. We recommend that existing structures in close proximity to the proposed building should be monitored for cracks or signs of distress during adjacent new excavation and building construction operations.

5. Strip the proposed construction limits of all grass, roots, topsoil, and other deleterious materials within 5 feet beyond the perimeter of the proposed area. Expect typical stripping at this site to depths of 6 to 12 inches. Deeper clearing and grubbing depths may be encountered in heavily vegetated areas. *As previously mentioned, deleterious material (glass/debris) was encountered at a depth of 2.5 feet below the existing grade in boring B-7. We recommend performing test pits during the clearing operation to determine the extent of the deleterious/debris materials.* All deleterious material, debris and organics that may be identified in this area of the site should be removed in its entirety. Stockpile the surficial sandy soils for later use as fill. Excavated areas should be set back to a stable configuration or shored, in either case in accordance with OSHA requirements. Adequate bracing should be provided if necessary to prevent side slope failures.

6. Following site clearing, grubbing and rough grading, the same project areas should be proof-rolled using a large, fully loaded rubber-tired vehicle (dump truck) or similar equipment. Proof-rolling will help locate any surficial zones of especially loose or soft or unsuitable soils not encountered in the soil test borings, and should help provide more uniformity in the sandy subsurface soil profile. Unusual or unanticipated conditions

identified during this process must be immediately brought to the attention of the UES Geotechnical Engineer. Field density testing is not required during proof-rolling operations.

7. Weak subgrade soils identified during proof-rolling operations should be excavated and removed from the site, and replaced with granular backfill/fill soils. We recommend that the bottom of all footings be probed as to confirm the suitability of the bearing soils. Backfill/fill soils used for this purpose should meet the material and placement specifications outlined below.

8. Proof-rolling operations should be followed by backfill compaction operation. Backfill compaction should be performed until an in-place soil density of 95 percent of the Modified Proctor maximum dry density (ASTM D-1557) is achieved to a depth of 2 feet below the final subgrade, or foundation bearing elevations, whichever is greater. If necessary to achieve the recommended soil compaction at depth, the entire project area may be undercut, the exposed subgrade soils compacted, and then the areas backfilled using 6-inch lifts to final subgrade elevation. The subgrade beneath slabs should be compacted to a depth of 1 foot below the beginning grade prior to placing fill.

9. Compaction operations should extend to the limits of the cleared/grubbed project areas. Compaction of the existing, near-surface sandy soils will provide for uniformity of foundation/slab settlements and improve the soils' bearing capacity conditions. Typically, the soils should exhibit moisture contents within ± 2 percent of the modified Proctor optimum moisture content during compaction. A minimum of eight (8) complete coverages (in perpendicular directions) should be made in the building area with the roller to improve the uniformity and increase the density of the underlying sandy soils. It should be anticipated that moisture will need to be added to the subgrade in order to achieve the required compaction.

10. Should the bearing level soils experience pumping and soil strength loss during the compaction operations, compaction work should be immediately terminated and (1) the disturbed soils removed and backfilled with dry structural fill soils which are then compacted, or (2) the excess pore pressures within the disturbed soils allowed to dissipate before recompacting.

11. Care should be exercised to avoid damaging any nearby structures while the compaction operation is underway. Prior to commencing compaction, occupants of adjacent structures should be notified and the existing conditions of the structures be documented with photographs and survey (if deemed necessary). Compaction should cease if deemed detrimental to adjacent structures. Universal Engineering Sciences can provide vibration monitoring services to help document and evaluate the effects of the surface compaction operation on existing structures. In the absence of vibration monitoring, it is recommended the vibratory roller remain a minimum of 100 feet from existing structures. Within this zone, use of a vibratory roller operating in the static mode is recommended.

12. Place fill material, as required. Offsite fill material should contain less than 10 percent passing the No. 200 sieve. Place backfill and fill in uniform 10- to 12-inch loose lifts and compact each lift to a minimum density of 95 percent of the modified Proctor maximum dry density. Each lift should be compacted and have density test results that verify the level of compaction is satisfactory before the next lift is placed.

13. Perform compliance tests within the fill/backfill at a frequency of not less than one test per 2,500 square feet per lift in the structure areas, or at a minimum of three tests per structure area, whichever is greater. Additionally, we recommend that you test every other column footing, and one test per every 50 lineal feet of wall footing. Footings should be visually inspected and probed with a static cone penetrometer to verify stability.

4.6 STORMWATER MANAGEMENT SYSTEM

The laboratory test data indicates that the surficial sandy soils in the proposed stormwater management area for this project generally have permeability rates between approximately 15 to 20 feet per day at the boring locations. Based upon the above findings, we recommend that you consider the soil parameters presented in Table 2 for design of the stormwater management system on the subject project site. It should be noted that the above referenced values are measured values and do not incorporate a factor of safety.

Table 2 – Stormwater Management System Soil Design Parameters	
Corresponding Soil Boring Test Locations	B-1/B-2
Average Depth to Confining Layer, feet	5.75
Unsaturated Vertical Infiltration rate, feet per day	12
Saturated Vertical Infiltration Rate, feet per day	17
Estimated Horizontal Hydraulic Conductivity, feet per day	22
Estimated Fillable Porosity, percentage	25
Average Estimated Depth of Seasonal High Water Table, feet[1]	5.5

[1]Normal seasonal high water table (SHWT) will be the result of perched conditions.

4.7 STORMWATER MANAGEMENT SYSTEM FILL SUITABILITY

The recovered soil samples were classified using visual and textural means, and limited laboratory testing. We offer below ***preliminary guidelines*** for the use of on-site soils, such as those excavated from the proposed retention ponds, as fill material for the project.

Soil materials excavated and classified as fine sands to sand with silt and sand with clay (SP, SP-SM, SP-SC), with typically 12% fines or less (silt/clay fraction), may be considered suitable for use as utility trench backfill, as well as building pad and pavement subgrade structural fill, provided the materials are properly dried, placed, and compacted.

Soil materials excavated and classified as silty fine sands [SM], with typically 12% to 25% fines, may also be considered suitable for use as utility trench backfill, as well as building pad and pavement subgrade structural fill, after significant drying and some mixing with the fine sand material described above. Proper placement, proof rolling and compaction must also be performed.

Soil materials excavated and classified as clayey sand, silt or clay (SC, ML, MH, CL, and CH) and any organic-laden soils (5% or greater organics by weight) should not be reused as fill beneath building or pavement sections. These materials could be used in green areas, if applicable and in non-structural applications where excessive ground subsidence will not create functional or aesthetic problems. It should be noted that silt and clay materials will retain water

and if used near the final grade may become saturated and soft for a significant period of time following a rain event.

Soil borings for a typical geotechnical report are widely spaced and generally not sufficient for reliably detecting the presence of isolated, anomalous surface or subsurface conditions, or reliably estimating unsuitable or suitable material quantities. Accordingly, UES does not recommend relying on our boring information to negate presence of anomalous materials or for estimation of material quantities unless our contracted services ***specifically*** include sufficient exploration for such purpose(s) and within the report we so state that the level of exploration provided should be sufficient to detect such anomalous conditions or estimate such quantities. Therefore, UES will not be responsible for any extrapolation or use of our data by others beyond the purpose(s) for which it is applicable or intended.

4.8 CONSTRUCTION RELATED SERVICES

We recommend the Owner retain Universal Engineering Sciences to perform construction materials tests and observations on this project. Field tests and observations include verification of foundation subgrade by performing quality assurance tests on the placement of compacted structural fill. We can also provide concrete testing, and general construction observation services.

The geotechnical engineering design does not end with the advertisement of the construction documents. The design is an on-going process throughout construction. Because of our familiarity with the site conditions and the intent of the engineering design, we are most qualified to address problems that might arise during construction in a timely and cost-effective manner.

5.0 REPORT LIMITATIONS

This Report has been prepared for the exclusive use of Alchem Laboratories Corp., and other members of the Design/Construction Team for the specific project discussed in this Report. This Report has been prepared in accordance with generally accepted local geotechnical engineering practices; no other warranty is expressed or implied.

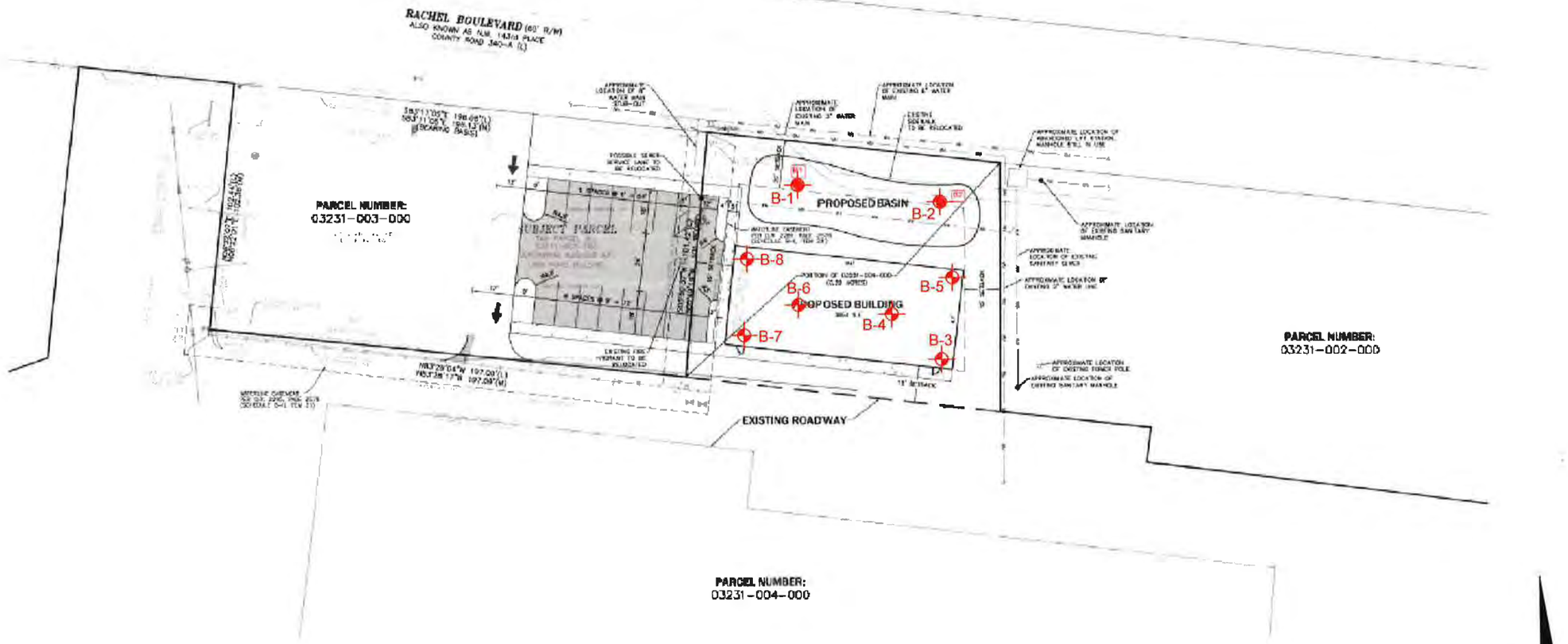
During the early stages of most construction projects, geotechnical issues not addressed in this report may arise. Because of the natural limitations inherent in working with the subsurface, it is not possible for a geotechnical engineer to predict and address all possible problems. A GBA publication, "Important Information About Your Geotechnical Engineering Report" appears in Appendix B, and will help explain the nature of geotechnical issues. Further, we present documents in Appendix B: Constraints and Restrictions, to bring to your attention the potential concerns and the basic limitations of a typical geotechnical report.

APPENDIX A

Boring Location Plan

Boring Logs

Key to Boring Logs



LEGEND

BORING LOCATION

NOTE: ALL SOIL TEST BORING LOCATIONS SHOWN ARE APPROXIMATE.

ALCHEM LABORATORIES CORP.

DRAWN BY: TK	DATE: 11/20/20
CHECKED BY: ES	DATE: 11/20/20
SCALE: 1"=50'	ACADFILE:0230 2000140-A
PROJECT NO: 0230 2000140.0000	REPORT NO: 1819436

ALCHEM BUILDING 4
TAX PARCEL: 03231-004-000
ALACHUA, FLORIDA

BORING LOCATION PLAN

UNIVERSAL
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PAGE NO:
A - 1

UNIVERSAL ENGINEERING SCIENCES
BORING LOG

PROJECT NO.:	0230.2000140.0000
REPORT NO.:	1819436
PAGE:	A-2

PROJECT: ALCHEM BUILDING 4
TAX PARCEL 03231-004-000
ALACHUA, FLORIDA

BORING NO: **B-1** SHEET: 1 of 1

CLIENT: ALCHEM LABORATORIES CORP
LOCATION: SEE BORING LOCATION PLAN
REMARKS:

SECTION: TOWNSHIP: RANGE:

GS ELEVATION(ft):
WATER TABLE (ft): NE
DATE OF READING: NA
EST. WSWT (ft):

DATE STARTED: 11/4/20
DATE FINISHED: 11/4/20
DRILLED BY: M. BOATRIGHT
TYPE OF SAMPLING: ASTM D-1452

DEPTH (FT.)	SAMPLE	BLOWS PER 6" INCREMENT	N VALUE	W.T.	SYMBOL	DESCRIPTION	-200 (%)	MC (%)	ATTERBERG LIMITS LL	ATTERBERG LIMITS PI	K (FT/ DAY)	ORG CONT. (%)
0						Brown SAND, with silt [SP-SM]						
1						Light brown and light orange SAND, with silt [SP-SM]						
2												
3							7	7			15	
4						Orange, brown and gray clayey SAND [SC]						
5												
6												
7												
8						Green, gray and orange sandy CLAY [CH], with seams of sand						
9												
10												
11												
12												
13												
14												
15						Boring Terminated at 15'						

UNIVERSAL ENGINEERING SCIENCES
BORING LOG

PROJECT NO.:	0230.2000140.0000
REPORT NO.:	1819436
PAGE:	A-3

PROJECT: ALCHEM BUILDING 4
TAX PARCEL 03231-004-000
ALACHUA, FLORIDA

BORING NO: **B-2** SHEET: 1 of 1

SECTION: TOWNSHIP: RANGE:

CLIENT: ALCHEM LABORATORIES CORP
LOCATION: SEE BORING LOCATION PLAN
REMARKS:

GS ELEVATION(ft):
WATER TABLE (ft): NE
DATE OF READING: NA
EST. WSWT (ft):

DATE STARTED: 11/4/20
DATE FINISHED: 11/4/20
DRILLED BY: M. BOATRIGHT
TYPE OF SAMPLING: ASTM D-1452

DEPTH (FT.)	SAMPLE	BLOWS PER 6" INCREMENT	N VALUE	W.T.	SYMBOL	DESCRIPTION	-200 (%)	MC (%)	ATTERBERG LIMITS LL	ATTERBERG LIMITS PI	K (FT/ DAY)	ORG CONT. (%)
0						Brown SAND, with silt [SP-SM]						
1						Brown and tan SAND, with silt [SP-SM]						
2												
3												
4							6	4			20	
5												
6						Brown, gray and orange clayey SAND [SC]						
7												
8												
9												
10												
11												
12												
13												
14												
15						Boring Terminated at 15'						

UNIVERSAL ENGINEERING SCIENCES
BORING LOG

PROJECT NO.:	0230.2000140.0000
REPORT NO.:	1819436
PAGE:	A-4

PROJECT: ALCHEM BUILDING 4
TAX PARCEL 03231-004-000
ALACHUA, FLORIDA

BORING NO: **B-3** SHEET: 1 of 1

CLIENT: ALCHEM LABORATORIES CORP
LOCATION: SEE BORING LOCATION PLAN
REMARKS:

SECTION: TOWNSHIP: RANGE:
GS ELEVATION(ft):
WATER TABLE (ft): NE
DATE OF READING: NA
EST. WSWT (ft):

DATE STARTED: 11/4/20
DATE FINISHED: 11/4/20
DRILLED BY: M. BOATRIGHT
TYPE OF SAMPLING: ASTM D-1586

DEPTH (FT.)	SAMPLE	BLOWS PER 6" INCREMENT	N VALUE	W.T.	SYMBOL	DESCRIPTION	-200 (%)	MC (%)	ATTERBERG LIMITS LL	ATTERBERG LIMITS PI	K (FT/ DAY)	ORG CONT. (%)
0						Loose brown SAND, with silt [SP-SM]						
1												
2		2-3-4	7									
3						Very loose orange and tan SAND [SP]						
4		3-2-1	3				4	3				
5												
		1-1-1	2									
6												
7		1/12"-1	1									
8												
		1-1-2	3									
9												
10		2-1-2	3									
11												
12						Medium dense gray and orange very clayey SAND to sandy CLAY [SC/CH]						
13												
14												
15		4-6-7	13			Boring Terminated at 15'						

UNIVERSAL ENGINEERING SCIENCES
BORING LOG

PROJECT NO.:	0230.2000140.0000
REPORT NO.:	1819436
PAGE:	A-5

PROJECT: ALCHEM BUILDING 4
TAX PARCEL 03231-004-000
ALACHUA, FLORIDA

BORING NO: **B-4** SHEET: 1 of 1

SECTION: TOWNSHIP: RANGE:

CLIENT: ALCHEM LABORATORIES CORP

LOCATION: SEE BORING LOCATION PLAN

REMARKS:

GS ELEVATION(ft):

WATER TABLE (ft): NE

DATE OF READING: NA

EST. WSWT (ft):

DATE STARTED: 11/4/20

DATE FINISHED: 11/4/20

DRILLED BY: M. BOATRIGHT

TYPE OF SAMPLING:ASTM D-1586

DEPTH (FT.)	SAMPLE	BLOWS PER 6" INCREMENT	N VALUE	WT	SYMBOL	DESCRIPTION	-200 (%)	MC (%)	ATTERBERG LIMITS LL	ATTERBERG LIMITS PI	K (FT/ DAY)	ORG CONT. (%)
0						Loose brown SAND, with silt [SP-SM]						
1												
2		1-2-3	5			Loose to very loose tan and light orange SAND [SP]						
3												
4		4-3-3	6									
5		2-1-2	3									
6												
7		1-1-1	2			Loose orange and brown very clayey SAND [SC]						
8		1-2-5	7			Firm orange and gray sandy CLAY [CL]						
9						Very stiff gray and orange sandy CLAY [CL], with limestone fragments						
10		9-13-13	26									
11												
12						Boring Terminated at 12' Due to Auger Refusal						

UNIVERSAL ENGINEERING SCIENCES
BORING LOG

PROJECT NO.:	0230.2000140.0000
REPORT NO.:	1819436
PAGE:	A-6

PROJECT: ALCHEM BUILDING 4
TAX PARCEL 03231-004-000
ALACHUA, FLORIDA

BORING NO: **B-5** SHEET: 1 of 1

SECTION: TOWNSHIP: RANGE:

CLIENT: ALCHEM LABORATORIES CORP
LOCATION: SEE BORING LOCATION PLAN
REMARKS:

GS ELEVATION(ft):
WATER TABLE (ft): NE
DATE OF READING: NA
EST. WSWT (ft):

DATE STARTED: 11/4/20
DATE FINISHED: 11/4/20
DRILLED BY: M. BOATRIGHT
TYPE OF SAMPLING:ASTM D-1586

DEPTH (FT.)	SAMPLE	BLOWS PER 6" INCREMENT	N VALUE	W.T.	SYMBOL	DESCRIPTION	-200 (%)	MC (%)	ATTERBERG LIMITS LL	ATTERBERG LIMITS PI	K (FT/ DAY)	ORG CONT. (%)
0						Brown SAND, with silt [SP-SM]						
1												
2		2-2-3	5			Loose to very loose light brown and light orange SAND [SP]						
3												
4		1-1-1	2									
5		1-1-1	2									
6												
7		1-1-2	3									
8		2-3-5	8			Loose to medium dense green, orange and brown clayey SAND [SC]						
9												
10		5-5-5	10									
11												
12						Stiff gray and orange sandy CLAY [CL], with limerock fragments						
13												
14												
15		4-5-9	14			Boring Terminated at 15'						

UNIVERSAL ENGINEERING SCIENCES
BORING LOG

PROJECT NO.:	0230.2000140.0000
REPORT NO.:	1819436
PAGE:	A-7

PROJECT: ALCHEM BUILDING 4
TAX PARCEL 03231-004-000
ALACHUA, FLORIDA

BORING NO: **B-6** SHEET: 1 of 1

CLIENT: ALCHEM LABORATORIES CORP
LOCATION: SEE BORING LOCATION PLAN
REMARKS: PERCHED WATER TABLE AT 7.5'

SECTION: TOWNSHIP: RANGE:

GS ELEVATION(ft):
WATER TABLE (ft): NE
DATE OF READING: NA
EST. WSWT (ft):

DATE STARTED: 11/4/20
DATE FINISHED: 11/4/20
DRILLED BY: M. BOATRIGHT
TYPE OF SAMPLING:ASTM D-1586

DEPTH (FT.)	SAMPLE	BLOWS PER 6" INCREMENT	N VALUE	W.T.	SYMBOL	DESCRIPTION	-200 (%)	MC (%)	ATTERBERG LIMITS LL	ATTERBERG LIMITS PI	K (FT/ DAY)	ORG CONT. (%)
0						Loose brown SAND, with silt [SP-SM]						
1												
2		1-2-3	5									
3						Loose tan and light orange SAND [SP]						
4		4-4-3	7									
5												
6		2-1-1	2									
7		1-1-1	2									
8						Loose orange and brown clayey SAND [SC]						
9		2-3-4	7									
10		4-1-2	3			Soft gray and orange sandy CLAY [CL], with trace of rock						
11												
12												
13												
14												
15		1-4-5	9			Loose gray and orange clayey SAND [SC]						
						Boring Terminated at 15'						

UNIVERSAL ENGINEERING SCIENCES
BORING LOG

PROJECT NO.:	0230.2000140.0000
REPORT NO.:	1819436
PAGE:	A-8

PROJECT: ALCHEM BUILDING 4
TAX PARCEL 03231-004-000
ALACHUA, FLORIDA

BORING NO: **B-7** SHEET: 1 of 1

SECTION: TOWNSHIP: RANGE:

CLIENT: ALCHEM LABORATORIES CORP

LOCATION: SEE BORING LOCATION PLAN

REMARKS:

GS ELEVATION(ft):

WATER TABLE (ft): NE

DATE OF READING: NA

EST. WSWT (ft):

DATE STARTED: 11/4/20

DATE FINISHED: 11/4/20

DRILLED BY: M. BOATRIGHT

TYPE OF SAMPLING:ASTM D-1586

DEPTH (FT.)	SAMPLE	BLOWS PER 6" INCREMENT	N VALUE	W.T.	SYMBOL	DESCRIPTION	-200 (%)	MC (%)	ATTERBERG LIMITS LL	ATTERBERG LIMITS PI	K (FT/ DAY)	ORG CONT. (%)
0						Brown SAND, with silt [SP-SM] and trace of debris						
1												
2		1-2-3	5			Crushed LIMEROCK Base/(FILL) Loose brown SAND [SP], with glass/debris						
3						Loose to very loose tan and light orange SAND [SP]						
4		2-2-2	4									
5		2-1-1	2									
6												
7		1-1-1	2									
8		1-1-1	2			Very loose orange and brown clayey SAND [SC]						
9						Medium dense gray, brown and orange very clayey SAND [SC]						
10		2-4-6	10									
11												
12												
13												
14												
15		4-5-7	12			Boring Terminated at 15'						

UNIVERSAL ENGINEERING SCIENCES
BORING LOG

PROJECT NO.:	0230.2000140.0000
REPORT NO.:	1819436
PAGE:	A-9

PROJECT: ALCHEM BUILDING 4
TAX PARCEL 03231-004-000
ALACHUA, FLORIDA

BORING NO: **B-8** SHEET: 1 of 1

SECTION: TOWNSHIP: RANGE:

CLIENT: ALCHEM LABORATORIES CORP
LOCATION: SEE BORING LOCATION PLAN
REMARKS:

GS ELEVATION(ft):
WATER TABLE (ft): NE
DATE OF READING: NA
EST. WSWT (ft):

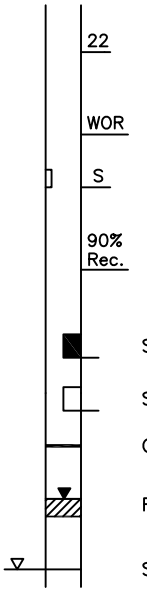
DATE STARTED: 11/4/20
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DEPTH (FT.)	SAMPLE	BLOWS PER 6" INCREMENT	N VALUE	W.T.	SYMBOL	DESCRIPTION	-200 (%)	MC (%)	ATTERBERG LIMITS LL	ATTERBERG LIMITS PI	K (FT/ DAY)	ORG CONT. (%)
0						Brown SAND, with silt [SP-SM]						
1						Loose to very loose tan and light orange SAND [SP]						
2		2-2-3	5									
3												
4		2-2-1	3									
5												
6		1-1-1	2			Loose orange and brown very clayey SAND [SC]	24	15	31	15		
7		1-1-3	4									
8		4-5-7	12			Medium dense light brown, gray and orange very clayey SAND [SC]						
9												
10		9-8-9	17									
11												
12												
13												
14												
15		6-7-7	14			Boring Terminated at 15'						

UNIVERSAL
ENGINEERING SCIENCES

KEY TO BORING LOGS

SYMBOLS



22	Number of Blows of a 140–lb Weight Falling 30 in. Required to Drive Standard Spoon One Foot
WOR	Weight of Drill Rods
S	Thin–Wall Shelby Tube Undisturbed Sampler Used
90% Rec.	Percent Core Recovery from Rock Core–Drilling Operations
	Sample Taken at this Level
	Sample Not Taken at this Level
	Change in Soil Strata
	Free Ground Water Level
	Seasonal High Ground Water Level

GRANULAR MATERIALS

Relative Density	Safety Hammer SPT N (Blows/Ft.)	Automatic Hammer SPT N (Blows/Ft.)
Very Loose	Less than 4	Less than 3
Loose	4–10	3–8
Medium Dense	10–30	8–24
Dense	30–50	24–40
Very Dense	>50	>40

COHESIVE MATERIALS

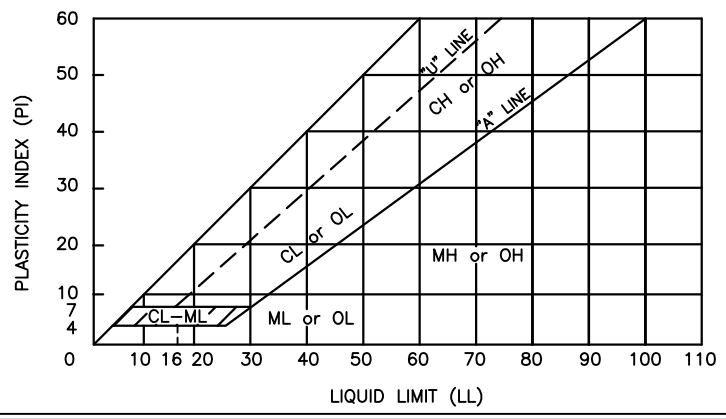
Consistency	Safety Hammer SPT N (Blows/Ft.)	Automatic Hammer SPT N (Blows/Ft.)
Very Soft	Less than 2	Less than 1
Soft	2–4	1–3
Firm	4–8	3–6
Stiff	8–15	6–12
Very Stiff	15–30	12–24
Hard	>30	>24

UNIFIED CLASSIFICATION SYSTEM

MAJOR DIVISIONS			GROUP SYMBOLS	TYPICAL NAMES
COARSE-GRAINED SOILS More than 50% retained on No. 200 sieve*	GRAVELS 50% or more of coarse fraction retained on No. 200 sieve	CLEAN GRAVELS	GW	Well–graded gravels and gravel–sand mixtures, little or no fines
			GP	Poorly graded gravels and gravel–sand mixtures, little or no fines
		GRAVELS WITH FINES	GM	Silty gravels, gravel–sand–silt mixtures
			GC	Clayey gravels, gravel–sand–clay mixtures
	SANDS More than 50% of coarse fraction passes No. 4 sieve	CLEAN SANDS	SW	Well–graded sands and gravelly sands, little or no fines
			SP	Poorly graded sands and gravelly sands, little or no fines
		SANDS WITH FINES	SM	Silty sands, sand–silt mixtures
			SC	Clayey sands, sand–clay mixtures
FINE-GRAINED SOILS 50% or more passes No. 200 sieve*	SILTS AND CLAYS Liquid limit 50% or less		ML	Inorganic silts, very fine sands, rock flour, silty or clayey fine sands
			CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays silty clays, lean clays
			OL	Organic silts and organic silty clays of low plasticity
	SILTS AND CLAYS Liquid limit greater than 50%		MH	Inorganic silts, micaceous or diatomacaceous fine sands or silts, elastic silts
			CH	Inorganic clays or high plasticity, fat clays
			OH	Organic clays of medium to high plasticity
Highly organic Soils			PT	Peat, muck and other highly organic soils

* Based on the material passing the 3–in. (75mm) sieve.

PLASTICITY CHART



APPENDIX B

Important Information About Your Geotechnical Engineering Report
Constraint and Restrictions

Important Information about This

Geotechnical-Engineering Report

Subsurface problems are a principal cause of construction delays, cost overruns, claims, and disputes.

While you cannot eliminate all such risks, you can manage them. The following information is provided to help.

The Geoprofessional Business Association (GBA) has prepared this advisory to help you – assumedly a client representative – interpret and apply this geotechnical-engineering report as effectively as possible. In that way, clients can benefit from a lowered exposure to the subsurface problems that, for decades, have been a principal cause of construction delays, cost overruns, claims, and disputes. If you have questions or want more information about any of the issues discussed below, contact your GBA-member geotechnical engineer. Active involvement in the Geoprofessional Business Association exposes geotechnical engineers to a wide array of risk-confrontation techniques that can be of genuine benefit for everyone involved with a construction project.

Geotechnical-Engineering Services Are Performed for Specific Purposes, Persons, and Project

Geotechnical engineers structure their services to meet the specific needs of their clients. A geotechnical-engineering study conducted for a given civil engineer will not likely meet the needs of a civil-works constructor or even a different civil engineer. Because each geotechnical-engineering study is unique, each geotechnical-engineering report is unique, prepared *solely* for the client. *Those who rely on a geotechnical-engineering report prepared for a different client can be seriously misled*. No one except authorized client representatives should rely on this geotechnical-engineering report without first conferring with the geotechnical engineer who prepared it. *And no one – not even you – should apply this report for any purpose or project except the one originally contemplated.*

Read this Report in Full

Costly problems have occurred because those relying on a geotechnical-engineering report did not read it *in its entirety*. Do not rely on an executive summary. Do not read selected elements only. *Read this report in full.*

You Need to Inform Your Geotechnical Engineer about Change

Your geotechnical engineer considered unique, project-specific factors when designing the study behind this report and developing the confirmation-dependent recommendations the report conveys. A few typical factors include:

- the client's goals, objectives, budget, schedule, and risk-management preferences;
- the general nature of the structure involved, its size, configuration, and performance criteria;
- the structure's location and orientation on the site; and
- other planned or existing site improvements, such as retaining walls, access roads, parking lots, and underground utilities.

Typical changes that could erode the reliability of this report include those that affect:

- the site's size or shape;
- the function of the proposed structure, as when it's changed from a parking garage to an office building, or from a light-industrial plant to a refrigerated warehouse;
- the elevation, configuration, location, orientation, or weight of the proposed structure;
- the composition of the design team; or
- project ownership.

As a general rule, *always* inform your geotechnical engineer of project changes – even minor ones – and request an assessment of their impact. *The geotechnical engineer who prepared this report cannot accept responsibility or liability for problems that arise because the geotechnical engineer was not informed about developments the engineer otherwise would have considered.*

This Report May Not Be Reliable

Do not rely on this report if your geotechnical engineer prepared it:

- for a different client;
- for a different project;
- for a different site (that may or may not include all or a portion of the original site); or
- before important events occurred at the site or adjacent to it; e.g., man-made events like construction or environmental remediation, or natural events like floods, droughts, earthquakes, or groundwater fluctuations.

Note, too, that it could be unwise to rely on a geotechnical-engineering report whose reliability may have been affected by the passage of time, because of factors like changed subsurface conditions; new or modified codes, standards, or regulations; or new techniques or tools. *If your geotechnical engineer has not indicated an "apply-by" date on the report, ask what it should be*, and, in general, *if you are the least bit uncertain* about the continued reliability of this report, contact your geotechnical engineer before applying it. A minor amount of additional testing or analysis – if any is required at all – could prevent major problems.

Most of the "Findings" Related in This Report Are Professional Opinions

Before construction begins, geotechnical engineers explore a site's subsurface through various sampling and testing procedures. *Geotechnical engineers can observe actual subsurface conditions only at those specific locations where sampling and testing were performed*. The data derived from that sampling and testing were reviewed by your geotechnical engineer, who then applied professional judgment to form opinions about subsurface conditions throughout the site. Actual sitewide-subsurface conditions may differ – maybe significantly – from those indicated in this report. Confront that risk by retaining your geotechnical engineer to serve on the design team from project start to project finish, so the individual can provide informed guidance quickly, whenever needed.

This Report's Recommendations Are Confirmation-Dependen

The recommendations included in this report – including any options or alternatives – are confirmation-dependent. In other words, *they are not final*, because the geotechnical engineer who developed them relied heavily on judgment and opinion to do so. Your geotechnical engineer can finalize the recommendations *only after observing actual subsurface conditions* revealed during construction. If through observation your geotechnical engineer confirms that the conditions assumed to exist actually do exist, the recommendations can be relied upon, assuming no other changes have occurred. *The geotechnical engineer who prepared this report cannot assume responsibility or liability for confirmation-dependent recommendations if you fail to retain that engineer to perform construction observation.*

This Report Could Be Misinterpreted

Other design professionals' misinterpretation of geotechnical-engineering reports has resulted in costly problems. Confront that risk by having your geotechnical engineer serve as a full-time member of the design team, to:

- confer with other design-team members,
- help develop specifications,
- review pertinent elements of other design professionals' plans and specifications, and
- be on hand quickly whenever geotechnical-engineering guidance is needed.

You should also confront the risk of constructors misinterpreting this report. Do so by retaining your geotechnical engineer to participate in prebid and preconstruction conferences and to perform construction observation.

Give Constructors a Complete Report and Guidance

Some owners and design professionals mistakenly believe they can shift unanticipated-subsurface-conditions liability to constructors by limiting the information they provide for bid preparation. To help prevent the costly, contentious problems this practice has caused, include the complete geotechnical-engineering report, along with any attachments or appendices, with your contract documents, *but be certain to note conspicuously that you've included the material for informational purposes only*. To avoid misunderstanding, you may also want to note that "informational purposes" means constructors have no right to rely on the interpretations, opinions, conclusions, or recommendations in the report, but they may rely on the factual data relative to the specific times, locations, and depths/elevations referenced. Be certain that constructors know they may learn about specific project requirements, including options selected from the report, *only* from the design drawings and specifications. Remind constructors that they may perform their own studies if they want to, and *be sure to allow enough time* to permit them to do so. Only then might you be in a position to give constructors the information available to you, while requiring them to at least share some of the financial responsibilities stemming from unanticipated conditions. Conducting prebid and preconstruction conferences can also be valuable in this respect.

Read Responsibility Provisions Closely

Some client representatives, design professionals, and constructors do not realize that geotechnical engineering is far less exact than other engineering disciplines. That lack of understanding has nurtured unrealistic expectations that have resulted in disappointments, delays, cost overruns, claims, and disputes. To confront that risk, geotechnical engineers commonly include explanatory provisions in their reports. Sometimes labeled "limitations," many of these provisions indicate where geotechnical engineers' responsibilities begin and end, to help others recognize their own responsibilities and risks. *Read these provisions closely*. Ask questions. Your geotechnical engineer should respond fully and frankly.

Geoenvironmental Concerns Are Not Covered

The personnel, equipment, and techniques used to perform an environmental study – e.g., a "phase-one" or "phase-two" environmental site assessment – differ significantly from those used to perform a geotechnical-engineering study. For that reason, a geotechnical-engineering report does not usually relate any environmental findings, conclusions, or recommendations; e.g., about the likelihood of encountering underground storage tanks or regulated contaminants. *Unanticipated subsurface environmental problems have led to project failures*. If you have not yet obtained your own environmental information, ask your geotechnical consultant for risk-management guidance. As a general rule, *do not rely on an environmental report prepared for a different client, site, or project, or that is more than six months old.*

Obtain Professional Assistance to Deal with Moisture Infiltration and Mol

While your geotechnical engineer may have addressed groundwater, water infiltration, or similar issues in this report, none of the engineer's services were designed, conducted, or intended to prevent uncontrolled migration of moisture – including water vapor – from the soil through building slabs and walls and into the building interior, where it can cause mold growth and material-performance deficiencies. Accordingly, ***proper implementation of the geotechnical engineer's recommendations will not of itself be sufficient to prevent moisture infiltration.*** Confront the risk of moisture infiltration by including building-envelope or mold specialists on the design team. ***Geotechnical engineers are not building-envelope or mold specialists.***

GEOPROFESSIONAL BUSINESS ASSOCIATION

Telephone: 301/565-2733
e-mail: info@geoprofessional.org www.geoprofessional.org

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CONSTRAINTS & RESTRICTIONS

The intent of this document is to bring to your attention the potential concerns and the basic limitations of a typical geotechnical report.

WARRANTY

Universal Engineering Sciences has prepared this report for our client for his exclusive use, in accordance with generally accepted soil and foundation engineering practices, and makes no other warranty either expressed or implied as to the professional advice provided in the report.

UNANTICIPATED SOIL CONDITIONS

The analysis and recommendations submitted in this report are based upon the data obtained from soil borings performed at the locations indicated on the Boring Location Plan. This report does not reflect any variations which may occur between these borings.

The nature and extent of variations between borings may not become known until excavation begins. If variations appear, we may have to re-evaluate our recommendations after performing on-site observations and noting the characteristics of any variations.

CHANGED CONDITIONS

We recommend that the specifications for the project require that the contractor immediately notify Universal Engineering Sciences, as well as the owner, when subsurface conditions are encountered that are different from those present in this report.

No claim by the contractor for any conditions differing from those anticipated in the plans, specifications, and those found in this report, should be allowed unless the contractor notifies the owner and Universal Engineering Sciences of such changed conditions. Further, we recommend that all foundation work and site improvements be observed by a representative of Universal Engineering Sciences to monitor field conditions and changes, to verify design assumptions and to evaluate and recommend any appropriate modifications to this report.

MISINTERPRETATION OF SOIL ENGINEERING REPORT

Universal Engineering Sciences is responsible for the conclusions and opinions contained within this report based upon the data relating only to the specific project and location discussed herein. If the conclusions or recommendations based upon the data presented are made by others, those conclusions or recommendations are not the responsibility of Universal Engineering Sciences.

CHANGED STRUCTURE OR LOCATION

This report was prepared in order to aid in the evaluation of this project and to assist the architect or engineer in the design of this project. If any changes in the design or location of the structure as outlined in this report are planned, or if any structures are included or added that are not discussed in the report, the conclusions and recommendations contained in this report shall not be considered valid unless the changes are reviewed and the conclusions modified or approved by Universal Engineering Sciences.

USE OF REPORT BY BIDDERS

Bidders who are examining the report prior to submission of a bid are cautioned that this report was prepared as an aid to the designers of the project and it may affect actual construction operations.

Bidders are urged to make their own soil borings, test pits, test caissons or other investigations to determine those conditions that may affect construction operations. Universal Engineering Sciences cannot be responsible for any interpretations made from this report or the attached boring logs with regard to their adequacy in reflecting subsurface conditions which will affect construction operations.

STRATA CHANGES

Strata changes are indicated by a definite line on the boring logs which accompany this report. However, the actual change in the ground may be more gradual. Where changes occur between soil samples, the location of the change must necessarily be estimated using all available information and may not be shown at the exact depth.

OBSERVATIONS DURING DRILLING

Attempts are made to detect and/or identify occurrences during drilling and sampling, such as: water level, boulders, zones of lost circulation, relative ease or resistance to drilling progress, unusual sample recovery, variation of driving resistance, obstructions, etc.; however, lack of mention does not preclude their presence.

WATER LEVELS

Water level readings have been made in the drill holes during drilling and they indicate normally occurring conditions. Water levels may not have been stabilized at the last reading. This data has been reviewed and interpretations made in this report. However, it must be noted that fluctuations in the level of the groundwater may occur due to variations in rainfall, temperature, tides, and other factors not evident at the time measurements were made and reported. Since the probability of such variations is anticipated, design drawings and specifications should accommodate such possibilities and construction planning should be based upon such assumptions of variations.

LOCATION OF BURIED OBJECTS

All users of this report are cautioned that there was no requirement for Universal Engineering Sciences to attempt to locate any man-made buried objects during the course of this exploration and that no attempt was made by Universal Engineering Sciences to locate any such buried objects. Universal Engineering Sciences cannot be responsible for any buried man-made objects which are subsequently encountered during construction that are not discussed within the text of this report.

TIME

This report reflects the soil conditions at the time of exploration. If the report is not used in a reasonable amount of time, significant changes to the site may occur and additional reviews may be required.

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Universal Engineering Sciences, LLC
GENERAL CONDITIONS

SECTION 1: RESPONSIBILITIES

1.1 *Universal Engineering Sciences, LLC, Universal Engineering Inspections, LLC, and GFA International Inc. ("UES"), have the responsibility for providing the services described under the Scope of Services section. The work is to be performed according to accepted standards of care and is to be completed in a timely manner. The term "UES" as used herein includes all of Universal Engineering Sciences, LLC, Universal Engineering Inspections, LLC, GFA International, Inc., its' agents, employees, professional staff, and subcontractors.*

1.2 The Client or a duly authorized representative is responsible for providing UES with a clear understanding of the project nature and scope. The Client shall supply UES with sufficient and adequate information, including, but not limited to, maps, site plans, reports, surveys and designs, to allow UES to properly complete the specified services. The Client shall also communicate changes in the nature and scope of the project as soon as possible during performance of the work so that the changes can be incorporated into the work product.

1.3 The Client acknowledges that UES's responsibilities in providing the services described under the Scope of Services section is limited to those services described therein, and the Client hereby assumes any collateral or affiliated duties necessitated by or for those services. Such duties may include, but are not limited to, reporting requirements imposed by any third party such as federal, state, or local entities, the provision of any required notices to any third party, or the securing of necessary permits or permissions from any third parties required for UES's provision of the services so described, unless otherwise agreed upon by both parties.

1.4 Universal will not be responsible for scheduling our services and will not be responsible for tests or inspections that are not performed due to a failure to schedule our services on the project or any resulting damages.

1.5 **PURSUANT TO FLORIDA STATUTES §558.0035, ANY INDIVIDUAL EMPLOYEE OR AGENT OF UES MAY NOT BE HELD INDIVIDUALLY LIABLE FOR NEGLIGENCE.**

SECTION 2: STANDARD OF CARE

2.1 Services performed by UES under this Agreement will be conducted in a manner consistent with the level of care and skill ordinarily exercised by members of UES's profession practicing contemporaneously under similar conditions in the locality of the project. No other warranty, express or implied, is made.

2.2 The Client recognizes that subsurface conditions may vary from those observed at locations where borings, surveys, or other explorations are made, and that site conditions may change with time. Data, interpretations, and recommendations by UES will be based solely on information available to UES at the time of service. UES is responsible for those data, interpretations, and recommendations, but will not be responsible for other parties' interpretations or use of the information developed.

2.3 Execution of this document by UES is not a representation that UES has visited the site, become generally familiar with local conditions under which the services are to be performed, or correlated personal observations with the requirements of the Scope of Services. It is the Client's responsibility to provide UES with all information necessary for UES to provide the services described under the Scope of Services, and the Client assumes all liability for information not provided to UES that may affect the quality or sufficiency of the services so described.

2.4 Should UES be retained to provide threshold inspection services under Florida Statutes §553.79, Client acknowledges that UES's services thereunder do not constitute a guarantee that the construction in question has been properly designed or constructed, and UES's services do not replace any of the obligations or liabilities associated with any architect, contractor, or structural engineer. Therefore it is explicitly agreed that the Client will not hold UES responsible for the proper performance of service by any architect, contractor, structural engineer or any other entity associated with the project.

SECTION 3: SITE ACCESS AND SITE CONDITIONS

3.1 Client will grant or obtain free access to the site for all equipment and personnel necessary for UES to perform the work set forth in this Agreement. The Client will notify any and all possessors of the project site that Client has granted UES free access to the site. UES will take reasonable precautions to minimize damage to the site, but it is understood by Client that, in the normal course of work, some damage may occur, and the correction of such damage is not part of this Agreement unless so specified in the Proposal.

3.2 The Client is responsible for the accuracy of locations for all subterranean structures and utilities. UES will take reasonable precautions to avoid known subterranean structures, and the Client waives any claim against UES, and agrees to defend, indemnify, and hold UES harmless from any claim or liability for injury or loss, including costs of defense, arising from damage done to subterranean structures and utilities not identified or accurately located. In addition, Client agrees to compensate UES for any time spent or expenses incurred by UES in defense of any such claim with compensation to be based upon UES's prevailing fee schedule and expense reimbursement policy.

SECTION 4: SAMPLE OWNERSHIP AND DISPOSAL

4.1 Soil or water samples obtained from the project during performance of the work shall remain the property of the Client.

4.2 UES will dispose of or return to Client all remaining soils and rock samples 60 days after submission of report covering those samples. Further storage or transfer of samples can be made at Client's expense upon Client's prior written request.

4.3 Samples which are contaminated by petroleum products or other chemical waste will be returned to Client for treatment or disposal, consistent with all appropriate federal, state, or local regulations.

SECTION 5: BILLING AND PAYMENT

5.1 UES will submit invoices to Client monthly or upon completion of services. Invoices will show charges for different personnel and expense classifications.

5.2 Payment is due 30 days after presentation of invoice and is past due 31 days from invoice date. Client agrees to pay a finance charge of one and one-half percent (1 ½ %) per month, or the maximum rate allowed by law, on past due accounts.

5.3 If UES incurs any expenses to collect overdue billings on invoices, the sums paid by UES for reasonable attorneys' fees, court costs, UES's time, UES's expenses, and interest will be due and owing by the Client.

SECTION 6: OWNERSHIP AND USE OF DOCUMENTS

6.1 All reports, boring logs, field data, field notes, laboratory test data, calculations, estimates, and other documents prepared by UES, as instruments of service, shall remain the property of UES.

6.2 Client agrees that all reports and other work furnished to the Client or his agents, which are not paid for, will be returned upon demand and will not be used by the Client for any purpose.

6.3 UES will retain all pertinent records relating to the services performed for a period of five years following submission of the report, during which period the records will be made available to the Client at all reasonable times.

6.4 All reports, boring logs, field data, field notes, laboratory test data, calculations, estimates, and other documents prepared by UES, are prepared for the sole and exclusive use of Client, and may not be given to any other party or used or relied upon by any such party without the express written consent of UES.

SECTION 7: DISCOVERY OF UNANTICIPATED HAZARDOUS MATERIALS

7.1 Client warrants that a reasonable effort has been made to inform UES of known or suspected hazardous materials on or near the project site.

7.2 Under this agreement, the term hazardous materials include hazardous materials (40 CFR 172.01), hazardous wastes (40 CFR 261.2), hazardous substances (40 CFR 300.6), petroleum products, polychlorinated biphenyls, and asbestos.

7.3 Hazardous materials may exist at a site where there is no reason to believe they could or should be present. UES and Client agree that the discovery of unanticipated hazardous materials constitutes a changed condition mandating a renegotiation of the scope of work. UES and Client also agree that the discovery of unanticipated hazardous materials may make it necessary for UES to take immediate measures to protect health and safety. Client agrees to compensate UES for any equipment decontamination or other costs incident to the discovery of unanticipated hazardous waste.

7.4 UES agrees to notify Client when unanticipated hazardous materials or suspected hazardous materials are encountered. Client agrees to make any disclosures required by law to the appropriate governing agencies. Client also agrees to hold UES harmless for any and all consequences of disclosures made by UES which are required by governing law. In the event the project site is not owned by Client, Client recognizes that it is the Client's responsibility to inform the property owner of the discovery of unanticipated hazardous materials or suspected hazardous materials.

7.5 Notwithstanding any other provision of the Agreement, Client waives any claim against UES, and to the maximum extent permitted by law, agrees to defend, indemnify, and save UES harmless from any claim, liability, and/or defense costs for injury or loss arising from UES's discovery of unanticipated hazardous materials or suspected hazardous materials including any costs created by delay of the project and any cost associated with possible reduction of the property's value. Client will be responsible for ultimate disposal of any samples secured by UES which are found to be contaminated.

SECTION 8: RISK ALLOCATION

8.1 Client agrees that UES's liability for any damage on account of any breach of contract, error, omission or other professional negligence will be limited to a sum not to exceed $50,000 or UES's fee, whichever is greater. If Client prefers to have higher limits on contractual or professional liability, UES agrees to increase the limits up to a maximum of $1,000,000.00 upon Client's written request at the time of accepting our proposal provided that Client agrees to pay an additional consideration of four percent of the total fee, or $400.00, whichever is greater. The additional charge for the higher liability limits is because of the greater risk assumed and is not strictly a charge for additional professional liability insurance.

SECTION 9: INSURANCE

9.1 UES represents and warrants that it and its agents, staff and consultants employed by it, is and are protected by worker's compensation insurance and that UES has such coverage under public liability and property damage insurance policies which UES deems to be adequate. Certificates for all such policies of insurance shall be provided to Client upon request in writing. Within the limits and conditions of such insurance, UES agrees to indemnify and save Client harmless from and against loss, damage, or liability arising from negligent acts by UES, its agents, staff, and consultants employed by it. UES shall not be responsible for any loss, damage or liability beyond the amounts, limits, and conditions of such insurance or the limits described in Section 8, whichever is less. The Client agrees to defend, indemnify and save UES harmless for loss, damage or liability arising from acts by Client, Client's agent, staff, and other UESs employed by Client.

SECTION 10: DISPUTE RESOLUTION

10.1 All claims, disputes, and other matters in controversy between UES and Client arising out of or in any way related to this Agreement will be submitted to alternative dispute resolution (ADR) such as mediation or arbitration, before and as a condition precedent to other remedies provided by law, including the commencement of litigation.

10.2 If a dispute arises related to the services provided under this Agreement and that dispute requires litigation instead of ADR as provided above, then:

(a) the claim will be brought and tried in judicial jurisdiction of the court of the county where UES's principal place of business is located and Client waives the right to remove the action to any other county or judicial jurisdiction, and

(b) The prevailing party will be entitled to recovery of all reasonable costs incurred, including staff time, court costs, attorneys' fees, and other claim related expenses.

SECTION 11: TERMINATION

11.1 This agreement may be terminated by either party upon seven (7) days written notice in the event of substantial failure by the other party to perform in accordance with the terms hereof. Such termination shall not be effective if that substantial failure has been remedied before expiration of the period specified in the written notice. In the event of termination, UES shall be paid for services performed to the termination notice date plus reasonable termination expenses.

11.2 In the event of termination, or suspension for more than three (3) months, prior to completion of all reports contemplated by the Agreement, UES may complete such analyses and records as are necessary to complete its files and may also complete a report on the services performed to the date of notice of termination or suspension. The expense of termination or suspension shall include all direct costs of UES in completing such analyses, records and reports.

SECTION 12: ASSIGNS

12.1 Neither the Client nor UES may delegate, assign, sublet or transfer their duties or interest in this Agreement without the written consent of the other party.

SECTION 13. GOVERNING LAW AND SURVIVAL

13.1 The laws of the State of Florida will govern the validity of these Terms, their interpretation and performance.

13.2 If any of the provisions contained in this Agreement are held illegal, invalid, or unenforceable, the enforceability of the remaining provisions will not be impaired. Limitations of liability and indemnities will survive termination of this Agreement for any cause.

SECTION 14. INTEGRATION CLAUSE

14.1 This Agreement represents and contains the entire and only agreement and understanding among the parties with respect to the subject matter of this Agreement, and supersedes any and all prior and contemporaneous oral and written agreements, understandings, representations, inducements, promises, warranties, and conditions among the parties. No agreement, understanding, representation, inducement, promise, warranty, or condition of any kind with respect to the subject matter of this Agreement shall be relied upon by the parties unless expressly incorporated herein.

14.2 This Agreement may not be amended or modified except by an agreement in writing signed by the party against whom the enforcement of any modification or amendment is sought.

Attachment B

Aerial Photograph



Project Aerial

Attachment C

Pre- and Post-Development Drainage Maps

COUNTY ROAD 340-A/RACHAEL BLVD
(66' R/W)

LIMITS OF TOPOGRAPHIC SURVEY

BROKEN YELLOW STRIPE

BROKEN WHITE STRIPE

WHITE STRIPE

EDGE OF PAVEMENT

24" rcp
inv.=58.95'

BOL (3)

WV
WM

GAS

WL

lo22

BFPR

CONC SIDEWALK

TELPED

FH

20'
BUILDING
SETBACK

24" rcp
inv.=59.20'

RIP RAP

(5/8", NOT LEGIBLE)

CONC

S 83°11'05" E 121.08' (D)
S 83°11'05" E 121.13' (M)

10.0'
10.0'

pa17

PAVERS (TBR)

UGT

UNK

WHITE STRIPE (TBR)

LT PL

CONC SIDEWALK (TBR.)

(5/8" LB 2903)
SITE BENCHMARK
ELEV.=63.17'

SV
(2)

ARVMH
TOP ELEV.=63.14'

R/W LINE
(BEARING BASIS)

LIMITS OF TOPOGRAPHIC SURVEY

TOP
ELEV.=66.61'

ONE-STORY BUILDING
13305 RACHAEL BLVD.

CONC

8" CONC WALL
W/HANDRAIL

F.F.EL.=66.28'

TOP
ELEV.=67.80'

UNKNOWN VERTICAL
METAL PIPE

ASPHALT PARKING (TBR)

N 05°50'37" E 101.42' (D)
N 05°52'57" E 101.39' (M)

WHITE STRIPE (TBR)

STEPS

COVERED AREA (TBR)

CO

(TO BE REMOVED)
PORTABLE BUILDING
F.F.EL.=71.50'

COVERED AREA (TBR)

TELPED

S 83°29'04" E 197.00' (D)

10.0'

6" CURB

P.O.B.
THIS SURVEY

(PRM LS 3784)

WV/CONC

6" CLF

1.1'

LIMITS OF TOPOGRAPHIC SURVEY

15'
BUILDING
SETBACK

N 82°54'46" W 130.28' (D)
N 82°53'05" W 130.34' (M)

CTVPED

PP

UGE, OHPL

ANCH

(PRM LS 3784)

ALACHUA COUNTY TAX PARCEL
03231-004-000
O.R. 2340, Pg. 1962

CONC

F.F.EL.=70.05'

ASPHALT PAVEMENT

OHPL

(NO ID)
S 72°26'26" W, 0.12'
FROM PROPERTY CORNER

MULTI-STORY
CONCRETE BLOCK
BUILDING

OLD STRUCTURE
W/METAL LID
(POSSIBLE STORM SEWER)
grate elev.=69.97'
6"dip? W inv=69.13'
6"dip? S inv=67.77'
8" vcp SW to NE THROUGH

BOL (2)

DRAIN

A/C

CONC

FDC

METAL DRAIN

WL /HRIB

BOL (2)

CONC PATCH

A/C

STEPS

CO

STEEL

INVEY

ALACHUA COUNTY TAX PARCEL
03231-002-000
O.R. 3433, Pg. 709

S 00°44'31" W 102.61' (D)
S 00°42'22" W 102.64' (M)

15'
BUILDING
SETBACK

OLD CONC STRUCTURE
W/METAL LID
(POSSIBLE STORM SEWER)
8"vcp W inv=66.56'
12"vcp E inv=65.12'
6"dip S inv=65.91'

TOP OF 6" METAL PIPE
ELEV.=66.39'

SANITARY MANHOLE
top elev=64.07'
8"pvc S inv=60.52'
8"pvc NE inv=60.52'

SANITARY MANHOLE
top elev=67.18'
8"pvc N inv=61.86'
8"pvc SE inv=64.68'
4"pvc S inv=62.10'
4"pvc SW inv=62.39'
4"pvc SE inv=62.29'

eda consultants inc.
EB 2389
720 S.W. 2nd Ave, South Tower, Suite 300
GAINESVILLE, FLORIDA 32601
TEL. (352) 373-3541
www.edafl.com mail@edafl.com

NORTH
SCALE: 1" = 20'

0 10 20 40
GRAPHIC SCALE

No.	Date	Comment

Professional Engineer of Record:

Sergio J. Reyes, P.E. — Engineer — 47311 — Certificate No.

Project No:

Project phase:

Project title:

ALCHEM BUILDING FOUR
CITY OF ALACHUA,
FLORIDA

Sheet title:

PRE DEVELOPMENT DRAINAGE

Designed: –
Drawn:
Checked:
Date:

Sheet No.:

1 OF 1

COUNTY ROAD 340-A/RACHAEL BLVD
(66' R/W)

LIMITS OF TOPOGRAPHIC SURVEY

EDGE OF PAVEMENT

24" rcp
inv.=58.95'

24" rcp
inv.=59.20'

BOL (2)

GAS

WL

lo22

CONC SIDEWALK (TBR)

RAP

S-4

S-2

S-3

XI

Z

65

66

10.0'

10.0'

PROPOSED BASIN

CONC SIDEWALK (TBR)

R/W LINE
(BEARING BASIS)

LIMITS OF TOPOGRAPHIC SURVEY

UGT

UGE

TELPED

ONE-STORY BUILDING
13305 RACHAEL BLVD.

F.F.EL.=66.28'

UNKNOWN VERTICAL
METAL PIPE

LOADING ZONE

PROPOSED
BUILDING
F.F.=67.50

ALACHUA COUNTY TAX PARCEL
03231-002-000
O.R. 3433, Pg. 709

6' CLF GATE

6" CURB

6" CURB

S 89°29'04" E 197.00' (D)

10.0'

LIMITS OF TOPOGRAPHIC SURVEY

6' CLF

1.1'

CTVPED

PP

UGE OHPL

ANCH

ALACHUA COUNTY TAX PARCEL
03231-004-000
O.R. 2340, Pg. 1962

F.F.EL.=70.05'

ASPHALT PAVEMENT

OHPL

MULTI-STORY
CONCRETE BLOCK
BUILDING

BOL (2)

A/C

FDC

BOL (2)

A/C

STEPS

CO

METAL DRAIN

LIMITS OF TOPOGRAPHIC SURVEY

OHPL

STEPS

eda consultants inc.
EB 2389
720 S.W. 2nd Ave, South Tower, Suite 300
GAINESVILLE, FLORIDA 32601
TEL. (352) 373-3541
www.edafl.com mail@edafl.com

NORTH
SCALE: 1" = 20'

0 10 20 40
GRAPHIC SCALE

No.	Date	Comment

Professional Engineer of Record:

Sergio J. Reyes, P.E. 47311
Engineer Certificate No.

Project No:

Project phase:

Project title:

ALCHEM BUILDING FOUR
CITY OF ALACHUA,
FLORIDA

Sheet title:

POST DEVELOPMENT DRAINAGE

Designed: –
Drawn:
Checked:
Date:

Sheet No.:

POST

I:\3engprojects\Alchem - Bldg 4\Design Reports\Drainage\Working DWG\Stormwater.dwg, POST DEVELOPMENT, 3/8/2021 5:18:51 PM, engineer13

Attachment D

Soils Map

Hydrologic Soil Group—Alachua County, Florida
(Alchem Building 4)



Map Scale: 1:1,320 if printed on A landscape (11" x 8.5") sheet.

N

Meters
0 15 30 60 90

Feet
0 50 100 200 300

Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 17N WGS84

MAP LEGEND

Area of Interest (AOI)
- Area of Interest (AOI)

Soils

Soil Rating Polygons
- A
- A/D
- B
- B/D
- C
- C/D
- D
- Not rated or not available

Soil Rating Lines
- A
- A/D
- B
- B/D
- C
- C/D
- D
- Not rated or not available

Soil Rating Points
- A
- A/D
- B
- B/D
- C
- C/D
- D
- Not rated or not available

Water Features
- Streams and Canals

Transportation
- Rails
- Interstate Highways
- US Routes
- Major Roads
- Local Roads

Background
- Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:15,800.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Alachua County, Florida
Survey Area Data: Version 21, Jun 5, 2020

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Nov 26, 2014—Dec 9, 2017

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
3	Arredondo fine sand, 0 to 5 percent slopes	A	0.3	3.7%
27	Urban land		7.6	96.3%
Totals for Area of Interest			**7.8**	**100.0%**

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

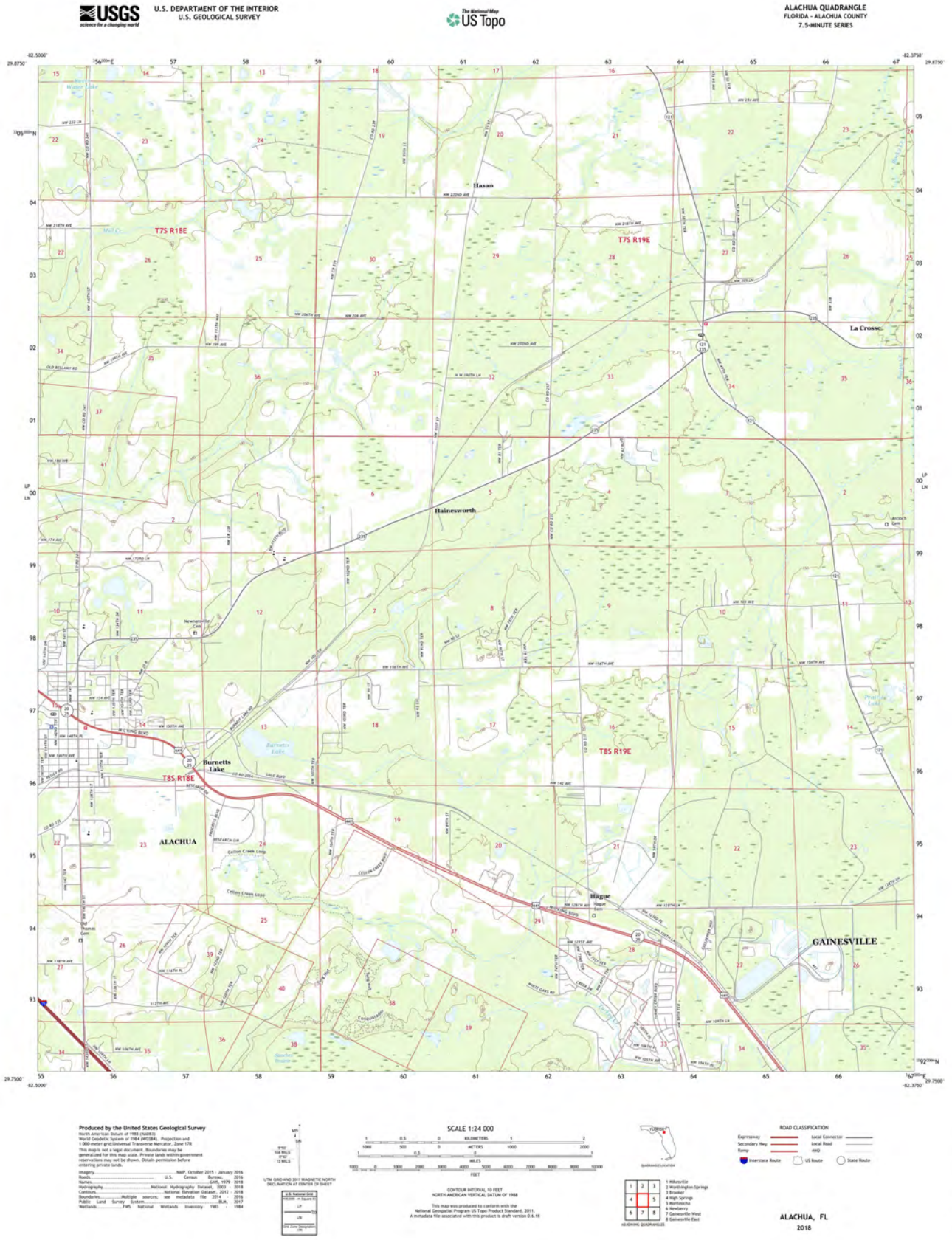
Aggregation Method: Dominant Condition

Component Percent Cutoff: None Specified

Tie-break Rule: Higher

Attachment E

Quad Map


USGS
science for a changing world
U.S. DEPARTMENT OF THE INTERIOR
U.S. GEOLOGICAL SURVEY
The National Map
US Topo
ALACHUA QUADRANGLE
FLORIDA - ALACHUA COUNTY
7.5-MINUTE SERIES
Hasan
Hainesworth
La Crosse
Burnetts Lake
ALACHUA
Hague
GAINESVILLE
T7S R18E
T7S R19E
T8S R18E
T8S R19E
Produced by the United States Geological Survey
SCALE 1:24 000
CONTOUR INTERVAL 10 FEET
NORTH AMERICAN VERTICAL DATUM OF 1988
ROAD CLASSIFICATION
ALACHUA, FL
2018

Attachment F

FEMA Map

National Flood Hazard Layer FIRMette

FEMA

82°29'31"W 29°47'25"N



82°28'53"W 29°46'54"N

0 250 500 1,000 1,500 2,000 Feet

1:6,000

Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

SPECIAL FLOOD HAZARD AREAS
- Without Base Flood Elevation (BFE) *Zone A, V, A99*
- With BFE or Depth *Zone AE, AO, AH, VE, AR*
- Regulatory Floodway

OTHER AREAS OF FLOOD HAZARD
- 0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile *Zone X*
- Future Conditions 1% Annual Chance Flood Hazard *Zone X*
- Area with Reduced Flood Risk due to Levee. See Notes. *Zone X*
- Area with Flood Risk due to Levee *Zone D*

OTHER AREAS
- NO SCREEN Area of Minimal Flood Hazard *Zone X*
- Effective LOMRs
- Area of Undetermined Flood Hazard *Zone D*

GENERAL STRUCTURES
- Channel, Culvert, or Storm Sewer
- Levee, Dike, or Floodwall

OTHER FEATURES
- 20.2 Cross Sections with 1% Annual Chance
- 17.5 Water Surface Elevation
- Coastal Transect
- Base Flood Elevation Line (BFE)
- Limit of Study
- Jurisdiction Boundary
- Coastal Transect Baseline
- Profile Baseline
- Hydrographic Feature

MAP PANELS
- Digital Data Available
- No Digital Data Available
- Unmapped

N

The pin displayed on the map is an approximate point selected by the user and does not represent an authoritative property location.

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on 12/4/2020 at 10:55 AM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.

Attachment G

Pre- and Post-Development ICPR Model

Nodes
A Stage/Area
V Stage/Volume
T Time/Stage
M Manhole

Basins
O Overland Flow
U SCS Unit CN
S SBUH CN
Y SCS Unit GA
Z SBUH GA

Links
P Pipe
W Weir
C Channel
D Drop Structure
B Bridge
R Rating Curve
H Breach
E Percolation
F Filter
X Exfil Trench



NODE DIAGRAM

Name	Group	Simulation	Max Time Stage hrs	Max Stage ft	Warning Stage ft	Max Delta Stage ft	Max Surf Area ft2	Max Time Inflow hrs	Max Inflow cfs	Max Time Outflow hrs	Max Outflow cfs
POST OUT	BASE	100YR-001HR	0.00	0.00	0.00	0.0000	0	0.99	0.16	0.00	0.00
PRE OUT	BASE	100YR-001HR	0.00	0.00	0.00	0.0000	0	0.75	0.60	0.00	0.00
SMF	BASE	100YR-001HR	0.99	63.57	64.00	0.0050	1649	0.58	2.33	0.89	0.30
POST OUT	BASE	100YR-002HR	0.00	0.00	0.00	0.0000	0	1.71	0.17	0.00	0.00
PRE OUT	BASE	100YR-002HR	0.00	0.00	0.00	0.0000	0	0.83	0.55	0.00	0.00
SMF	BASE	100YR-002HR	1.71	63.70	64.00	0.0050	1677	0.83	1.84	1.71	0.29
POST OUT	BASE	100YR-004HR	0.00	0.00	0.00	0.0000	0	3.06	0.36	0.00	0.00
PRE OUT	BASE	100YR-004HR	0.00	0.00	0.00	0.0000	0	2.50	0.48	0.00	0.00
SMF	BASE	100YR-004HR	3.06	63.98	64.00	-0.0050	1738	2.00	1.01	3.06	0.49
POST OUT	BASE	100YR-008HR	0.00	0.00	0.00	0.0000	0	4.25	0.31	0.00	0.00
PRE OUT	BASE	100YR-008HR	0.00	0.00	0.00	0.0000	0	4.00	0.58	0.00	0.00
SMF	BASE	100YR-008HR	4.25	63.93	64.00	0.0050	1727	4.00	1.01	4.25	0.44
POST OUT	BASE	100YR-024HR	0.00	0.00	0.00	0.0000	0	15.09	0.14	0.00	0.00
PRE OUT	BASE	100YR-024HR	0.00	0.00	0.00	0.0000	0	11.99	0.21	0.00	0.00
SMF	BASE	100YR-024HR	15.09	63.35	64.00	0.0050	1601	11.99	0.33	12.15	0.20
POST OUT	BASE	100YR-072HR	0.00	0.00	0.00	0.0000	0	60.10	0.14	0.00	0.00
PRE OUT	BASE	100YR-072HR	0.00	0.00	0.00	0.0000	0	59.98	0.18	0.00	0.00
SMF	BASE	100YR-072HR	60.10	63.34	64.00	0.0050	1600	59.92	0.21	60.05	0.17
POST OUT	BASE	100YR-168HR	0.00	0.00	0.00	0.0000	0	160.02	0.12	0.00	0.00
PRE OUT	BASE	100YR-168HR	0.00	0.00	0.00	0.0000	0	159.92	0.13	0.00	0.00
SMF	BASE	100YR-168HR	160.02	63.16	64.00	0.0043	1560	159.92	0.15	160.00	0.14
POST OUT	BASE	100YR-240HR	0.00	0.00	0.00	0.0000	0	184.03	0.14	0.00	0.00
PRE OUT	BASE	100YR-240HR	0.00	0.00	0.00	0.0000	0	183.92	0.17	0.00	0.00
SMF	BASE	100YR-240HR	184.03	63.42	64.00	0.0050	1615	183.90	0.19	184.02	0.18

==========
==== Basins ====
==========

Name: POST BASIN Node: SMF Status: Onsite
Group: BASE Type: SCS Unit Hydrograph CN

Unit Hydrograph: Uh484 Peaking Factor: 484.0
Rainfall File: Storm Duration(hrs): 0.00
Rainfall Amount(in): 0.000 Time of Conc(min): 10.00
Area(ac): 0.310 Time Shift(hrs): 0.00
Curve Number: 89.30 Max Allowable Q(cfs): 999999.000
DCIA(%): 0.00

Name: PRE BASIN Node: PRE OUT Status: Onsite
Group: BASE Type: SCS Unit Hydrograph CN

Unit Hydrograph: Uh323 Peaking Factor: 323.0
Rainfall File: Storm Duration(hrs): 0.00
Rainfall Amount(in): 0.000 Time of Conc(min): 10.00
Area(ac): 0.310 Time Shift(hrs): 0.00
Curve Number: 58.60 Max Allowable Q(cfs): 999999.000
DCIA(%): 0.00

==========
==== Nodes ====
==========

Name: GROUNDWATER Base Flow(cfs): 0.000 Init Stage(ft): 0.000
Group: BASE Warn Stage(ft): 0.000
Type: Time/Stage

Time(hrs)	Stage(ft)
0.00	0.000
99999.00	0.000

Name: POST OUT Base Flow(cfs): 0.000 Init Stage(ft): 0.000
Group: BASE Warn Stage(ft): 0.000
Type: Time/Stage

Time(hrs)	Stage(ft)
0.00	0.000
99999.00	0.000

Name: PRE OUT Base Flow(cfs): 0.000 Init Stage(ft): 0.000
Group: BASE Warn Stage(ft): 0.000
Type: Time/Stage

Time(hrs)	Stage(ft)
0.00	0.000
99999.00	0.000

Name: SMF Base Flow(cfs): 0.000 Init Stage(ft): 61.000
Group: BASE Warn Stage(ft): 64.000
Type: Stage/Area

Stage(ft)	Area(ac)
61.000	0.0150
62.000	0.0230
63.000	0.0350
64.000	0.0400
65.000	0.0500

==========
==== Drop Structures ====
==========

```
        Name: DROP STRUCTURE      From Node: SMF               Length(ft): 24.00
       Group: BASE                  To Node: POST OUT              Count: 1

              UPSTREAM       DOWNSTREAM                 Friction Equation: Automatic
    Geometry: Circular       Circular                 Solution Algorithm: Most Restrictive
    Span(in): 8.00           8.00                                Flow: Both
    Rise(in): 8.00           8.00                   Entrance Loss Coef: 0.000
   Invert(ft): 62.000        61.000                     Exit Loss Coef: 1.000
  Manning's N: 0.013000      0.013000                 Outlet Ctrl Spec: Use dc or tw
 Top Clip(in): 0.000         0.000                     Inlet Ctrl Spec: Use dc
 Bot Clip(in): 0.000         0.000                       Solution Incs: 10

Upstream FHWA Inlet Edge Description:
Circular Concrete: Square edge w/ headwall

Downstream FHWA Inlet Edge Description:
Circular Concrete: Square edge w/ headwall


*** Weir 1 of 2 for Drop Structure DROP STRUCTURE ***
                                                                                TABLE
                  Count: 1                      Bottom Clip(in): 0.000
                   Type: Horizontal                Top Clip(in): 0.000
                   Flow: Both                   Weir Disc Coef: 3.200
              Geometry: Circular             Orifice Disc Coef: 0.600

              Span(in): 2.50                       Invert(ft): 62.650
              Rise(in): 2.50                  Control Elev(ft): 62.650

*** Weir 2 of 2 for Drop Structure DROP STRUCTURE ***
                                                                                TABLE
                  Count: 1                      Bottom Clip(in): 0.000
                   Type: Vertical: Mavis           Top Clip(in): 0.000
                   Flow: Both                   Weir Disc Coef: 3.200
              Geometry: Rectangular          Orifice Disc Coef: 0.600

              Span(in): 6.00                       Invert(ft): 63.750
              Rise(in): 3.00                  Control Elev(ft): 63.750


================================================================================
==== Percolation Links =========================================================
================================================================================

        Name: PERC               From Node: SMF                     Flow: Both
       Group: BASE                 To Node: GROUNDWATER            Count: 1

        Surface Area Option: Use 1st Point in Stage/Area Table
   Vertical Flow Termination: Horizontal Flow Algorithm
       Aquifer Base Elev(ft): 57.850                    Perimeter 1(ft): 222.000
        Water Table Elev(ft): 58.100                    Perimeter 2(ft): 410.000
  Ann Recharge Rate(in/year): 0.000                     Perimeter 3(ft): 2295.000
  Horiz Conductivity(ft/day): 11.000                Distance 1 to 2(ft): 30.000
   Vert Conductivity(ft/day): 8.500                 Distance 2 to 3(ft): 300.000
     Effective Porosity(dec): 0.250                    Num Cells 1 to 2: 10
           Suction Head(in): 0.000                     Num Cells 2 to 3: 100
         Layer Thickness(ft): 2.900



================================================================================
==== Hydrology Simulations =====================================================
================================================================================

        Name: 003YR-001HR
    Filename: \\Server3\network\DJ\Templates\ICPR\003YR-001HR.R32

     Override Defaults: Yes
   Storm Duration(hrs): 1.00
         Rainfall File: Fdot-1
   Rainfall Amount(in): 2.60

Time(hrs)       Print Inc(min)
--------------- ---------------
73.000          5.00

--------------------------------------------------------------------------------------------------------------
        Name: 003YR-002HR
    Filename: \\Server3\network\DJ\Templates\ICPR\003YR-002HR.R32

     Override Defaults: Yes
   Storm Duration(hrs): 2.00
         Rainfall File: Fdot-2
   Rainfall Amount(in): 3.20

Time(hrs)       Print Inc(min)
```

74.000	5.00

Name: 003YR-004HR
Filename: \\Server3\network\DJ\Templates\ICPR\003YR-004HR.R32

Override Defaults: Yes
Storm Duration(hrs): 4.00
Rainfall File: Fdot-4
Rainfall Amount(in): 3.80

Time(hrs)	Print Inc(min)
76.000	5.00

Name: 003YR-008HR
Filename: \\Server3\network\DJ\Templates\ICPR\003YR-008HR.R32

Override Defaults: Yes
Storm Duration(hrs): 8.00
Rainfall File: Fdot-8
Rainfall Amount(in): 4.48

Time(hrs)	Print Inc(min)
80.000	5.00

Name: 003YR-024HR
Filename: \\Server3\network\DJ\Templates\ICPR\003YR-024HR.R32

Override Defaults: Yes
Storm Duration(hrs): 24.00
Rainfall File: Fdot-24
Rainfall Amount(in): 6.00

Time(hrs)	Print Inc(min)
96.000	5.00

Name: 003YR-072HR
Filename: \\Server3\network\DJ\Templates\ICPR\003YR-072HR.R32

Override Defaults: Yes
Storm Duration(hrs): 72.00
Rainfall File: Fdot-72
Rainfall Amount(in): 7.60

Time(hrs)	Print Inc(min)
144.000	5.00

Name: 003YR-168HR
Filename: \\Server3\network\DJ\Templates\ICPR\003YR-168HR.R32

Override Defaults: Yes
Storm Duration(hrs): 168.00
Rainfall File: Fdot-168
Rainfall Amount(in): 9.50

Time(hrs)	Print Inc(min)
240.000	5.00

Name: 003YR-240HR
Filename: \\Server3\network\DJ\Templates\ICPR\003YR-240HR.R32

Override Defaults: Yes
Storm Duration(hrs): 240.00
Rainfall File: Fdot-240
Rainfall Amount(in): 10.80

Time(hrs)	Print Inc(min)
312.000	5.00

Name: 005YR-001HR
Filename: \\Server3\network\DJ\Templates\ICPR\005YR-001HR.R32

Override Defaults: Yes
Storm Duration(hrs): 1.00
Rainfall File: Fdot-1

Rainfall Amount(in): 2.80

Time(hrs)	Print Inc(min)
73.000	5.00

Name: 005YR-002HR
Filename: \\Server3\network\DJ\Templates\ICPR\005YR-002HR.R32

Override Defaults: Yes
Storm Duration(hrs): 2.00
Rainfall File: Fdot-2
Rainfall Amount(in): 3.40

Time(hrs)	Print Inc(min)
74.000	5.00

Name: 005YR-004HR
Filename: \\Server3\network\DJ\Templates\ICPR\005YR-004HR.R32

Override Defaults: Yes
Storm Duration(hrs): 4.00
Rainfall File: Fdot-4
Rainfall Amount(in): 4.00

Time(hrs)	Print Inc(min)
76.000	5.00

Name: 005YR-008HR
Filename: \\Server3\network\DJ\Templates\ICPR\005YR-008HR.R32

Override Defaults: Yes
Storm Duration(hrs): 8.00
Rainfall File: Fdot-8
Rainfall Amount(in): 4.80

Time(hrs)	Print Inc(min)
80.000	5.00

Name: 005YR-024HR
Filename: \\Server3\network\DJ\Templates\ICPR\005YR-024HR.R32

Override Defaults: Yes
Storm Duration(hrs): 24.00
Rainfall File: Fdot-24
Rainfall Amount(in): 6.48

Time(hrs)	Print Inc(min)
96.000	5.00

Name: 005YR-072HR
Filename: \\Server3\network\DJ\Templates\ICPR\005YR-072HR.R32

Override Defaults: Yes
Storm Duration(hrs): 72.00
Rainfall File: Fdot-72
Rainfall Amount(in): 8.00

Time(hrs)	Print Inc(min)
144.000	5.00

Name: 005YR-168HR
Filename: \\Server3\network\DJ\Templates\ICPR\005YR-168HR.R32

Override Defaults: Yes
Storm Duration(hrs): 168.00
Rainfall File: Fdot-168
Rainfall Amount(in): 9.60

Time(hrs)	Print Inc(min)
240.000	5.00

Name: 005YR-240HR
Filename: \\Server3\network\DJ\Templates\ICPR\005YR-240HR.R32

```
      Override Defaults: Yes
    Storm Duration(hrs): 240.00
          Rainfall File: Fdot-240
    Rainfall Amount(in): 11.00

Time(hrs)       Print Inc(min)
--------------- ---------------
312.000         5.00

-------------------------------------------------------------------------------------------------------
         Name: 010YR-001HR
     Filename: \\Server3\network\DJ\Templates\ICPR\010YR-001HR.R32

      Override Defaults: Yes
    Storm Duration(hrs): 1.00
          Rainfall File: Fdot-1
    Rainfall Amount(in): 3.20

Time(hrs)       Print Inc(min)
--------------- ---------------
73.000          5.00

-------------------------------------------------------------------------------------------------------
         Name: 010YR-002HR
     Filename: \\Server3\network\DJ\Templates\ICPR\010YR-002HR.R32

      Override Defaults: Yes
    Storm Duration(hrs): 2.00
          Rainfall File: Fdot-2
    Rainfall Amount(in): 4.00

Time(hrs)       Print Inc(min)
--------------- ---------------
74.000          5.00

-------------------------------------------------------------------------------------------------------
         Name: 010YR-004HR
     Filename: \\Server3\network\DJ\Templates\ICPR\010YR-004HR.R32

      Override Defaults: Yes
    Storm Duration(hrs): 4.00
          Rainfall File: Fdot-4
    Rainfall Amount(in): 4.80

Time(hrs)       Print Inc(min)
--------------- ---------------
76.000          5.00

-------------------------------------------------------------------------------------------------------
         Name: 010YR-008HR
     Filename: \\Server3\network\DJ\Templates\ICPR\010YR-008HR.R32

      Override Defaults: Yes
    Storm Duration(hrs): 8.00
          Rainfall File: Fdot-8
    Rainfall Amount(in): 5.84

Time(hrs)       Print Inc(min)
--------------- ---------------
80.000          5.00

-------------------------------------------------------------------------------------------------------
         Name: 010YR-024HR
     Filename: \\Server3\network\DJ\Templates\ICPR\010YR-024HR.R32

      Override Defaults: Yes
    Storm Duration(hrs): 24.00
          Rainfall File: Fdot-24
    Rainfall Amount(in): 7.92

Time(hrs)       Print Inc(min)
--------------- ---------------
96.000          5.00

-------------------------------------------------------------------------------------------------------
         Name: 010YR-072HR
     Filename: \\Server3\network\DJ\Templates\ICPR\010YR-072HR.R32

      Override Defaults: Yes
    Storm Duration(hrs): 72.00
          Rainfall File: Fdot-72
    Rainfall Amount(in): 8.90

Time(hrs)       Print Inc(min)
--------------- ---------------
144.000         5.00

-------------------------------------------------------------------------------------------------------
```

Name: 010YR-168HR
Filename: \\Server3\network\DJ\Templates\ICPR\010YR-168HR.R32

Override Defaults: Yes
Storm Duration(hrs): 168.00
Rainfall File: Fdot-168
Rainfall Amount(in): 11.00

Time(hrs)	Print Inc(min)
240.000	5.00

Name: 010YR-240HR
Filename: \\Server3\network\DJ\Templates\ICPR\010YR-240HR.R32

Override Defaults: Yes
Storm Duration(hrs): 240.00
Rainfall File: Fdot-240
Rainfall Amount(in): 12.50

Time(hrs)	Print Inc(min)
312.000	5.00

Name: 025YR-001HR
Filename: \\Server3\network\DJ\Templates\ICPR\025YR-001HR.R32

Override Defaults: Yes
Storm Duration(hrs): 1.00
Rainfall File: Fdot-1
Rainfall Amount(in): 3.60

Time(hrs)	Print Inc(min)
73.000	5.00

Name: 025YR-002HR
Filename: \\Server3\network\DJ\Templates\ICPR\025YR-002HR.R32

Override Defaults: Yes
Storm Duration(hrs): 2.00
Rainfall File: Fdot-2
Rainfall Amount(in): 4.40

Time(hrs)	Print Inc(min)
74.000	5.00

Name: 025YR-004HR
Filename: \\Server3\network\DJ\Templates\ICPR\025YR-004HR.R32

Override Defaults: Yes
Storm Duration(hrs): 4.00
Rainfall File: Fdot-4
Rainfall Amount(in): 5.28

Time(hrs)	Print Inc(min)
76.000	5.00

Name: 025YR-008HR
Filename: \\Server3\network\DJ\Templates\ICPR\025YR-008HR.R32

Override Defaults: Yes
Storm Duration(hrs): 8.00
Rainfall File: Fdot-8
Rainfall Amount(in): 6.56

Time(hrs)	Print Inc(min)
80.000	5.00

Name: 025YR-024HR
Filename: \\Server3\network\DJ\Templates\ICPR\025YR-024HR.R32

Override Defaults: Yes
Storm Duration(hrs): 24.00
Rainfall File: Fdot-24
Rainfall Amount(in): 8.64

Time(hrs)	Print Inc(min)

96.000 5.00

Name: 025YR-072HR
Filename: \\Server3\network\DJ\Templates\ICPR\025YR-072HR.R32

Override Defaults: Yes
Storm Duration(hrs): 72.00
Rainfall File: Fdot-72
Rainfall Amount(in): 11.00

Time(hrs)	Print Inc(min)
144.000	5.00

Name: 025YR-096HR
Filename: \\Server3\network\DJ\Templates\ICPR\025YR-096HR.R32

Override Defaults: Yes
Storm Duration(hrs): 96.00
Rainfall File: Sjrwmd96
Rainfall Amount(in): 9.60

Time(hrs)	Print Inc(min)
168.000	5.00

Name: 025YR-168HR
Filename: \\Server3\network\DJ\Templates\ICPR\025YR-168HR.R32

Override Defaults: Yes
Storm Duration(hrs): 168.00
Rainfall File: Fdot-168
Rainfall Amount(in): 13.00

Time(hrs)	Print Inc(min)
240.000	5.00

Name: 025YR-240HR
Filename: \\Server3\network\DJ\Templates\ICPR\025YR-240HR.R32

Override Defaults: Yes
Storm Duration(hrs): 240.00
Rainfall File: Fdot-240
Rainfall Amount(in): 15.00

Time(hrs)	Print Inc(min)
312.000	5.00

Name: 050YR-001HR
Filename: \\Server3\network\DJ\Templates\ICPR\050YR-001HR.R32

Override Defaults: Yes
Storm Duration(hrs): 1.00
Rainfall File: Fdot-1
Rainfall Amount(in): 3.95

Time(hrs)	Print Inc(min)
73.000	5.00

Name: 050YR-002HR
Filename: \\Server3\network\DJ\Templates\ICPR\050YR-002HR.R32

Override Defaults: Yes
Storm Duration(hrs): 2.00
Rainfall File: Fdot-2
Rainfall Amount(in): 4.80

Time(hrs)	Print Inc(min)
74.000	5.00

Name: 050YR-004HR
Filename: \\Server3\network\DJ\Templates\ICPR\050YR-004HR.R32

Override Defaults: Yes
Storm Duration(hrs): 4.00
Rainfall File: Fdot-4
Rainfall Amount(in): 5.92

Time(hrs) Print Inc(min)
--- ---
76.000 5.00

Name: 050YR-008HR
Filename: \\Server3\network\DJ\Templates\ICPR\050YR-008HR.R32

Override Defaults: Yes
Storm Duration(hrs): 8.00
Rainfall File: Fdot-8
Rainfall Amount(in): 6.96

Time(hrs) Print Inc(min)
--- ---
80.000 5.00

Name: 050YR-024HR
Filename: \\Server3\network\DJ\Templates\ICPR\050YR-024HR.R32

Override Defaults: Yes
Storm Duration(hrs): 24.00
Rainfall File: Fdot-24
Rainfall Amount(in): 9.60

Time(hrs) Print Inc(min)
--- ---
96.000 5.00

Name: 050YR-072HR
Filename: \\Server3\network\DJ\Templates\ICPR\050YR-072HR.R32

Override Defaults: Yes
Storm Duration(hrs): 72.00
Rainfall File: Fdot-72
Rainfall Amount(in): 12.00

Time(hrs) Print Inc(min)
--- ---
144.000 5.00

Name: 050YR-168HR
Filename: \\Server3\network\DJ\Templates\ICPR\050YR-168HR.R32

Override Defaults: Yes
Storm Duration(hrs): 168.00
Rainfall File: Fdot-168
Rainfall Amount(in): 14.00

Time(hrs) Print Inc(min)
--- ---
240.000 5.00

Name: 050YR-240HR
Filename: \\Server3\network\DJ\Templates\ICPR\050YR-240HR.R32

Override Defaults: Yes
Storm Duration(hrs): 240.00
Rainfall File: Fdot-240
Rainfall Amount(in): 16.00

Time(hrs) Print Inc(min)
--- ---
312.000 5.00

Name: 100YR-001HR
Filename: \\Server3\network\DJ\Templates\ICPR\100YR-001HR.R32

Override Defaults: Yes
Storm Duration(hrs): 1.00
Rainfall File: Fdot-1
Rainfall Amount(in): 4.40

Time(hrs) Print Inc(min)
--- ---
721.000 5.00

Name: 100YR-002HR
Filename: \\Server3\network\DJ\Templates\ICPR\100YR-002HR.R32

Override Defaults: Yes

```
    Storm Duration(hrs): 2.00
          Rainfall File: Fdot-2
    Rainfall Amount(in): 5.40

Time(hrs)       Print Inc(min)
--------------- ---------------
722.000         5.00

-------------------------------------------------------------------------------------------------------
         Name: 100YR-004HR
     Filename: \\Server3\network\DJ\Templates\ICPR\100YR-004HR.R32

      Override Defaults: Yes
    Storm Duration(hrs): 4.00
          Rainfall File: Fdot-4
    Rainfall Amount(in): 6.72

Time(hrs)       Print Inc(min)
--------------- ---------------
724.000         5.00

-------------------------------------------------------------------------------------------------------
         Name: 100YR-008HR
     Filename: \\Server3\network\DJ\Templates\ICPR\100YR-008HR.R32

      Override Defaults: Yes
    Storm Duration(hrs): 8.00
          Rainfall File: Fdot-8
    Rainfall Amount(in): 8.00

Time(hrs)       Print Inc(min)
--------------- ---------------
728.000         5.00

-------------------------------------------------------------------------------------------------------
         Name: 100YR-024HR
     Filename: \\Server3\network\DJ\Templates\ICPR\100YR-024HR.R32

      Override Defaults: Yes
    Storm Duration(hrs): 24.00
          Rainfall File: Fdot-24
    Rainfall Amount(in): 11.04

Time(hrs)       Print Inc(min)
--------------- ---------------
744.000         5.00

-------------------------------------------------------------------------------------------------------
         Name: 100YR-072HR
     Filename: \\Server3\network\DJ\Templates\ICPR\100YR-072HR.R32

      Override Defaults: Yes
    Storm Duration(hrs): 72.00
          Rainfall File: Fdot-72
    Rainfall Amount(in): 13.80

Time(hrs)       Print Inc(min)
--------------- ---------------
792.000         5.00

-------------------------------------------------------------------------------------------------------
         Name: 100YR-168HR
     Filename: \\Server3\network\DJ\Templates\ICPR\100YR-168HR.R32

      Override Defaults: Yes
    Storm Duration(hrs): 168.00
          Rainfall File: Fdot-168
    Rainfall Amount(in): 16.00

Time(hrs)       Print Inc(min)
--------------- ---------------
888.000         5.00

-------------------------------------------------------------------------------------------------------
         Name: 100YR-240HR
     Filename: \\Server3\network\DJ\Templates\ICPR\100YR-240HR.R32

      Override Defaults: Yes
    Storm Duration(hrs): 240.00
          Rainfall File: Fdot-240
    Rainfall Amount(in): 18.00

Time(hrs)       Print Inc(min)
--------------- ---------------
960.000         5.00

-------------------------------------------------------------------------------------------------------
         Name: MEAN ANNUAL
```

Filename: \\Server3\network\DJ\Templates\ICPR\MEAN ANNUAL.R32

Override Defaults: Yes
Storm Duration(hrs): 24.00
Rainfall File: Flmod
Rainfall Amount(in): 4.20

Time(hrs)	Print Inc(min)
96.000	5.00

Name: WQTV
Filename: \\server3\engprojects\Alchem - Bldg 4\Design Reports\Drainage\Current Model\WQTV.R32

Override Defaults: Yes
Storm Duration(hrs): 24.00
Rainfall File: Flmod
Rainfall Amount(in): 0.00

Time(hrs)	Print Inc(min)
72.000	5.00

==========
==== Routing Simulations ==========
==========

Name: 003YR-001HR Hydrology Sim: 003YR-001HR
Filename: \\Server3\network\DJ\Templates\ICPR\003YR-001HR.I32

Execute: No Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 73.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs)	Print Inc(min)
999.000	15.000

Group	Run
BASE	Yes

Name: 003YR-002HR Hydrology Sim: 003YR-002HR
Filename: \\Server3\network\DJ\Templates\ICPR\003YR-002HR.I32

Execute: No Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 74.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs)	Print Inc(min)
999.000	15.000

Group	Run
BASE	Yes

Name: 003YR-004HR Hydrology Sim: 003YR-004HR
Filename: \\Server3\network\DJ\Templates\ICPR\003YR-004HR.I32

Execute: No Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 78.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs) Print Inc(min)
--- ---
999.000 15.000

Group Run
--- ---
BASE Yes

Name: 003YR-008HR Hydrology Sim: 003YR-008HR
Filename: \\Server3\network\DJ\Templates\ICPR\003YR-008HR.I32

Execute: No Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 80.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs) Print Inc(min)
--- ---
999.000 15.000

Group Run
--- ---
BASE Yes

Name: 003YR-024HR Hydrology Sim: 003YR-024HR
Filename: \\Server3\network\DJ\Templates\ICPR\003YR-024HR.I32

Execute: No Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 96.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs) Print Inc(min)
--- ---
999.000 15.000

Group Run
--- ---
BASE Yes

Name: 003YR-072HR Hydrology Sim: 003YR-072HR
Filename: \\Server3\network\DJ\Templates\ICPR\003YR-072HR.I32

Execute: No Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 792.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs) Print Inc(min)
--- ---
999.000 15.000

Group Run
--- ---
BASE Yes

Name: 003YR-168HR Hydrology Sim: 003YR-168HR
Filename: \\Server3\network\DJ\Templates\ICPR\003YR-168HR.I32

Execute: No Restart: No Patch: No

Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 888.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs)	Print Inc(min)
999.000	15.000

Group	Run
BASE	Yes

Name: 003YR-240HR Hydrology Sim: 003YR-240HR
Filename: \\Server3\network\DJ\Templates\ICPR\003YR-240HR.I32

Execute: No Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 960.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs)	Print Inc(min)
999.000	15.000

Group	Run
BASE	Yes

Name: 005YR-001HR Hydrology Sim: 005YR-001HR
Filename: \\Server3\network\DJ\Templates\ICPR\005YR-001HR.I32

Execute: No Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 73.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs)	Print Inc(min)
999.000	15.000

Group	Run
BASE	Yes

Name: 005YR-002HR Hydrology Sim: 005YR-002HR
Filename: \\Server3\network\DJ\Templates\ICPR\005YR-002HR.I32

Execute: No Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 74.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs)	Print Inc(min)
999.000	15.000

Group	Run

BASE Yes

Name: 005YR-004HR Hydrology Sim: 005YR-004HR
Filename: \\Server3\network\DJ\Templates\ICPR\005YR-004HR.I32

Execute: No Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 76.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs)	Print Inc(min)
999.000	15.000

Group	Run
BASE	Yes

Name: 005YR-008HR Hydrology Sim: 005YR-008HR
Filename: \\Server3\network\DJ\Templates\ICPR\005YR-008HR.I32

Execute: No Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 80.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs)	Print Inc(min)
999.000	15.000

Group	Run
BASE	Yes

Name: 005YR-024HR Hydrology Sim: 005YR-024HR
Filename: \\Server3\network\DJ\Templates\ICPR\005YR-024HR.I32

Execute: No Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 96.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs)	Print Inc(min)
999.000	15.000

Group	Run
BASE	Yes

Name: 005YR-072HR Hydrology Sim: 005YR-072HR
Filename: \\Server3\network\DJ\Templates\ICPR\005YR-072HR.I32

Execute: No Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 144.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs) Print Inc(min)
--- ---
999.000 15.000

Group Run
--- ---
BASE Yes

Name: 005YR-168HR Hydrology Sim: 005YR-168HR
Filename: \\Server3\network\DJ\Templates\ICPR\005YR-168HR.I32

Execute: No Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 240.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs) Print Inc(min)
--- ---
999.000 15.000

Group Run
--- ---
BASE Yes

Name: 005YR-240HR Hydrology Sim: 005YR-240HR
Filename: \\Server3\network\DJ\Templates\ICPR\005YR-240HR.I32

Execute: No Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 312.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs) Print Inc(min)
--- ---
999.000 15.000

Group Run
--- ---
BASE Yes

Name: 010YR-001HR Hydrology Sim: 010YR-001HR
Filename: \\Server3\network\DJ\Templates\ICPR\010YR-001HR.I32

Execute: No Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 73.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs) Print Inc(min)
--- ---
999.000 15.000

Group Run
--- ---
BASE Yes

Name: 010YR-002HR Hydrology Sim: 010YR-002HR
Filename: \\Server3\network\DJ\Templates\ICPR\010YR-002HR.I32

Execute: No Restart: No Patch: No

Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 74.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs)	Print Inc(min)
999.000	15.000

Group	Run
BASE	Yes

Name: 010YR-004HR Hydrology Sim: 010YR-004HR
Filename: \\Server3\network\DJ\Templates\ICPR\010YR-004HR.I32

Execute: No Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 76.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs)	Print Inc(min)
999.000	15.000

Group	Run
BASE	Yes

Name: 010YR-008HR Hydrology Sim: 010YR-008HR
Filename: \\Server3\network\DJ\Templates\ICPR\010YR-008HR.I32

Execute: No Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 80.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs)	Print Inc(min)
999.000	15.000

Group	Run
BASE	Yes

Name: 010YR-024HR Hydrology Sim: 010YR-024HR
Filename: \\Server3\network\DJ\Templates\ICPR\010YR-024HR.I32

Execute: No Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 96.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs)	Print Inc(min)
999.000	15.000

Group	Run

BASE Yes

Name: 010YR-072HR Hydrology Sim: 010YR-072HR
Filename: \\Server3\network\DJ\Templates\ICPR\010YR-072HR.I32

Execute: No Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 144.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs)	Print Inc(min)
999.000	15.000

Group	Run
BASE	Yes

Name: 010YR-168HR Hydrology Sim: 010YR-168HR
Filename: \\Server3\network\DJ\Templates\ICPR\010YR-168HR.I32

Execute: No Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 240.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs)	Print Inc(min)
999.000	15.000

Group	Run
BASE	Yes

Name: 010YR-240HR Hydrology Sim: 010YR-240HR
Filename: \\Server3\network\DJ\Templates\ICPR\010YR-240HR.I32

Execute: No Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 312.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs)	Print Inc(min)
999.000	15.000

Group	Run
BASE	Yes

Name: 025YR-001HR Hydrology Sim: 025YR-001HR
Filename: \\Server3\network\DJ\Templates\ICPR\025YR-001HR.I32

Execute: No Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 73.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs) Print Inc(min)
--- ---
999.000 15.000

Group Run
--- ---
BASE Yes

Name: 025YR-002HR Hydrology Sim: 025YR-002HR
Filename: \\Server3\network\DJ\Templates\ICPR\025YR-002HR.I32

Execute: No Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 74.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs) Print Inc(min)
--- ---
999.000 15.000

Group Run
--- ---
BASE Yes

Name: 025YR-004HR Hydrology Sim: 025YR-004HR
Filename: \\Server3\network\DJ\Templates\ICPR\025YR-004HR.I32

Execute: No Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 76.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs) Print Inc(min)
--- ---
999.000 15.000

Group Run
--- ---
BASE Yes

Name: 025YR-008HR Hydrology Sim: 025YR-008HR
Filename: \\Server3\network\DJ\Templates\ICPR\025YR-008HR.I32

Execute: No Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 80.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs) Print Inc(min)
--- ---
999.000 15.000

Group Run
--- ---
BASE Yes

Name: 025YR-024HR Hydrology Sim: 025YR-024HR
Filename: \\Server3\network\DJ\Templates\ICPR\025YR-024HR.I32

Execute: No Restart: No Patch: No

Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 96.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs)	Print Inc(min)
999.000	15.000

Group	Run
BASE	Yes

Name: 025YR-072HR Hydrology Sim: 025YR-072HR
Filename: \\Server3\network\DJ\Templates\ICPR\025YR-072HR.I32

Execute: No Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 144.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs)	Print Inc(min)
999.000	15.000

Group	Run
BASE	Yes

Name: 025YR-096HR Hydrology Sim: 025YR-096HR
Filename: \\Server3\network\DJ\Templates\ICPR\025YR-096HR.I32

Execute: No Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 168.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs)	Print Inc(min)
999.000	15.000

Group	Run
BASE	Yes

Name: 025YR-168HR Hydrology Sim: 025YR-168HR
Filename: \\Server3\network\DJ\Templates\ICPR\025YR-168HR.I32

Execute: No Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 240.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs)	Print Inc(min)
999.000	15.000

Group	Run

BASE Yes

Name: 025YR-240HR Hydrology Sim: 025YR-240HR
Filename: \\Server3\network\DJ\Templates\ICPR\025YR-240HR.I32

Execute: No Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 312.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs)	Print Inc(min)
999.000	15.000

Group	Run
BASE	Yes

Name: 050YR-001HR Hydrology Sim: 050YR-001HR
Filename: \\Server3\network\DJ\Templates\ICPR\050YR-001HR.I32

Execute: No Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 73.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs)	Print Inc(min)
999.000	15.000

Group	Run
BASE	Yes

Name: 050YR-002HR Hydrology Sim: 050YR-002HR
Filename: \\Server3\network\DJ\Templates\ICPR\050YR-002HR.I32

Execute: No Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 74.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs)	Print Inc(min)
999.000	15.000

Group	Run
BASE	Yes

Name: 050YR-004HR Hydrology Sim: 050YR-004HR
Filename: \\Server3\network\DJ\Templates\ICPR\050YR-004HR.I32

Execute: No Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 76.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs) Print Inc(min)
--- ---
999.000 15.000

Group Run
--- ---
BASE Yes

Name: 050YR-008HR Hydrology Sim: 050YR-008HR
Filename: \\Server3\network\DJ\Templates\ICPR\050YR-008HR.I32

Execute: No Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 80.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs) Print Inc(min)
--- ---
999.000 15.000

Group Run
--- ---
BASE Yes

Name: 050YR-024HR Hydrology Sim: 050YR-024HR
Filename: \\Server3\network\DJ\Templates\ICPR\050YR-024HR.I32

Execute: No Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 96.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs) Print Inc(min)
--- ---
999.000 15.000

Group Run
--- ---
BASE Yes

Name: 050YR-072HR Hydrology Sim: 050YR-072HR
Filename: \\Server3\network\DJ\Templates\ICPR\050YR-072HR.I32

Execute: No Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 144.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs) Print Inc(min)
--- ---
999.000 15.000

Group Run
--- ---
BASE Yes

Name: 050YR-168HR Hydrology Sim: 050YR-168HR
Filename: \\Server3\network\DJ\Templates\ICPR\050YR-168HR.I32

Execute: No Restart: No Patch: No

Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 240.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs)	Print Inc(min)
999.000	15.000

Group	Run
BASE	Yes

Name: 050YR-240HR Hydrology Sim: 050YR-240HR
Filename: \\Server3\network\DJ\Templates\ICPR\050YR-240HR.I32

Execute: No Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 312.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs)	Print Inc(min)
999.000	15.000

Group	Run
BASE	Yes

Name: 100YR-001HR Hydrology Sim: 100YR-001HR
Filename: \\Server3\network\DJ\Templates\ICPR\100YR-001HR.I32

Execute: Yes Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 721.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs)	Print Inc(min)
999.000	15.000

Group	Run
BASE	Yes

Name: 100YR-002HR Hydrology Sim: 100YR-002HR
Filename: \\Server3\network\DJ\Templates\ICPR\100YR-002HR.I32

Execute: Yes Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 722.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs)	Print Inc(min)
999.000	15.000

Group	Run

BASE Yes

Name: 100YR-004HR Hydrology Sim: 100YR-004HR
Filename: \\Server3\network\DJ\Templates\ICPR\100YR-004HR.I32

Execute: Yes Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 724.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs)	Print Inc(min)
999.000	15.000

Group	Run
BASE	Yes

Name: 100YR-008HR Hydrology Sim: 100YR-008HR
Filename: \\Server3\network\DJ\Templates\ICPR\100YR-008HR.I32

Execute: Yes Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 728.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs)	Print Inc(min)
999.000	15.000

Group	Run
BASE	Yes

Name: 100YR-024HR Hydrology Sim: 100YR-024HR
Filename: \\Server3\network\DJ\Templates\ICPR\100YR-024HR.I32

Execute: Yes Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 744.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs)	Print Inc(min)
999.000	15.000

Group	Run
BASE	Yes

Name: 100YR-072HR Hydrology Sim: 100YR-072HR
Filename: \\Server3\network\DJ\Templates\ICPR\100YR-072HR.I32

Execute: Yes Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 792.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs) Print Inc(min)
--- ---
999.000 15.000

Group Run
--- ---
BASE Yes

Name: 100YR-168HR Hydrology Sim: 100YR-168HR
Filename: \\Server3\network\DJ\Templates\ICPR\100YR-168HR.I32

Execute: Yes Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 888.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs) Print Inc(min)
--- ---
999.000 15.000

Group Run
--- ---
BASE Yes

Name: 100YR-240HR Hydrology Sim: 100YR-240HR
Filename: \\Server3\network\DJ\Templates\ICPR\100YR-240HR.I32

Execute: Yes Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 960.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs) Print Inc(min)
--- ---
999.000 15.000

Group Run
--- ---
BASE Yes

Name: MEAN ANNUAL Hydrology Sim: MEAN ANNUAL
Filename: \\Server3\network\DJ\Templates\ICPR\MEAN ANNUAL.I32

Execute: No Restart: No Patch: No
Alternative: No

Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 96.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs) Print Inc(min)
--- ---
999.000 15.000

Group Run
--- ---
BASE Yes

Name: WQTV Hydrology Sim: WQTV
Filename: \\server3\engprojects\Alchem - Bldg 4\Design Reports\Drainage\Current Model\WQTV.I32

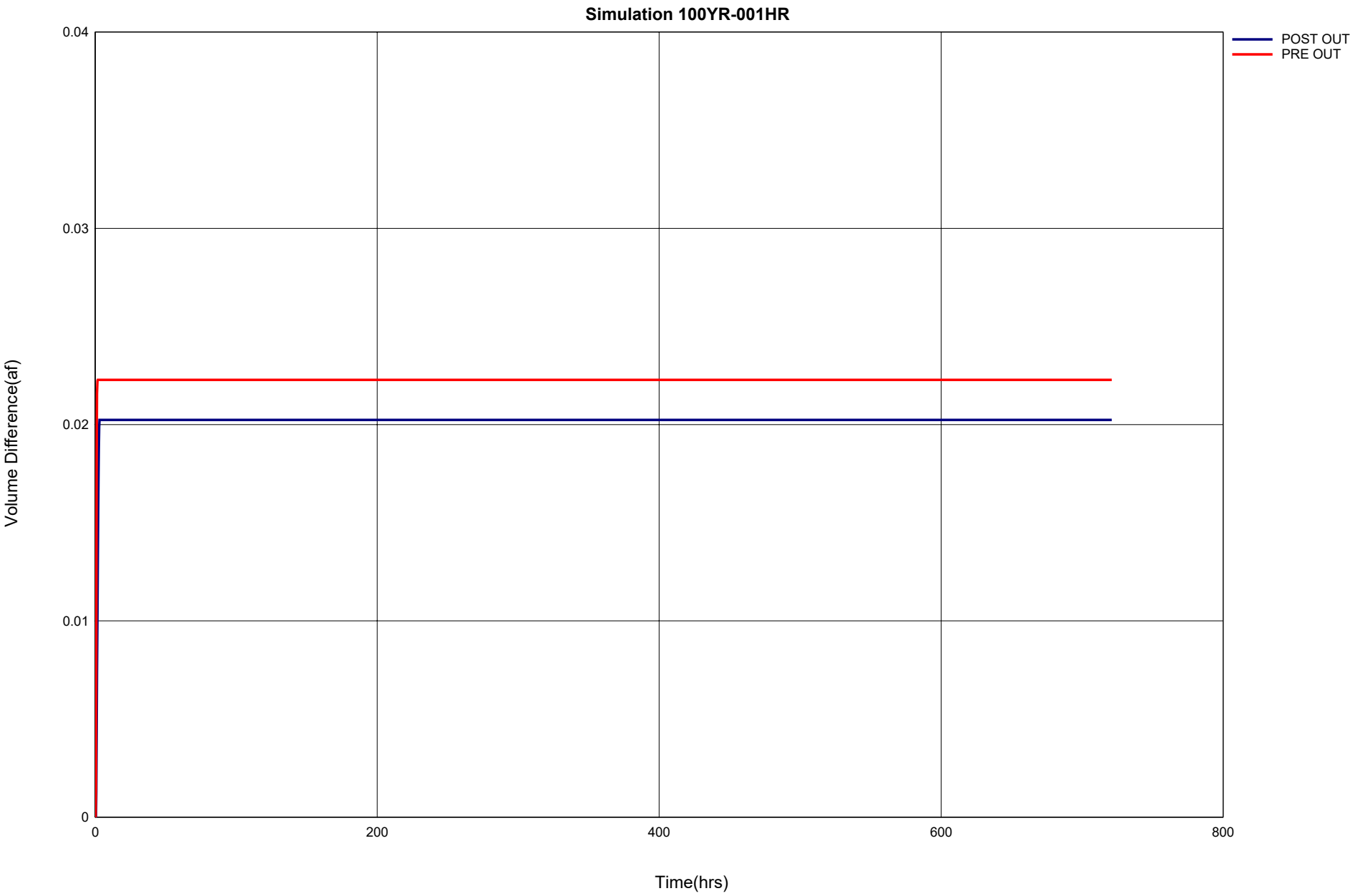
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Alternative: No

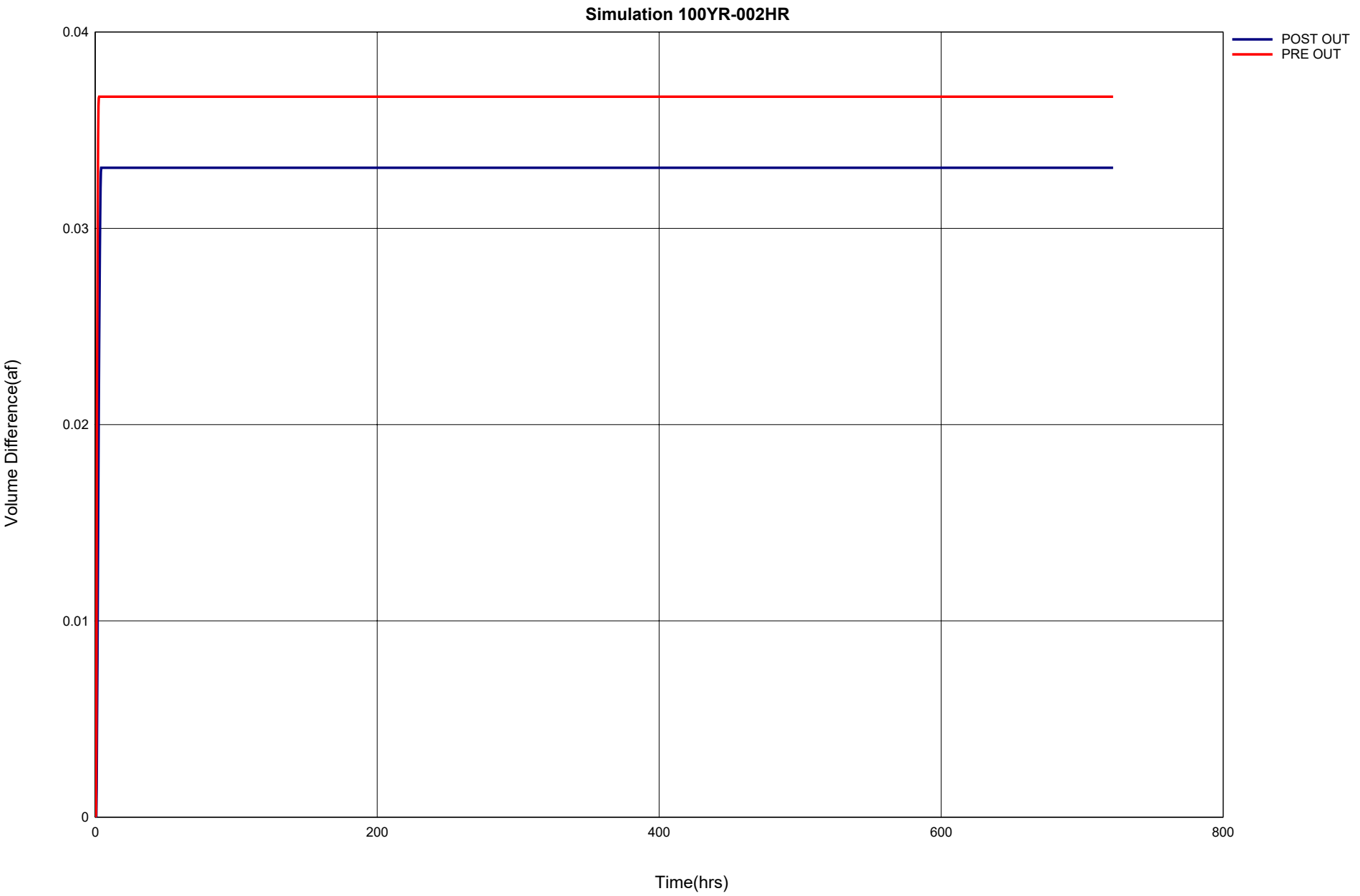
Max Delta Z(ft): 1.00 Delta Z Factor: 0.00500
Time Step Optimizer: 10.000
Start Time(hrs): 0.000 End Time(hrs): 72.00
Min Calc Time(sec): 0.5000 Max Calc Time(sec): 60.0000
Boundary Stages: Boundary Flows:

Time(hrs)	Print Inc(min)
999.000	15.000

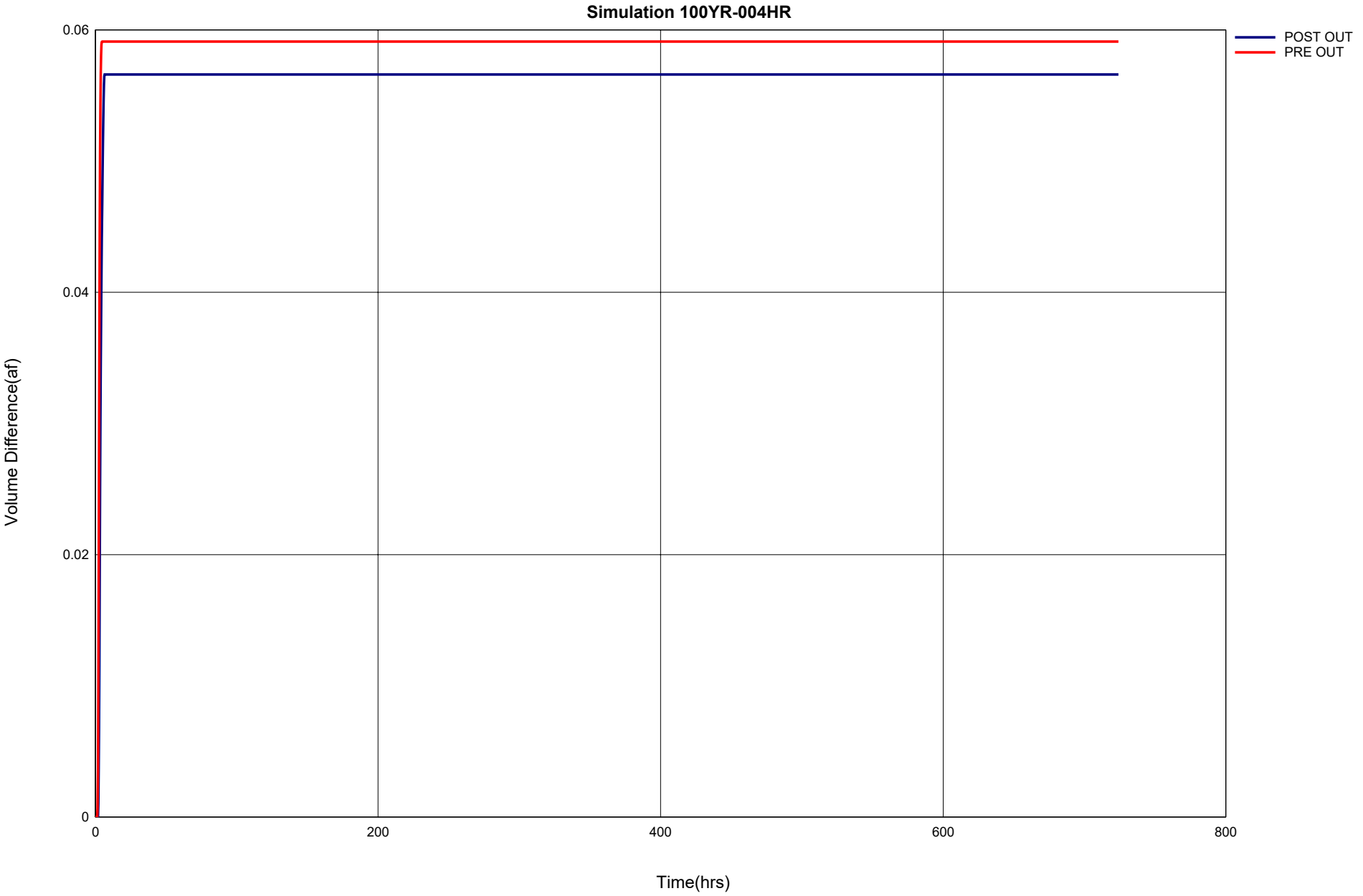
Group	Run
BASE	Yes



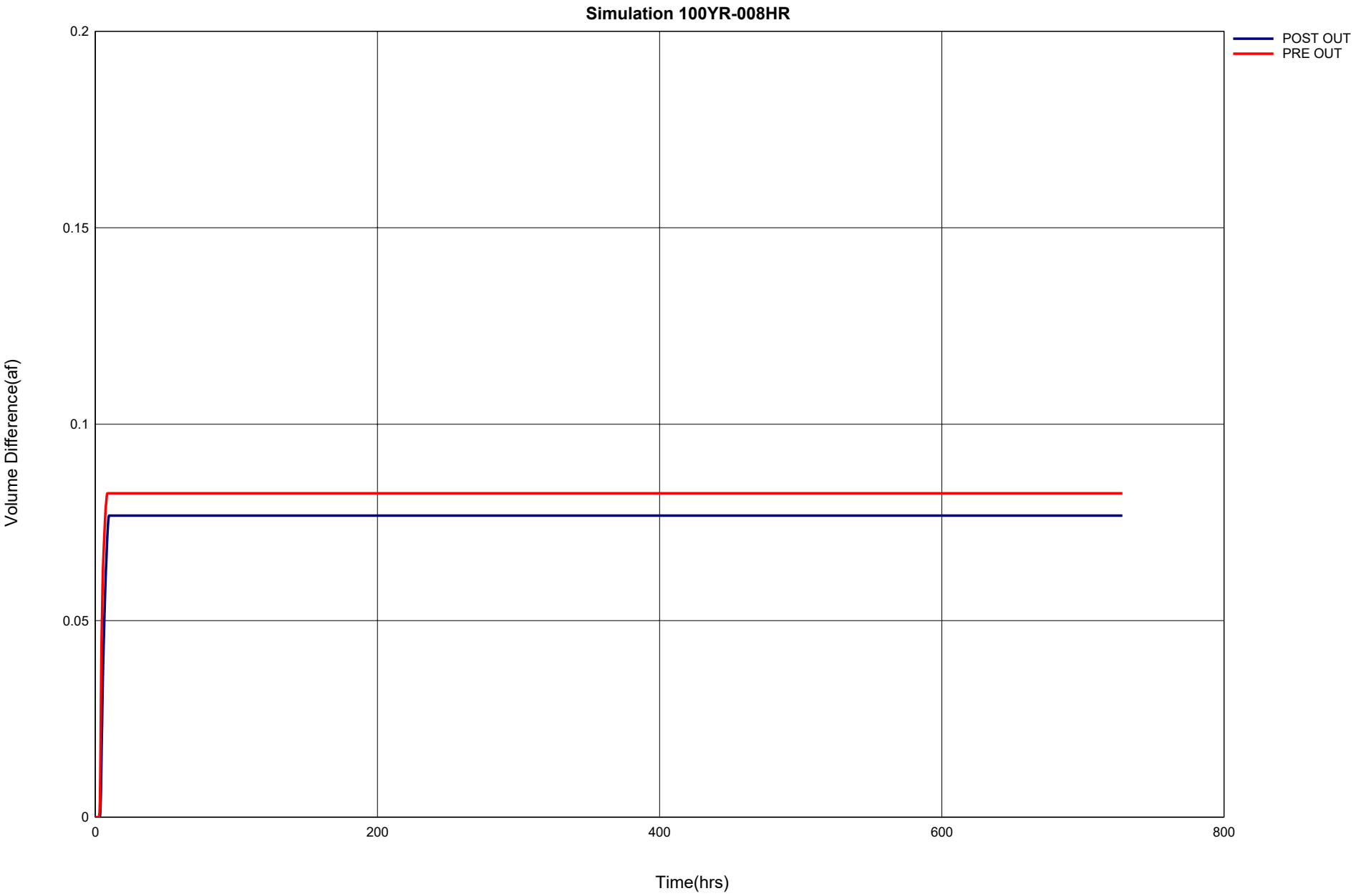
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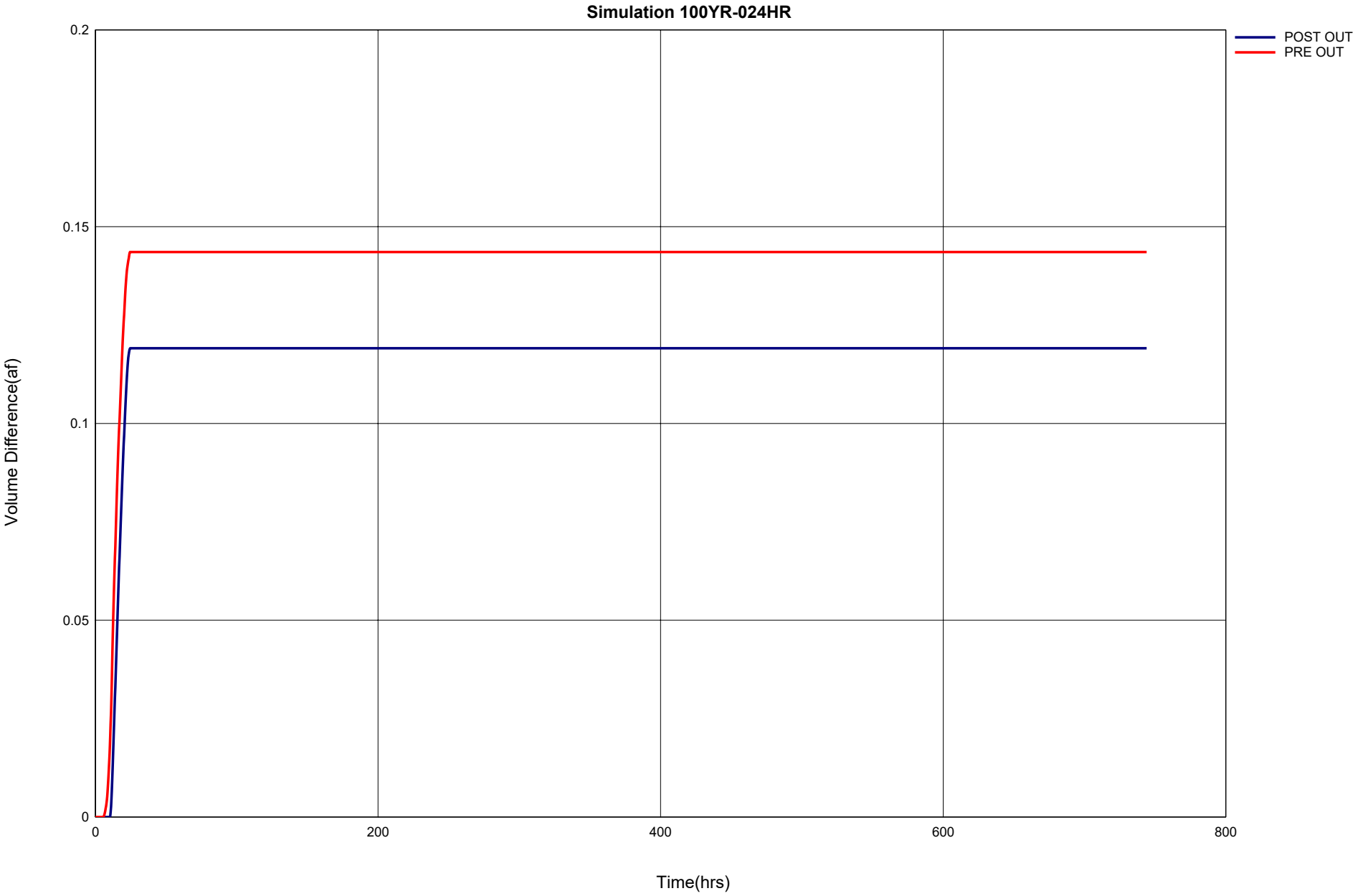
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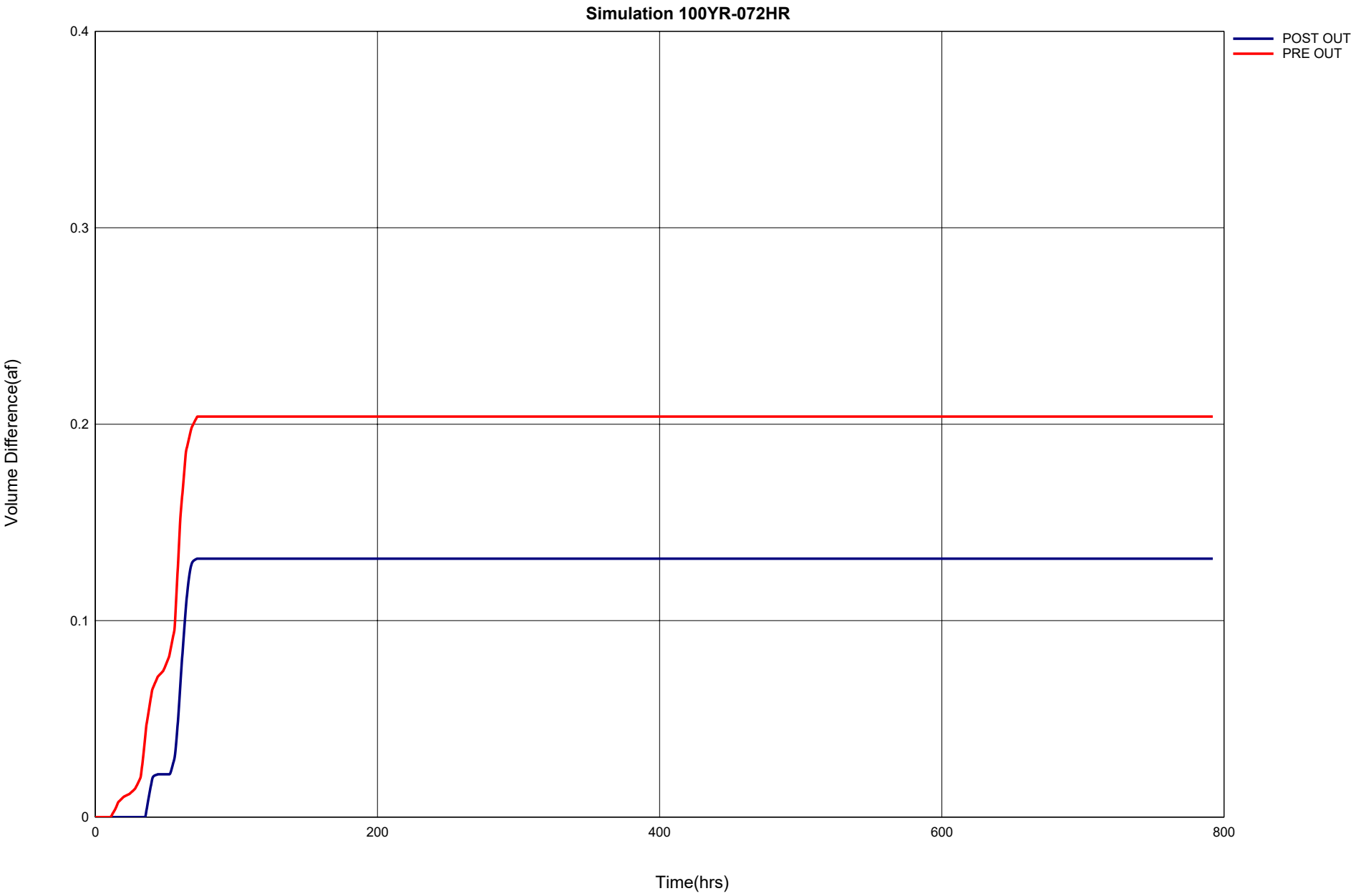
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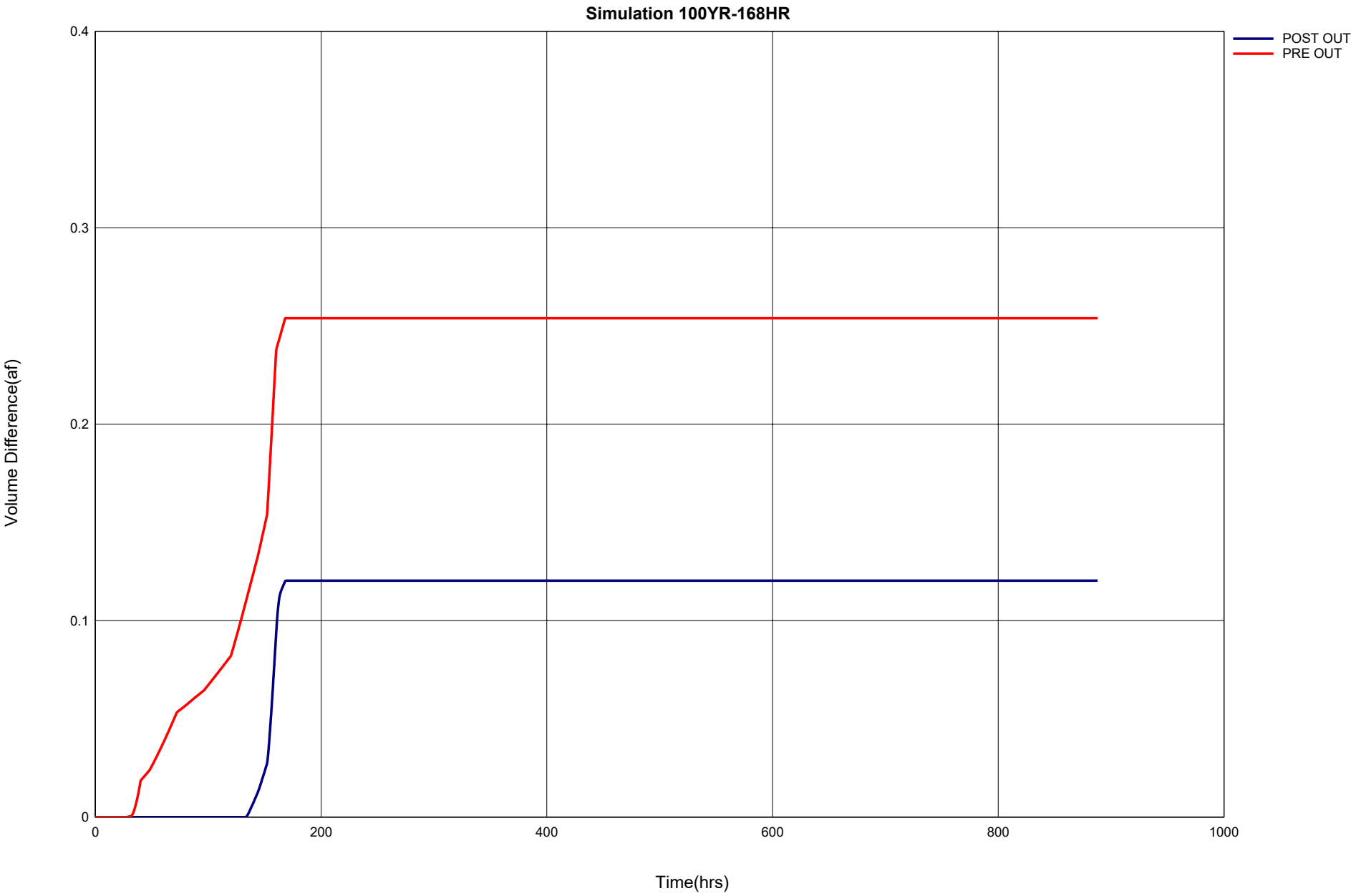
Interconnected Channel and Pond Routing Model (ICPR) ©2002 Streamline Technologies, Inc.



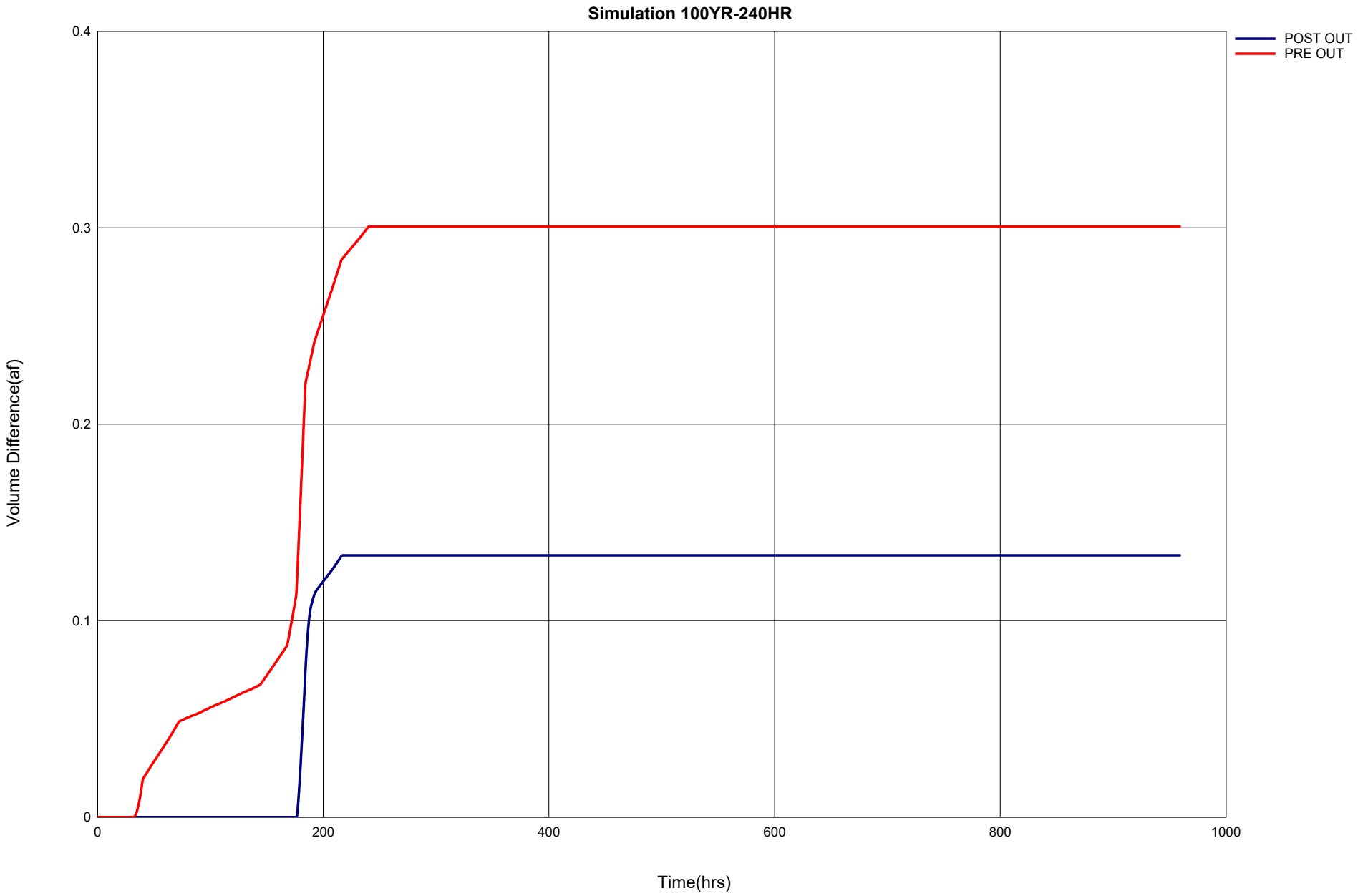
Interconnected Channel and Pond Routing Model (ICPR) ©2002 Streamline Technologies, Inc.



Interconnected Channel and Pond Routing Model (ICPR) ©2002 Streamline Technologies, Inc.



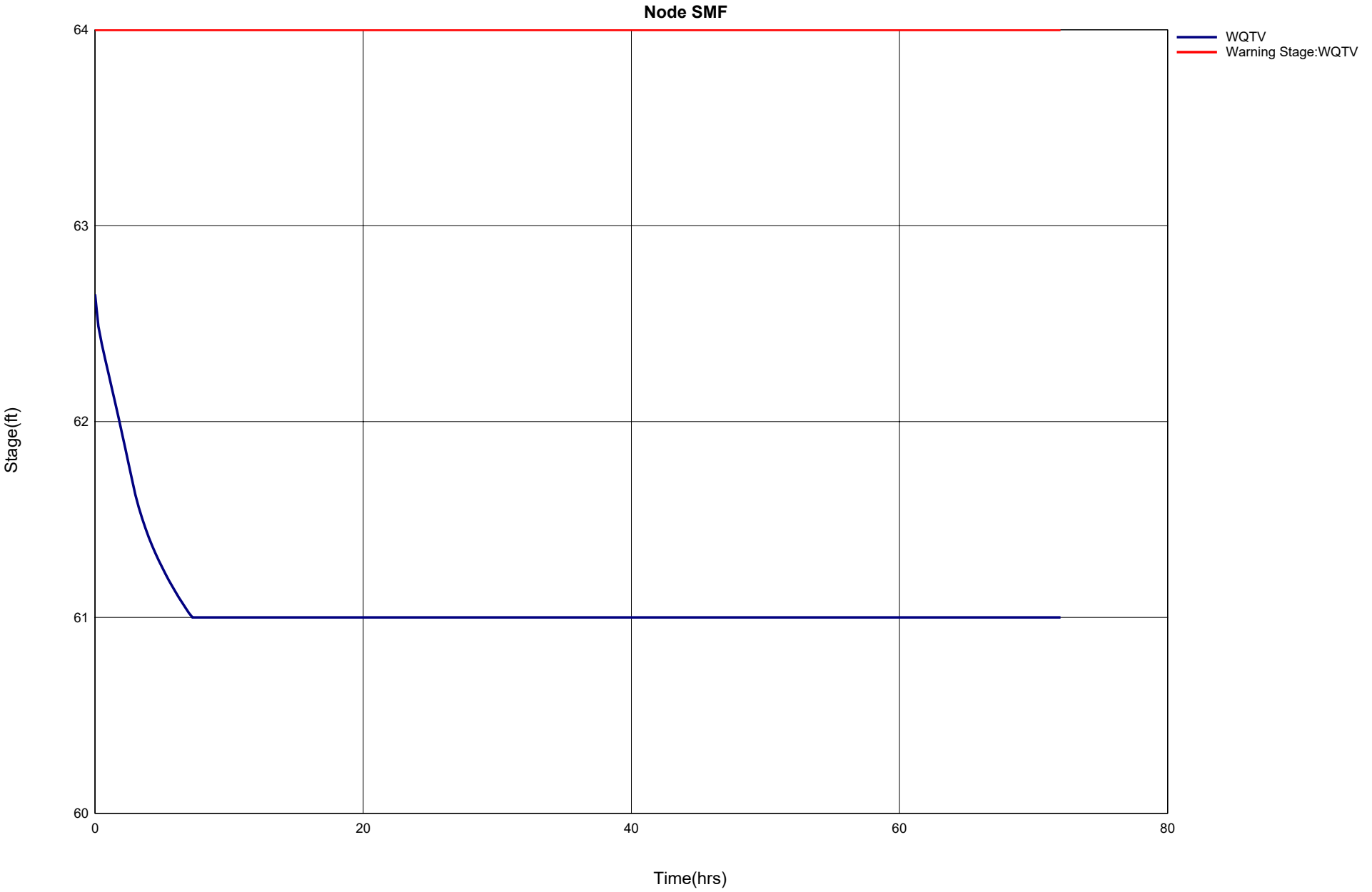
Interconnected Channel and Pond Routing Model (ICPR) ©2002 Streamline Technologies, Inc.



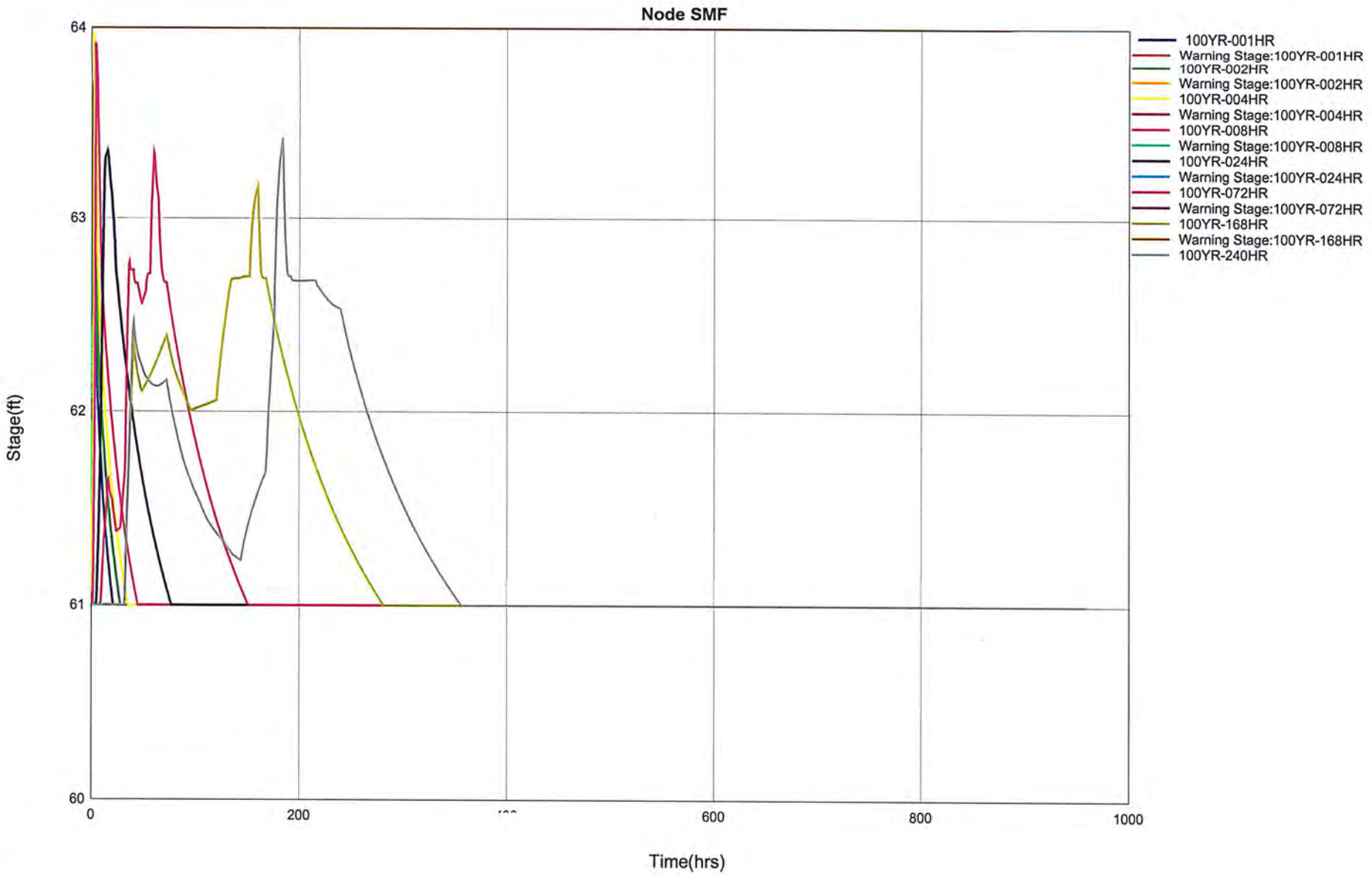
Interconnected Channel and Pond Routing Model (ICPR) ©2002 Streamline Technologies, Inc.

Attachment H

Recovery Analysis



WQTV RECOVERY



SMF RECOVERY

Attachment I

Pipe Calculations

PIPE SIZE CALCULATIONS

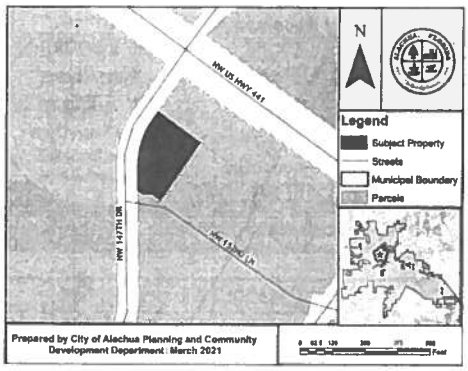
Alchem Laboratories: Building 4 Site Plan - Alachua, FL

Max Discharge CFS	n	Required Pipe Size				Proposed Pipe Size			
		Slope %	Pipe (in)	Q CFS	V ft/sec	Slope %	Pipe (in)	Q CFS	V ft/sec
0.36	0.012	0.30	8.00	0.72	2.06	0.96	12.00	3.79	4.83

City of ALACHUA

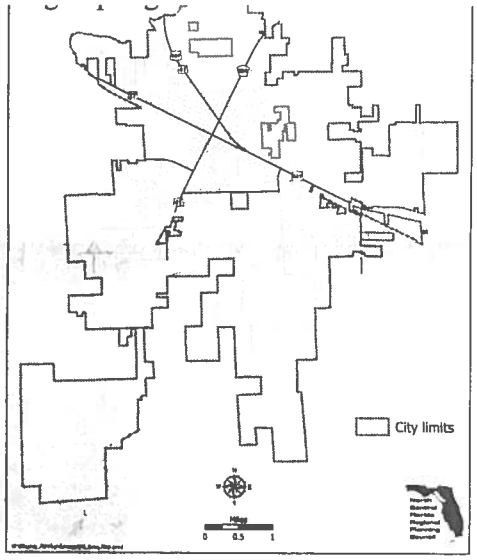
NOTICE OF PUBLIC HEARING BEFORE THE PLANNING AND ZONING BOARD OF THE CITY OF ALACHUA, FLORIDA

Notice is hereby given that the Planning & Zoning Board of the City of Alachua will hold a public hearing on April 20, 2021, at 6:00 p.m. The hearing will be held in the James A. Lewis Commission Chambers in City Hall, located at 15100 NW 142nd Terrace, Alachua, Florida, to consider the following: A request by Christopher Potts, P.E., of JBrown Professional Group, Inc., applicant and agent for Shakti Management, LLC, property owner, for consideration of a Site-Specific Amendment to the Official Zoning Atlas (Rezoning) to amend the Official Zoning Atlas from Central Business District (CBD) to Commercial Intensive (CI) on a ±1.09 acre subject property. The property subject to the proposed amendment is located south of US HWY 441, to the east of NW 147th Drive, and north of the Hitchcock's Plaza; Tax Parcel Number 03869-004-000; Existing Future Land Use Map (FLUM) Designation: Commercial. Existing Zoning: Central Business District (CBD).



At the public hearing, all interested parties may appear and be heard with respect to the application. Copies of the application are available for public inspection at the Planning and Community Development Department, 15100 NW 142nd Terrace, Alachua, Florida, on any regular business day between the hours of 7:30 a.m. to 6:00 p.m. Written comments on the application may be sent to the following address: City of Alachua, Planning and Community Development, P.O. Box 9, Alachua, FL 32616. Notice is given pursuant to Section 286.0105, Florida Statutes, that, in order to appeal any decision made at the public hearing, you will need a record of the proceedings, and that, for such purpose, you may need to ensure that a verbatim record of the proceedings is made, which includes the testimony and evidence upon which the appeal is to be based. In accordance with the Americans with Disabilities Act, any persons with a disability requiring reasonable accommodation in order to participate in this meeting should call the City Clerk at (386) 418-6100 x 1501 at least 48 hours prior to the public hearing.

(Published: Alachua County Today - April 08, 2021)



The first of two public hearings on the proposed amendment and the ordinance adopting said amendment will be held April 22, 2021 at 6:30 p.m., or as soon thereafter as the matter can be heard, in the City Commission Chambers, City Hall located at 23718 West U.S. Highway 27, High Springs, Florida. The title of said ordinance shall read, as follows:

ORDINANCE NO. LDC 21-02

AN ORDINANCE OF THE CITY OF HIGH SPRINGS, FLORIDA, AMENDING THE CITY OF HIGH SPRINGS LAND DEVELOPMENT CODE, APPENDIX B, LAND DEVELOPMENT CODE; ARTICLE XI ADMINISTRATION; PART 11.00.00 GENERALLY; TABLE 11.00.01 TYPE OF PROCEDURE BY DEVELOPMENT OPTION; PART 11.01.00 PROCEDURE FOR OBTAINING PERMITS; SECTION 11.01.07.01 DETERMINATION OF REQUIRED REVIEW; PART 11.02.00 PROCEDURES FOR OBTAINING DEVELOPMENT ORDERS; SECTION 11.02.02.01 LEVEL ONE, ACTION BY SITE PLAN REVIEW COMMITTEE; PART 11.04.00 SITE PLAN REVIEW; SECTION 11.04.04 PRELIMINARY SITE PLAN REQUIRED; SECTION 11.04.05 FINAL SITE PLAN REQUIRED; STRIKING PART 11.01.00 PROCEDURE FOR OBTAINING PERMITS; SECTION 11.01.07.03 PLATTED SUBDIVISION WITH NO IMPROVEMENT FACILTIIES, PROVIDING FOR SEVERABILTY; REPEALING ALL ORDINANCES IN CONFLICT AND PROVIDING AN EFFECTIVE DATE.

The public is advised to visit the website: https://www.highsprings.us for up-to-date information concerning the public hearing. If the public hearing is also held via communications media technology, all pertinent information will be made available via the City website https://www.highsprings.us or may be obtained by calling the Office of City Clerk at 386.454.1416.

The public hearing may be continued to one or more future dates. Any interested party shall be advised that the date, time and place of any continuation of the public hearing shall be announced during the public hearing and that no further notice concerning the matter will be published, unless said continuation exceeds six calendar weeks from the date of the above referenced public hearing.

At the aforementioned public hearing, all interested parties may appear to be heard with respect to the amendment.

Copies of the amendment and the ordinance adopting said amendment are available for public inspection at the Office of the City Clerk, 23718 West U.S. Highway 27, High Springs, Florida, during regular business hours.

All persons are advised that if they decide to appeal any decision made at the above referenced public hearing, they will need a record of the proceedings, and that for such purpose they may need to ensure that a verbatim record of the proceedings is made, which record includes the testimony and evidence upon which the appeal is to be based.

In accordance with the American with Disabilities Act, any person with disabilities needing any special accommodation to participate in City meetings should contact the Office of the City Clerk, 23718 West U.S. Highway 27, High Springs, Florida, 32643 or via telephone: 386.454.1416, ext. 7237, at least 48 hours prior to the date of the public hearing.

(Published: Alachua County Today - April 08, 2021)

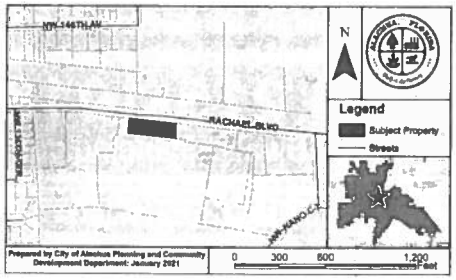
(Published: Alachua County Today - April 08, 2021)

Alachua County Today
is your best source for local news - 386.462.3355

City of ALACHUA

NOTICE OF PUBLIC HEARING BEFORE THE PLANNING AND ZONING BOARD OF THE CITY OF ALACHUA, FLORIDA

Notice is hereby given that the Planning and Zoning Board of the City of Alachua will hold a public hearing on April 20, 2021, at 6:00 p.m. The hearing will be held in the James A. Lewis Commission Chambers in City Hall, located at 15100 NW 142nd Terrace, Alachua, Florida, to consider the following: A request by Claudia Vega, P.E., of EDA Consultants, Inc., applicant and agent for Alchem Laboratories Corporation, property owner, for consideration of a Site Plan which proposes the construction of a ±3,788 square foot building and associated site improvements on a ±0.76 acre property, located at 13305 Rachael Boulevard; Tax Parcel Nos. 03231-003-000 and 03231-004-001; FLUM: Industrial; Zoning: Light & Warehouse Industrial (ILW).



At the public hearing, all interested parties may appear and be heard with respect to the application. Copies of the application are available for public inspection at the Planning and Community Development Department, 15100 NW 142nd Terrace, Alachua, Florida, on any regular business day between the hours of 7:30 a.m. to 6:00 p.m. Written comments on the application may be sent to the following address: City of Alachua, Planning and Community Development, P.O. Box 9, Alachua, FL 32616. Notice is given pursuant to Section 286.0105, Florida Statutes, that, in order to appeal any decision made at the public hearing, you will need a record of the proceedings, and that, for such purpose, you may need to ensure that a verbatim record of the proceedings is made, which includes the testimony and evidence upon which the appeal is to be based. In accordance with the Americans with Disabilities Act, any persons with a disability requiring reasonable accommodation in order to participate in this meeting should call the City Clerk at (386) 418-6100 x 101 at least 48 hours prior to the public hearing.

(Published: Alachua County Today - April 08, 2021)

AFFIDAVIT FOR POSTED LAND USE SIGN

I MELISSA WATSON, POSTED THE LAND USE
(name)
SIGN ON 4/1/2021 FOR THE Alchem Building 4 Site Plan
(date) (state type of action and project name)
LAND USE ACTION.

AS PER ARTICLE 2.2.9 D OF THE LAND DEVELOPMENT REGULATIONS.

THIS WILL BE INCLUDED IN THE STAFF REPORT.

Melissa Watson
(signature)

2
(number of signs)

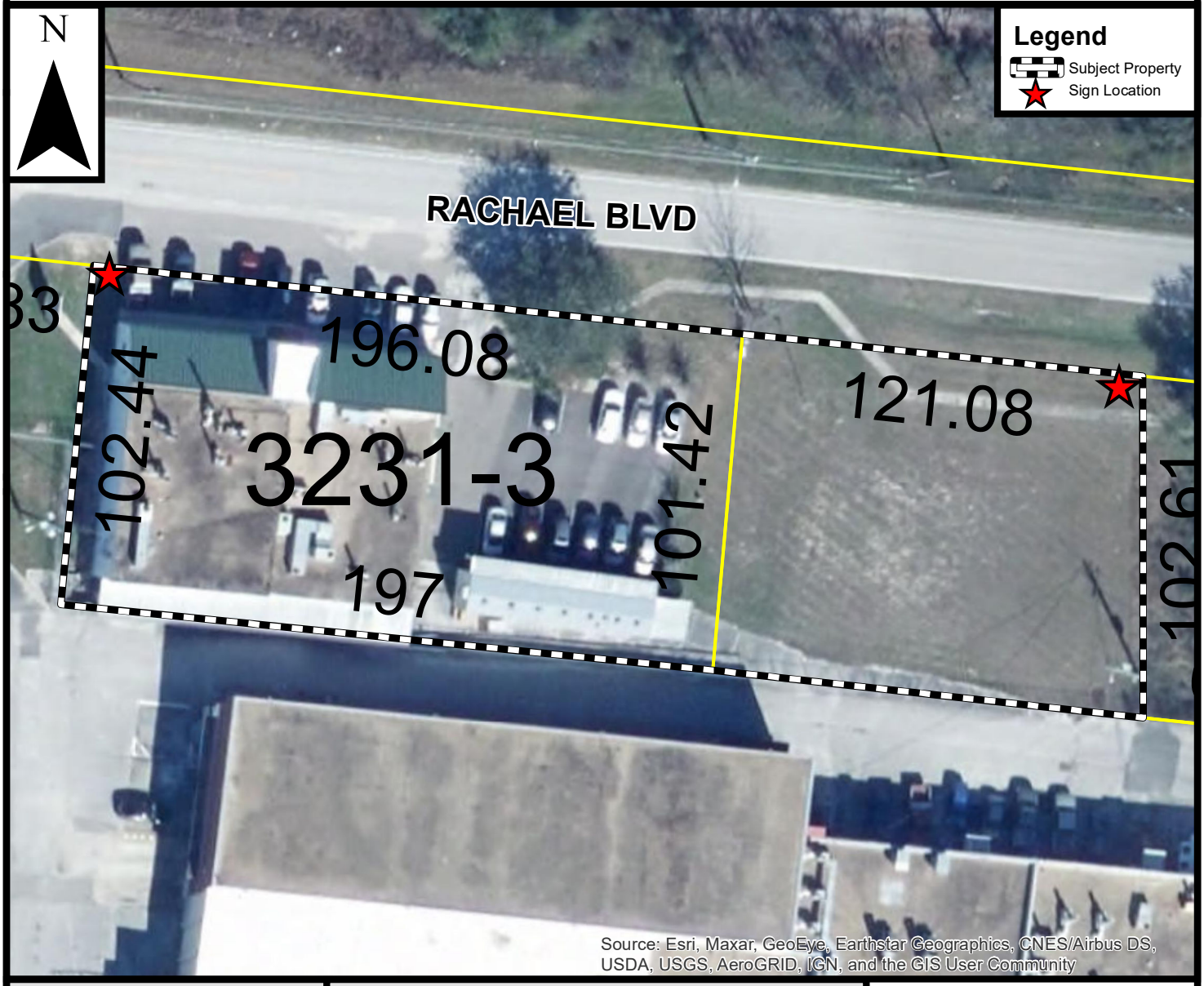
IMPORTANT INFORMATION REGARDING POSTED NOTICE SIGNS

Pursuant to Section 2.2.9(D) of the City's Land Development Regulations, posted notice signs must be placed on the land that is the subject of the application, along each street which is adjacent to or runs through the land in a manner that makes them clearly visible. Signs shall be posted at intervals of not more than 400 feet when the land subject to the application has less than 1,500 feet of road frontage. When the land subject to the application has 1,500 feet or more of road frontage, signs shall be posted at intervals of not more than 1,320 feet.

Signs shall be inspected by the City subsequent to their posting. The applicant shall be responsible for ensuring that the posted notice is maintained on the land subject to the application until the completion of the final public hearing on the application.

Signs must be removed by the applicant and returned to the City within ten days after the final decision on the application.

Alchem Building 4
Site Plan



Prepared by the City of Alachua
Planning & Community Development Department

Prepared February 2021

DISCLAIMER: Data is provided "as is" without warranty of any representation of accuracy, timeliness, or completeness. The burden of determining accuracy, timeliness, or completeness for use rests solely on the requestor. The City makes no warranties, expressed or implied, as to the use of the data. The requestor acknowledges and accepts the limitations of the data, including the fact that the data is dynamic and in a constant state of maintenance, correction, and update.

0 15 30 60 Feet

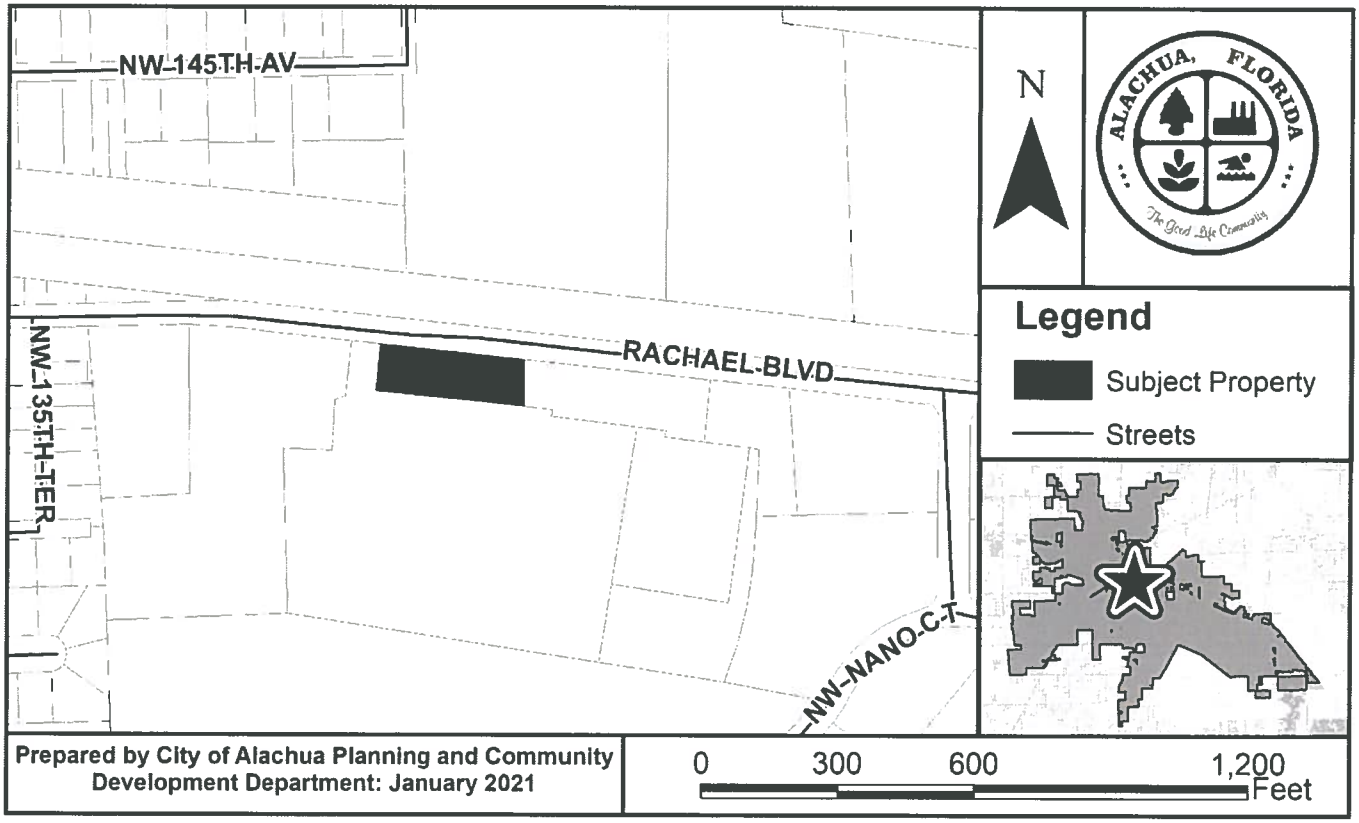
Mailed 4/5/21 KO

City of ALACHUA

THE GOOD LIFE COMMUNITY

NOTICE OF PUBLIC HEARING BEFORE THE PLANNING AND ZONING BOARD OF THE CITY OF ALACHUA, FLORIDA

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P.O. Box 9 ♦ Alachua, Florida 32616-0009
Phone: (386) 418-6121 ♦ Fax: (386) 418-6130

03231-002-000
BLUMBERG GLENN
11421 NW 120TH TER
ALACHUA, FL 32615

03934-001-020
ALLEN & ALLEN & ALLEN & FREENEY & JENKINS
12008 NW 148TH PL
ALACHUA, FL 32615

03231-004-000
DUAL ENTERPRISES LLC
132 NW 76TH DR
GAINESVILLE, FL 32607

03231-003-000
ALCHEM LABORATORIES CORP
13305 RACHEL BLVD
ALACHUA, FL 32615-6687

03230-001-031
WHITE JOANNA
13417 NW 145TH AVE
ALACHUA, FL 32615

03934-001-019
JENKINS & MCDANIEL-JENKINS H/W
13510 NW 141ST PL
ALACHUA, FL 32615

03934-001-001
TOUCHE NOLA L
13515 NW 142ND AVE
ALACHUA, FL 32615

03927-012-000
SNH MEDICAL OFFICE PROPERTIES TRUST
255 WASHINGTON ST STE 300
NEWTON, MA 02458

03230-001-034
JOHNSON KENNON KEITH
269 CROSSGATE DR
ELMORE, AL 36025-1039

03231-000-000
CONTEXT DEVELOPMENT CO
3020 CORAL WAY
MIAMI, FL 33145

03384-200-000
CSX TRANSPORTATION INC
500 WATER ST TAX DEPARTMENT J-910
JACKSONVILLE, FL 32202-4423

92060-517-900
CSX TRANSPORTATION INC
500 WATER ST TAX DEPARTMENT J-910
JACKSONVILLE, FL 32202-4423

03229-000-000
MARLOW ALACHUA HOLDINGS LLC
5212 SNEAD ISLAND RD
PALMETTO, FL 34221

03229-003-000
PHOENIX COMMERCIAL PARK
PO BOX 1000
ALACHUA, FL 32616

03231-005-001
ADC DEVELOPMENT & INVESTMENT
PO BOX 238
LAKE BUTLER, FL 32054

03230-002-000
ADC DEVELOPMENT & INVESTMENT GROUP LLC
PO BOX 238
LAKE BUTLER, FL 32054

03231-005-000
ADC DEVELOPMENT & INVESTMENT GROUP LLC
PO BOX 238
LAKE BUTLER, FL 32054

03927-000-000
ADC DEVELOPMENT & INVESTMENT GROUP LLC
PO BOX 238
LAKE BUTLER, FL 32054

03231-001-000
BARBER LUMBER SALES OF ALACHUA INC
PO BOX 263
ALACHUA, FL 32616-0263

03927-006-000
BARBER LUMBER SALES OF ALACHUA INC
PO BOX 263
ALACHUA, FL 32616-0263

03231-005-003
COPELAND INDUSTRIAL PARK OWNERS ASSOCIATION INC
PO BOX 365
LAKE BUTLER, FL 32054

03518-001-000
BRYANT FLORA M
PO BOX 556
ALACHUA, FL 32616-0556

Antoinette Endelicato
5562 NW 93rd Avenue
Gainesville, FL 32653

Dan Rhine
288 Turkey Creek
Alachua, FL 32615

Tom Gorman
9210 NW 59th Street
Alachua, FL 32653

Richard Gorman
5716 NW 93rd Avenue
Alachua, FL 32653

Peggy Arnold
410 Turkey Creek
Alachua, FL 32615

David Forest
23 Turkey Creek
Alachua, FL 32615

President of TCMOA
1000 Turkey Creek
Alachua, FL 32615

Linda Dixon, AICP
Assistant Director of Planning
PO Box 115050
Gainesville, FL 32611

Craig Parenteau
FL Dept. of Environmental Protection
4801 Camp Ranch Road
Gainesville, FL 32641

Jeannette Hinsdale
P.O. Box 1156
Alachua, FL 32616

Lynn Coullias
7406 NW 126th Ave
Alachua, FL 32615

Lynda Coon
7216 NW 126 Avenue
Alachua, FL 32615

Tamara Robbins
PO Box 2317
Alachua, FL 32616

Michele L. Lieberman
County Manager
12 SE 1st Street
Gainesville, FL 32601

Bonnie Flynn
16801 NW 166th Drive
Alachua, FL 32615

Hugh & Jean Calderwood
P.O. Box 2307
Alachua, FL 32616

Lisia Jenkins
PO Box 1071
Alachua, FL 32616

City of Alachua
THE GOOD LIFE COMMUNITY

Board/Committee Agenda Item

MEETING DATE: 4/20/2021
SUBJECT: Site-Specific Amendment to the Official Zoning Atlas: A request by Christopher Potts, P.E. of JBrown Professional Group, LLC, applicant and agent for Shakti Management, LLC, property owner, to amend the Official Zoning Atlas from Central Business District (CBD) to Commercial Intensive (CI) on a ±1.09 acre subject property. Tax Parcel Number 03869-004-000 (Quasi-Judicial Hearing).

PREPARED BY: Adam Hall, AICP, Planner

RECOMMENDED ACTION:

Staff recommends that the Planning & Zoning Board transmit the Site-Specific Amendment to the Official Zoning Atlas submitted by JBrown Professional Group, Inc. on behalf of Shakti Management, LLC to the City Commission with a recommendation to approve. upon making the following motion:

Based upon the competent substantial evidence presented at this hearing, the presentation before this Board, and Staff's recommendation, this Board finds the application for a Site-Specific Amendment to the Official Zoning Atlas submitted by JBrown Professional Group, Inc. on behalf of Shakti Management, LLC to be consistent with the City of Alachua Comprehensive Plan and in compliance with the Land Development Regulations and transmits the application to the City Commission, with a recommendation to approve.

Summary

The proposed Site Specific Amendment to the City of Alachua Official Zoning Atlas (Rezoning) is a request by Christopher Potts, P.E., of JBrown Professional Group, LLC, for the consideration of the rezoning of the subject property from Central Business District (CBD) to Commercial Intensive (CI).

The Future Land Use Map designation of the subject property is Commercial.

The subject property is located south of US HWY 441, east of NW 147th Drive, north of the Hitchcock's Shopping Plaza as an outparcel of that development. It is currently a cleared vacant lot.

The property is surrounded by commercial uses including Hitchcock's plaza and Rolling Oaks Plaza. See Map 5, Vicinity Map, and Table 2, Surrounding Land Uses, below.

The subject property has been zoned with a commercial zoning designation since at least the 1980s.

The general purposes of the Business zone districts are established and described in Section 3.5.1 of the City's Land Development Regulations (LDRs):

> *General purposes.* The business zone districts are established for the general purpose of ensuring there are lands in the City that provide a wide range of office, retail, service, light industrial and related uses to meet household and business needs, and more specifically:
> (A)*Provide appropriately located lands for business uses consistent with Comprehensive Plan.* Provide

appropriately located lands in areas served by water and sewer for the full range of business uses needed by Alachua's residents, businesses, and workers, consistent with the goals, objectives, and policies of the Comprehensive Plan;

(B)*Strengthen economic base.* Strengthen the City's economic base, and provide employment opportunities close to home for residents of the City and surrounding communities;

(C)*Provide suitable environment for business uses.* Create suitable environments for various types of business uses, and protect them from the adverse effects of incompatible uses; and

(D)*Minimize impact of business development on residential districts and uses.* Minimize the impact of business development on residential districts and uses.

The specific purpose of the Commercial Intensive (CI) zone district is established and described in Section 3.5.2 (E) of the City's Land Development Regulations (LDRs):

(E)CI, Commercial Intensive District. The CI District is established and intended to provide lands and facilitate highway-oriented development opportunities within the City, for uses that require high public visibility and an accessible location. The CI district should be located along major arterials or highways and at the US 441/Interstate-75 interchange.

ATTACHMENTS:

Description

- Draft Ordinance
- Staff Report and Staff Supporting Materials
- Application and Supporting Materials
- Public Notice Materials for 04/20/21 Planning and Zoning Board Hearing

ORDINANCE 21-XX

AN ORDINANCE OF THE CITY OF ALACHUA, FLORIDA, RELATING TO THE AMENDMENT OF THE OFFICIAL ZONING ATLAS; AMENDING THE OFFICIAL ZONING ATLAS FROM CENTRAL BUSINESS DISTRICT (CBD) TO COMMERCIAL INTENSIVE (CI) ON A ±1.09 ACRE PROPERTY; LOCATED SOUTH OF US HWY 441, TO THE EAST OF NW 147TH DRIVE, AND NORTH OF THE HITCHCOCK'S PLAZA; CONSISTING OF TAX PARCEL NUMBER 03869-004-000; REPEALING ALL ORDINANCES IN CONFLICT; PROVIDING SEVERABILITY; AND PROVIDING AN EFFECTIVE DATE.

R E C I T A L S

WHEREAS, an application for a Site-Specific Amendment to the Official Zoning Atlas ("Amendment"), as described below, has been filed with the City; and

WHEREAS, on April 8, 2021, the City advertised a public hearing to be held before the Planning and Zoning Board, sitting as the Local Planning Agency ("LPA"); and

WHEREAS, on April 20, 2021 the LPA conducted a quasi-judicial public hearing on the proposed Amendment and the LPA reviewed and considered all comments received during the public hearing concerning the proposed Amendment and made its recommendation to the City Commission; and

WHEREAS, on _____, 2021, and on _____, 2021, the City advertised public hearings to be held before the City Commission; and

WHEREAS, on _____, 2021, and on _____, 2021, the City Commission conducted quasi-judicial public hearings on the proposed Amendment and provided for and received public participation at both public hearings; and

WHEREAS, the City Commission has determined and found the proposed Amendment to be consistent with the City's Comprehensive Plan and the City's Land Development Regulations ("LDRs"); and

WHEREAS, for reasons set forth in this ordinance that is hereby adopted and incorporated as findings of fact, the Alachua City Commission finds and declares that the enactment of this Amendment is in the furtherance of the public health, safety, morals, order, comfort, convenience, appearance, prosperity, or general welfare.

NOW THEREFORE BE IT ORDAINED BY THE CITY COMMISSION OF THE CITY OF ALACHUA, FLORIDA:

Section 1. Findings of Fact and Conclusions of Law

1. The above recitals are true and correct and are incorporated herein by reference.
2. The proposed Amendment is in compliance with the City's LDRs.
3. The proposed Amendment will not cause a reduction in the adopted level of service standards for transportation, water, sewer, waste, stormwater, recreation, and public schools.

Section 2. Official Zoning Atlas Amended

The Official Zoning Atlas is hereby amended from Central Business District (CBD) to Commercial Intensive (CI) for Tax Parcel Number 03869-004-000, in accordance with the legal description found in Exhibit "A" and map found in Exhibit "B" attached hereto and incorporated herein.

Section 3. Ordinance to be Construed Liberally

This ordinance shall be liberally construed in order to effectively carry out the purposes hereof which are deemed to be in the best interest of the public health, safety, and welfare of the citizens and residents of Alachua, Florida.

Section 4. Repealing Clause

All ordinances or parts of ordinances in conflict herewith are, to the extent of the conflict, hereby repealed.

Section 5. Severability

It is the declared intent of the City Commission of the City of Alachua that, if any section, sentence, clause, phrase, or provision of this ordinance is for any reason held or declared to be unconstitutional, void, or inoperative by any court or agency of competent jurisdiction, such holding of invalidity or unconstitutionality shall not affect the remaining provisions of this ordinance, and the remainder of the ordinance after the exclusions of such part or parts shall be deemed to be valid.

Section 6. Effective Date

This ordinance shall take effect immediately upon its adoption by the City Commission and the signature of the Mayor.

PASSED on First Reading the _____ day of _____ 2021.

PASSED and **ADOPTED**, in regular session, with a quorum present and voting, by the City Commission, upon second and final reading this _____ day of _____ 2021.

CITY COMMISSION OF THE
CITY OF ALACHUA, FLORIDA

_____
Gib Coerper, Mayor
SEAL

ATTEST:

_____
Adam Boukari, City Manager/Clerk

APPROVED AS TO FORM

_____
Marian B. Rush, City Attorney

EXHIBIT "A"

LEGAL DESCRIPTION:

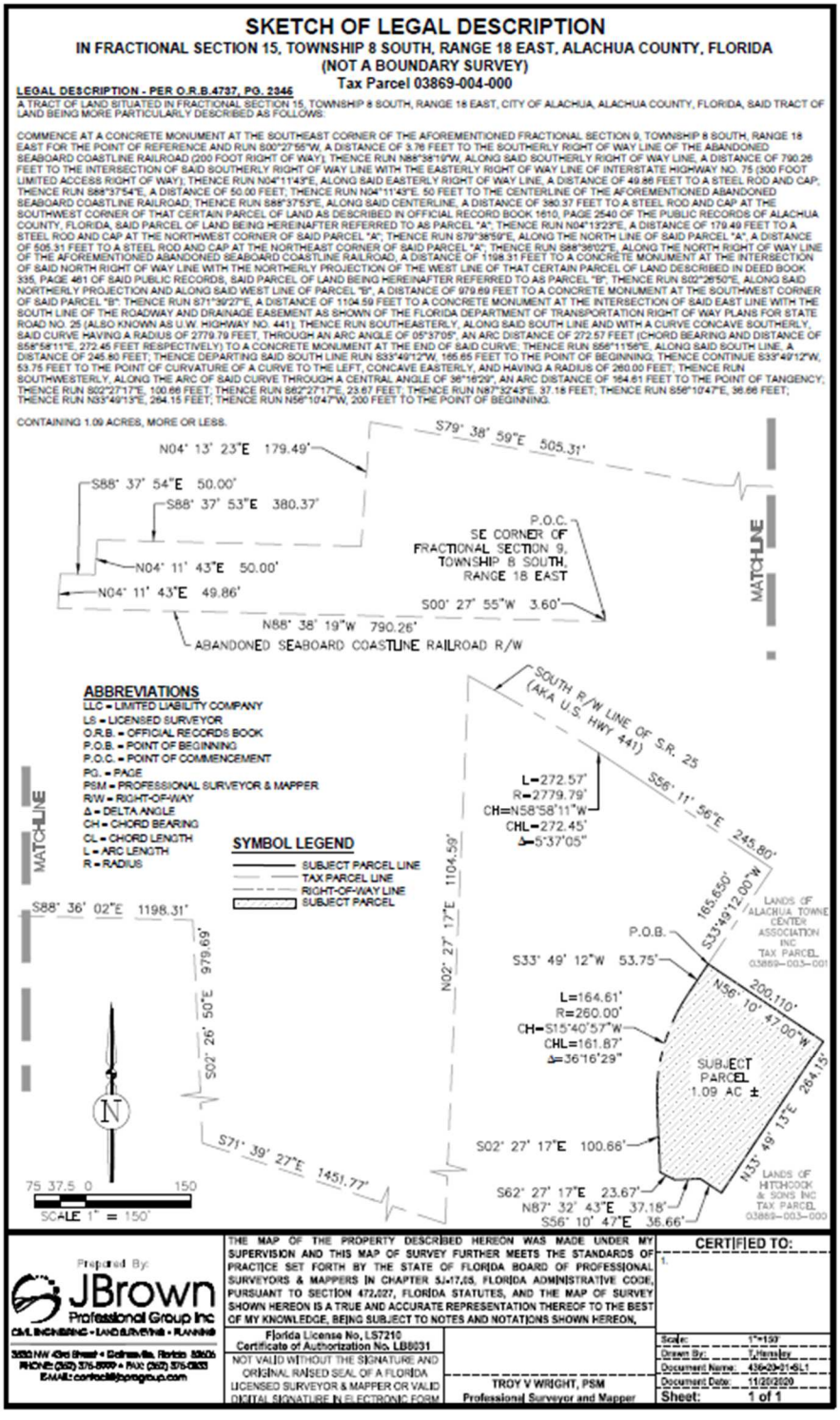
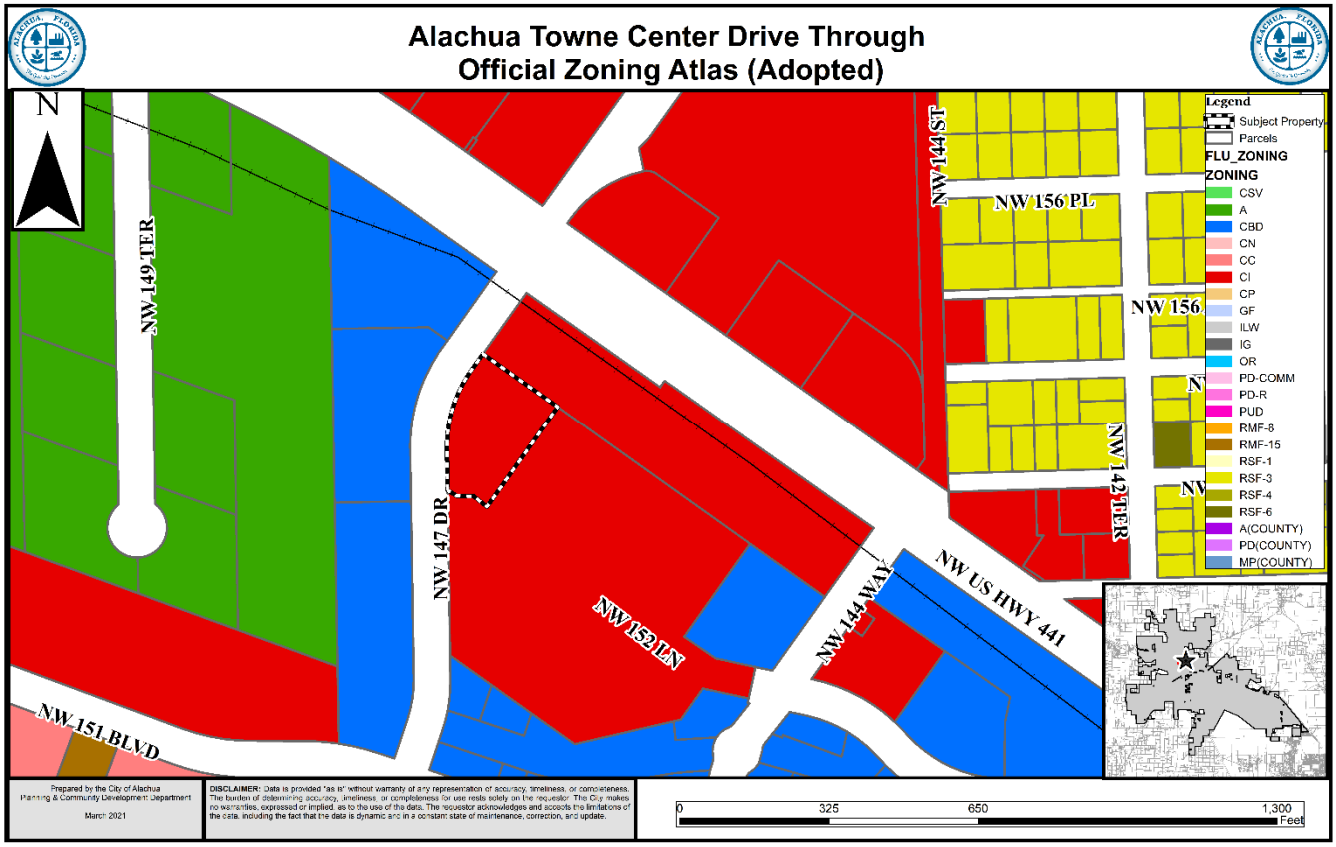


EXHIBIT "B"


Alachua Towne Center Drive Through
Official Zoning Atlas (Adopted)
N
NW 149 TER
NW 147 DR
NW 152 LN
NW 151 BLVD
NW 144 WAY
NW US HWY 441
NW 142 TER
NW 144 ST
NW 156 PL
NW 156
Legend
Subject Property
Parcels
FLU_ZONING
ZONING
CSV
A
CBD
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PD-COMM
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RMF-8
RMF-15
RSF-1
RSF-3
RSF-4
RSF-6
A(COUNTY)
PD(COUNTY)
MP(COUNTY)
Prepared by the City of Alachua
Planning & Community Development Department
March 2021
DISCLAIMER: Data is provided "as is" without warranty of any representation of accuracy, timeliness, or completeness. The burden of determining accuracy, timeliness, or completeness for use rests solely on the requestor. The City makes no warranties, expressed or implied, as to the use of the data. The requestor acknowledges and accepts the limitations of the data, including the fact that the data is dynamic and in a constant state of maintenance, correction, and update.
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City of Alachua
Planning & Community Development Department
Staff Report

Planning & Zoning Board Hearing Date: April 20, 2021
Quasi-Judicial Hearing

SUBJECT:	A request to amend the Official Zoning Atlas from Central Business District (CBD) to Commercial Intensive (CI)
APPLICANT/AGENT:	Christopher Potts, P.E. JBrown Professional Group, Inc.
PROPERTY OWNER:	Shakti Management, LLC
LOCATION:	South of US HWY 441, east of NW 147th Drive, north of the Hitchcock's Shopping Plaza
PARCEL ID NUMBER:	03869-004-000
ACREAGE:	±1.09 acres
PROJECT PLANNER:	Adam Hall, AICP
RECOMMENDATION:	Staff recommends that the Planning & Zoning Board transmit the Site-Specific Amendment to the Official Zoning Atlas submitted by JBrown Professional Group, Inc. on behalf of Shakti Management, LLC to the City Commission with a recommendation to approve.
RECOMMENDED MOTION:	*Based upon the competent substantial evidence presented at this hearing, the presentation before this Board, and Staff's recommendation, this Board finds the application for a Site-Specific Amendment to the Official Zoning Atlas submitted by JBrown Professional Group, Inc. on behalf of Shakti Management, LLC to be consistent with the City of Alachua Comprehensive Plan and in compliance with the Land Development Regulations and transmits the application to the City Commission, with a recommendation to approve.*

SUMMARY

The proposed Site Specific Amendment to the City of Alachua Official Zoning Atlas (Rezoning) is a request by Christopher Potts, P.E., of JBrown Professional Group, LLC, for the consideration of the rezoning of the subject property from Central Business District (CBD) to Commercial Intensive (CI).

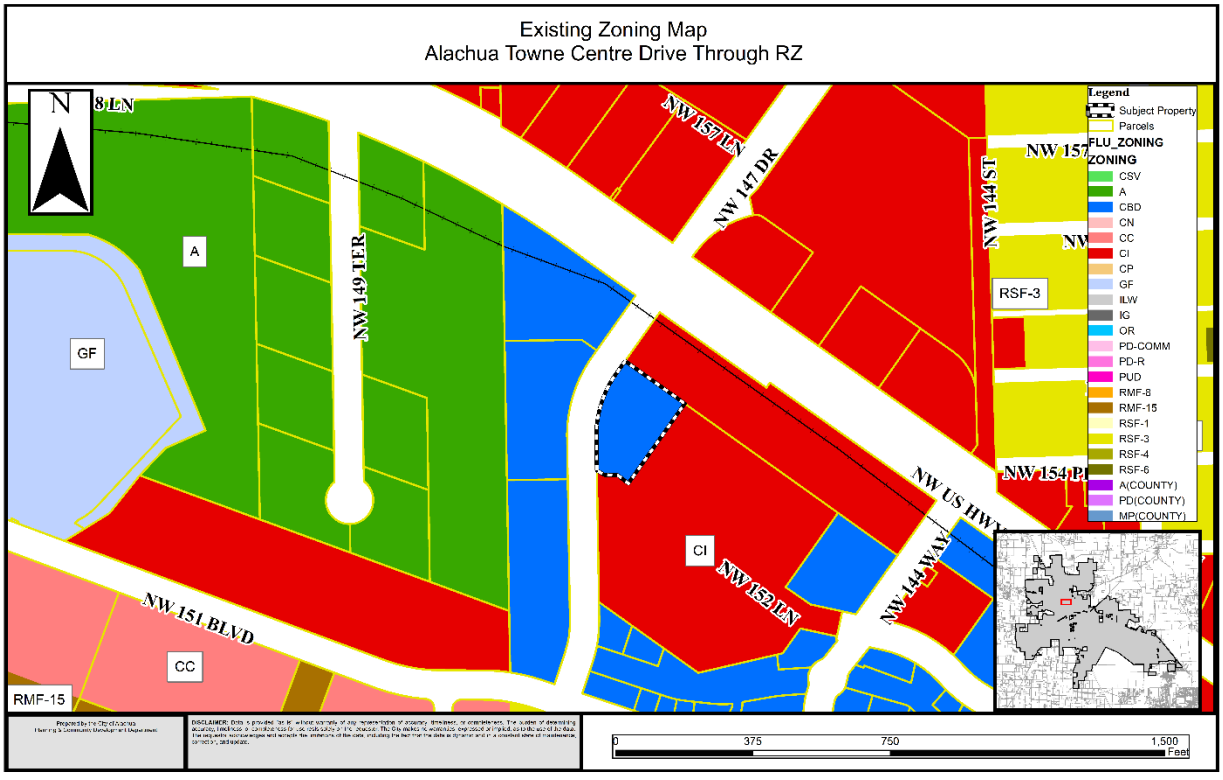
The Future Land Use Map designation of the subject property is Commercial.

The subject property is located south of US HWY 441, east of NW 147th Drive, north of the Hitchcock's Shopping Plaza as an outparcel of that development. It is currently a cleared vacant lot.

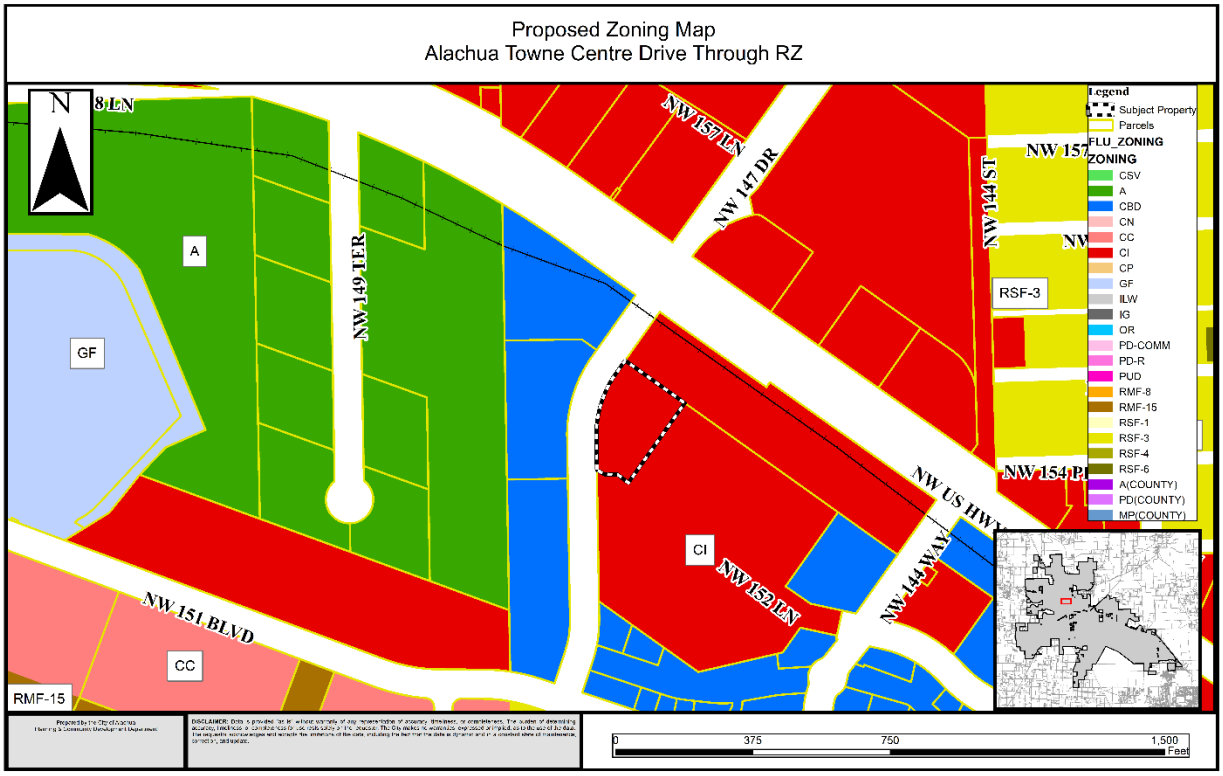
The property is surrounded by commercial uses including Hitchcock's plaza and Rolling Oaks Plaza. See Map 5, Vicinity Map, and Table 2, Surrounding Land Uses, below.

The subject property has been zoned with a commercial zoning designation since at least the 1980s.

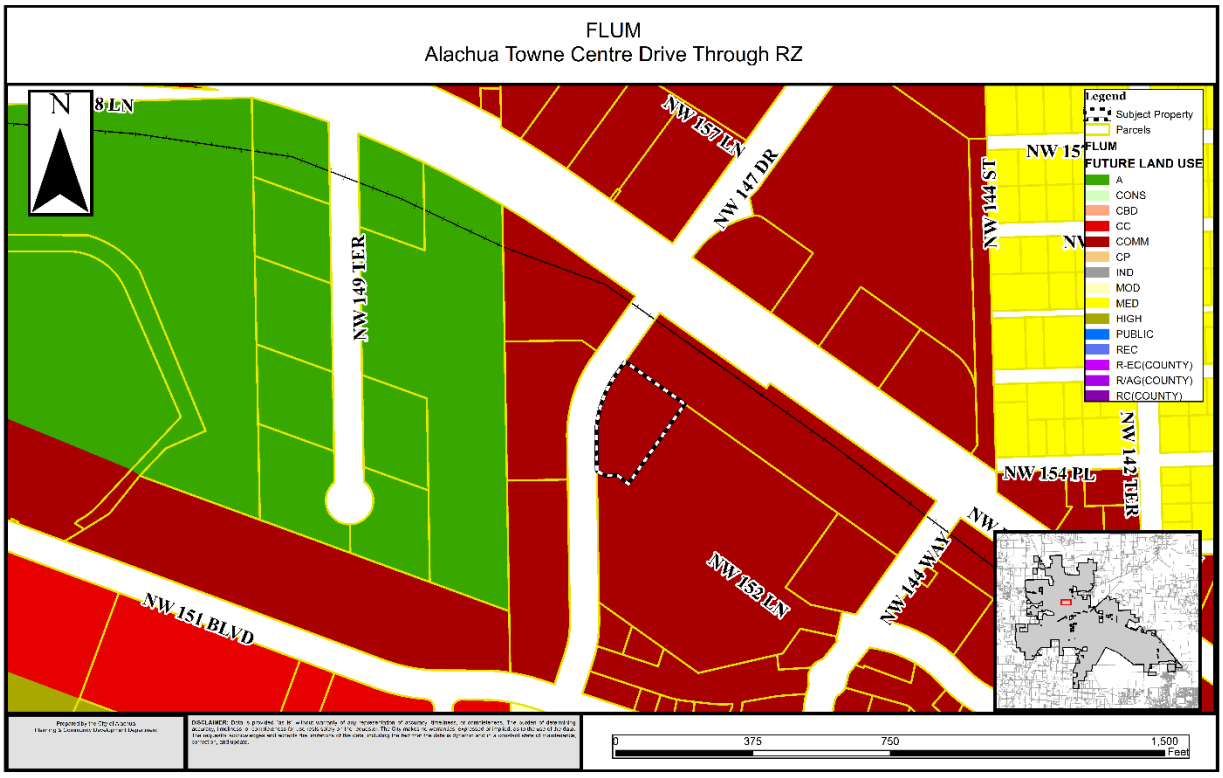
Map 1. Existing Zoning Map with Subject Property



Map 2. Proposed Zoning with Subject Property



Map 3. Existing Future Land Use Map with Subject Property



The general purposes of the Business zone districts are established and described in Section 3.5.1 of the City's Land Development Regulations (LDRs):

> *General purposes.* The business zone districts are established for the general purpose of ensuring there are lands in the City that provide a wide range of office, retail, service, light industrial and related uses to meet household and business needs, and more specifically:
> (A)*Provide appropriately located lands for business uses consistent with Comprehensive Plan.* Provide appropriately located lands in areas served by water and sewer for the full range of business uses needed by Alachua's residents, businesses, and workers, consistent with the goals, objectives, and policies of the Comprehensive Plan;
> (B)*Strengthen economic base.* Strengthen the City's economic base, and provide employment opportunities close to home for residents of the City and surrounding communities;
> (C)*Provide suitable environment for business uses.* Create suitable environments for various types of business uses, and protect them from the adverse effects of incompatible uses; and
> (D)*Minimize impact of business development on residential districts and uses.* Minimize the impact of business development on residential districts and uses.

The specific purpose of the Commercial Intensive (CI) zone district is established and described in Section 3.5.2 (E) of the City's Land Development Regulations (LDRs):

> (E)CI, Commercial Intensive District. The CI District is established and intended to provide lands and facilitate highway-oriented development opportunities within the City, for uses that require high public visibility and an accessible location. The CI district should be located along major arterials or highways and at the US 441/Interstate-75 interchange.

EXISTING USES

The existing site is currently a cleared vacant lot.

EXISTING/PROPOSED ZONING DISTRICT COMPARISON

The matrix below provides an analysis of the maximum gross density, floor area ratio, and typical uses permitted within the existing and proposed zoning districts:

	Existing Zoning District	Proposed Zoning District
Zoning District:	Central Business District (CBD)	Commercial Intensive (CI)
Max. Gross Intensity	0.75 floor to area ratio 35,610 square feet	0.75 floor to area ratio 35,610 square feet
Permitted Uses:	Single family attached and detached, residential offices, business and professional services, financial institution, eating establishments, retail sales and services that serve community, hotels, motels	Retail sales and services, personal services, financial institutions, tourist-related uses, hotels, motels, commercial shopping centers, auto-oriented uses, office/business parks, eating establishments, including drive through/drive in uses

SURROUNDING USES

The subject property is located in a primarily commercial area. Santa Fe Station and Baywood are the closest residential uses.

The existing uses, Future Land Use Map ("FLUM") Designations, and zone districts of the surrounding area are identified in Table 1. Map 5 provides an overview of the vicinity of the subject property.

NOTE: The information below is intended to provide a general overview of the area surrounding the subject property and to generally orient the reader. It is not intended to be all-inclusive, and may not identify all existing uses, FLUM Designations, and/or zoning districts surrounding the subject property.

Map 4. Vicinity Map

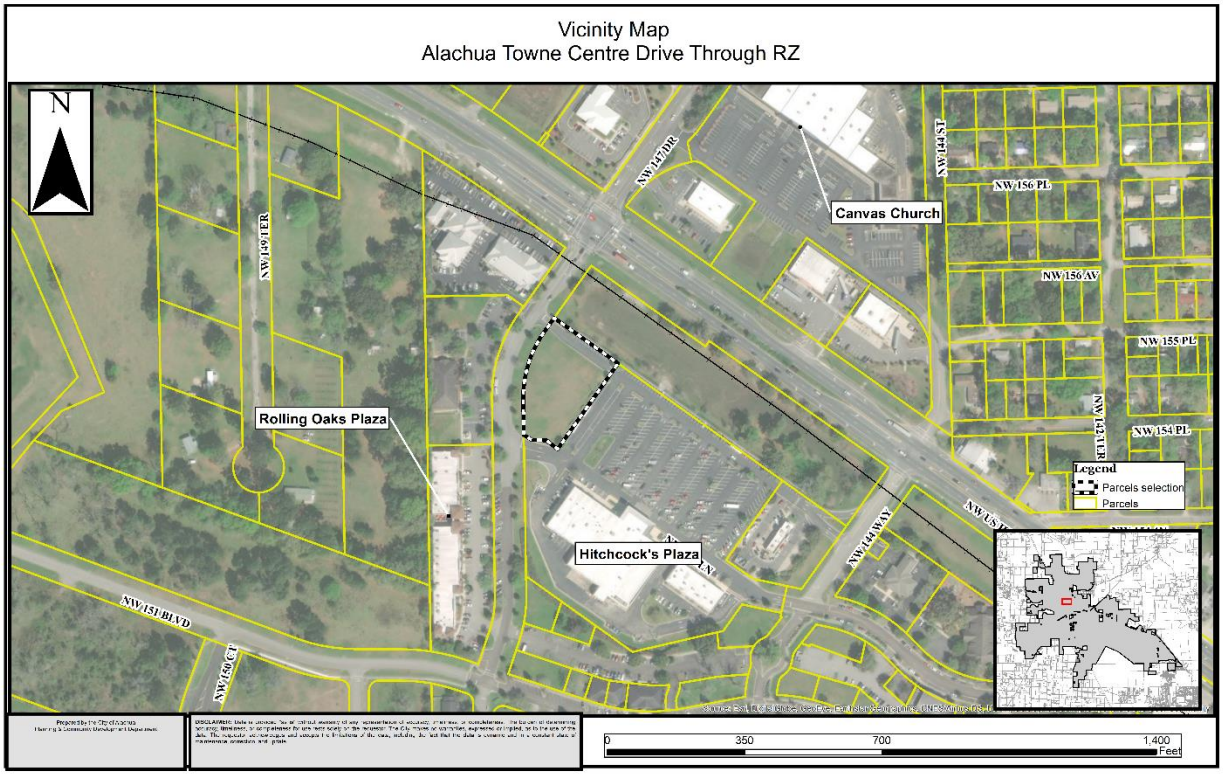


Table 2. Surrounding Land Uses

Direction	Existing Use(s)	FLUM Designation(s)	Zoning District(s)
North	Commercial (stormwater DRA for Alachua Towne Center	Commercial	Commercial Intensive (CI)
South	Commercial (Hitchcock's Plaza)	Commercial	Commercial Intensive (CI)
East	Commercial (Hitchcock's Plaza)	Commercial	Commercial Intensive (CI)
West	Commercial (Rolling Oaks Plaza)	Commercial	Central Business District (CBD)

NEIGHBORHOOD MEETING

The purpose of a Neighborhood Meeting is to educate the owners of nearby land and any other interested members of the public about the project and to receive comments regarding the project. As required by Section 2.2.4 of the LDRs, all property owners within 400 feet of the subject property and those persons / organizations registered with the City were notified of the meeting. Notice of the meeting was also published in a newspaper of general circulation.

The Neighborhood Meeting was held virtually on November 12, 2020 via Zoom. The applicant was present and available to answer questions. As evidenced by materials submitted by the applicant, no members of the general public attended the meeting.

CONSISTENCY WITH THE COMPREHENSIVE PLAN

The applicant proposes to amend the zoning designation from Central Business District (CBD) to Commercial Intensive (CI). Table 3 shows the existing FLUM Designation and the proposed corresponding zoning designation. The proposed zoning designation is consistent with the existing FLUM Designation for the properties subject to this application.

Table 3. Subject Property & Consistency with FLUM Designation

Parcel No.	Acreage	Existing FLUM	Proposed Zoning Designation	Consistent
03869-004-000	±1.09	Commercial	Commercial Intensive (CI)	☑

The Goals, Objectives, and Policies (GOPs) identified below are provided to establish a basis of the application's consistency with the Comprehensive Plan. There may be additional GOPs which the application is consistent with that are not identified within this report. An evaluation and findings of consistency with the identified GOPs is also provided below.

Future Land Use Element

GOAL 1: Future Land Use Map 2025:

The City of Alachua shall maintain a Future Land Use Map in order to effectively guide development in a sustainable manner and to ensure economic prosperity and stability while maintaining a high quality of life for all of its present and future citizens.

Objective 1.3: *Commercial*

The City shall establish three commercial land use categories: Community Commercial, Commercial, and Central Business District. These land use categories shall provide a broad range of commercial uses, including retail sales and services, personal services, offices, and tourist-oriented uses, in order to provide for the availability of goods and services, both to the citizens and visitors of Alachua and to the citizens of the North Central Florida region.

Policy 1.3.b: *Commercial*: The Commercial land use category is established to provide for general commercial uses, as well as more intense commercial and highway commercial uses. This is the land use category in which large-scale, regional commercial uses may locate. The following uses are allowed within the Commercial land use category:

1. Retail sales and services;
2. Personal services;
3. Financial institutions;
4. Outdoor recreation and entertainment;
5. Tourist-related uses;
6. Hotels and motels;
7. Commercial shopping centers;

8. Auto-oriented uses;
9. Traditional Neighborhood Planned Developments;
10. Employment Center Planned Developments;
11. Commercial recreation centers;
12. Office/business parks;
13. Limited industrial services; and,
14. Eating establishments.

Policy 1.3.d: *Design and performance standards*: The following criteria shall apply when evaluating commercial development proposals:

1. Integration of vehicular and non-vehicular access into the site and access management features of site in terms of driveway cuts and cross access between adjacent sites, including use of frontage roads and/or shared access;
2. Buffering from adjacent existing/potential uses;
3. Open space provisions and balance of proportion between gross floor area and site size;
4. Adequacy of pervious surface area in terms of drainage requirements;
5. Placement of signage;
6. Adequacy of site lighting and potential impacts of lighting upon the surrounding area. Lighting should be designed to minimize impacts to preserve the ambiance and quality of the nighttime sky by reducing light trespass and light pollution on adjacent properties by utilizing lighting at appropriate intensities, directions and times to ensure light is not overused or impacting areas where it is not intended;
7. Safety of on-site circulation patterns (patron, employee, and delivery vehicles), including parking layout, drive aisles, and points of conflict;
8. Landscaping, as it relates to the requirements and goals of the Comprehensive Plan and Land Development Regulations;
9. Unique features and resources which may constrain site development such as soils, existing vegetation and historic significance;
10. Performance based zoning requirements, which may serve as a substitute for or accompany land development regulations in attaining acceptable site design; and,
11. Commercial uses shall be limited to an intensity of less than or equal to 0.50 floor area ratio for parcels 5 acres or greater, 0.75 floor area ratio for parcels less than 5 acres but greater than 1 acre, and 1.0 floor area ratio for parcels 1 acre or less.

Policy 1.3.e: The creation/promotion of strip pattern commercial development shall be discouraged. Infill within established commercial areas is preferred over extension of a strip commercial pattern. Extension of a commercial land use designation may be considered in circumstances where the extension is compatible with the existing land uses surrounding the land or the land use categories of surrounding lands. Extensions of a commercial land use category shall not encroach into a residential area. The following criteria shall be

applied to limit the impacts of commercial development upon surrounding areas:

1. Impacts upon traffic circulation should be anticipated and mitigated through the reservation of right-of-way for road widening and marginal access streets when appropriate.
2. Access points for commercial development shall minimize points of conflict by utilizing frontage roads, providing cross access between parcels, or installing shared use ingress and egress access driveways connections to roadways.
3. Setbacks and landscaped or other appropriate buffers shall be established to mitigate the visual impacts of commercial development upon surrounding land uses.
4. A sidewalk or bicycle path shall be required where appropriate, to provide convenient access to the development from surrounding areas and to reduce traffic volumes on the roadways.

Policy 1.3.f: The City shall pursue the establishment of activity centers and similar criteria to guide the placement and design of commercial and business areas.

1. The City shall establish and maintain a Gateway Activity Center at the interchange of US 441 and Interstate 75. The boundaries of the Gateway Activity Center shall be based on the Generalized US 441/I-75 Activity Center Map provided in the Future Land Use Map Series. The intent of this activity center is to welcome existing and future residents and visitors to the City, and to promote Alachua as an attractive, vibrant, and economically prosperous community.
2. The City shall establish and maintain the US 441 corridor as a Corporate Corridor Activity Center. The intent of establishing this activity center is to implement economic development objectives and to promote a coordinated development plan to maximize existing and future land use patterns and preserve the function of the US 441 corridor.

Analysis of Consistency with Objective 1.3 and Policies 1.3.b, 1.3.d, 1.3.e, and 1.3.f: The proposed amendment would apply the Commercial Intensive (CI) zoning designation to the subject property. The existing Future Land Use Map designation is Commercial. The proposed zoning designation is compatible with this Future Land Use Map designation. At time of a development application, development review will ensure compliance with the design standards found in Policy 1.3.d.

Objective 2.4: *Landscaping and Tree Protection Standards*

The City shall adopt landscaping and tree protection standards in order to achieve the aesthetic design values of the community and preserve tree canopies, as well as to protect exceptional specimens and champion trees.

Policy 2.4.a: *Landscaping*: *General* – The City shall require landscaping plans to be submitted with each nonresidential and multiple family residential site plan.

The minimum landscaped area shall be 10% of the development site, not inclusive of any designated open space areas. Landscaping designs shall incorporate principles of xeriscaping, where feasible. The City shall develop a list of preferred planting materials to assist in the landscape design. Landscape plans shall include perimeter and internal site landscaping.

Policy 2.4.b: *Landscaping*: *Buffering* – A buffer consists of horizontal space (land) and vertical elements (plants, berms, fences, walls) that physically separate and visually screen adjacent land uses. The City shall establish buffer yard requirements that are based on the compatibility of the adjacent uses and the desired result of the buffer.

Policy 2.4.c: *Tree Protection: Heritage Trees and Champion Trees* – The City shall require the preservation of heritage trees and champion trees when possible. Standards shall be set for determining the health and safety risks associated with heritage and champion trees both on individual residential lots, and existing and proposed developments.

Policy 2.4.d: *Tree Protection: Regulated Trees* – The City shall establish standards for the preservation of regulated trees. Particular attention shall be given to preserving specimen and preferred species of regulated trees, where feasible.

Policy 2.4.e *Tree Protection: Removal and Mitigation* – Along with establishing standards for tree removal and mitigation, the City shall establish a tree banking program to provide flexibility for re-planting trees through the mitigation process. Funds within the tree bank may be utilized to plant landscaping on city-owned properties, in public parks, and in road rights of way, where appropriate.

Objective 2.5: *Open Space Standards*

The City shall utilize open space requirements to preserve the rural character of Alachua, protect natural resources, and provide spaces for people to recreate and gather.

Policy 2.5.a: There shall be a minimum of 10% percent open space required. The City shall establish incentives for the provision of open space beyond minimum requirements.

Policy 2.5.b: Open space shall not be limited to unusable portions of project sites. A portion of open space shall be usable and functional.

Analysis of Consistency with Objectives 2.4 and 2.5: At time of development application, development review will ensure compliance with these landscaping and open space requirements.

Goal 4: *Infill and Redevelopment Standards*

The City shall encourage new development and redevelopment to occur within developed areas of the City, such as the Community Redevelopment Area and the Central City Area, to utilize vacant and abandoned properties, prevent blight, and make the best use of available resources.

Objective 4.1: *Infill Development*

Infill development shall be encouraged in order to protect the unique character of existing neighborhoods and commercial developments, provide for a safe urban environment, increase densities and intensities in a manner compatible with existing uses, provide open spaces, and restore or maintain economic vitality and cultural diversity.

Policy 4.1.a: Infill within existing neighborhoods may occur at a higher density. Design standards will be adopted in the Land Development Regulations to protect the traditional character of existing neighborhoods and development. Design standards may include, but are not limited to, utilizing similar architectural design features, enhanced landscaping, and placement of the building on the lot in a manner consistent with surrounding buildings.

Policy 4.1.b: The City shall establish flexible standards related to landscaping, buffering, stormwater (to the extent allowable), and parking requirements to encourage infill development.

Policy 4.1.c: Compatible infill development shall be encouraged within the Central City Area.

Analysis of Consistency with Goal 4, Objective 4.1: The proposed amendment would encourage infill development, which is supported by this Goal, Objective, and Policies.

Objective 5.1: Natural features: The City shall coordinate Future Land Use designations with appropriate topography, soils, areas of seasonal flooding, wetlands and habitat during review of proposed amendments to the Future Land Use Map and the development review process. Natural features may be included as amenities within a development project.

Policy 5.1.a: Topography: The City shall protect the natural topography of the City, including steep and seepage slopes, by requiring new development to include techniques to minimize negative impacts on the natural terrain. An emphasis will be placed on retaining the natural function of seepage slopes during development. Additionally, retention of existing native vegetation will be encouraged as one method of protecting slopes.

Policy 5.1.b: Soils: The City shall ensure soil protection and intervention measures are included in the development review process.

Policy 5.1.c: Flood prone areas: The City shall require as part of the development review process the identification of FEMA flood zone areas. Where necessary, base flood elevations and minimum finished floor elevations shall be established. The City shall also require finished floor elevations on subdivision plats, site plans and building permit plans when necessary to determine compliance with flood prone area regulations. The City shall establish standards for a limitation on filling in flood prone areas.

Policy 5.1.d: Wetlands: The City shall utilize statewide wetland delineation methodology in accordance with Florida Administrative Code (FAC) and regulations adopted by the FDEP and the Suwannee River Water Management District.

Policy 5.1.e: Habitat: The City shall require as part of the development review process, an inventory of listed species for all new developments in areas identified as known habitat for listed species if listed species are known to exist in close proximity to the development. The survey shall include detailed information regarding type, quantity, location, and habitat requirements for any listed species identified. A de minimus threshold for properties required to complete the inventory shall be established in the City's Land Development Regulations.

Analysis of Consistency with Objective 5.1 and Policies 5.1.a – e: The subject property is not known to contain any environmental features which would significantly affect future development.

Objective 5.2: Availability of facilities and services: All new development shall be planned and constructed concurrently with the availability of facilities and services necessary for the development.

Policy 5.2.a: All new development shall meet level of service requirements for roadways, potable water and sanitary sewer, stormwater, solid waste, public schools, and improved recreation in accordance with LOS standards adopted in the elements addressing these facilities.

Evaluation and Findings of Consistency with Objective 5.2 and Policy 5.2.a: An analysis of the proposed amendment's potential impact to public facilities has been provided within this report. This analysis demonstrates that, based upon current facility capacities, the development would not adversely affect the Level of Service (LOS) standard of any monitored public facilities. Based on the provided traffic impact analysis report, the proposed rezoning will not reduce the level of service for the intersection of US 441 and NW 147th Drive. At time of development application, the traffic impact analysis will be reviewed again to determine if any improvements will be needed to the intersection to maintain operability.

GOAL 9: Water and Wastewater Service:

The City will ensure that new development within the corporate limits, where potable water and wastewater service are available, as defined in Policy 1.2.a and Policy 4.2.a of the Community Facilities and Natural Groundwater Aquifer Recharge Element of the Comprehensive Plan, shall connect to the City of Alachua's potable water and wastewater system.

Policy 9.1: Any new development within a Commercial or Industrial Future Land Use Map Designation within the corporate limits, where potable water and wastewater service are available, as defined in Policy 1.2.a and Policy 4.2.a of the Community Facilities and Natural Groundwater Aquifer Recharge Element of the City of Alachua Comprehensive Plan, shall connect to the City of Alachua's potable water and wastewater system.

Analysis of Consistency with Goal 9 and Policy 9.1: The subject property is located within the City's potable water and wastewater service areas. Future development would be required to be served by City potable water and wastewater services.

Transportation Element

Objective 1.1: Level of Service: The City shall establish a safe, convenient and efficient level of service standard for all motorized and non-motorized transportation systems.

Analysis of Consistency with Objective 1.1: An analysis of the impacts to transportation facilities is provided within this report. Based upon current demand, future development of the subject property would not adversely affect the Level of Service (LOS) standards for transportation facilities.

Community Facilities & Natural Groundwater Aquifer Recharge Element

Policy 1.2.a: The City shall establish a Community Wastewater Service Area, which includes all areas where wastewater service is available. Wastewater service shall be deemed available if:

1. A gravity wastewater system, wastewater pumping station, or force main exists within ¼ mile of the property line of any residential subdivision with more than 5 units, or any multi-family residential development, or any commercial development, or any industrial development and the gravity wastewater system, wastewater pumping station, or force main can be accessed through public utility easements or right of ways. The distance shall be measured as required for construction of the infrastructure along public utility easements and right of ways.

Analysis of Consistency with Policy 1.2.a: The subject property is located within the City's wastewater service area. Future development would be required to be served by City wastewater services.

Analysis of Consistency with Objective 2.1.a: An analysis of the impacts to solid waste facilities is provided within this report. Based upon current demand, development of the subject property will not adversely affect the Level of Service (LOS) standards for solid waste facilities.

Policy 4.1.b: The City shall establish a Community Potable Water Service Area, which includes all areas where potable water service is available. Water service shall be deemed available if:

1. A water main exists within ¼ mile of any residential subdivision with more than 5 units, or any multi-family residential development, or any commercial development, or any industrial development and water service can be accessed through public utility easements or right of ways. The distance shall be measured as required for construction of the infrastructure along public utility easements and right of ways.

Analysis of Consistency with Policy 4.1.b: The subject property is located within the City's potable water service area. Future development would be required to be served by City potable water services.

Conservation and Open Space Element

OBJECTIVE 1.3: Listed Species

The City shall protect species listed by State and Federal agencies as endangered, threatened or of special concern, and their habitats.

Policy 1.3.a: The City shall ensure that its ordinances, regulations and policies protect listed species and their habitats.

Policy 1.3.b: The City shall utilize the development review process, land acquisition programs, environmental regulatory partnerships, stewardship programs and public education to protect listed species and their habitat, and prevent extinction of or reduction in populations of listed species.

Policy 1.3.c: The City shall obtain data from the Florida Fish and Wildlife Conservation Commission, Alachua County Environmental Protection Department, Florida Department of Environmental Protection, to maintain a periodically updated inventory of listed species and habitats located within City limits or immediately

adjacent to City limits. The City will use the Florida Natural Areas Inventory as a base inventory.

Policy 1.3.d: The City shall require prior to development approval, an inventory of listed species for all new developments in areas identified as known habitat for listed species. The inventory shall include detailed information regarding type, quantity, location and habitat requirements for any listed species identified. De minimus threshold for properties required to complete the inventory shall be established in the City's Land Development Regulations.

Policy 1.3.e: The City's land use designations shall provide for the protection of threatened and endangered species.

Analysis of Consistency with Objective 1.3 and Policies 1.3.a – e: There are no known listed species inhabiting the subject property. Future development of the site must comply with the environmental protections established in the City of Alachua Comprehensive Plan and Land Development Regulations.

ENVIRONMENTAL CONDITIONS & SITE SUITIBILITY ANALYSIS

Wetlands

According to best available data, there are no wetlands identified on the subject property.

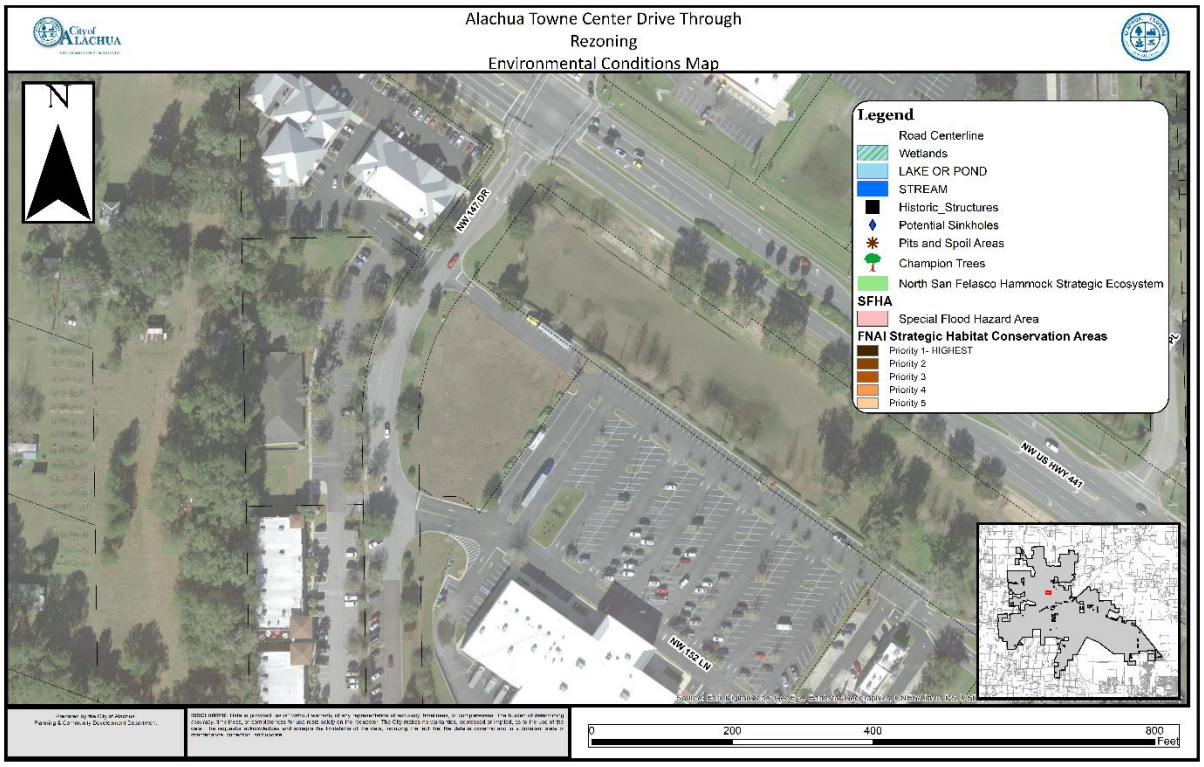
Evaluation: If any wetlands are identified on subject property at a later time and as part of the development review process, the applicable protection standards in the City's Comprehensive Plan and Land Development Regulations, as well as all applicable Suwannee River Water Management District (SRWMD) regulations, would apply to those areas identified as wetlands.

Creeks and Streams

There are no creeks or streams located on the subject property.

Evaluation: Objective 1.12 and Policy 1.12.d of the City of Alachua Comprehensive Plan Conservation & Open Space Element require minimum buffers from surface water bodies. Should a surface water body be found to exist on or proximate to the subject property, buffers as set forth in Policy 1.12.d shall be required.

Map 3. Environmental Features



Strategic Ecosystems

Strategic Ecosystems were identified by an ecological inventory project in a report prepared for Alachua County Department of Growth Management in 1987 and updated in 1996. The purpose of the inventory was to identify, inventory, map, describe, and evaluate the most significant natural biological communities in private ownership in Alachua County.

Evaluation: The subject property is not located within or adjacent to a Strategic Ecosystem, therefore, the development will have no impact upon any Strategic Ecosystem(s) identified within the ecological inventory report.

Regulated Plant & Animal Species

The subject property is not known to contain any species identified as endangered, threatened, or of special concern. The Florida Natural Areas Inventory (FNAI) has identified areas throughout the State of Florida which may contain good quality natural communities. This data layer is known as the Potential Natural Areas (PNA) data layer, and identifies privately owned lands that are not managed or listed for conservation purposes. These areas were delineated by FNAI scientific staff through interpretation of natural vegetation from 1988-1993 FDOT aerial photographs and from input received during Regional Ecological Workshops held for each regional planning council. These workshops were attended by experts familiar with natural areas in the region. Potential Natural Areas were assigned ranks of Priority 1 through Priority 5 based on size, perceived quality, and type of natural community present. The areas included in Priority 5 are exceptions to the above criteria.

These areas were identified through the same process of aerial photographic interpretation and regional workshops as the PNA 1 through 4 ranked sites, but do not meet the standard criteria.

Evaluation: The subject property does not contain any FNAI Strategic Habitat Conservation Area Priority lands. The FNAI PNA data is not intended for use in a regulatory decision making process. The data must be referenced only as a resource to indicate the potential of land to support wildlife. If a regulated plant or animal species is identified during any redevelopment of the subject property, the applicant must adhere to the applicable standards in the City of Alachua Comprehensive Plan and the Land Development Regulations.

Soil Survey

Each soil type found on the subject property is identified below. The hydrologic soil group is an indicator of potential soil limitations. The hydrologic soil group, as defined for each specific soil, refers to a group of soils which have been categorized according to their runoff-producing characteristics. These hydrologic groups are defined by the Soil Survey of Alachua County, Florida, dated August 1985. The chief consideration with respect to runoff potential is the capacity of each soil to permit infiltration (the slope and kind of plant cover are not considered, but are separate factors in predicting runoff). There are four hydrologic groups: A, B, C, and D. "Group A" soils have a higher infiltration rate when thoroughly wet and therefore have a lower runoff potential. "Group D" soils have very lower infiltration rates and therefore a higher runoff potential.

There are 3 soil types found on the subject property:

Soil Type	Hydrologic Group	Drainage Class	Small Commercial Buildings	% of Subject Property (may not total to 100% due to rounding)
Kendrick Sand, 2 to 5 percent slopes	B	Well drained	Somewhat limited	100.0

Evaluation: The soil type poses some limitation for small commercial buildings based on data and analysis from the Soil Survey of Alachua County. The soil has features that are moderately favorable for small commercial buildings, and, generally, any limitations can be overcome by design and engineering considerations.

Flood Potential

Panel 0120D of the Federal Emergency Management Agency (FEMA) Flood Insurance Rate Map (FIRM) Series, dated June 16, 2006, indicates that the subject property contains areas with Flood Zone "X" designations (areas determined to be outside of the 500-year floodplain).

Evaluation: The subject property is located in Flood Zone "X" (areas determined to be outside of the 500-year floodplain). Therefore, there are no issues related to flood potential.

Karst-Sensitive Features

Karst sensitive areas include geologic features, such as fissures, sinkholes, underground streams, and caverns, and are generally the result of irregular limestone formations. The subject property is located within an area where sinkholes may potentially allow hydrologic access to the Floridan Aquifer System.

Evaluation: There are no known geologic features that would pose an issue related to potential development.

Wellfield Protection Zone

Policy 7.2.1 of the Future Land Use Element of the Comprehensive Plan establishes a 500 foot radial buffer around city-owned potable water well.

Evaluation: The subject property is not located within a City of Alachua wellhead protection zone as identified on the City of Alachua Wellfield Primary Protection Zones Map of the Comprehensive Plan, therefore, there are no issues related to wellfield protection.

Historic Structures and Markers

The subject property does not contain any historic structures or markers as determined by the State of Florida and the Alachua County Historic Resources Inventory.

Evaluation: There are no issues related to historic markers or structures.

FINDINGS OF FACT:
COMPLIANCE WITH LAND DEVELOPMENT REGULATIONS

REZONING STANDARDS

Section 2.4.2(E)(1) of the Land Development Regulations ("LDRs") establishes standards with which all rezoning applications must be found to be compliant. Staff's evaluation of the application's compliance with the applicable standards of Section 2.4.2(E)(1) is provided below.

(a) ***Consistent with Comprehensive Plan*** – The proposed amendment is consistent with and furthers the goals, objectives, and policies of the Comprehensive Plan.

Evaluation and Findings: An analysis of the application's consistency with the Comprehensive Plan is provided within this report.

(b) ***Consistent with Ordinances*** – The amendment is not in conflict with any portion of these LDRs or any of the City Code of Ordinances.

Evaluation and Findings: An analysis of the application's compliance with the LDRs is provided within this report. The amendment does not conflict with any applicable requirements of the City's Code of Ordinances.

(c) ***Logical Development Pattern*** – The proposed amendment would result in a logical and orderly development pattern.

Evaluation and Findings: The property is contiguous to lands zoned Commercial Intensive (CI) and existing commercial uses. The subject property is located in proximity to drive through restaurants and financial institutions with drive through facilities. The subject property is located in close proximity to US Highway 441.

(d) ***Pre-Mature Development*** – The proposed amendment will not create premature development in undeveloped or rural areas.

Evaluation and Findings: The subject property is located in an area with existing commercial development. Future development of the property would not create premature development in an undeveloped or rural area.

(e) ***Incompatible with Adjacent Lands*** – The uses permitted by the proposed amendment are not incompatible with existing land uses of adjacent lands and/or the uses permitted by the zone district classifications of adjacent lands.

Evaluation and Findings: The property is contiguous to lands zoned Commercial Intensive (CI) and existing commercial uses. The subject property is located in proximity to drive through restaurants and financial institutions with drive through facilities. The subject property is located in close proximity to US Highway 441.

(f) ***Adverse Effect on Local Character*** – The proposed amendment will not adversely effect the character of the general area where it is proposed to be located by creating excessive traffic, density and/or intensities of use, building height and bulk, noise, lights, or other physical effects or nuisances.

Evaluation and Findings: While development of the property will have an impact on traffic and intensity of the use of the property, these are not anticipated to be excessive. Development will be required to meet the standards of the Land Development Regulations.

(g) ***Not Deviate from Pattern of Development*** – The uses permitted by the proposed amendment will not deviate from the development pattern (both established and as proposed by the surrounding zone districts) of the area where the proposed amendment is located.

Evaluation and Findings: The property is contiguous to lands zoned Commercial Intensive (CI) and existing commercial uses. The subject property is located in proximity to drive through restaurants and financial institutions with drive through facilities. The subject property is located in close proximity to US Highway 441.

(h) ***Encourage Sprawl*** – The proposed amendment will not encourage urban sprawl, either by resulting in strip or ribbon commercial development, leap-frog development or low density single dimensional development.

Evaluation and Findings: Chapter 163.3164(51), Florida Statues, defines "urban sprawl" as, "a development pattern characterized by low density, automobile-dependent development with either a single use or multiple uses that are not functionally related, requiring the extension of public facilities and services in an inefficient manner, and failing to provide a clear separation between urban and rural uses." Staff finds that the proposed amendment does not constitute urban sprawl.

(i) ***Spot Zoning*** – The proposed amendment will not result in the creation of an isolated zone district unrelated to adjacent and surrounding zone districts (spot zoning).

Evaluation and Findings: The property is contiguous to lands zoned Commercial Intensive (CI) and existing commercial uses. The subject property is located in proximity to drive through restaurants and financial institutions with drive through facilities. The subject property is located in close proximity to US Highway 441.

(j) ***Public Facilities*** – The proposed amendment will not result in development in a location where there are no plans by the City or other governmental entities to provide public facilities to serve the development (roads, potable water, wastewater, parks, storm water management, and solid wastes), and there are no assurances by the private sector that public facilities are planned and will be available to adequately accommodate development.

Evaluation and Findings: The subject property is located within the City of Alachua's utility service area. Future development would be served by the City's potable water and wastewater systems. Any future development will be required to connect to public water and wastewater infrastructure. As demonstrated in the public facilities analysis provided in this report, based upon current facility capacity, the proposed amendment would not result in any adverse impacts to roads, parks, potable water, sanitary sewer, or solid waste facilities.

(k) ***No Adverse Effect on the Environment*** – The proposed amendment would not result in significantly adverse impacts on the natural environment, including but not limited to water, air, noise, storm water management, wildlife, vegetation, wetlands, and the natural functioning of the environment.

Evaluation and Findings: A comprehensive analysis of environmental features has been provided in this report. The proposed amendment would not result in significant adverse impacts to any environmental features.

PUBLIC FACILITIES IMPACT

The existing maximum development potential and proposed maximum development potential is provided within the following matrix.

	Existing Zoning Designation	Proposed Zoning Designation
Zoning Designation	Central Business District (CBD)	Commercial Intensive (CI)
Max. Gross Intensity (FAR):	0.75	0.75
Maximum Intensity (Square feet) :	35,610	35,610

The analysis of each public facility provided below represents an analysis of them maximum realistic development potential proposed by the amendment. The maximum realistic development potential includes 5,000 square feet of retail uses, and 1,500 square feet of a restaurant with drive through services.

At the maximum development potential, the proposed amendment would not degrade the Level of Service (LOS) standard of any public facilities to an unacceptable level. This analysis is preliminary and based upon the maximum development potential of the proposed amendment. If development is proposed in the future, the applicant will be required to provide a comprehensive analysis of the impacts generated by such development upon public facilities. Facility capacity must be available to support the proposed development prior to the issuance of a final development order.

Table 3. Affected Comprehensive Plan Roadway Segments[1]

Segment Number[2, 3]	Segment Description	Lanes	Functional Classification	Area Type	LOS
5 (107)	US HWY 441 (From SR 235 to I-75)	4/D	Arterial	Urban	D

1 Source: City of Alachua Comprehensive Plan, Transportation Element.
2 For developments generating less than 1,000 trips or greater, affected roadway segments are identified as all those wholly or partially located within ½ mile of the development's ingress/egress, or to the nearest intersecting major street, whichever is greater [Section 2.4.14(H)(2)(b) of the LDRs].
3 FDOT roadway segment number shown in parenthesis. For the purposes of concurrency management, COA Comprehensive Plan segments that make up a portion of a larger FDOT roadway segment will be evaluated together when determining post development roadway capacity.

Table 4. Potential Trip Generation[1]

Land Use	AADT (Enter/Exit)	AM Peak Hour (Enter/Exit)	PM Peak Hour (Enter/Exit)
Retail Center[2] (ITE Code 820)	189 (95/ 95)	5 (3 / 2)	19 (9 / 10)
Restaurant with Drive through service[3] (ITE Code 934)	706 (353/353)	60 (34/31)	49 (25/25)
Total	895 (448/448)	65 (34/31)	68 (34/34)

1 Source: ITE Trip Generation, 10th Edition.
2 Formula: ITE Code 820: AADT –37.75 trips per 1ksf x 5 units (50% entering/50% exiting); AM Peak Hour –0.94 trips per 1ksf x 5 units (62% entering/38% exiting); PM Peak Hour – 3.81 trips per 1ksf x 5 units (48% entering/52% exiting).
3 Formula: ITE Code 934: AADT –470.95 trips per 1ksf x 1.5 units (50% entering/50% exiting); AM Peak Hour –40.19 trips per 1ksf x 1.5 units (51% entering/49% exiting); PM Peak Hour – 32.67 trips per 1ksf x 1.5 units (50% entering/50% exiting).

Table 5a. Projected Impact on Affected Comprehensive Plan Roadway Segments (AADT)

Traffic System Category	US HWY 441 (From SR 235 to I75)
Average Annual Daily Trips	
Maximum Service Volume[2]	39,000
Existing Traffic[3]	24,199
Reserved Trips[4]	3,745
Available Capacity[4]	11,056
Maximum AADT Generated by Amendment	895
Residual Capacity After Potential Development's Impacts[5]	10,161

1 FDOT roadway segment number shown in parenthesis (when applicable.) For the purposes of concurrency management, COA Comprehensive Plan segments that make up a portion of a larger FDOT roadway segment will be evaluated together when determining post development roadway capacity.
2 AADT & Peak Hour MSVs calculated using LOSPLAN 2012. County Facility AADT counts provided by Alachua County Public Works, April 2020.
3 Florida State Highway System Level of Service Report, Florida Department of Transportation, District Two.
4 Source: City of Alachua October 2020 Development Monitoring Report.

Table 5b. Projected Impact on Affected Comprehensive Plan Roadway Segments (Peak Hour)

Traffic System Category	US HWY 441 (From SR 235 to I75)
PM Peak Hour Trips	
Maximum Service Volume[2]	3,510
Existing Traffic[3]	2,299
Reserved Trips[4]	558
Available Capacity[4]	653
PM Peak Hour Trips Generated by Amendment	68
Residual Capacity After Potential Development's Impacts[5]	585

1 FDOT roadway segment number shown in parenthesis (when applicable.) For the purposes of concurrency management, COA Comprehensive Plan segments that make up a portion of a larger FDOT roadway segment will be evaluated together when determining post development roadway capacity.
2 AADT & Peak Hour MSVs calculated using LOSPLAN 2012. County Facility AADT counts provided by Alachua County Public Works, April 2020.
3 Florida State Highway System Level of Service Report, Florida Department of Transportation, District Two.
4 Source: City of Alachua October 2020 Development Monitoring Report.

Evaluation: It is anticipated that the proposed amendment will not adversely affect the Level of Service ("LOS") of any affected roadway segments and the development of the property at its maximum development potential is therefore acceptable. Concurrency and impacts to the City's transportation system will be reevaluated at the site plan or preliminary plat review stage.

Potable Water Impacts

Table 6. Potable Water Impacts

System Category	Gallons Per Day
Current Permitted Capacity[1]	2,300,000
Less Actual Potable Water Flows[1]	1,295,000
Reserved Capacity[2]	181,743
Available Capacity	823,257
Projected Potable Water Demand from Application[3]	1,750
Residual Capacity	**821,507**
Percentage of Permitted Design Capacity Utilized	**64.28%**

Sources:
1 City of Alachua Public Services Department, April 2020.
2 City of Alachua March 21 Development Monitoring Report.
*3 FAC 64E-6. Restaurant (40 gpd/ seat = 100*10) ; Retail (15 gpd/100 sq ft- 5,000 x 0.15)*

Evaluation: It is anticipated that the proposed amendment will not adversely affect the Level of Service ("LOS") of the potable water system and the development of the property at its maximum development potential is therefore acceptable. Concurrency and impacts to the City's potable water system will be reevaluated at the site plan or preliminary plat review stage.

Sanitary Sewer Impacts

Table 7. Sanitary Sewer Impacts

System Category	Gallons Per Day
Treatment Plant Current Permitted Capacity	1,500,000
Less Actual Treatment Plant Flows[1]	675,000
Reserved Capacity[2]	161,357
Available Capacity	663,643
Projected Sanitary Sewer Demand from Application[3]	1,750
Residual Capacity	**661,893**
Percentage of Permitted Design Capacity Utilized	**55.87%**

Sources:
1 City of Alachua Public Services Department, April 2020.
2 City of Alachua March 2021 Development Monitoring Report.
*3 FAC 64E-6. Restaurant (40 gpd/ seat = 100*10) ; Retail (15 gpd/100 sq ft- 5,000 x 0.15)*

Evaluation: It is anticipated that the proposed amendment will not adversely affect the Level of Service ("LOS") of the wastewater system and the development of the property at its maximum development potential is therefore acceptable. Concurrency and impacts to the City's wastewater system will be reevaluated at the site plan or preliminary plat review stage.

Solid Waste Impacts

Table 8. Solid Waste Impacts

System Category	Pounds Per Day	Tons Per Year
Demand from Existing Development[1]	41,192.00	7,517.54
Reserved Capacity[2]	18,923.62	3,453.56
Demand Generated by Application[3]	78.00	14.20
New River Solid Waste Facility Capacity[4]	**50 years**	

Sources:
1 *University of Florida, Bureau of Economic & Business Research, Estimates of Population by County and City in Florida (2018); Policy 2.1.a, CFNGAR Element (Formula: 10,298 persons x 0.73 tons per person per year).*
2 *City of Alachua March 2021 Development Monitoring Report.*
3 *United States EPA Facts and Figures about Materials, Waste and Recycling.*
4 *New River Solid Waste Facility, April 2020.*

Evaluation: It is anticipated that the proposed amendment will not adversely affect the Level of Service ("LOS") for solid waste facilities, and the impacts are therefore acceptable. This analysis is based on the maximum development potential proposed by the amendment. Concurrency and impacts to the City's solid waste system will be reevaluated at site plan review or preliminary plat review stage.

EXHIBIT "A"

TO

ALACHUA TOWNE CENTER DRIVE THROUGH RESTAURANT
SITE SPECIFIC AMENDMENT TO THE OFFICIAL ZONING ATLAS
STAFF REPORT

Table 4.1-1. Table of Allowed Uses

P = Permitted use S = Special exception permit A = Allowed in the PD districts Blank cell = Prohibited

Use Category/Use Type	CSV	A	Residential								Business									Planned Development				Use Specific Standards (Sec. 4.3)
			Single-Family (RSF)				Mobile Home (RMH)		Multiple Family (RMF)		OR	CN	CC	CBD	CI	CP	ILW	IG	GF	COMM	R	TND	EC	
			1	3	4	6	5	P	8	15														
RESIDENTIAL USES																								
Household living																								
Dwelling, live/work		P							P	P	P	P	P	P		P	P			A	A	A	A	
Dwelling, manufactured home		P	P	P	P	P	P	P	P	P											A	A	A	4.3.1(A)(1), (2)
Dwelling, mobile home		P					P	P																4.3.1(A)(1)

Dwelling, multiple-family						S			P	P	P		P	P	P	P				A	A	A	A	4.3.1(A)(3)
Dwelling, single-family attached						P			P	P	P			P	P	P				A	A	A		4.3.1(A)(3)
Dwelling, single-family detached		P	P	P	P	P			P	P	P			P		P					A	A		4.3.1(A)(4)
Dwelling, townhouse				S	S	S			P	P	P				P	P				A	A	A		4.3.1(A)(3)
Dwelling, two- to four-family				S	S	P			P	P	P			P	P	P				A	A	A		4.3.1(A)(3)
Mobile home park								P																4.3.1(A)(1)
Upper story dwelling									P	P	P	P	P	P	P	P				A		A	A	
Group living																								
Co-housing		P				P	P	P																4.3.1(B)(1)
Community residential home (6 or fewer residents)		P	P	P	P	P	P	P	P	P	P			P							A	A		4.3.1(B)(2)(b)
Community residential home (7—14 residents)					S	S			P	P	P													4.3.1(B)(2)(c)
Dormitory		P							S	P			P				S					A	A	4.3.1(B)(3)
Group home (15 or more residents)		S				S			S	S	S											A		
Roominghouse				S	S	S			P	P														

PUBLIC AND INSTITUTIONAL USES																								
Community services																								
Community center		S				S	S	S	S	S		P		P					P	A	A	A		4.3.2(A)
Cultural facility									S	S	P	P	P	P		P			P	A	A	A	A	4.3.2(A)
Library						S			P	P	P	P	P	P					P	A	A	A	A	4.3.2(A)
Senior center						S	P	P	P	P		P	P	P					P		A	A		4.3.2(A)
Youth club facility						S	P	P	P	P		P	P	P					P		A	A		4.3.2(A)
Day care																								
Adult care center		S	S	S	S	S	S	S	S	S	S	S	S	S						A	A	A	A	
Child care center		S		S	S	S	S	S	P	P	P	P	P	P		P				A	A	A	A	4.3.2(B)(1)
Day care home (up to and including 6 persons)		S	S	S	S	S	S	S	S	S	P	P	P	P							A			
Overnight child care center		S				S	S	S	P	P	P	P	P	P									A	4.3.2(B)(1)
Educational facilities																								
College or university							S		P	P	S		P	P	P	P	P	P	P	A	A	A	A	
School		P	S	S	S	P	P	P	P	P			P	P					P	A	A	A		4.3.2(C)(1)

Vocational school						S	S	S	P	P	S	P	P	P	P	P	P	P	P	A	A	A	A	4.3.2(C)(1)
Government facilities																								
Government maintenance, storage, and distribution facility													S	S	P		P	P	P	A			A	4.3.2(D)
Government office											P		P	P	P	P			P	A		A	A	4.3.2(D)
Post office						S	S	S	S	S	P	P	P	P	P	P	P		P	A	A	A	A	4.3.2(D)
Health care facilities																								
Blood collection facility															P	P	P	P		A			A	
Birth center											P	S	P	P	P	P				A		A	A	
Hospital		S							S	S			P	P			P		P	A		A	A	4.3.2(E)(1)
Medical and dental clinic											P	S	P	P	P	P				A		A	A	
Medical and dental lab													P		P	P	P	P		A			A	
Medical marijuana dispensing																								
Outpatient facility									S	S		S	P	P	P					A		A	A	4.3.2(E)(2)
Institutions																								

Assisted living facility		S			S	S			P	P	S									A	A	A	A	
Auditorium												S	P	P	S	P	P		P	A			A	4.3.2(F)(1)
Convention center												S	P	P	S	P	S		P	A			A	4.3.2(F)(1)
Drug and alcohol treatment facility													P											
Nursing home		S							S	S	P		P	P						A		A		
Psychiatric treatment facility													P							A				
Religious institution, with seating capacity less than 300 in sanctuary or main activity area		P	S	S	S	P	P	P	P	P	P	P	P	P	P	P	P	P		P	A	A	A	A	4.3.2(F)(2)
Religious institution, with seating capacity of 300 or greater in sanctuary or main activity area, or with accessory schools, day care centers with more than 50 children, or recreational facilities		P	S	S	S	S	S	S	S	S	S	S	S	P	P	P	P	P		P	A	A	A	A	4.3.2(F)(2)
Parks and open areas																								
Arboretum		P	P	P	P	P	P	P	P	P	P	P	P	P	P	P	P	P		P	A	A	A	A	
Botanical garden		P	P	P	P	P	P	P	P	P	P	P	P	P	P	P	P			P	A	A	A	A	
Cemetery, columbaria, mausoleum		P				S	S	S	S	S			P		P					P				

Community garden		P	P	P	P	P	P	P	P	P						P			P		A	A	A	
Golf course, public		P	P	P	P	P	P	P	P	P						P	P		P	A	A	A	A	
Park, private and public	P	P	P	P	P	P	P	P	P	P	P	P	P	P	P	P	P	P	P	A	A	A	A	
Public square		S	S	S	S	S	P	P	P	P	P	P	P	P	P	P	P	P	P	A	A	A	A	
Recreational trail	P	P	P	P	P	P	P	P	P	P	P	P	P	P	P	P	P	P	P	A	A	A	A	
Resource-based recreation uses, nonintensive	P	P	P	P	P	P	P	P	P	P	P	P	P			P	P		P	A	A	A	A	
Resource-based recreation uses	S	S	P	P	P	P	P	P	P	P						P			P	A	A	A	A	
Public safety																								
Fire and EMS		P		S	S	S	S	S	P	P	P	P	P	P	P	P	P		P	A	A	A	A	4.3.2(G)
Police station		P		S	S	S	S	S	P	P	P	P	P	P	P	P	P	P	P	A	A	A	A	4.3.2(G)
Substation for fire and City police		P			S	P	P	P	P	P	P	P	P	P	P	P	P	P	P	A	A	A	A	4.3.2(G)
Transportation																								
Airport		S											S		S			S	P				A	
Airplane landing strip		S											S		S			S	P	A	A		A	4.3.2(H)(1)
Helicopter landing facilities		S							S	S			S	S	S	S	S	S	P	A			A	4.3.2(H)(2)

Passenger terminal, surface transportation													S		P	P	P	P	P	A				
Utilities																										
Wireless communication tower and/or antenna, freestanding	S	P	S	S	S	S	S	S	S	S	S	S	P	S	P	P	P	P	P	A	A	A	A	4.3.2(I)(1)
Wireless communication antenna, collocation on existing tower	P	P	P	P	P	P	P	P	P	P	P	P	P	P	P	P	P	P	P	A	A	A	A	4.3.2(I)(1)
Wireless communication antenna, placement on existing building	P	P	P	P	P	P	P	P	P	P	P	P	P	P	P	P	P	P	P	A	A	A	A	4.3.2(I)(1)
Railroad right-of-way	P	P	P	P	P	P	P	P	P	P	P	P	P	P	P	P	P	P	P	A	A	A	A	
Utility, major		S	S	S	S	S	S	S	S	S			S	S	S	S	S	S	P	A	A	A	A	4.3.2(I)(3)
Utility, minor		P	P	P	P	P	P	P	P	P	P	P	P	P	P	P	P	P	P	A	A	A	A	4.3.2(I)(4)
AGRICULTURE																										
Agriculture																										
General use category		P																								
Medical marijuana cultivation																										
Non-medical marijuana cultivation																										

Animal husbandry																									
General use category		P																							
Horticulture																									
General use category		P																							
Agriculture support and services (directly related)																									
Agricultural processing		P																							4.3.3(A)(1), (2)
Agri-education		P																							4.3.3(A)(1)
Agri-entertainment		P																							4.3.3(A)(1)
Custom operator		P																							4.3.3(A)(1)
Direct market business for sale of products produced on site, including but not limited to produce stands or PYO (pick-your-own) establishments		P																							4.3.3(A)(1), (3)
Equestrian facility		P													S		S								4.3.3(A)(1)
Farm co-op		P																							4.3.3(A)(1)
Farm machinery repair		S																							4.3.3(A)(1)

Farm market		P																		A				4.3.3(A)(1)
Farm produce stand		P																		A				4.3.3(A)(1), (3)
Feedlot (for ongoing, on-site animal husbandry activities)		P																						4.3.3(A)(1)
Nursery, commercial		S																		A				4.3.3(A)(1), (4)
Nursery, production		P	S	S	S	S	S								P									4.3.3(A)(1), (4)
Pet farm		P																						4.3.3(A)(1)
Sawmill		S																						4.3.3(A)(1), (5)
Stable		P																						4.3.3(A)(1), (6)
Agriculture support and services (not directly related)																								
Agricultural research facility		P														P	P	P						4.3.3(B)(1)
Animal care business		P															P							
Auction arena for livestock		S															S	S						

Central farm distribution hub for agricultural products		P															P	P						4.3.3(B)(1)
Equestrian facility		P																						4.3.3(B)(2)
Fair grounds		S																		A				
Farm machinery repair		P													P		P							
Farm machinery sales, rental, and service		P													P		P			A				
Stable		P																						4.3.3(B)(3)
Animal sales, service and care																								
Animal hospital		P											P		S					A				4.3.3(C)(1)
Animal shelter		S											S							A				4.3.3(C)(2)
Animal grooming		P											P	P	P			P		A				
Kennel, indoor		P							S	S		S	P		S	P	P			A		A		4.3.3(C)(3)
Kennel, outdoor		P											P		P		P	P		A				4.3.3(C)(4)
Veterinary clinic		P											P	P	P	P				A		A		4.3.3(C)(5)
BUSINESS																								

Eating establishments																									
Ice cream shop									S	S	S	P	P	P	P	P				A	A	A		
Restaurant, indoor seating only									S	S	S	P	P	P	P	P	P			A	A	A	A	
Restaurant, with outdoor seating									S	S	S	P	P	P	S	P	S			A	A	A	A	4.3.4(A)(1)
Restaurant, with drive-through or drive-in service													P		P	S	S			A			A	4.3.4(A)(2)
Specialty eating establishment									S	S	S	P	P	P	P	P				A	A	A	A	
Conference and training centers																								
Conference center		S							S	S			P	P	P	P	P		S	A		A	A	4.3.4(B)(1)
Rural agricultural corporate retreat		P																						4.3.4(B)(2)
Industrial services																								
Building, heating, plumbing, or electrical contractors																P	P	P		A				
Electric motor repair																		P						4.3.4(C)(1)
Fuel oil distributor																		P						
General industrial service													P	P	P	P	P	P					A	

Heavy equipment sales, rental, or repair															P			P						4.3.4(C)(2)
Laundry, dry cleaning, and carpet cleaning facilities													P	P	P								A	4.3.4(C)(3)
Machine shop															S			P						4.3.4(C)(1)
Repair of scientific or professional instruments													S		S	P	P	P		A			A	
Tool repair															S	P	P	P						4.3.4(C)(1)
Manufacturing and production																								
Asphalt/concrete batch plant																		S						4.3.4(D)(1)
Manufacturing, heavy [1]																		P						4.3.4(D)(1)
Manufacturing, light																P	P	P		A			A	
Medical marijuana processing																								
Medical radioisotope laboratory																P								
Non-medical marijuana processing																								
Offices																								

Business services											P	P	P	P	P	P				A		A	A	
Financial services											P	P	P	P	P	P				A		A	A	
Professional services										S	P	P	P	P	P	P				A		A	A	
Radio and television broadcasting studio													P	P	P	P	P	P		A			A	
Sales													P	P	P	P				A		A	A	
Parking, commercial																								
Parking lot													P	P	P	P	P	P	P	A	A	A	A	4.3.4(E)(1)
Parking structure													P	P	P	P	P	P	P	A	A	A	A	4.3.4(E)(2)
Recreation/entertainment, indoor																								
Banquet hall													P	P	P	P			P	A		A	A	4.3.4(F)(1)
Commercial recreation, indoor									S	S			P	P	P	P		P	P	A	A	A	A	
Internet Café/simulated gaming establishment																								
Neighborhood recreation center				S	S	S			P	P		P				P			P	A	A	A		
Private club or lodge with seating capacity of less than 300 in main activity area		P	S	S	S	P	P	P	S	S	P	P	P	P	P	P	P			A	A	A	A	

Private club or lodge, with seating capacity of 300 or greater in main activity area		P					S	S	S	S	S	S	P	P	P	P	P			A	A	A	A	
Theater												S	P	P	P	P	P		S	A		A	A	
Recreation/entertainment, outdoor																								
Archery range		P													P				P					
Arena, amphitheater, auditorium, stadium		S											S	S	S		S	S	P	A	A	A	A	4.3.4(F)(1)
Commercial recreation, outdoor		S													P				P	A		A	A	
Golf course, private		P	P	P	P	P	P	P	P	P						P	P				A	A	A	
Retail sales and services																								
Auction house		P													P					A				4.3.4(G)
Bar, nightclub, or cocktail lounge												S	P	P	P					A		A		4.3.4(G)(1)
Convenience store									S	S		P	P	S	P	P	S			A	A	A		4.3.4(G)(2)
Department or discount store													P	P	P					A		A		4.3.4(G)
Drug store or pharmacy (stand alone)													P	P	P	P				A				4.3.4(G)(3)
Crematory		S													S		P	P					A	4.3.4(G)

Entertainment establishment													P	P	P	P				A				4.3.4(G)
Financial institution										S	S	P	P	P	P	P	P			A	A	A	A	4.3.4(G)(4)
Funeral home											S		P	P	P					A				4.3.4(G)
General media store													P	P	P	P				A	A	A		4.3.4(G)
Liquor store													S	P	P	P				A		A		4.3.4(G)
Laundromat															P	P				A				4.3.4(G)(5)
Non-medical marijuana dispensing																								
Pawn shop															P					A				4.3.4(G)(8)
Precious metals dealer															P					A				4.3.4(G)(9)
Personal services establishment									S	S		P	P	P	P	P				A		A		4.3.4(G)(6)
Repair establishment												S	P	P	P					A				4.3.4(G)
Sales establishment									S	S		S	P	P	P	P				A	A	A		4.3.4(G)
Tattoo parlor/Body-piercing studio															P					A				4.3.4(G)(10)
Large-scale retail establishments ≥ 20,000 sf, but < 80,000 sf													P		P					A		A		4.3.4(G)(7)

Large-scale retail establishments ≥ 80,000 sf													S		S					A				4.3.4(G)(7)
Self-service storage																								
All uses													P		P	S		P		A				4.3.4(H)
Sexually oriented business																								
Sexually oriented cabaret															P									4.3.4(I)
Sexually oriented media store															P									4.3.4(I)
Sexually oriented motion picture theater															P									4.3.4(I)
Sex shop															P									4.3.4(I)
Vehicles, sales and services																								
Automobile body shop															P		P	P						4.3.4(J)(1)
Automobile parts sales													S		P		P	P		A				
Automobile rental and sales													S		P					A				4.3.4(J)(2)
Automobile repair and servicing		S													S		P	P		A				4.3.4(J)(3)
Automobile service station												S	S	P	P		P	P		A				

Automobile service station with wash and detail															P					A				
Boat and marine rental and sales													P		P					A				4.3.4(J)(4)
Carwash or auto detailing															P					A				4.3.4(J)(5)
Gasoline sales												S	P	P	P		S	S		A		A		4.3.4(J)(6)
Recreational vehicle rental and sales															P			P		A				4.3.4(J)(2)
Taxicab service													P	P	P					A				
Tire sales and mounting													P		P					A				4.3.4(J)(7)
Towing service															P					A				4.3.4(J)(8)
Transmission or muffler shop															P					A				4.3.4(J)(7)
Truck or tractor rental or sales															P			P						4.3.4(J)(2)
Visitor accommodations																								
Bed and breakfast		P	S	S	S	S			S	S	S	S		P		P				A	A	A		4.3.4(K)(1)
Bed and breakfast inn									S	S	S	S		S		P				A		A		4.3.4(K)(2)
Hotel or motel												S	P	P	P	P				A		A	A	4.3.4(K)(4)
Warehouse and freight movement																								

Cold storage plant																		P						
Parcel services															P	P	P	P		A				4.3.4(L)(1)
Truck or freight terminal																	S	P						4.3.4(L)(1)
Warehouse (distribution)																P	P	P						4.3.4(L)(1)
Warehouse (storage)																P	P	P		A				4.3.4(L)(1)
Outdoor storage (as a principal use)																	S	S						4.3.4(L)(2)
Waste-related services																								
Energy recovery plant		S																						
Hazardous waste collection sites																		P						
Incinerator		S																S						
Landfill		S																S	S					
Landspreading of wastes		S																S						
Recycling dropoff center															S		S	S		A		A	A	4.3.4(M)(1)
Recycling and salvage center		S																P						4.3.4(M)(2)
Salvage and junkyard																		P						4.3.4(M)(3)

Tire disposal or recycling																		S						4.3.4(M)(3)
Waste composting		S																P						
Wholesale sales																								
All uses															S	P	P	P						

[1] This use type prohibits petroleum refining, rendering, mining and manufacture of chemicals, fertilizers, paint, turpentine, etc., but allows manufacture of automobiles and computers. See definitions for further details. Asphalt/concrete batch plants are considered a subuse type of manufacturing, heavy.

EXHIBIT "B"

TO

ALACHUA TOWNE CENTER DRIVE THROUGH

SITE SPECIFIC AMENDMENT TO THE OFFICIAL ZONING ATLAS

STAFF REPORT

SUPPORTING APPLICATION MATERIALS SUBMITTED BY CITY STAFF TO THE PLANNING AND ZONING BOARD

City of Alachua

ADAM BOUKARI
CITY MANAGER

PLANNING & COMMUNITY DEVELOPMENT
DIRECTOR KATHY WINBURN, AICP

March 31, 2021

Also sent by electronic mail to chris.potts@jbprogroup.com

Christopher Potts, P.E.
JBrown Professional Group, Inc.
3530 NW 43rd Street
Gainesville, FL 32606

RE: Notice of Public Hearing to be Scheduled for:
Alachua Towne Center Drive Through Site-Specific Amendment to the Official Zoning Atlas (Rezoning)

Dear Mr Potts:

On March 25, 2021 the City of Alachua received your updated application for the above referenced project. Based on review of the materials submitted, the City has determined that the application can now be scheduled for a hearing before the Planning and Zoning Board.

You must provide three (3) *double-sided, three-hole punched, color sets* of the **complete** application package, and a digital copy of all materials (including plans) in PDF format on a CD *no later than 10 business days prior to the PZB Meeting at which your application is scheduled to be heard*. The application has been scheduled for the **April 20, 2021** Planning and Zoning Board meeting; therefore, the above referenced materials must be submitted to the City no later than 12 PM on ***Tuesday, April 6, 2021.*** Materials may be submitted earlier than this date.

In addition, Section 2.2.9(D) of the Land Development Regulations requires the applicant to place posted notice signs on the subject property at least 14 days prior to the public hearing. Therefore, posted notice signs must be placed on the property no later than ***Monday, April 5, 2021***. These signs will be available at City Hall after 12 PM on Thursday, April 1, 2021.

If you plan to utilize a PowerPoint presentation or would like other materials to be available for reference during the public hearing, please submit the presentation or materials no later than 12:00 PM on the last business day prior the PZB meeting (no later ***Monday, April 19, 2021***). Any presentation or materials may be submitted by emailing them to planning@cityofalachua.com.

Should you have any questions, please feel free to contact me at (386) 418-6100, ext. 1603 or via email at ahall@cityofalachua.com.

Sincerely,

Adam Hall, AICP
Planner
c: Kathy Winburn, AICP, Planning Director
File

City of Alachua

ADAM BOUKARI
CITY MANAGER

PLANNING & COMMUNITY DEVELOPMENT
DIRECTOR KATHY WINBURN, AICP

March 11, 2021

Also sent by electronic mail to chris.potts@jbprogroup.com

Christopher Potts, P.E.
JBrown Professional Group, Inc.
3530 NW 43rd Street
Gainesville, FL 32606

RE: Development Review Team (DRT) Comments for:
Alachua Towne Center Drive Through Site-Specific Amendment to the Official Zoning Atlas (Rezoning)

Dear Mr. Potts:

On February 16, 2021, the City of Alachua received your revised Site Specific Amendment to the Official Zoning Atlas (Rezoning) application, which proposes rezoning the subject property from Central Business District (CBD) to Commercial Intensive (CI) on a ±1.09 acre subject property (Parcel No. 03869-004-000).

The application has been reviewed for compliance with the applicable review standards, including the City's Comprehensive Plan and Land Development Regulations (LDRs). Based upon Staff's review, revisions must be made to the application before it may be scheduled for a public hearing before the City's Planning & Zoning Board (PZB). Please address all insufficiencies outlined below in writing and provide an indication as to how they have been addressed by 5:00 PM on **Thursday, March 25, 2021.** A total of four (4) copies of the complete application package (i.e., all application materials and attachments) and a CD containing a PDF (or cloudlink/ FTP link provided by the applicant) of all application materials must be provided by this date. If all comments are addressed by the resubmission date above, the application may be scheduled for the April 20, 2021 PZB Meeting. Submission of an incomplete package may delay your tentative hearing at the Planning and Zoning Board. Upon receipt of your revised application, Staff will notify you of any remaining insufficiencies which must be resolved, if any, before the item may be scheduled for a public hearing before the PZB.

Please address the following insufficiencies (begins on next page):

Deficiencies to be Addressed

1. *Generally*
   a. Mailing labels not provided. Please provide physical versions of mailing labels (not just copy of labels on standard paper).

2. *Comprehensive Plan Consistency Analysis*
   a. Comprehensive Plan Consistency Analysis includes evaluation of Policy 1.3.c- Central Business District. Future Land Use Designation of property, however, is Commercial (Policy 1.3.b). Please revise.

3. *Concurrency Impact Analysis*
   a. Please provide sources for data provided in concurrency analysis. These should include citing the data provided by the City in its Development Monitoring Report (provide month and year of report), calculations for water and wastewater demand, and calculations for solid waste.
   b. Please provide analysis of impacted transportation segment(s) to demonstrate available capacity. Note: This information is provided in traffic study, but should also be included in the concurrency analysis section as well.

4. *Traffic Impact Study*
   a. Provided traffic impact study will also be reviewed at time of a development application.

If you have any questions regarding the information above, please contact me at 386-418-6100 x 1603 or via e-mail at ahall@cityofalachua.com. We look forward to receiving your revised application.

Sincerely,

Adam Hall, AICP
Planner

cc: Kathy Winburn, AICP, Planning & Community Development Director
Justin Tabor, AICP, Principal Planner
Project File

City of Alachua

ADAM BOUKARI
CITY MANAGER

PLANNING & COMMUNITY DEVELOPMENT
DIRECTOR KATHY WINBURN, AICP

February 22, 2021

Also sent by electronic mail to chris.potts@jbprogroup.com

Christopher Potts, P.E.
JBrown Professional Group, Inc.
3530 NW 43rd Street
Gainesville, FL 32606

RE: Completeness Review for: Alachua Towne Center Drive Through Site-Specific Amendment to the Official Zoning Atlas (Rezoning)

Dear Mr. Potts:

On February 16, 2021 the City of Alachua received your revised Site Specific Amendment to the Official Zoning Atlas (Rezoning) application, which proposes rezoning the subject property from Central Business District (CBD) to Commercial Intensive (CI) on a ±1.09 acre subject property (Parcel No. 03869-004-000).

According to Section 2.2.6 of the Land Development Regulations (LDRs), upon receipt of an application, a completeness review shall be conducted to determine that the application contains all the necessary information and materials, is in proper form and of sufficient detail, and is accompanied by the appropriate fee. The Planning Department has reviewed the aforementioned application and finds that the application is complete.

The contents of the application **has not** been thoroughly reviewed. An in-depth review of the content of the application will be performed and any further issues with content will be provided to you at the Development Review Team meeting.

Should you have any questions, please feel free to contact me at (386) 418-6100, ext. 1603 or via email at ahall@cityofalachua.com.

Sincerely,

Adam Hall, AICP
Planner

c: Kathy Winburn, AICP, Planning & Community Development Director (by electronic mail)
Justin Tabor, AICP, Principal Planner (by electronic mail)
Project File

City of Alachua

ADAM BOUKARI
CITY MANAGER

PLANNING & COMMUNITY DEVELOPMENT
DIRECTOR KATHY WINBURN, AICP

November 25, 2020

Also sent by electronic mail to chris.potts@jbprogroup.com

Christopher Potts, P.E.
JBrown Professional Group, Inc.
3530 NW 43rd Street
Gainesville, FL 32606

RE: Completeness Review for: Alachua Towne Center Drive Through Site-Specific Amendment to the Official Zoning Atlas (Rezoning)

Dear Mr. Potts:

On November 23, 2020 the City of Alachua received your Site Specific Amendment to the Official Zoning Atlas (Rezoning) application, which proposes rezoning the subject property from Central Business District (CBD) to Commercial Intensive (CI) on a ±1.09 acre subject property (Parcel No. 03869-004-000).

According to Section 2.2.6 of the Land Development Regulations (LDRs), upon receipt of an application, a completeness review shall be conducted to determine that the application contains all the necessary information and materials, is in proper form and of sufficient detail, and is accompanied by the appropriate fee. The Planning Department has reviewed the aforementioned application for completeness and finds that the application is incomplete.

Please note, the comments below are based solely on a preliminary review of your application for completeness. The contents of the application **has not** been thoroughly reviewed. An in-depth review of the content of the application will be performed after the application is deemed complete and any further issues with content will be provided to you at the Development Review Team meeting. Please address the following comments:

1. Application Attachment D.6- Please provide three sets of mailing labels for all property owners within 400' of the subject property and those individuals and organizations registered with the City to receive public notice.
2. As discussed during the pre-application meeting for this project, please provide a more detailed analysis of traffic impacts that would potentially be caused by the proposed rezoning application. It appears that the net increase in potential trips for this site is approximately 4,400 AADT/ 650 PM peak based on the existing and proposed zoning. Please provide intended methodology prior to completing this analysis. This analysis may be submitted to an independent expert consultant for review and comment (outside review). Per Section 2. 10 of the City's Fee

Resolution (Resolution 20-13), all costs associated with the outside review shall be paid prior to any hearing related to the application.

Failure to respond to the identified deficiencies above within 45 days of this letter shall result in withdrawal of the applications. Should you have any questions, please feel free to contact me at (386) 418-6100, ext. 108 or via email at ahall@cityofalachua.com.

Sincerely,

Adam Hall, AICP
Planner

c: Kathy Winburn, AICP, Planning & Community Development Director (by electronic mail)
Justin Tabor, AICP, Principal Planner (by electronic mail)
Project File

JBrown Professional Group
CIVIL ENGINEERING • PLANNING • LAND SURVEYING
3530 NW 43rd Street • Gainesville, FL 32606-6104 • Office: 352.375.8999

March 25, 2021

Mr. Adam Hall, AICP
Planner, City of Alachua
15100 NW 142nd Terrace / PO Box 9
Alachua, FL 32616

RE: Alachua Towne Center Parcel ID 03869-004-000 Rezoning Application

Dear Adam:

We are pleased to provide a resubmittal package for **Alachua Towne Center Parcel ID 03869-004-000 Rezoning Application**. This resubmittal package is provided to address the RAI comments you provided in your March 11, 2021 summary. Included in this submittal package are copies of the following:

The resubmittal package includes hard and digital copies of the following items.

- Rezoning Narrative Application
- 4 Copies of Mailing Labels

Please find below responses to the comments provided via email, dated March 11, 2021 for the above referenced project. The comments and responses are numbered in accordance with the numbering provided. The comments have been replicated below followed by responses in bold.

1. Mailing labels not provided. Please provide physical versions of mailing labels (not just copy of labels on standard paper).
   Response: Four sets of the mailing labels have been included.
2. Comprehensive Plan Consistency Analysis includes evaluation of Policy 1.3.c- Central Business District. Future Land Use Designation of property, however, is Commercial (Policy 1.3.b). Please revise.
   Response: Policy 1.3.b has been added to the analysis.
3. Please provide sources for data provided in concurrency analysis. These should include citing the data provided by the City in its Development Monitoring Report (provide month and year of report), calculations for water and wastewater demand, and calculations for solid waste.
   Response: The data has been cited in the concurrency section.
4. Please provide analysis of impacted transportation segment(s) to demonstrate available capacity. Note: This information is provided in traffic study, but should also be included in the concurrency analysis section as well.
   Response: The traffic study information has been added to the concurrency.
5. Provided traffic impact study will also be reviewed at time of a development application.
   Response: So noted.

In addition, the electronic files of all the submittal items were provided through electronic link. If you have any questions or require any additional information, please contact me. Thank you for your help with this project.

Sincerely,

Christopher Potts

Christopher Potts, P.E.
Director of Engineering
JBrown Professional Group Inc.

ALACHUA TOWNE CENTER

PARCEL 03869-004-000

Rezoning Application Package

March 25, 2021

Christopher Potts, P.E.

FL Registration No. 73842

Certificate of Authorization No. 30495

3530 NW 43rd Street

Gainesville, FL 32606

(352) 375-8999

JBrown Professional Group

CIVIL ENGINEERING • PLANNING • LAND SURVEYING

Table of Contents

SECTION 1: APPLICATION DOCUMENTS 3

SECTION 2: STATEMENT OF PROPOSED CHANGE 3

SECTION 3: AERIAL MAP 5

SECTION 4: CONCURRENCY IMPACT ANALYSIS 5

SECTION 5: COMPREHENSIVE PLAN ANALYSIS AND JUSTIFICATION 9

SECTION 6: ANALYSIS OF COMPLIANCE 19

SECTION 7: PROPERTY OWNER LABELS 21

SECTION 8: NEIGBHORHOOD MEETING SUMMARY 21

SECTION 9: LEGAL DESCRIPTION WITH TAX PARCEL NUMBER(S) 22

SECTION 10: PROOF OF OWNERSHIP 22

SECTION 11: PROOF OF TAXES 22

SECTION 12: APPLICATION FEE 22

List of Attachments

Attachment A: Rezoning Amendment Application

Attachment B: Authorized Agent Affidavit

Attachment C: Property Owner Labels

Attachment D: Neighborhood Meeting Summary

Attachment E: Sketch of Legal Description

Attachment F: Proof of Ownership

Attachment G: Proof of Payment of Taxes

Attachment H: MPH - Traffic Impact Study

SECTION 1: APPLICATION DOCUMENTS

The Rezoning Amendment Application is included as **Attachment A** to this submittal package. Also provided is the Authorized Agent Affidavit included as **Attachment B.**

SECTION 2: STATEMENT OF PROPOSED CHANGE

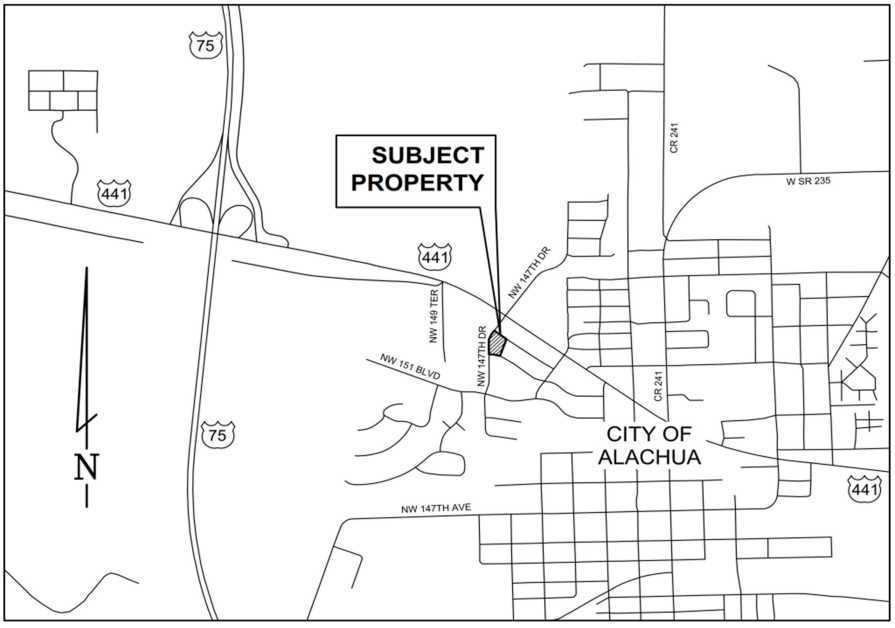
A. PREAPPLICATION CONFERENCE

A pre-application conference was held with City staff on 10/12/20. The proposed project was discussed in detail.

B. EXISTING CONDITIONS:

The property consists of 1 parcel totaling 1.09 acres of land, and is located on the east side of NW 147th Drive to the south of US-441 in the City of Alachua. The property includes tax parcel 03869-004-000.

FIGURE 1 - Location Map



The subject property is currently undeveloped. As shown in the following figures, the property has a current Zoning designation of Central Business District (C-CBD), which does not allow for a drive-thru use.

FIGURE 2: EXISTING ZONING MAP



Alachua County GIS

LOCATION	EXISTING USE	ZONING
North	Community Shopping Center	(C-CBD) Commercial, Central Business District
South	Community Shopping Center	(CI) Commercial, Intensive
East	Commercial Services Building	(CI) Commercial, Intensive
West	Professional Services Building	(C-CBD) Commercial, Central Business District

C. PROPOSED CHANGE:

The applicant requests approval of a Site Specific Amendment to the Official Zoning Atlas to change 1.09 acres from Central Business District (C-CBD) to a City of Alachua designation of Commercial Intensive (CI).

SECTION 3: AERIAL MAP

MAP



2020 Aerial Alachua County GIS Database

SECTION 4: CONCURRENCY IMPACT ANALYSIS

In accordance with *LDR Article 2, Section 2.4.14(H)* and *Comprehensive Plan Future Land Use Element Policy 5.2.1*, adequate public facilities are available to serve a maximum on-site development scenario. Specifically, the LDR states that "the necessary public facilities will be deemed available concurrent with the impacts of the proposed development if the sum of proposed development impacts when added to the existing demand and the capacity reservations are less than the maximum service volume on the affected facilities." Based upon maximum development scenarios, any proposed development allowed in the proposed Commercial land use designation will

operate within the level of service standards outlined in the Code. In addition, since there is no residential component, there is no impact on public schools facilities and recreation.

A. Potable Water - City of Alachua Municipal System

SYSTEM CATEGORY	GALLONS PER DAY
Current Permitted Capacity	2,300,000
Less Actual Flows	1,295,000
Less Reserved Capacity	181,743
Total (Actual Flow plus Reserved)	1,476,743
Available Capacity (Permitted - Total Flow)	823,257
Projected Potable Water Demand from Project	1,750
Residual Capacity	821,507

Usage Calculations by proposed uses within the development

- Restaurant w/Drive-thru: (40 gpd / seat) = 100 * 10 = 1,000 gpd
- Office Retail: (15 gpd/100 sq.ft.) = 5,000 * 0.15 = 750 gpd

Result: Adequate potable water capacity exists for the proposed project

Source:

- City of Alachua March 2021 Development Monitoring Report.
- F.A.C. 64E-6 for flow calculations.

B. Sanitary Sewer

SYSTEM CATEGORY	GALLONS PER DAY
Treatment Plant Current Permitted Capacity	1,500,000
Less Actual Treatment Plant Flows	675,000
Less Reserved Capacity	161,357
Available Capacity (Permitted - Actual - Reserve)	663,643
Projected Sanitary Sewer Demand from Project	1,750
Residual Capacity	661,893

Usage Calculations by proposed uses within the development

- Restaurant w/Drive-thru: (10 gpd / seat) = 100 * 10 = 1,000 gpd
- Office Retail: (15 gpd/100 sq.ft.) = 5,000 * 0.15 = 750 gpd

Result: Adequate sanitary sewer capacity exists for the proposed project

Source:

- City of Alachua March 2021 Development Monitoring Report.
- F.A.C. 64E-6 for flow calculations.

C. Solid Waste

SYSTEM CATEGORY	LBS PER DAY	TONS/YR
Existing Demand	41,192.00	7,517.54
Reserved Capacity	18,923.62	3,453.56
Proposed Demand	78.00	14.20
Residual Capacity	**22,190.38**	**4,049.78**

Usage Calculations by proposed uses within the development

- Restaurant: 6,500 SF * 12 lbs/day/1000 SF = 78 lbs/day = 14.2 tons/yr

Result: Adequate solid waste capacity exists for the proposed project.

Source:

- City of Alachua March 2021 Development Monitoring Report.
- United States Environmental Protection Agency Facts and Figures about Materials, Waste and Recycling.

D. Stormwater

The project shall comply with all City of Alachua stormwater design criteria and regulations, the permitting requirements of the Suwannee River Water Management District (SRWMD), the requirements of the Florida Department of Environmental Protection, and the requirements of the Alachua County Environmental Protection Department. Stormwater management facilities will be designed for the 100 year critical duration storm event and will ensure that post-development runoff levels do not exceed pre-development runoff levels. A complete stormwater management design report will be provided for each development phase.

E. Transportation

A Traffic Study was performed by MPH Transportation Planning, Inc. that addressed trip generation and trip distribution on the nearby roadway corridors. **Table 1**, as provided within the MPH Study, is provided below to address the Trip Generation for the proposed development.

TABLE 1: Project Trip Generation
Alachua Towne Center Retail Expansion
Alachua, Florida

Land Use					Distribution		Trips		*820* 34% Pass	*934* 50% Net
Description	ITE	ITE Trip Rates	Unit*	Trips	In	Out	In	Out	By	Trips
Retail Center	820	(T) = 37.75 (X)	5.000	189	50%	50%	95	95	64	125
Fast Food w/Drive-thru	934	(T) = 470.95 (X)	1.500	706	50%	50%	353	353	353	353
		Daily Trips	**6.500**	**895**			448	448	448	478
Retail Center	820	(T) = 0.94 (X)	5.000	5	62%	38%	3	2	2	3
Fast Food w/Drive-thru	934	(T) = 40.19 (X)	1.500	60	51%	49%	31	29	30	30
		AM Peak Hour Trips		**65**			34	31	32	33
Retail Center	820	(T) = 3.81 (X)	5.000	19	48%	52%	9	10	6	13
Fast Food w/Drive-thru	934	(T) = 32.67 (X)	1.500	49	50%	50%	25	25	25	25
		PM Peak Hour Trips		**68**			34	34	31	37

*Units: ksf = 1,000 sq. ft. Source: *ITE 10th Edition Trip Generation*

The distribution of these project trips to SR 20 and NW 147th Drive was also analyzed and provided within the MPH Study for the proposed rezoning. **Table 2**, included in the MPH Study, includes Daily Trip Distribution, AM Peak Hour Trip Distribution, and PM Peak Hour Trip Distribution for both SR 20 and NW 147th Drive.

TABLE 2:Project Trip Distribution
Alachua Towne Center Retail Expansion

Daily Project Trips		Enter	239	Exit	239	2-Way
Roadway	**Segment**	**%**	**Trips**	**%**	**Trips**	**Total**
SR 20 (US 441)	North of Project	35%	84	35%	84	168
	South of Project	50%	119	50%	119	238
NW 147th Drive	East of Project	5%	12	5%	12	24
	West of Project	10%	24	10%	24	48
	Total	100%	239	100%	239	478

AM Peak Hour Project Trips		Enter	34	Exit	31	2-Way
Roadway	**Segment**	**%**	**Trips**	**%**	**Trips**	**Total**
SR 20 (US 441)	North of Project	35%	12	35%	11	23
	South of Project	50%	17	50%	15	32
NW 147th Drive	East of Project	5%	2	5%	2	4
	West of Project	10%	3	10%	3	6
	Total	100%	34	100%	31	65

PM Peak Hour Project Trips		Enter	34	Exit	34	2-Way
Roadway	**Segment**	**%**	**Trips**	**%**	**Trips**	**Total**
SR 20 (US 441)	North of Project	35%	12	35%	12	24
	South of Project	50%	17	50%	17	34
NW 147th Drive	East of Project	5%	2	5%	2	4
	West of Project	10%	3	10%	3	6
	Total	100%	34	100%	34	68

Project Distributions are estimated from adjacent street traffic & interacting land uses
Peak hour project trips represent the gross driveway impacts from Table 1

Table 3, included in the MPH Study, includes the level of services of the surrounding roadways.

Table 3 - Roadway Level of Service (LOS)
Alachua Towne Center Retail Expansion
Cocoa, Florida

Roadway	Segment #/Description	Period	MSV	Existing	Res'vd.	Project	V/C	LOS
SR 20 - US 441	SR 235 to I-75	PM Peak	3,510	2,299	558	34	82%	C
	FDOT #107, CoA #5							

Source: Maximum Service Volume (MSV) from City of Alachua Traffic Database (2-way peak hour).
Existing Traffic from City Traffic Counts 2020. (MPH count on 2/9/21 = 2,278)
Reserved trips from Pending and approved Development Orders
Project trips represent PM Peak Hour two way traffic from Table 2.
Project trips are gross impacts not accounting for pass-by trip reductions.

Table 4, included in the MPH Study, the impact to the intersection of NW 147th Drive and US 441.

Table 4 - Intersection Impacts
Signal Controlled Intersection Impacts

Approach	*SR 20 - US 441*						*NW 147th Drive*						
PM Peak	Northbound			Southbound			Eastbound			Westbound			
Movement	Lt	Thru	Rt	Lt	Thru	Rt	Left	Thru	Rt	Left	Thru	Rt	Totals
2021	94	1252	61	62	739	119	200	24	43	55	11	87	2747
Project	**17**					**12**	**12**	**2**	**17**		**2**		**62**
2022	111	1252	61	62	739	131	212	26	60	55	13	87	2809

Notes: 2021 traffic volumes from Figure 1, Project Traffic (BOLD) from Figure 2.

Table 5, included in the MPH Study, includes the level of services of the intersection of NW 147th Drive and US 441.

Table 5 - Intersection Level of Service (LOS)

Traffic		SR 20 - US 441				NW 147th Drive					
Signal		NB		SB		EB		WB		Intersection	
Control		LOS	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS	Delay
2021	PM	B	20.5	B	14.2	D	47.7	C	27.8	C	21.2
2022	PM	B	20.4	B	14.0	D	51.7	C	28.3	C	21.8

Delay expressed in AVERAGE seconds per vehicle.

Result:

1. The proposed retail land use expansion on the Alachua Towne Center vacant outparcel is planned to include a fast food with drive thru facility as a component of the proposed 6,500 sq. ft. building. The entire project is estimated to generate

478 new daily trip impacts. The PM Peak hour will have approximately 68 total trips (37 net new) impacting adjacent roadways.

2. This segment of SR 20 (US 441) currently meets LOS standards (LOS "C") and will remain well below this threshold with the addition of project trips.

3. The proposed project driveways are located within the internal roadway network of the Alachua Towne Center south of NW 147th Drive.

4. Project trips will not adversely impact either roadway.

5. Right turn lane analysis indicates that no modifications are required to any of the three right turn lanes at the SR 20 (US 441)/NW 147th Drive intersection.

6. Left turn lane analysis indicates that all four left turn lanes at the subject intersection have adequate storage capacity for existing and project vehicle demand impacts.

SECTION 5: COMPREHENSIVE PLAN ANALYSIS AND JUSTIFICATION

Pursuant to *Article 2, Section 2.4.2(E)(1)(a)* of the City of Alachua's ("City") Land Development regulations ("LDRs"), the applicant is required to demonstrate how the proposed rezoning from Central Business District (C-CBD) to Commercial Intensive (CI), "…is consistent with, and furthers the goals, objectives and policies (G.O.P.'s) of the [City's] Comprehensive Plan." The following analysis will provide competent, and substantial evidence that the proposed project is consistent with the G.O.P's set forth in the following Comprehensive Plan Elements: Future Land Use, FLUM, Transportation, Community Facilities, Conservation/Open Space, Intergovernmental Coordination, and Capital Improvements.

I. Future Land Use Element:

The following G.O.P.'s are applicable to the proposed rezoning application:

> ***Goal 1: Future Land Use Map 2035****. The City shall maintain a Future Land Use Map in order to effectively guide development in a sustainable manner and to ensure economic prosperity and stability while maintaining a high quality of life for all of its present and future citizens, businesses, and visitors.*
>
> > **The project will aim to, "…ensure economic prosperity and stability, while maintaining a high quality of life for all of [the City's] present and future citizens, businesses, and visitors," by proposing to rezone the subject parcel to zoning designation which still complies with its Commercial (COMM) Future Land Use classification. The CI District is established and intended to facilitate highway-oriented**

development opportunities for uses that require high public visibility and an accessible location.

Objective 1.3: Commercial*. The City shall establish three commercial land use categories: Community Commercial, Commercial, and Central Business District. These land use categories shall provide a broad range of commercial uses, including retail sales and services, personal services, offices, and tourist-oriented uses, in order to provide for the availability of goods and services, both to the citizens and visitors of Alachua and to the citizens of the North Central Florida region.*

The proposed rezoning from Central Business District (C-CBD) to Commercial Intensive (CI) will permit a "drive thru" use that will increase the availability of goods and services for both citizens and tourists, traveling to and through the City along Highway 441.

Policy 1.3.b: Commercial. *The Commercial land use category is established to provide for general commercial uses, as well as more intense commercial and highway commercial uses. This is the land use category in which largescale, regional commercial uses may locate. The following uses are allowed within the Commercial land use category:*

1. *Retail sales and services;*
2. *Personal services;*
3. *Financial institutions;*
4. *Outdoor recreation and entertainment;*
5. *Tourist-related uses;*
6. *Hotels and motels;*
7. *Commercial shopping centers;*
8. *Auto-oriented uses;*
9. *Traditional Neighborhood Planned Developments;*
10. *Employment Center Planned Developments;*
11. *Commercial recreation centers;*
12. *Office/business parks;*
13. *Limited industrial services; and,*
14. *Eating establishments.*

Future development of the subject parcel will be consistent with the uses listed above, and will therefore be consistent with the intent of the Commercial (COMM) Future Land Use classification.

Policy 1.3.d: Design and performance standards: *The following criteria shall apply when evaluating commercial development proposals:*

1. Integration of vehicular and non-vehicular access into the site and access management features of site in terms of driveway cuts and cross access between adjacent sites, including use of frontage roads and/or shared access;

2. Buffering from adjacent existing/potential uses;

3. Open space provisions and balance of proportion between gross floor area and site size;

4. Adequacy of pervious surface area in terms of drainage requirements;

5. Placement of signage;

6. Adequacy of site lighting and potential impacts of lighting upon the surrounding area. Lighting should be designed to minimize impacts to preserve the ambiance and quality of the nighttime sky by reducing light trespass and light pollution on adjacent properties by utilizing lighting at appropriate intensities, directions and times to ensure light is not overused or impacting areas where it is not intended;

7. Safety of on-site circulation patterns (patron, employee, and delivery vehicles), including parking layout, drive aisles, and points of conflict;

8. Landscaping, as it relates to the requirements and goals of the Comprehensive Plan and Land Development Regulations;

9. Unique features and resources which may constrain site development such as soils, existing vegetation and historic significance;

10. Performance based zoning requirements, which may serve as a substitute for or accompany land development regulations in attaining acceptable site design; and,

11. Commercial uses shall be limited to an intensity of less than or equal to 0.50 floor area ratio for parcels 5 acres or greater, 0.75 floor area ratio for parcels less than 5 acres but greater than 1 acre, and 1.0 floor area ratio for parcels 1 acre or less.

Future development of the subject parcel will be consistent with the design and performance standards defined under the Commercial (COMM) Future Land Use classification, so as to ensure that the site is developed in a manner that respects and enhances the built environment.

Policy 1.3.e: The creation/promotion of strip pattern commercial development shall be discouraged. Infill within established commercial areas is preferred over extension of a strip commercial pattern. *Extension of a commercial land use designation may be considered in circumstances where the extension is compatible with the existing land uses surrounding the land or the land use categories of surrounding lands. Extensions of a commercial land use category shall not encroach into a residential area. The following criteria shall be applied to limit the impacts of commercial development upon surrounding areas:*

1. Impacts upon traffic circulation should be anticipated and mitigated through the reservation of right-of-way for road widening and marginal access streets when appropriate.

2. Access points for commercial development shall minimize points of conflict by utilizing frontage roads, providing cross access between

parcels, or installing shared use ingress and egress access driveways connections to roadways.

3. Setbacks and landscaped or other appropriate buffers shall be established to mitigate the visual impacts of commercial development upon surrounding land uses.

4. A sidewalk or bicycle path shall be required where appropriate, to provide convenient access to the development from surrounding areas and to reduce traffic volumes on the roadways.

Future development of the subject parcel will be compatible with the existing land uses surrounding the land, and shall be designed in manner that promotes multi-modal transit options that are safe for all, while also providing satisfactory aesthetic and landscaping elements.

Objective 2.4: Landscaping and Tree Protection Standards. *The City shall adopt landscaping and tree protection standards in order to achieve the aesthetic design values of the community and preserve tree canopies, as well as to protect exceptional specimens and champion trees.*

Policy 2.4.a: Landscaping: General. *The City shall require landscaping plans to be submitted with each nonresidential and multiple family residential site plan. The minimum landscaped area shall be 10% of the development site, not inclusive of any designated open space areas. Landscaping designs shall incorporate principles of xeriscaping, where feasible. The City shall develop a list of preferred planting materials to assist in the landscape design. Landscape plans shall include perimeter and internal site landscaping.*

Policy 2.4.b: Landscaping: Buffering. *A buffer consists of horizontal space (land) and vertical elements (plants, berms, fences, walls) that physically separate and visually screen adjacent land uses. The City shall establish buffer yard requirements that are based on the compatibility of the adjacent uses and the desired result of the buffer.*

Future development of the subject parcel will be consistent with the landscaping and buffering standards required for all non-residential uses permitted under the Commercial (COMM) Future Land Use classification.

Policy 2.4.c: Tree Protection: Heritage Trees and Champion Trees. *The City shall require the preservation of heritage trees and champion trees when possible. Standards shall be set for determining the health and safety risks associated with heritage and champion trees both on individual residential lots, and existing and proposed developments.*

Policy 2.4.d: Tree Protection: Regulated Trees. *The City shall establish standards for the preservation of regulated trees. Particular attention shall be given to preserving specimen and preferred species of regulated trees, where feasible.*

Policy 2.4.e Tree Protection: Removal and Mitigation. *Along with establishing standards for tree removal and mitigation, the City shall establish a tree banking program to provide flexibility for re-planting trees through the mitigation process. Funds within the tree bank may be utilized to plant landscaping on city-owned properties, in public parks, and in road rights of way, where appropriate.*

> **Future development of the subject parcel will ensure that all Heritage Trees, Champion Trees, and Regulated trees are fully preserved and protected.**

Objective 2.5: Open Space Standards. *The City shall utilize open space requirements to preserve the rural character of Alachua, protect natural resources, and provide spaces for people to recreate and gather.*

Policy 2.5.a: *There shall be a minimum of 10% percent open space required. The City shall establish incentives for the provision of open space beyond minimum requirements.*

Policy 2.5.b: *Open space shall not be limited to unusable portions of project sites. A portion of open space shall be usable and functional.*

> **Future development of the subject parcel will comply with the City's open space standards that require a minimum 10% of open space, in order to provide spaces for constituents and visitors alike, to recreate, gather, and enjoy the amenities provided on site.**

Objective 2.7: Discouragement of Urban Sprawl. *The City shall discourage the proliferation of urban sprawl as set for the in Chapter 163, Florida Statutes.*

Policy 2.7.a.: *The City shall discourage the proliferation of urban sprawl. Applications for an amendment to the land use category of a property shall include an analysis of the application's discouragement of the proliferation of urban sprawl. Such analysis shall consider the primary indicators or the urban form criteria as set forth in Chapter 163.317(6)(a)9.a. and b., Florida Statutes, respectively.*

> **The proposed project will not encourage urban sprawl, and will instead be part of a logical and cohesive growth pattern of commercial uses along the U.S. 441 Highway. Additionally, since the site is located 715' to the North of residential uses, 200' to the East of Professional Offices, and 200' to the South and West of Commercial Shopping Centers, its development will provide a great opportunity to promote principles of smart growth, and new urbanism, since these uses are all within walking distance to one other.**

Goal 4: Infill and Redevelopment Standards*. The City shall encourage new development and redevelopment to occur within developed areas of the City, such as the Community Redevelopment Area and the Central City Area, to utilize vacant and abandoned properties, prevent blight, and make the best use of available resources.*

Objective 4.2: Community Redevelopment Area. *The City shall encourage development and redevelopment within the Community Redevelopment Area in accordance with the adopted redevelopment plan.*

Policy 4.2.a: *The City shall include the Community Redevelopment Area in its Future Land Use Map Series.*

Policy 4.2.b: *The City shall consider establishing standards and regulations to promote a vibrant mix of land uses, including commercial, residential, and public spaces, establish design guidelines for building renovation and new construction, provide for increased landscaping standards, flexible parking requirements, pedestrian amenities, lighting, and requirements for stormwater management, including opportunities for off-site stormwater management.*

> **The proposed project is located on a parcel of land located within the City's Redevelopment Area ("CRA") as shown within the Future Land Use Map Series. In the future, the applicant will be able to develop a commercial use on the site that will aim to implement the design guidelines for parking, landscaping, pedestrian walkability, and lighting promoted in *Policy 4.2.b.* of the Future Land Use Element.**

Objective 5.2: Availability of facilities and services. *The City shall utilize a concurrency management system to ensure that the adopted levels of service standards are maintained.*

Policy 5.2.a: *All new development shall meet level of service requirements for roadways, potable water, sanitary sewer, stormwater, solid waste, and public schools in accordance with LOS standards adopted in the elements addressing these facilities.*

Policy 5.2.b: *The concurrency management system shall specify the period for which certification of level of service compliance shall be valid until actual development pursuant to a final development order or Chapter 163 Development Agreement has commenced.*

> **The applicant has completed a concurrency analysis for the site, and has determined that any future development will meet satisfactory levels of service with respects to roadways, potable water, sanitary sewer, storm water, solid waste, and other applicable facilities. Since the parcel has a Commercial (COMM) Future Land Use classification, there will be no impacts to Public School facilities.**

Objective 9.1: Connections to Water and Wastewater Systems. *The City will require new development and significant redevelopment projects to connect to the City's potable water and wastewater systems when such connections can be made to such systems.*

Policy 9.1: *Any new development or redevelopment within a Commercial or Industrial land use category within the corporate limits, where potable water and wastewater service are available, as defined in Policy 1.2.a and Policy 4.2.b of the Community Facilities Element of the City's Comprehensive Plan, shall connect to the City's potable water and wastewater system.*

Future development of the site (it is currently vacant), will be required to connect to public potable water and wastewater facilities, as outlined above.

Transportation Element:

The following G.O.P.'s are applicable to the proposed rezoning application:

Goal 1: Traffic Circulation System. *The City shall provide for an efficient traffic circulation system which serves existing and future land uses.*

Objective 1.1: Level of Service. *The City shall establish a safe, convenient, and efficient level of service standard for all motorized and non-motorized transportation systems.*

The project site is located near an adjacent collector roadway with adequate capacity to serve future on-site development and therefore, the proposed zoning change will not result in an increase in traffic impacts that will exceed the adopted Level of Service standards consistent with the Transportation Element.

Objective 1.3: Parking. *The City shall require innovative parking lot design for multiple-family and nonresidential developments, including the provision of bicycle parking, pedestrian friendly design, and landscaping.*

Policy 1.3.a: *The City shall establish minimum and maximum parking standards in order to avoid excessive amounts of underutilized parking areas.*

Policy 1.3.b: *The City shall establish standards that emphasize shared parking and parking reduction incentives to provide flexibility for businesses and to discourage excessive parking areas.*

Policy 1.3.c: *The City shall establish incentives for the use of pervious surfaces in parking lot design for parking which exceeds the minimum parking standards.*

Policy 1.3.d: *The City shall require landscaping within parking areas, with an emphasis on canopy trees. The City shall consider establishing incentives for landscaping in excess of minimum standards.*

Policy 1.3.e: *The City shall establish standards for parking facility design that adequately separates pedestrians from vehicular traffic and delineates pedestrian crossing zones.*

Policy 1.3.f: *The City shall establish bicycle parking facility standards based on type of use within developments.*

Policy 1.3.g: *The City shall require spaces to accommodate persons with physical disabilities as required by the Americans with Disabilities Act.*

Future development of the subject parcel will include an innovative parking lot design that will include provisions for bicycle parking, pedestrian walkways, and landscaping.

Objective 1.4: Bicycle and Pedestrian Standards. *The City shall work to develop a network of bicycle, pedestrian, and other multimodal (non-vehicular) facilities which connect all areas of the City.*

Policy 1.4.a: *The City shall require any development which must obtain site plan or subdivision approval to provide additional right-of-way width for bicycle and pedestrian ways along all proposed collector and arterial streets.*

Policy 1.4.b: *The City shall require any commercial, industrial, or residential development which must obtain site plan or subdivision approval to provide a sidewalk along the frontage of any collector street or arterial street when a sidewalk is not present along the frontage of the street. A sidewalk shall not be required if a sidewalk does not exist within 2,640 feet of the property on the same side of the collector or arterial street.*

Policy 1.4.c: *The City shall consider establishing regulations to encourage safe and continuous multimodal transportation facilities, including bicycle lanes, pedestrian pathways, trails, and multiple-purpose pathways.*

Policy 1.4.d: *The City shall require pedestrian paths within subdivisions and within new developments to be connected to paths outside the development.*

Policy 1.4.e: *The City shall work with the Suwannee River Water Management District, Alachua County, and other public or private entities to implement accessible and functional regional trail systems and transportation plans related to non-vehicular transportation.*

Future development of the subject parcel will include bicycle and pedestrian design features that are safe, accessible, and meant to encourage the creation of continuous multimodal transportation facilities throughout the City.

Community Facilities Element

The following G.O.P.'s are applicable to the proposed rezoning application:

Goal 1: Wastewater. *The City shall plan for and provide adequate and economical wastewater service while protecting the environment, especially groundwater resources.*

Objective 1.2: Wastewater Service. *New development may only occur within areas where wastewater service is available or shall be made available as a part of the proposed development.*

Goal 2: Solid Waste. *The City will provide for solid waste disposal service in a sanitary, economic, and environmentally safe manner.*

Goal 3: Stormwater. *The City shall develop and maintain a stormwater management system that minimizes flooding and protects, preserves, and enhances desirable water quality conditions, and, where possible, preserves and utilizes existing natural features.*

Goal 4: Potable Water. *The City shall provide an adequate supply of potable water to customers throughout the water service area.*

Objective 4.1: Water System Level of Service Standards. *The City shall achieve and maintain acceptable levels of service for potable water quantity and quality.*

Compliance with this Element has been demonstrated within the "Concurrency Management Analysis found herein. Any future development of the subject parcel will be required to connect to both potable water, and wastewater facilities, in a manner consistent with the above goals and objectives. Stormwater management facilities shall also be required at the time of development, per standard engineering principles.

Conservation & Open Space Element

The following G.O.P.'s are applicable to the proposed rezoning application:

Objective 1.1: Conservation Designations. *The City shall provide additional protection to particularly sensitive natural resources, habitats and ecosystems by establishing conservation designations, with specific uses, buffers and management protocol.*

There are no existing or proposed conservation areas within the subject parcel.

Objective 1.2: Native Communities and Ecosystems. *The City shall preserve and protect native communities and ecosystems, particularly those considered endangered or threatened.*

Based on available information, there are no significant native communities or sensitive ecosystems identified on the subject parcel.

Objective 1.3: Listed Species*. The City shall protect species listed by State and Federal agencies as endangered, threatened or of special concern, and their habitats.*

Based on available information, there are no listed species identified on the subject parcel.

Objective 1.4: Air Quality. *The City shall institute the following measures to maintain air quality at the levels established in the National Ambient Air Quality Standards (NAAQS).*

Since there are no development requests associated with this rezoning application, there are no impacts to existing air quality exist. Any future development of the site however, will be sure to comply with the City's air quality standards.

Objective 1.6: Mineral Resources. *The City shall ensure that extraction activities do not pose a threat to natural resources, protected habitats and ecosystems, and human health.*

Based on available information, there are no known mineral resources on the subject parcel.

Objective 1.7: Geological Resources. *The City shall identify, protect and conserve significant geological resources and their natural functions.*

Based on available information, there are no known geographical features identified on the subject parcel.

Objective 1.11: Open and Green Space. *The City shall work to preserve native ecosystems and the natural aesthetic beauty and charm of Alachua by ensuring the provision of open spaces and green linkages throughout the City, designed for the enjoyment of the citizenry.*

Future development of the site shall comply with all open and green space requirements.

Capital Improvement Element

The following G.O.P.'s are applicable to the proposed rezoning application:

Goal 1: Capital Improvements Program. *The City shall maintain a Capital Improvements Program to provide needed public facilities and services to its citizens in a manner that protects investments in existing facilities, maximizes the use of existing facilities, and promotes orderly, compact growth.*

Objective 1.2: Public Facilities Monitoring and Level of Service Standards. *The City shall require that all decisions regarding the issuance of development orders and permits shall be consistent with the development requirements of the Comprehensive Plan, the Land Development Regulations and that public facilities and services necessary to support such development are available while also maintaining the adopted level of service standards.*

Although there is no development proposed as part of this rezoning application, future development activity on the subject parcel will require an updated analysis of the availability of public facilities consistent with adopted LOS standards that are subject to development review, and concurrency analysis by the City. At this time public facilities are currently operating within the acceptable level of service standards outline within the Comprehensive Plan, and as dictated within the Concurrency Impact Analysis provide herein.

SECTION 6: ANALYSIS OF COMPLIANCE WITH STANDARDS FOR SITE SPECIFIC AMENDMENTS TO THE OFFICIAL ZONING ATLAS

Pursuant to *Article 2, Section 2.4.2* of the City's LDR's the applicant has demonstrated in the following ways that the proposed project is consistent with the standards found therein, for a Site Specific Amendment to the Official Zoning Atlas:

i. ***Consistent with Comprehensive Plan.*** *The proposed amendment is consistent with and furthers the goals, objectives and policies of the Comprehensive Plan.*

The proposed project is consistent with the City's Comprehensive Plan as described in the compliance analysis found herein.

ii. ***Consistent with ordinances****. The proposed amendment is not in conflict with any portion of these LDRs or any of the City Code of Ordinances.*

The proposed project fully complies with all applicable LDRs and Code of Ordinances that pertain to the review and approval of Site Specific Amendments to the Official Zoning Map.

iii. ***Logical development pattern.*** *The proposed amendment would result in a logical and orderly development pattern.*

The proposed project will further advance the existing developmental pattern of the area, which currently provides for various commercial uses to the North, South, and West of the subject parcel.

iv. ***Premature development.*** *The proposed amendment will not create premature development in undeveloped or rural areas.*

The proposed project is consistent with the existing commercial developments adjacent to the subject parcel, and will not contribute to the premature development of undeveloped lands. Although the parcel to the Easterly side of the proposed project is vacant, it has a Commercial Intensive (CI) Zoning classification, and a Commercial (COMM) Future Land Use classification.

v. ***Incompatible with adjacent lands.*** *The uses permitted by the proposed amendment are not incompatible with existing land uses of adjacent lands and/or the uses permitted by the zone district classifications of adjacent lands.*

The proposed project is compatible with the existing land uses found on the properties adjacent to the subject parcel. To the North and South, there are currently Community Shopping Centers, whereas the property to the West currently houses a Professional Services Building. As previously mentioned, the property to the East of the subject parcel is vacant, but zoned to permit commercial uses.

i. ***Adverse effect on local character.*** *The proposed amendment will not adversely affect the character of the general area where it is proposed to*

be located by creating excessive traffic, density and/or intensities of use, building height and bulk, noise, lights or other physical effects or nuisances.

The proposed project will not adversely affect the general area in which it is located in, in any way that would negatively impact traffic, density, building height and bulk, noise, lights or any other physical nuisances. The applicant is solely requesting the approval of a Site Specific Amendment to the Official Zoning Atlas, which will fully comply with all the regulations set forth in the City's Comprehensive Plan and LDRs. Any future development of the site will be designed to prohibit the creation of any nuisances that would negatively impact the local character of the general area.

ii. ***Not deviate from pattern of development.*** *The uses permitted by the proposed amendment will not deviate from the development pattern (both established and as proposed by surrounding zone districts) of the area where the proposed amendment is located.*

Any future development of the subject parcel will be based on the uses permitted within the CI Zoning classification, which are commercial in nature. As shown in the Contextual Neighborhood Zoning Analysis, all the surrounding and adjacent uses are also commercial.

iii. ***Encourage sprawl****. The proposed amendment will not encourage urban sprawl, either by resulting in strip or ribbon commercial development, leap-frog development or low-density single dimensional development.*

The proposed project will not encourage urban sprawl, and will instead be part of a logical and cohesive growth pattern of commercial uses along the U.S. 441 Highway. Additionally, since the site is located 715' to the North of residential uses, 200' to the East of Professional Offices, and 200' to the South and West of Commercial Shopping Centers, its development will provide a great opportunity to promote principles of smart growth, and new urbanism, since these uses are all within walking distance of each other.

iv. ***Spot zoning.*** *The proposed amendment will not result in the creation of an isolated zone district unrelated to adjacent and surrounding zone districts (spot zoning).*

The proposed request to amend the Official Zoning Atlas to Commercial Intensive (CI) will result in the parcels to the East and South of the site, both sharing the same Zoning classification as the subject parcel. As a result, the proposed project will not promote the creation of "spot zoning."

v. ***Public facilities.*** *The proposed amendment will not result in development in a location where there are no plans by the City or other governmental entities to provide public facilities to serve the development (roads, potable water, wastewater, parks, stormwater*

management, and solid wastes), and there are no assurances by the private sector that public facilities are planned and will be available to adequately accommodate development.

The proposed project will be developed within an area that currently has sufficient public facilities to serve the site.

vi. ***No adverse effect on the environment.*** *The proposed amendment would not result in significantly adverse impacts on the natural environment, including, but not limited to, water, air, noise, stormwater management, wildlife, vegetation, wetlands, and the natural functioning of the environment.*

The proposed project will not result in any adverse effects to the natural environment. Future development of the site will be designed to ensure that nearby conservation areas, and/or natural resources, will be protected from any proverbial impacts of its growth.

Recommendation

In view of the forgoing analysis, which is based upon the City's Comprehensive Plan and applicable LDR(s), we respectfully recommend approval of the applicant's request to rezone the subject parcel (Parcel No. 03869-004-000) from Central Business District (C-CBD) to Commercial Intensive (CI).

SECTION 7: PROPERTY OWNER LABELS

See **Attachment C** for Property Owner Labels.

SECTION 8: NEIGHBORHOOD MEETING SUMMARY

NEIGHBORHOOD MEETING

A neighborhood meeting was held on 11/12/20 at 6:00 PM. One (1) person attended the meeting. A copy of the Neighborhood Meeting Summary is included in **Attachment D**.

SECTION 9: LEGAL DESCRIPTION WITH TAX PARCEL NUMBERS

LEGAL DESCRIPTION

COM SE COR SEC 9-8-18 S 00 DEG 27 MIN 55 SEC W 3.76 FT N 88 DEG 38 MIN 19 SEC W 790.26 FT N 04 DEG 11 MIN 43 SEC E 49.86 FT S 88 DEG 37 MIN 54 SEC E 50 FT N 04 DEG 11 MIN 43 SEC E 50 FT S 88 DEG 37 MIN 53 SEC E 380.37 FT N 04 DEG 13 MIN 23 SEC E 179.49 FT

See **Attachment E** for the Sketch of Legal Description.

SECTION 10: PROOF OF OWNERSHIP

Copies of the property deeds are provided as proof of ownership. See **Attachment F**.

SECTION 11: PROOF OF PAYMENT OF TAXES

Copies of tax payments are included. See **Attachment G**.

SECTION 12: FEE

A copy of the $4,500.00 check for this rezoning application fee is provided as check no. 1055 from Shakti Management, LLC.

ATTACHMENT A:

REZONING AMENDMENT APPLICATION

City of ALACHUA

THE GOOD LIFE COMMUNITY

FOR PLANNING USE ONLY
Case #: _____
Application Fee: $_____
Filing Date: _____
Acceptance Date: _____
Review Type: P&Z; CC

Site Specific Amendment to the Official Zoning Atlas Amendment (Rezoning) Application

Reference City of Alachua Land Development Regulations Article 2.4.2

A. PROJECT

1. Project Name: Alachua Towne Center Drive-Thru
2. Address of Subject Property: 15560 NW U.S. Highway 441, Alachua FL, 32615
3. Parcel ID Number(s): 03869-004-000
4. Existing Use of Property: Vacant
5. Future Land Use Map Designation : Commercial (COMM)
6. Existing Zoning Designation: Central Business District (C-CBD)
7. Proposed Zoning Designation: Commercial Intensive (CI)
8. Acreage: 1.09 +/-

B. APPLICANT

1. Applicant's Status ☐ Owner (title holder) ☒ Agent
2. Name of Applicant(s) or Contact Person(s): Christopher A. Potts, P.E. Title: Sr. Engineering Manager
   Company (if applicable): JBrown Professional Group Inc.
   Mailing address: 3530 NW 43rd Street
   City: Gainesville State: FL ZIP: 32606
   Telephone: (352) 375-8999 FAX: (352) 375-0833 e-mail: Chris.Potts@jbprogroup.com
3. If the applicant is agent for the property owner*:
   Name of Owner (title holder): Shakti Management LLC c/o Parth Chaudhari
   Mailing Address: 2631 SW 118th Terrace
   City: Gainesville State: FL ZIP: 32606
   * Must provide executed Property Owner Affidavit authorizing the agent to act on behalf of the property owner.

C. ADDITIONAL INFORMATION

1. Is there any additional contact for sale of, or options to purchase, the subject property? ☐ Yes ☒ No
   If yes, list names of all parties involved: _____
   If yes, is the contract/option contingent or absolute? ☐ Contingent ☐ Absolute

D. ATTACHMENTS

1. Statement of proposed change, including a map showing the proposed zoning change and zoning designations on surrounding properties
2. A current aerial map or plat of the property. (may be obtained from the Alachua County Property Appraiser.)
3. Concurrency Impact Analysis showing the impact on public facilities, including potable water, sanitary sewer, transportation, solid waste, recreation, stormwater, and public schools in accordance with Article 2.4.14 of the Land Development Regulations.
4. Analysis of Consistency with the City of Alachua Comprehensive Plan (analysis must identify specific Goals, Objectives, and Policies and describe in detail how the application complies with the noted Goal, Objective, or Policy.)

5. Analysis of compliance with the Standards for Site Specific Amendments to the Official Zoning Atlas, as defined in Section 2.4.2 of the Land Development Regulations (LDRs), and listed below:

   i. Consistent with Comprehensive Plan
   The proposed amendment is consistent with and furthers the goals, objectives, and policies of the Comprehensive Plan.

   ii. Consistent with Ordinances
   The proposed amendment is not in conflict with any portion of these LDRs or any of the City Code of Ordinances.

   iii. Logical Development Pattern
   The proposed amendment would result in a logical and orderly development pattern.

   iv. Pre-Mature Development
   The proposed amendment will not create premature development in undeveloped or rural areas.

   v. Incompatible with Adjacent Lands
   The uses permitted by the proposed amendment are not incompatible with existing land uses of adjacent lands and/or the uses permitted by the zone district classifications of adjacent lands.

   vi. Adverse Effect on Local Character
   The proposed amendment will not adversely effect the character of the general area where it is proposed to be located by creating excessive traffic, density and/or intensities of use, building height and bulk, noise, lights, or other physical effects or nuisances.

   vii. Not Deviate from Pattern of Development
   The uses permitted by the proposed amendment will not deviate from the development pattern (both established and as proposed by surrounding zone districts) of the area where the proposed amendment is located.

   viii. Encourage Sprawl
   The proposed amendment will not encourage urban sprawl, either by resulting in strip or ribbon commercial development, leap-frog development or low density single dimensional development.

   ix. Spot Zoning
   The proposed amendment will not result in the creation of an isolated zone district unrelated to adjacent and surrounding zone districts (spot zoning).

   x. Public Facilities
   The proposed amendment will not result in development in a location where there are no plans by the City or other governmental entities to provide public facilities to serve the development (roads, potable water, wastewater, parks, storm water management, and solid wastes), and there are no assurances by the private sector that public facilities are planned and will be available to adequately accommodate development.

   xi. No Adverse Effect on the Environment
   The proposed amendment would not result in significantly adverse impacts on the natural environment, including but not limited to water, air, noise, storm water management, wildlife, vegetation, wetlands, and the natural functioning of the environment.

6. Three (3) sets of labels for all property owners within 400 feet of the subject property boundaries – even if property within 400 feet falls outside of City limits. (Obtain from the Alachua County Property Appraiser).

7. Neighborhood Meeting Materials, including:
   i. Copy of the required published notice (advertisement) – must be published a newspaper of general circulation, as defined in Article 10 of the City's Land Development Regulations
   ii. Copy of written notice (letter) sent to all property owners within 400 feet, and mailing labels or list of those who received written notice
   iii. Written summary of meeting – must include (1) those in attendance; (2) a summary of the issues related to the development proposal discussed; (3) comments by those in attendance about the development proposal; and, (4) any other information deemed appropriate.

8. For applications requesting a zoning which permits residential uses, Public School Student Generation Form.

9. Legal description with tax parcel number.

10. Proof of ownership.

11. Proof of payment of taxes.

12. **Fee.** Please see fee schedule for fee determination. No application shall be accepted for processing until the required application fee is paid in full by the applicant. Any necessary technical review will be billed to the applicant at the rate of the reviewing entity. The invoice shall be paid in full prior to any legislative and/or quasi-judicial action of any kind on the petition, appeal, or development application.

All 12 attachments are required for a complete application. A completeness review of the application will be conducted within five (5) business days of receipt. If the application is determined to be incomplete, the application will be returned to the applicant.

I/We certify and acknowledge that the information contained herein is true and correct to the best of my/our knowledge.

Signature of Applicant

Signature of Co-applicant

Parth Chaudhari Owner

Typed or printed name and title of applicant

Typed or printed name of co-applicant

State of Florida County of Alachua

The foregoing application is acknowledged before me this 2nd day of Nov, 2023 by _____

_____, who is/are personally known to me, or who has/have produced _____

as identification.

NOTARY SEAL

Maximillian I. Palmer
NOTARY PUBLIC
STATE OF FLORIDA
Comm# GG964843
Expires 3/3/2024

Signature of Notary Public, State of Florida

ATTACHMENT C:

PROPERTY OWNER LABELS

McCauley Properties LLC
3727 SW 92nd Drive
Gainesville, FL 32608

Young Estates LLC
14414 NW 60th Avenue
Alachua, FL 32615

Starmax FDA LLC
C/O Family Dollar Stores Inc.
500 Volvo Parkway
Chesapeake, VA 23320

Canvas Church FL INC
P.O. Box 219
Alachua, FL 32616

Hitchcock & Sons INC
29220 NW 122nd Street
Alachua, FL 32615

Wiggins Ethel J Trustee
P.O. Box 1857
Alachua, FL 32616

Alachua Towne Centre Association INC
13505 NW 88th Place
Alachua, FL 32615

Advance Stores Co INC #9236
P.O. Box 2710 ATTN: TAX ACCOUNTING
Roanoke, VA 24001

Tomlinson Property Group LLC
3280 N. Main Street
Gainesville, FL 32609

Alachua Professional Plaza LLC
15260 NW 147th Drive, Suite A
Alachua, FL 32615

Campus USA Credit Union
14007 NW 1st Road
Newberry, FL 32669

Christopher Allan Korosic
15710 NW US Highway 441
Alachua, FL 32615

ATTACHMENT D:

NEIGHBORHOOD MEETING MATERIALS

JBrown Professional Group
CIVIL ENGINEERING • LAND SURVEYING • PLANNING
3530 NW 43rd Street • Gainesville, FL 32606 • 352.375.8999 • JBProGroup.com

Neighborhood Workshop Summary For The Rezoning Application Of Alachua County Tax Parcel No. 3869-004-000

The neighborhood workshop for the Development Review Application of Alachua County Tax Parcel No. 03869-004-000 was held on Thursday November 12th, 2020 at 6pm digitally as a Zoom conference. The workshop was noticed in the Alachua County Today newspaper and mailers were sent out to the required property owners in advance of the workshop in accordance with City of Alachua regulations and requirements. Chris Potts and Logan Peters of JBrown Professional Group Inc. were in attendance to manage the workshop.

The Zoom conference began promptly at 6:00 pm. A PowerPoint Presentation containing visual references of the Alachua County Future Land Use Map, Zoning Map and the subdivision layout were available for attendees to view. Mr. Potts gave the presentation and Mrs. Peters attended and took notes. The only attendee was the applicant, Dhairya Chaudhari. Mr. Potts waited until about 6:20pm to begin the presentation in the event neighbors arrived late. The presentation was discussed, and the Zoom meeting was recorded to allow neighbors to view online at a later date.

The neighborhood meeting was adjourned at approximately 6:25 pm. The following items are attached to further document the Neighborhood Workshop.

1) Copy of the PowerPoint Presentation used for visual aid during the conference
2) Copy of Newspaper Advertisement
3) Copy of Public Notice Mailed

JBrown Professional Group
CIVIL ENGINEERING • LAND SURVEYING • PLANNING
3530 NW 43rd Street • Gainesville, FL 32606 • 352.375.8999 • JBProGroup.com

Neighborhood Workshop Invitation

You are cordially invited to attend a Neighborhood Workshop as described below.

A Neighborhood Workshop will be held to discuss an application for a Rezoning of Alachua County Tax Parcel No. 03869-004-000. The parcel is located in front of the Hitchcock's Grocery Store on US-441. The property is approximately 1.09 acres in size and currently has a Zoning designation of CBD (Commercial – Central Business District). The application is to change the Zoning designation to CI (Commercial Intensive). The workshop will be held virtually as a Zoom video conference. The purpose of the Neighborhood Workshop is to inform neighboring property owners about the nature of the application and to seek comments. The details to log into the Zoom video conference are provided below.

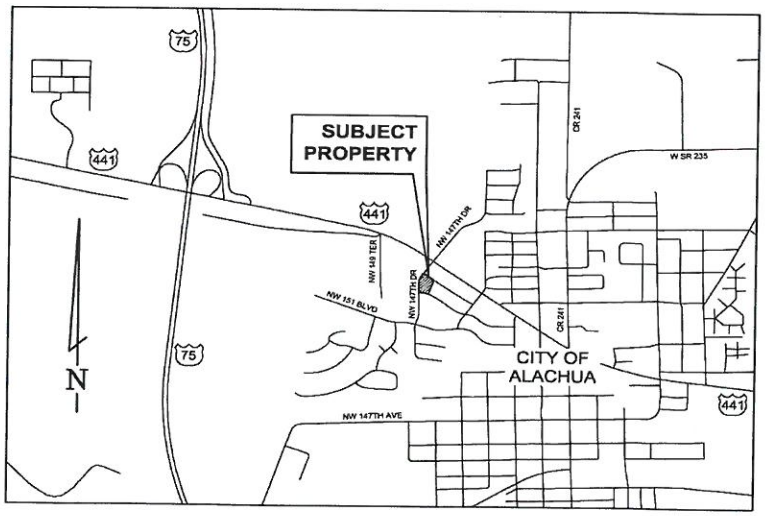
Workshop Information: The workshop will be held virtually as a Zoom video conference. The details to log into the Zoom video conference are provided below.

Date: Thursday November 12, 2020
Time: 6:00 PM
Web link: **https://us02web.zoom.us/j/83371110791**
Zoom Meeting ID: 833 7111 0791
Telephone Call-in No.: 1 (929) 205-6099

Contact Person: Chris Potts, PE, JBrown Professional Group Inc.
(352) 375-8999 chris.potts@jbprogroup.com

This is not a public hearing. The purpose of the workshop is to inform neighboring property owners about the nature of the application and to seek comments. Please feel free to reach out to JBrown Professional Group if you have any questions regarding the workshop or the Zoom conference. A recording of the workshop will be available at the JBrown Professional Group YouTube Channel (https://www.youtube.com/channel/UCZic3JnkYOjx1W0b2fkgZuw/videos). Workshop materials can also be mailed upon request.

LOCATION MAP



Mailed October 28, 2020

JBrown Professional Group Inc.
3530 NW 43rd Street
Gainesville, FL 32606

B83846 02
$0.50 0
US POSTAGE
FIRST-CLASS
062S0000883215
32606

stamps.com stamp2.com

J Brown Professional Group
3530 NW 43rd Street
Gainesville, FL 32606

326068104 R014

Neighborhood Workshop

FOR THE

Rezoning of Parcel 03869-004-000 Located at Alachua Towne Center (Hitchcock's Plaza)

15560 NW U.S. Highway 441

November 12, 2020

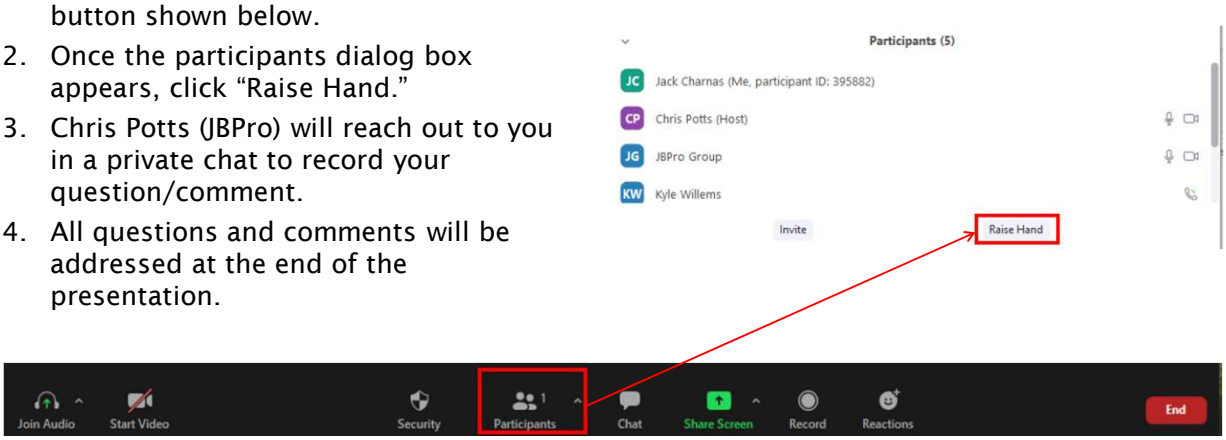
JBrown PROFESSIONAL GROUP

Presentation Outline

1. Zoom Communication Instructions
2. Project Location
3. Zoning
4. Conceptual Site Plan
5. Neighborhood Considerations

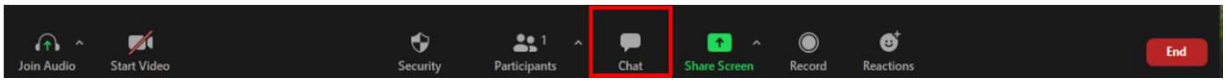
Zoom Instructions: Raise Hand

1. If you have a question or would like to comment, please click the participants button shown below.
2. Once the participants dialog box appears, click "Raise Hand."
3. Chris Potts (JBPro) will reach out to you in a private chat to record your question/comment.
4. All questions and comments will be addressed at the end of the presentation.



Zoom Instructions: Chat

1. To respond to Chris Potts's chat, please click the chat button shown in red below.



2. The chat box should appear on the side of your screen. Please ensure the recipient is "Chris Potts" as shown below. If Chris is not the recipient, you can edit the recipient by clicking the dropdown arrow above the dialog box as shown below.



Project Location

General Information

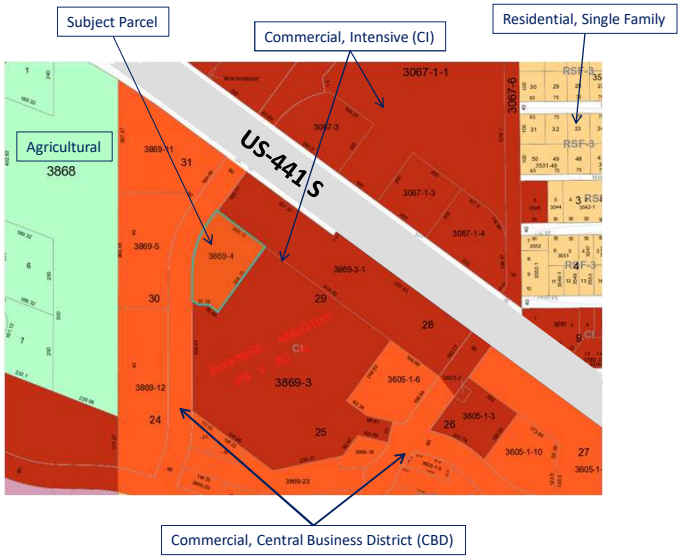
- Parcel 03869-004-000
- 1.09 total acres

Surrounding Properties

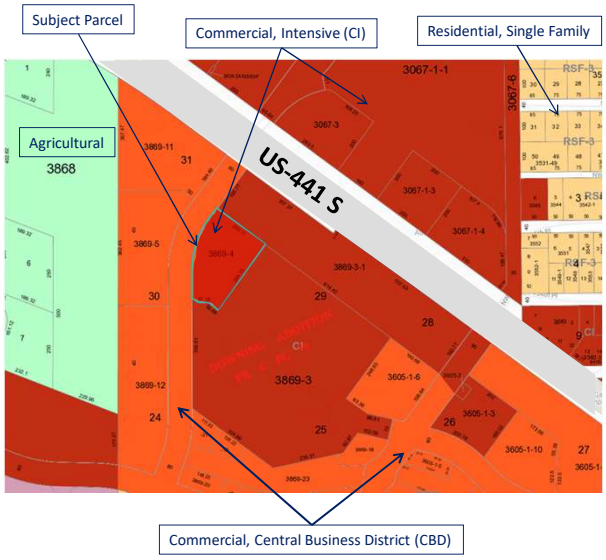
- Hitchcock's Grocery Store
- McCauley and Associates Medical Group



Existing Zoning



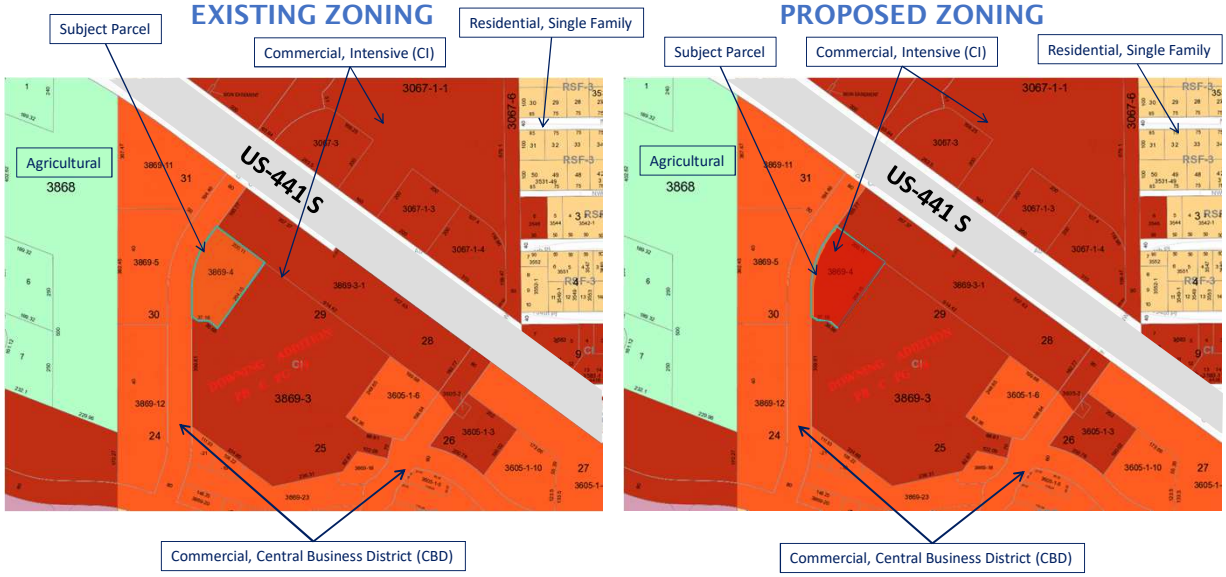
Proposed Zoning



Existing & Proposed Zoning

EXISTING ZONING

PROPOSED ZONING



Conceptual Site Plan



Proposed Next Steps

- ✓ Submit Applications to City of Alachua for Rezoning (November 2020)
- ✓ Planning & Zoning Hearing (February 2021)
- ✓ Board of City Commissioners (BOCC) Public Hearings:
- ✓ (1st) Transmittal Hearing & (2nd) Adoption Hearing (March/April 2021 & April/May 2021)
- ✓ Site Plan Application – July/August 2021
- ✓ Obtain All Permits – City of Alachua, SRWMD (EOY 2021)
- ✓ Begin Construction (EOY 2021 / BOY 2022)

Questions & Comments

Chris Potts – chris.potts@jbprogroup.com
(352) 375-8999 x 113

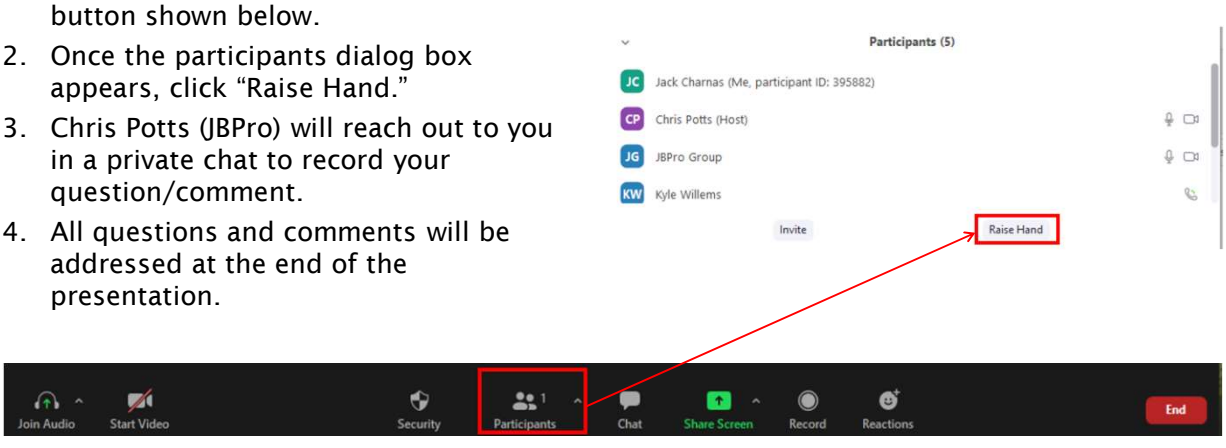
Logan Peters– logan.peters@jbprogroup.com
(352) 375-8999 x 105

JBPro YouTube Channel Link:
https://www.youtube.com/channel/UCZic3JnkYOjx1W0b2fkgZuw

How to ask questions or comment:
Please use the "Raise hand" feature under the participants tab on your bottom bar and someone will reach out to you in the Chat to get your input.

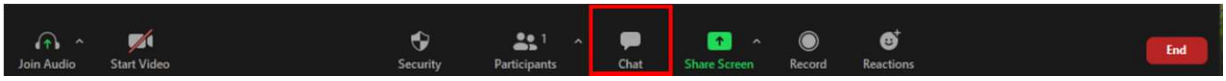
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ATTACHMENT E:

SKETCH OF LEGAL DESCRIPTION

SKETCH OF LEGAL DESCRIPTION

IN FRACTIONAL SECTION 15, TOWNSHIP 8 SOUTH, RANGE 18 EAST, ALACHUA COUNTY, FLORIDA

(NOT A BOUNDARY SURVEY)

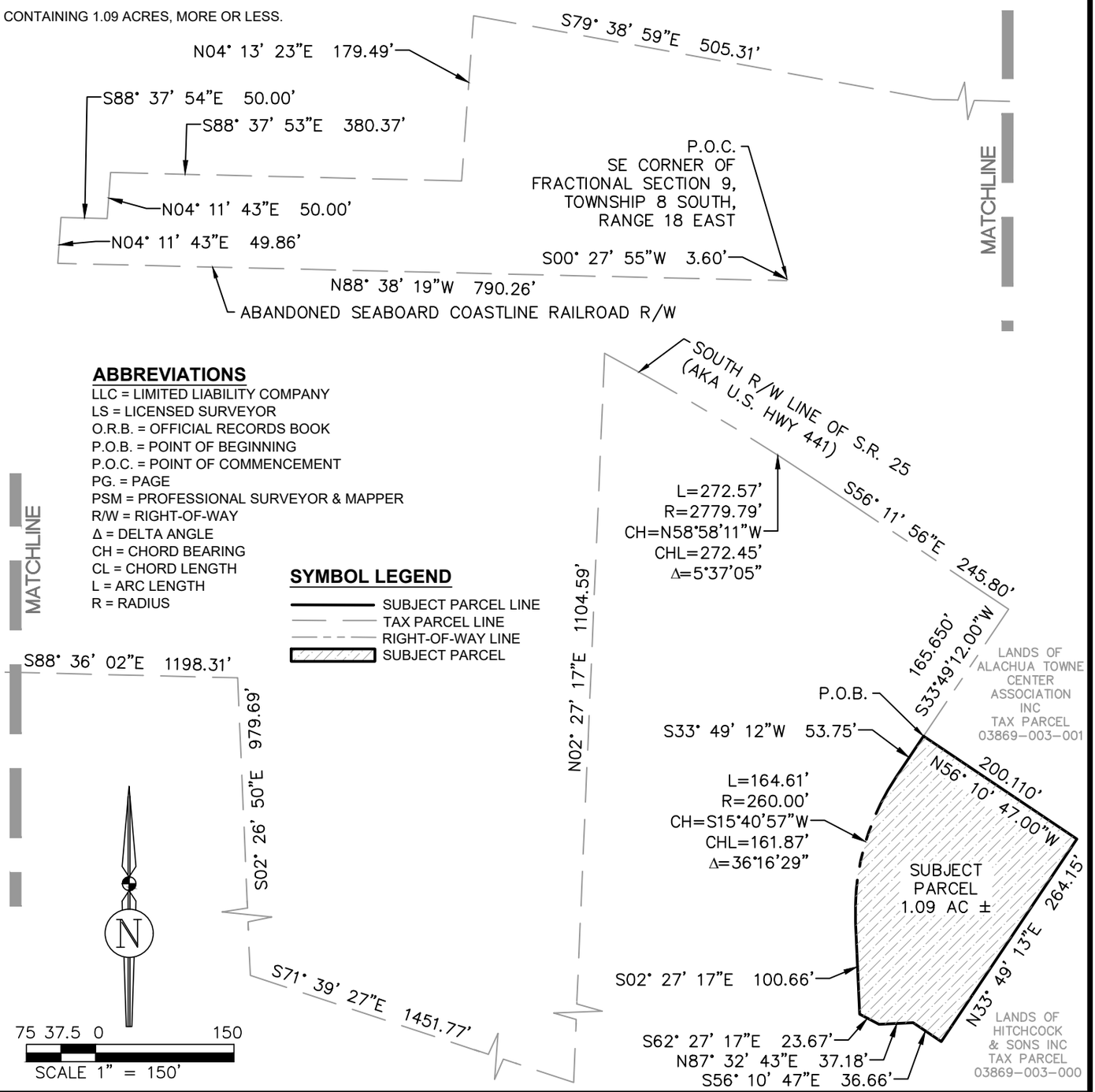
Tax Parcel 03869-004-000

LEGAL DESCRIPTION - PER O.R.B.4737, PG. 2345

A TRACT OF LAND SITUATED IN FRACTIONAL SECTION 15, TOWNSHIP 8 SOUTH, RANGE 18 EAST, CITY OF ALACHUA, ALACHUA COUNTY, FLORIDA, SAID TRACT OF LAND BEING MORE PARTICULARLY DESCRIBED AS FOLLOWS:

COMMENCE AT A CONCRETE MONUMENT AT THE SOUTHEAST CORNER OF THE AFOREMENTIONED FRACTIONAL SECTION 9, TOWNSHIP 8 SOUTH, RANGE 18 EAST FOR THE POINT OF REFERENCE AND RUN S00°27'55"W, A DISTANCE OF 3.76 FEET TO THE SOUTHERLY RIGHT OF WAY LINE OF THE ABANDONED SEABOARD COASTLINE RAILROAD (200 FOOT RIGHT OF WAY); THENCE RUN N88°38'19"W, ALONG SAID SOUTHERLY RIGHT OF WAY LINE, A DISTANCE OF 790.26 FEET TO THE INTERSECTION OF SAID SOUTHERLY RIGHT OF WAY LINE WITH THE EASTERLY RIGHT OF WAY LINE OF INTERSTATE HIGHWAY NO. 75 (300 FOOT LIMITED ACCESS RIGHT OF WAY); THENCE RUN N04°11'43"E, ALONG SAID EASTERLY RIGHT OF WAY LINE, A DISTANCE OF 49.86 FEET TO A STEEL ROD AND CAP; THENCE RUN S88°37'54"E, A DISTANCE OF 50.00 FEET; THENCE RUN N04°11'43"E. 50 FEET TO THE CENTERLINE OF THE AFOREMENTIONED ABANDONED SEABOARD COASTLINE RAILROAD; THENCE RUN S88°37'53"E, ALONG SAID CENTERLINE, A DISTANCE OF 380.37 FEET TO A STEEL ROD AND CAP AT THE SOUTHWEST CORNER OF THAT CERTAIN PARCEL OF LAND AS DESCRIBED IN OFFICIAL RECORD BOOK 1610, PAGE 2540 OF THE PUBLIC RECORDS OF ALACHUA COUNTY, FLORIDA, SAID PARCEL OF LAND BEING HEREINAFTER REFERRED TO AS PARCEL "A"; THENCE RUN N04°13'23"E, A DISTANCE OF 179.49 FEET TO A STEEL ROD AND CAP AT THE NORTHWEST CORNER OF SAID PARCEL "A"; THENCE RUN S79°38'59"E, ALONG THE NORTH LINE OF SAID PARCEL "A", A DISTANCE OF 505.31 FEET TO A STEEL ROD AND CAP AT THE NORTHEAST CORNER OF SAID PARCEL "A"; THENCE RUN S88°36'02"E, ALONG THE NORTH RIGHT OF WAY LINE OF THE AFOREMENTIONED ABANDONED SEABOARD COASTLINE RAILROAD, A DISTANCE OF 1198.31 FEET TO A CONCRETE MONUMENT AT THE INTERSECTION OF SAID NORTH RIGHT OF WAY LINE WITH THE NORTHERLY PROJECTION OF THE WEST LINE OF THAT CERTAIN PARCEL OF LAND DESCRIBED IN DEED BOOK 335, PAGE 461 OF SAID PUBLIC RECORDS, SAID PARCEL OF LAND BEING HEREINAFTER REFERRED TO AS PARCEL "B"; THENCE RUN S02°26'50"E, ALONG SAID NORTHERLY PROJECTION AND ALONG SAID WEST LINE OF PARCEL "B", A DISTANCE OF 979.69 FEET TO A CONCRETE MONUMENT AT THE SOUTHWEST CORNER OF SAID PARCEL "B": THENCE RUN S71°39'27"E, A DISTANCE OF 1104.59 FEET TO A CONCRETE MONUMENT AT THE INTERSECTION OF SAID EAST LINE WITH THE SOUTH LINE OF THE ROADWAY AND DRAINAGE EASEMENT AS SHOWN OF THE FLORIDA DEPARTMENT OF TRANSPORTATION RIGHT OF WAY PLANS FOR STATE ROAD NO. 25 (ALSO KNOWN AS U.W. HIGHWAY NO. 441); THENCE RUN SOUTHEASTERLY, ALONG SAID SOUTH LINE AND WITH A CURVE CONCAVE SOUTHERLY, SAID CURVE HAVING A RADIUS OF 2779.79 FEET, THROUGH AN ARC ANGLE OF 05°37'05", AN ARC DISTANCE OF 272.57 FEET (CHORD BEARING AND DISTANCE OF S58°58'11"E, 272.45 FEET RESPECTIVELY) TO A CONCRETE MONUMENT AT THE END OF SAID CURVE; THENCE RUN S56°11'56"E, ALONG SAID SOUTH LINE, A DISTANCE OF 245.80 FEET; THENCE DEPARTING SAID SOUTH LINE RUN S33°49'12"W, 165.65 FEET TO THE POINT OF BEGINNING; THENCE CONTINUE S33°49'12"W, 53.75 FEET TO THE POINT OF CURVATURE OF A CURVE TO THE LEFT, CONCAVE EASTERLY, AND HAVING A RADIUS OF 260.00 FEET; THENCE RUN SOUTHWESTERLY, ALONG THE ARC OF SAID CURVE THROUGH A CENTRAL ANGLE OF 36°16'29", AN ARC DISTANCE OF 164.61 FEET TO THE POINT OF TANGENCY; THENCE RUN S02°27'17"E, 100.66 FEET; THENCE RUN S62°27'17"E, 23.67 FEET; THENCE RUN N87°32'43"E. 37.18 FEET; THENCE RUN S56°10'47"E, 36.66 FEET; THENCE RUN N33°49'13"E, 264.15 FEET; THENCE RUN N56°10'47"W, 200 FEET TO THE POINT OF BEGINNING.

CONTAINING 1.09 ACRES, MORE OR LESS.



ABBREVIATIONS

LLC = LIMITED LIABILITY COMPANY
LS = LICENSED SURVEYOR
O.R.B. = OFFICIAL RECORDS BOOK
P.O.B. = POINT OF BEGINNING
P.O.C. = POINT OF COMMENCEMENT
PG. = PAGE
PSM = PROFESSIONAL SURVEYOR & MAPPER
R/W = RIGHT-OF-WAY
Δ = DELTA ANGLE
CH = CHORD BEARING
CL = CHORD LENGTH
L = ARC LENGTH
R = RADIUS

SYMBOL LEGEND

SUBJECT PARCEL LINE
TAX PARCEL LINE
RIGHT-OF-WAY LINE
SUBJECT PARCEL

Prepared By:
JBrown Professional Group Inc
CIVIL ENGINEERING • LAND SURVEYING • PLANNING
3530 NW 43rd Street • Gainesville, Florida 32606
PHONE: (352) 375-8999 • FAX: (352) 375-0933
E-MAIL: contact@jbprogroup.com

THE MAP OF THE PROPERTY DESCRIBED HEREON WAS MADE UNDER MY SUPERVISION AND THIS MAP OF SURVEY FURTHER MEETS THE STANDARDS OF PRACTICE SET FORTH BY THE STATE OF FLORIDA BOARD OF PROFESSIONAL SURVEYORS & MAPPERS IN CHAPTER 5J-17.05, FLORIDA ADMINISTRATIVE CODE, PURSUANT TO SECTION 472.027, FLORIDA STATUTES, AND THE MAP OF SURVEY SHOWN HEREON IS A TRUE AND ACCURATE REPRESENTATION THEREOF TO THE BEST OF MY KNOWLEDGE, BEING SUBJECT TO NOTES AND NOTATIONS SHOWN HEREON.

Florida License No. LS7210
Certificate of Authorization No. LB8031

NOT VALID WITHOUT THE SIGNATURE AND ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR & MAPPER OR VALID DIGITAL SIGNATURE IN ELECTRONIC FORM

TROY V WRIGHT, PSM
Professional Surveyor and Mapper

CERTIFIED TO:

1.

Scale:	1"=150'
Drawn By:	T.Hensley
Document Name:	436-20-01-SL1
Document Date:	11/20/2020
Sheet:	1 of 1

ATTACHMENT F:

PROOF OF OWNERSHIP

RECORDED IN OFFICIAL RECORDS
INSTRUMENT # 3230773 3 PG(S)
12/9/2019 10:56 AM
BOOK 4737 PAGE 2345
J.K. JESS IRBY, ESQ.
Clerk of the Court, Alachua County, Florida
ERECORDED Receipt # 924737
Doc Stamp-Mort: $0.00
Doc Stamp-Deed: $1,960.00
Intang. Tax: $0.00

Prepared By and Return To:

Mowitz Title, LLC
5341 SW 91st Terrace Suite A
Gainesville, FL 32608

For the issuance of title insurance, file #: 1941207

[Space Above This Line For Recording Data]

CORPORATE WARRANTY DEED

This Corporate Warranty Deed made **November 25, 2019** between

Grantor: Ameris Bank, a Foreign Profit Corporation — mailing address: 7825 Baymeadows Way, Suite 200-B, Jacksonville, FL 32556

Grantee: Shakti Management, LLC, a Florida limited liability company — mailing address: 2631 SW 118th Terrace, Gainesville, FL 32608

Witnesseth, that said grantor, for and in consideration of the sum of Ten Dollars and no/cents ($10.00) and other good and valuable considerations to said grantor in hand paid by said grantee, the receipt whereof is hereby acknowledged, has granted, bargained, and sold to the said grantee, and grantee's heirs and assigns forever, the following described land, situate, lying and being in Alachua County, Florida to-wit:

See Exhibit "A" attached hereto and made a part thereof.

Parcel No. 03869-004-000

Grantor warrants that at the time of this conveyance, the subject property is not the Grantor's homestead within the meaning set forth in the constitution of the state of Florida, nor is it contiguous to or a part of homestead property.

Subject to easements, restrictions, reservations and limitations of record, if any.

Together with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining

To Have and to Hold, the same in fee simple forever.

And the grantor hereby covenants with said grantee that the grantor is lawfully seized of said land in fee simple; that the grantor has good right and lawful authority to sell and convey said land; that the grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances, except taxes accruing subsequent to December 31, 2019.

□

In Witness Whereof, grantor has hereunto set grantor's hand and seal the day and year first above written.

Signed, sealed and delivered in our presence:

x Juanita Robinson

Witness Name: Juanita Robinson

x

Witness Name: Sonja Mosley

Robert W. "Skip" Mullins, as Senior Vice President of Ameris Bank, a Foreign Profit Corporation

STATE OF Florida
COUNTY OF Duval

This instrument was sworn to and acknowledged before me on 20 day of November, 2019, by Robert W. "Skip" Mullins, as Senior Vice President of Ameris Bank, a Foreign Profit Corporation , who produced a driver's license as identification.

x

(Notary Seal)

NADINE F MCGUNIGALE
Notary Public - State of Florida
Commission # GG 199112
My Comm. Expires Mar 27, 2022
Bonded through National Notary Assn.

EXHIBIT A
LEGAL DESCRIPTION

Issuing Office File No. 1941207

A Tract of land situated in fractional Sections 15, Township 8 South, Range 18 East, City of Alachua, Alachua County, Florida, said Tract of land being more particularly described as follows:

Commence at a concrete monument at the Southeast corner of the aforementioned fractional Section 9, Township 8 South, Range 18 East for the Point of Reference and run South 00 degrees 27 minutes 55 seconds West, a distance of 3.76 feet to the Southerly right of way line of the Abandoned Seaboard Coastline Railroad (200 foot right of way); thence run North 88 degrees 38 minutes 19 seconds West, along said Southerly right of way line, a distance of 790.26 feet to the intersection of said Southerly right of way line with the Easterly right of way line of Interstate Highway No. 75 (300 foot limited access right of way); thence run North 04 degrees 11 minutes 43 seconds East , along said Easterly right of way line, a distance of 49.86 feet to a steel rod and cap; thence run South 88 degrees 37 minutes 54 seconds East, a distance of 50.00 feet; thence run North 04 degrees 11 minutes 43 seconds East, 50.00 feet to the centerline of the aforementioned abandoned seaboard coastline railroad; thence run South 88 degrees 37 minutes 53 seconds East, along said centerline, a distance of 380.37 feet to a steel rod and cap at the Southwest corner of that certain Parcel of land as described in Official Record Book 1610, Page 2540 of the Public Records of Alachua County, Florida, said Parcel of land being hereinafter referred to as Parcel "A"; thence run North 04 degrees 13 minutes 23 seconds East, a distance of 179.49 feet to a steel rod and cap at the Northwest corner of said Parcel "A"; thence run South 79 degrees 38 minutes 59 seconds East, along the North line of said Parcel "A", a distance 0f 505.31 feet to a steel rod and cap at the Northeast corner of said Parcel "A"; thence run South 88 degrees 36 minutes 02 seconds East, along the North right of way line of the aforementioned abandoned seaboard coastline railroad, a distance of 1198.31 feet to a concrete monument at the intersection of said North right of way line with the Northerly projection of the West line of that certain Parcel of land as described in Deed Book 335, Page 461 of said Public Records, said Parcel of land being hereinafter referred to as Parcel "B"; thence run South 02 degrees 26 minutes 50 seconds East, along said Northerly projection and along said West line of Parcel "B", a distance of 979.69 feet to a concrete monument at the Southwest corner of said Parcel "B"; thence run South 71 degrees 39 minutes 27 seconds East, a distance of 1451.77 feet to a concrete monument at the Southeast corner of said Parcel "B"; thence run North 02 degrees 27 minutes 17 seconds West, along the East line of said Parcel "B", a distance of 1104.59 feet to a concrete monument at the intersection of said East line with the South line of the roadway and drainage easement as shown on the Florida Department of Transportation right of way plans for State Road No.25 (also known as U.S Highway No. 441); thence run Southeasterly, along said South line and with a curve concave Southerly, said curve having a radius of 2779.79 feet, through an arc angle of 05 degrees 37 minutes 05 seconds, an arc distance of 272.57 feet (chord bearing and distance of South 58 degrees 58 minutes 11 seconds East, 272.45 feet respectively) to a concrete monument at the end of said curve; thence run South 56 degrees 11 minutes 56 seconds East, along said South line, a distance of 245.80 feet; thence departing said South line run South 33 degrees 49 minutes 12 seconds West, 165.65 feet to the Point of Beginning; thence continue South 33 degrees 49 minutes 12 seconds West, 53.75 feet to the Point of Curvature of a curve to the left, concave Easterly, and having a radius of 260.00 feet; thence run Southwesterly, along the arc of said curve through a central angle of 36 degrees 16 minutes 29 seconds, an arc distance of 164.61 feet to the Point of Tangency; thence run South 02 degrees 27 minutes 17 seconds East, 100.66 feet; thence run South 62 degrees 27 minutes 17 seconds East, 23.67 feet; thence run North 87 degrees 32 minutes 43 seconds East, 37.18 feet; thence run South 56 degrees 10 minutes 47 seconds East, 36.66 feet; thence run North 33 degrees 49 minutes 13 seconds East, 264.15 feet; thence run North 56 degrees 10 minutes 47 seconds West, 200.11 feet to the Point of Beginning.

ATTACHMENT G:

PROOF OF PAYMENT OF TAXES

ALATB15081F

John Power TAX COLLECTOR Serving Alachua County

2019 PAID REAL ESTATE 15057

NOTICE OF AD VALOREM TAXES AND NON-AD VALOREM ASSESSMENTS

ACCOUNT NUMBER	ESCROW CD		MILLAGE CODE
03869 004 000		APPLICABLE VALUES AND EXEMPTIONS BELOW	1700

SHAKTI MANAGEMENT LLC
2631 SW 118TH TER
GAINESVILLE, FL 32608

15560 NW US HWY 441

COM SE COR SEC 9-8-18 S 00 DEG 27 MIN 55 SEC W 3.76 FT N
See Additional Legal on Tax Roll

AD VALOREM TAXES

TAXING AUTHORITY	MILLAGE RATE	ASSESSED VALUE	EXEMPTION(S)	TAXABLE VALUE	TAXES LEVIED
BOARD OF COUNTY COMMISSIONERS					
CNTY GENERAL	8.2729	332,363	0	332,363	2,749.61
ALACHUA CNTY LIBRARY DISTRICT					
LIBRARY GENERAL	1.1825	332,363	0	332,363	393.02
SCHOOL BOARD OF ALACHUA COUNTY					
SCHL CAP35 PROJECT (S01)	1.5000	332,363	0	332,363	498.54
SCHL DISCRNRY & CN (S01)	0.7480	332,363	0	332,363	248.61
SCHL GENERAL	3.8960	332,363	0	332,363	1,294.89
SCHOOL VOTED (S01)	1.0000	332,363	0	332,363	332.36
SUWANNEE RIVER WATER MGT DIST	0.3840	332,363	0	332,363	127.63
17 CITY OF ALACHUA	5.3900	332,363	0	332,363	1,791.44
CHILDREN'S TRUST	0.5000	332,363	0	332,363	166.18
TOTAL MILLAGE	22.8734		**AD VALOREM TAXES**		$7,602.28

WANT TO RECEIVE YOUR BILL ELECTRONICALLY NEXT YEAR? VISIT www.AlachuaCollector.com AND SIGN UP FOR E-BILLS!

PAY ONLINE WITH E-CHECK

SCAN TO PAY

NON-AD VALOREM ASSESSMENTS

LEVYING AUTHORITY		UNIT	RATE	AMOUNT
C550	550 COUNTY FIRE SERVICES	1.000	Varies	83.34
NON-AD VALOREM ASSESSMENTS				$83.34

PAY ONLY ONE AMOUNT.

COMBINED TAXES AND ASSESSMENTS $7,685.62

If Paid By	Dec 31, 2019				
Please Pay	$0.00				

Please Retain this Portion for your Records. Receipt Available Online.

JOHN POWER, CFC
ALACHUA COUNTY TAX COLLECTOR

2019 PAID REAL ESTATE 15057

NOTICE OF AD VALOREM TAXES AND NON-AD VALOREM ASSESSMENTS

PLEASE PAY IN U.S. FUNDS (NO POSTDATED CHECKS) TO JOHN POWER, TAX COLLECTOR • 12 SE 1st ST, GAINESVILLE, FL 32601

ACCOUNT NUMBER	SITUS	MESSAGE
03869 004 000	15560 NW US HWY 441	

SHAKTI MANAGEMENT LLC
2631 SW 118TH TER
GAINESVILLE, FL 32608

IF PAID BY	PLEASE PAY
☐ Dec 31, 2019	$0.00
☐	
☐	
☐	
☐	

Receipt # 19-0059732 $7,455.05 Paid 12/12/2019

ATTACHMENT H:

MPH – TRAFFIC IMPACT STUDY

TRAFFIC IMPACT STUDY
Alachua Towne Center
Retail Expansion
Alachua, Florida

February 11, 2021

prepared for:

CITY OF ALACHUA PLANNING DEPARTMENT

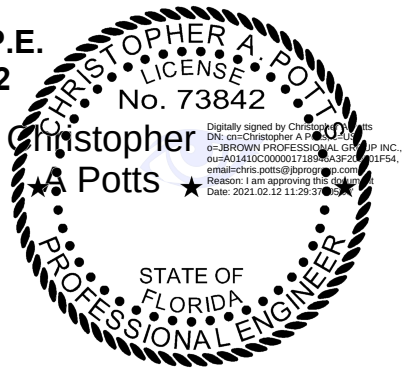
submitted on behalf of:

Shakti Management LLC

prepared by:

MPH Transportation Planning & JB Professional Group

QA/QC Engineer:
Christopher Potts
Christopher Potts, P.E.
FLA License # 73842



Signature of Preparer:
Michael Hemmen
Michael Hemmen, AICP
Certificate # 012190

TABLE OF CONTENTS

LIST OF FIGURES AND TABLES 2
EXECUTIVE SUMMARY 3
INTRODUCTION 4
EXISTING CONDITIONS 6
TRIP GENERATION 7
TRIP DISTRIBUTION 8
ROADWAY LEVEL OF SERVICE (LOS) ANALYSIS 10
INTERSECTION ANALYSIS 11
Right Turn Lane Analysis 12
Left Turn Lane Analysis 12
CONCLUSIONS and RECOMMENDATIONS 13
APPENDICES: Correspondence and Documentation 14
Appendix A: Correspondence 15
Appendix B: Data Collection 16
Appendix C: Highway Capacity Software Reports 17

LIST OF FIGURES AND TABLES

FIGURES

1. Project Location Map with Existing Traffic Data……5
2. PM Peak Hour Project Impacts……9

TABLES

1. Project Trip Generation……7
2. Project Trip Distribution... ……8
3. Roadway Level Of Service Analysis……..10
4. Intersection Impacts……..11
5. Intersection Level Of Service (LOS)……..11

EXECUTIVE SUMMARY

The results of the Traffic Impact Study conducted for the Alachua Towne Center Retail Expansion in Alachua, Florida provides the following conclusions. The actual *net new* trip assessment for the proposed new 6,500 sq. ft. retail building is **478 daily trips** and **37 p.m. peak hour trips**. The total traffic generated by this proposed project will be dispersed on the area roadway network so that the maximum peak hour volume on any roadway segment of **SR 20** (**US 441)** will be **34 trips**. The project is a new building on a commercial outparcel within the Alachua Towne Center. The concept plan indicates no new external roadway connections are required for this project. Two driveways will be provided from within the internal circulating roadways of the Alachua Towne Center.

The project is located within the City of Alachua, Florida on NW 147th Drive west of SR 20 (US 441) under the jurisdiction of City of Alachua. This study utilized historical traffic from the City of Alachua and FDOT traffic database. A traffic study methodology was discussed and approved by the City of Alachua Planning staff. A critical component of the study is intersection analysis and turn lane evaluations at the SR 20 (US 441)/NW 147th Drive signal controlled intersection.

The existing SR 20 (US 441) roadway segments are operating within the current level of service (LOS) standards. They will continue to do so with the minimal peak hour impacts of the proposed retail expansion.

Traffic at the SR 20 (US 441)/NW 147th Drive signal controlled intersection operates very well during the p.m. peak hour demand with the fully actuated signal timings. Highway Capacity Software analysis determined it will continue to operate within level of service standards based on the data collection and field observations. The minimal increase in project impacts do not require modification to the current signal timing plan. No off-site transportation improvements are required for project approval.

INTRODUCTION

MPH Transportation Planning, Inc. (MPH) is assisting J.Brown Professional Group (JBProGroup) project engineers with transportation impacts for the proposed new 6,500 sq. ft. retail expansion in the Alachua Towne Center located within the City of Alachua, Florida. This project is a new building to be located on a vacant outparcel at the northwest corner of the existing retail center. A traffic study methodology was discussed with City Planning staff to determine the acceptable trip generation, distribution and analysis procedures for this project. It was determined that the p.m. peak hour trip impacts were the determining criteria for assessing intersection impacts.

Figure 1 - Project Location Map with Existing Traffic Data provides location information. Available historical traffic data from the City of Alachua and FDOT was reviewed. MPH staff conducted a field review and data collection of p.m. peak period turn movement counts at the project intersection. This data was used to establish directional traffic volumes for the project and used in turn lane analysis. Access to the project site will be from within the Alachua Towne Center internal roadway network. External roadway network access will be to NW 147th Drive to the intersection with SR 20 (US 441). There is secondary ingress/egress on NW 144th Way at the southern end of the Alachua Towne Center.

This study contains an assessment of the proposed development utilizing trip rates from the Institute of Transportation Engineers (ITE) Trip Generation 10th Edition to determine daily and peak hour project traffic for the new land use. Project trip distributions are based on data collected for adjacent street traffic and locations of interacting land uses within the project's impact area. Utilizing this information, an assignment of a.m. and p.m. peak hour project trips onto the local area roadway network are made to the adjacent roadway segments.

Gross project trip volumes will be used in assessing intersection traffic operations and roadway segment level of service (LOS). Pass-by and internal trip reductions are applicable to these land uses. However, to present a worst case scenario gross project trips for the p.m. peak hour were used in intersection analysis.



MPH

Alachua Towne Center Retail Store – Alachua, FL

Project Location & Existing Traffic

Figure 1

EXISTING CONDITIONS

Roadway traffic volumes and peak hour directional traffic data were reviewed for this project at locations indicated on ***Figure 1 – Project Location with Existing Traffic Data.*** Roadway segment daily volumes on SR 20 (US 441) are 24,000. The immediate roadway network, as depicted in Figure 1, consists of SR 20 (US 441) to the east, NW 147th Drive to the north, and NW 144th Way to the south of the project site.

SR 20 (US 441) is a 4-lane divided rural principal arterial roadway with a posted speed of 45 mph in the area of the project. It has sidewalks on either side of the highway but no bike lanes. There is no regularly scheduled transit service in the area. Traffic signal control is at the SR 20 (US 441)/NW 147th Drive less than 0.1 miles northeast of the project parcel.

NW 147th Drive is a 2-lane undivided local road with a 25 mph posted speed. It is bordered by commercial development adjacent to SR 20 (US 441) then transitions to a residential neighborhood street beyond the two commercial centers that border SR 20 (US 441).

Figure 1 also displays the critical p.m. peak hour volumes collected at the SR 20 (US 441/NW 147th Drive intersection. These volumes are used later with new project traffic to analyze intersection operations and turn lane requirements.

TRIP GENERATION

The Institute of Transportation Engineers (ITE) Trip Generation 10th Edition was used to calculate project trip estimates for the new land uses for Daily, AM & PM peak hours. Trip information is summarized in ***Table 1 – Project Trip Generation*** below.

TABLE 1: Project Trip Generation
Alachua Towne Center Retail Expansion
Alachua, Florida

Land Use					Distribution		Trips		***820*** 34% Pass	***934*** 50% **Net**
Description	ITE	ITE Trip Rates	Unit*	Trips	In	Out	In	Out	By	**Trips**
Retail Center	820	(T) = 37.75 (X)	5.000	189	50%	50%	95	95	64	125
Fast Food w/Drive-thru	934	(T) = 470.95 (X)	1.500	706	50%	50%	353	353	353	353
		Daily Trips	**6.500**	**895**			448	448	448	478
Retail Center	820	(T) = 0.94 (X)	5.000	5	62%	38%	3	2	2	3
Fast Food w/Drive-thru	934	(T) = 40.19 (X)	1.500	60	51%	49%	31	29	30	30
		AM Peak Hour Trips		**65**			34	31	32	33
Retail Center	820	(T) = 3.81 (X)	5.000	19	48%	52%	9	10	6	13
Fast Food w/Drive-thru	934	(T) = 32.67 (X)	1.500	49	50%	50%	25	25	25	25
		PM Peak Hour Trips		**68**			34	34	31	37

*Units: ksf = 1,000 sq. ft. Source: *ITE 10th Edition Trip Generation*

The evaluating criteria is gross building square footage. Pass-by reductions are applicable to both land uses. Both the fast food with drive thru and generic retail land uses are detailed in Table 1. The total project trips are used in both roadway level of service and intersection analyses not just the new trips to present a conservative estimate of future traffic once the project is completed.

This data is used to develop external distribution of project trips onto the adjacent roadway network from the project site as discussed in the next section of this report.

TRIP DISTRIBUTION

The distribution of project trips on the roadway is a manual assignment derived from the site specific intersection turn movements collected by MPH on the adjacent roadways and review of existing locations of interacting land-uses. Trip generation numbers from Table 1 are factored by these nominal distribution criteria to develop trip impact estimates for the adjacent roadway segments to the project site. The results of these calculations are displayed in ***Table 2 – Project Trip Distribution*** below and in ***Figure 2–Project Traffic PM Peak Hour Impacts***.

TABLE 2:Project Trip Distribution
Alachua Towne Center Retail Expansion

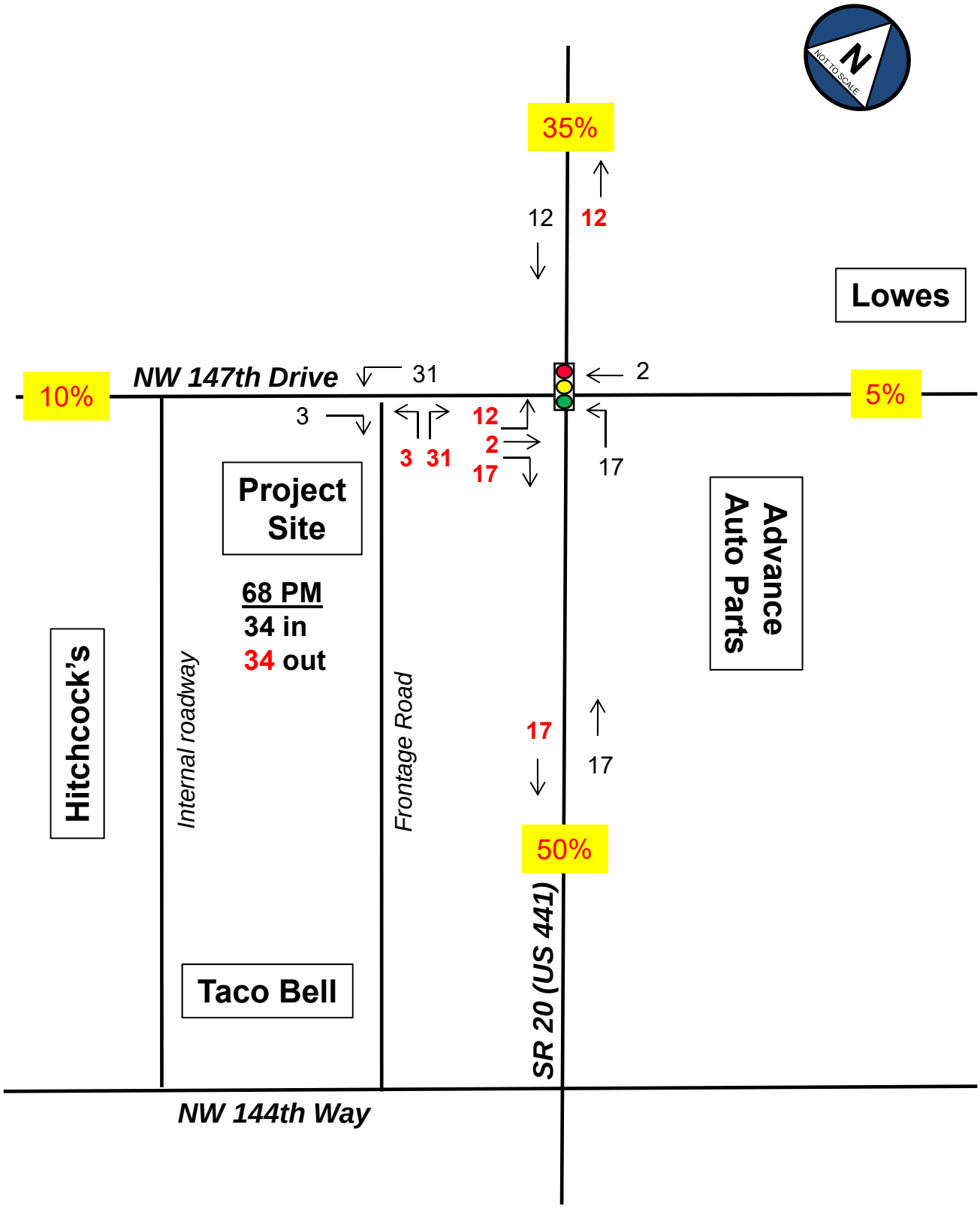
Daily Project Trips		**Enter**	**239**	**Exit**	**239**	**2-Way**
Roadway	**Segment**	**%**	**Trips**	**%**	**Trips**	**Total**
SR 20 (US 441)	North of Project	35%	84	35%	84	168
	South of Project	50%	119	50%	119	238
NW 147th Drive	East of Project	5%	12	5%	12	24
	West of Project	10%	24	10%	24	48
	Total	100%	239	100%	239	478

AM Peak Hour Project Trips		**Enter**	**34**	**Exit**	**31**	**2-Way**
Roadway	**Segment**	**%**	**Trips**	**%**	**Trips**	**Total**
SR 20 (US 441)	North of Project	35%	12	35%	11	23
	South of Project	50%	17	50%	15	32
NW 147th Drive	East of Project	5%	2	5%	2	4
	West of Project	10%	3	10%	3	6
	Total	100%	34	100%	31	65

PM Peak Hour Project Trips		**Enter**	**34**	**Exit**	**34**	**2-Way**
Roadway	**Segment**	**%**	**Trips**	**%**	**Trips**	**Total**
SR 20 (US 441)	North of Project	35%	12	35%	12	24
	South of Project	50%	17	50%	17	34
NW 147th Drive	East of Project	5%	2	5%	2	4
	West of Project	10%	3	10%	3	6
	Total	100%	34	100%	34	68

Project Distributions are estimated from adjacent street traffic & interacting land uses
Peak hour project trips represent the gross driveway impacts from Table 1

Peak-hour trip impacts provide driveway detail for intersection and turn lane analysis for site plan review. Intersection analysis consists of the gross trips entering or exiting the site. The highest two-way roadway segment project traffic impact during the peak hour would be: **34** vehicles on SR 20 (US 441) in the PM peak hour.



MPH ***Alachua Towne Center Retail Store***

PM Peak Hour Project Traffic

MPH 21-01 2/10/21

Figure 2
Page 9

ROADWAY LEVEL OF SERVICE (LOS) ANALYSIS

Data for City of Alachua roadway level of service (LOS) for the adjacent roadways was extracted from the City's 2020 traffic database. The City of Alachua Planning Department maintains a Concurrency database that includes existing traffic and that reserved from Pending and Approved Development Orders. The LOS analysis for the SR 20 (US 441) segment fronting the Alachua Towne Center is provided below. ***Table 3: Roadway Level of Service*** provides the most recent available data for roadway segments adjacent to the project site.

Table 3 - Roadway Level of Service (LOS)

Alachua Towne Center Retail Expansion

Cocoa, Florida

Roadway	Segment #/Description	Period	MSV	Existing	Res'vd.	Project	V/C	LOS
SR 20 - US 441	SR 235 to I-75	PM Peak	3,510	2,299	558	34	82%	C
	FDOT #107, CoA #5							

Source: Maximum Service Volume (MSV) from City of Alachua Traffic Database (2-way peak hour).
Existing Traffic from City Traffic Counts 2020. (MPH count on 2/9/21 = 2,278)
Reserved trips from Pending and approved Development Orders
Project trips represent PM Peak Hour two way traffic from Table 2.
Project trips are gross impacts not accounting for pass-by trip reductions.

As indicated in the table above, reserved trips are recorded for any Pending or Approved Development Order impacting the subject roadway segment. This analysis uses the p.m. peak hour traffic count collected in 2020. The peak hour two-way volume on SR 20 (US 441) collected by MPH on Feb. 9, 2021 is 2,278 vehicles. This is within 1% of the p.m. peak volume collected by the City in 2020. This is significant in light of the 5-10% reduction FDOT has recorded on most state roads due to the Covid impact. Gross 2-way p.m. peak hour project trips were utilized to represent a conservative analysis. This roadway segment remains below the standard LOS "C" threshold associated with this roadway.

Additionally, 2020 & 2021 traffic remains below 2019 volumes as recorded on FDOT permanent count stations as a result of the Covid19 pandemic. There is speculation that a significant reduction in traffic volumes will be observed for an extended period and potentially may never return to historical volumes as work from home increasingly becomes the norm for a significant population of the work force.

INTERSECTION ANALYSIS

City of Alachua Planning staff only requested intersection analysis at the SR 20 (US 441) intersection with NW 147th Drive. Intersection turn movement count data was collected by MPH staff between 4-6 p.m. on Tuesday, February 9, 2021. It is detailed in ***Table 4: Intersection Impacts*** below. A copy of all data collected is provided in the report appendix. Direct access to the project parcel will be from the internal Alachua Towne Center roadways. No new external roadway connections are required for this development project. City staff principal concern was for the turn lanes at the subject intersection.

Table 4 - Intersection Impacts

Signal Controlled Intersection Impacts

Approach	***SR 20 - US 441***						***NW 147th Drive***						
PM Peak	Northbound			Southbound			Eastbound			Westbound			
Movement	Lt	Thru	Rt	Lt	Thru	Rt	Left	Thru	Rt	Left	Thru	Rt	Totals
2021	94	1252	61	62	739	119	200	24	43	55	11	87	2747
Project	**17**					**12**	**12**	**2**	**17**		**2**		**62**
2022	111	1252	61	62	739	131	212	26	60	55	13	87	2809

Notes: 2021 traffic volumes from Figure 1, Project Traffic (BOLD) from Figure 2.

The most significant project movements at the SR 20 (US 441)/NW 147th Drive intersection approaches are the eastbound right turn and northbound left turn movements. Field observations indicated no significantly delayed turning movements on any approach during the PM peak periods. An On-site Field Observation report is included in the appendix.

This analysis is conservative in that project trip impacts do not account for pass-by reductions. Gross driveway trips are used without reducing existing trips with pass-by estimates.

The results of the HCS traffic signal controlled intersection analysis are summarized in ***Table 5 – Intersection Level of Service (LOS)***. They indicate that there is sufficient capacity to absorb project impacts without degrading level of service below acceptable standards.
The HCS detailed printouts are included in the report appendix.

Table 5 - Intersection Level of Service (LOS)

Traffic		SR 20 - US 441				NW 147th Drive					
Signal		NB		SB		EB		WB		Intersection	
Control		LOS	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS	Delay
2021	PM	B	20.5	B	14.2	D	47.7	C	27.8	C	21.2
2022	PM	B	20.4	B	14.0	D	51.7	C	28.3	C	21.8

Delay expressed in AVERAGE seconds per vehicle.

Right Turn Lane Analysis

There are dedicated right turn lanes at three of the four intersection approaches. Only eastbound NW 147th Drive has a thru-right lane and dedicated left turn lane. However, this is not a problem as the significant volume for the eastbound approach is the left turn movement which represents 75% of the total approach volume in the peak hour. Also, as recorded in the turn movement counts right turns on red (RoR) are significant for all four approaches. The eastbound through volume is so minimal that the right turn vehicles often have the ability to make RoR movements. No modifications are required to any of the right turn lanes. ..

Left Turn Lane Analysis

There are dedicated left turn lanes at all four intersection approaches. The eastbound NW 147th Drive approach has a short 150 foot thru-right lane as the left turn movement is the dominant vehicle movement for that approach. Access to the eastbound thru-right turn lane is obstructed when 6 or more vehicles are in the left turn queue. This restriction is not a problem as the signal timing allows for a leading eastbound movement (unopposed) and all vehicles are able to proceed through the intersection without delay and no residual queue. It is especially helpful that the new signal phasing allows for a flashing yellow turn arrow to assist left turning vehicles when no conflicting vehicle movements are present. The Gainesville Traffic Management Center (G-TMC) has done an exceptional job in providing an efficient and equitable signal timing plan for this intersection. State highways will always get the dominant portion of traffic cycle green time, however, NW 147th Drive traffic is always allocated sufficient signal phasing time to clear all stopped vehicles.

CONCLUSIONS and RECOMMENDATIONS

Based on the data and analysis provided in the previous sections of this report the following conclusions on traffic operations and recommendations are provided below.

Conclusions:

1. The proposed retail land use expansion on the Alachua Towne Center vacant outparcel is planned to include a fast food with drive thru facility as a component of the proposed 6,500 sq. ft. building. The entire project is estimated to generate 478 new daily trip impacts. The PM Peak hour will have approximately 68 total trips (37 net new) impacting adjacent roadways.
2. This segment of SR 20 (US 441) currently meets LOS standards (LOS "C") and will remain well below this threshold with the addition of project trips.
3. The proposed project driveways are located within the internal roadway network of the Alachua Towne Center south of NW 147th Drive.
4. Project trips will not adversely impact either roadway.
5. Right turn lane analysis indicates that no modifications are required to any of the three right turn lanes at the SR 20 (US 441)/NW 147th Drive intersection.
6. Left turn lane analysis indicates that all four left turn lanes at the subject intersection have adequate storage capacity for existing and project vehicle demand. Impacts.

Recommendations:

1. Approve the project as proposed with a fast food restaurant with a drive thru window as a component of the proposed 6,500 sq. ft. retail expansion within the Alachua Towne Center commercial development.

APPENDICES: Correspondence and Documentation

Appendix A: Correspondence

MPH Transportation Planning Inc.
1725 Riverbirch Hollow
Tallahassee, FL 32308

Telephone (850) 510-6461
mphemmen@comcast.net

February 8, 2021

Methodology Memo: Traffic Impact Analysis (TIA) for Alachua Towne Plaza Retail Store
Proposed Location: SR 20 (US 441) at NW 147th Drive, Alachua, FL [MPH 21-01]

Project Description:
MPH is providing transportation support services to J.Brown Professional Group, Inc. (JBProGroup) for a proposed 6,500 sf Retail Strip Center on the northeast outparcel of the Alachua Towne Plaza on SR 20 (US 441) at NW 147th Drive, Alachua, Florida. The project is a new development on a vacant outparcel within the commercial center as displayed in Figure 1. The following information provides the basic data required for an assessment of the daily, a.m. & p.m. peak hour trip impacts required to address traffic operations concerns.

Level of TIA:
The project has 895 gross daily trips, reduced to 478 net new trips with pass-by reductions. Driveway connections are restricted to the internal roadways within the commercial center. Normally a simple traffic statement is sufficient. However, since the project is seeking approval for a food service operation with a drive-thru window a rezoning of this parcel is required for approval of the proposed land use. The City of Alachua Planning Department staff has requested p.m. peak period data collection and intersection analysis to determine if existing turn lanes are adequate to absorb project impacts.

Site Location Map:
The attached Figure 1 shows the physical location of the proposed project.

Study Area:
A pre-app discussion for this project was held with city planning staff. The critical concerns were for the existing intersection and specifically the turn lane capacity at the SR 20 (US 441)/NW 147th Drive intersection. It was agreed that there would be both pass-by and internal trip capture from the existing land uses within the Alachua Towne Plaza and adjacent residential developments on NW 147th Drive to the west.

Signalized or major study area intersections:
SR 20 (US 441) at NW 147th Drive is under traffic signal control

Site Plan:
The attached Figure 2 shows the proposed building and access driveways.

Critical project issues:
None anticipated from project impacts. SR 20 (US 441) Segment %: SR 235 to I-75 is currently under operating capacity based on latest data from the City of Alachua database that includes existing traffic and approved and pending development orders. Available capacity on this roadway segment is greater than the estimated project trip impacts. The intersection lane geometry requires analysis.

ITE 10th Edition Trip Generation:

ITE Land Use Code 820 Retail Center and Land Use Code 934 Fast Food Restaurant with Drive Thru Window are used to estimate project trip impacts. The evaluating criteria is gross building square footage. A pass-by rate of 34% is applicable for the retail center and 50% for the fast food component of the development

TABLE 1: Project Trip Generation

Alachua Towne Plaza Retail Center

Alachua, Florida

									820	***934***
									34%	50%
Land Use					Distribution		Trips		Pass	**Net**
Description	ITE	ITE Trip Rates	Unit*	Trips	In	Out	In	Out	By	**Trips**
Retail Center	820	(T) = 37.75 (X)	5.000	189	50%	50%	95	95	64	125
Fast Food w/Drive-thru	934	(T) = 470.95 (X)	1.500	706	50%	50%	353	353	353	353
		Daily Trips	**6.500**	**895**			448	448	448	478
Retail Center	820	(T) = 0.94 (X)	5.000	5	62%	38%	3	2	2	3
Fast Food w/Drive-thru	934	(T) = 40.19 (X)	1.500	60	51%	49%	31	29	30	30
		AM Peak Hour Trips		**65**			34	31	32	33
Retail Center	820	(T) = 3.81 (X)	5.000	19	48%	52%	9	10	6	13
Fast Food w/Drive-thru	934	(T) = 32.67 (X)	1.500	49	50%	50%	25	25	25	25
		PM Peak Hour Trips		**68**			34	34	31	37

*Units: ksf = 1,000 sq. ft. Source: *ITE 10th Edition Trip Generation*

Trip Distribution:

Trip distribution will be based on historical traffic data and the turn movement count collected at the project intersection. The local area is populated with commercial development on SR 20 (US 441) and residential neighborhoods on NW 147th Drive beyond the commercial clusters. Trip distribution will reflect this concentration of interacting land uses when assigning project trips to the adjacent roadway network.

Proposed Growth Rate:

None. Project is normally constructed and open for business within 180-270 days of permit approvals. No increase in adjacent street traffic is anticipated within the next 6-9 months of 2021 that is not already accounted for within the approved or pending development orders. FDOT historical traffic counts indicate a steady volume over the past 5 years. Daily 2019 traffic counts on SR 20 (US 441) are 24,000 vehicles. No major projects underway or with confirmed trip reservations within the immediate area beyond those already in the City's traffic database that includes trip reservations.

Traffic Counts:

The latest FDOT counts available from the FDOT Traffic Online database will be utilized along with any the City of Alachua may have from recent traffic study submittals. City staff has requested p.m. peak period turn movement counts be collected to analyze intersection operations and turn lane demand. This project is in an established area of the City of Alachua and historical traffic volumes are well below the roadways assigned capacity. The minimal new p.m. peak hour traffic (37 trips) should not create any problems on SR 20 (US 441) or NW 147th Drive.

Data Analysis and Reports:

Vehicle turn movement data collected at the project intersection will be analyzed using Highway Capacity Software (HCS) for existing and existing plus project scenarios for the p.m. peak period. Analysis will include the turn lane 95th percentile queue and storage requirements for the intersection approaches. Field measurement of traffic signal timings will be collected and confirmation of signal timings with the Gainesville Traffic Management Center will be included in the traffic report. Field data collection will include

measurement and documentation of the eastern approach turn lanes and any residual traffic demand during the peak period signal phasing.

A comprehensive traffic impact analysis (TIA) will be produced that includes all field data collection and analysis. The final TIA report will undergo a thorough QA/QC review and it will be signed and sealed by a Florida Registered Professional Engineer (P.E.).

If further consultation or modification to any of the items above is required, please call me at (850) 510-6461 anytime during normal business hours to discuss.

Sincerely,

Mike Hemmen

Mike Hemmen, AICP – President
Certificate # 012190
MPH Transportation Planning, Inc.

Responsible Transportation Engineer of Traffic Study:

Anthony J. Brown Jr.

Anthony J. Brown Jr., P.E.
FLA License # 43879
JBProGroup

Copies distributed electronically to:

Kathy Winburn	Alachua Planning Director	kwinburn@cityof alachua.com
Adam Hall	Alachua Planner	ahall@cityof alachua.com
Chris Potts, P.E.	JBProGroup – Project Engineer	chris.potts@jbprogroup.com
Jay Brown, P.E.	JBProGroup – Principal	jay.brown@jbprogroup.com

Attachments: Figure 1: Project Location
Figure 2: Site Plan



MPH

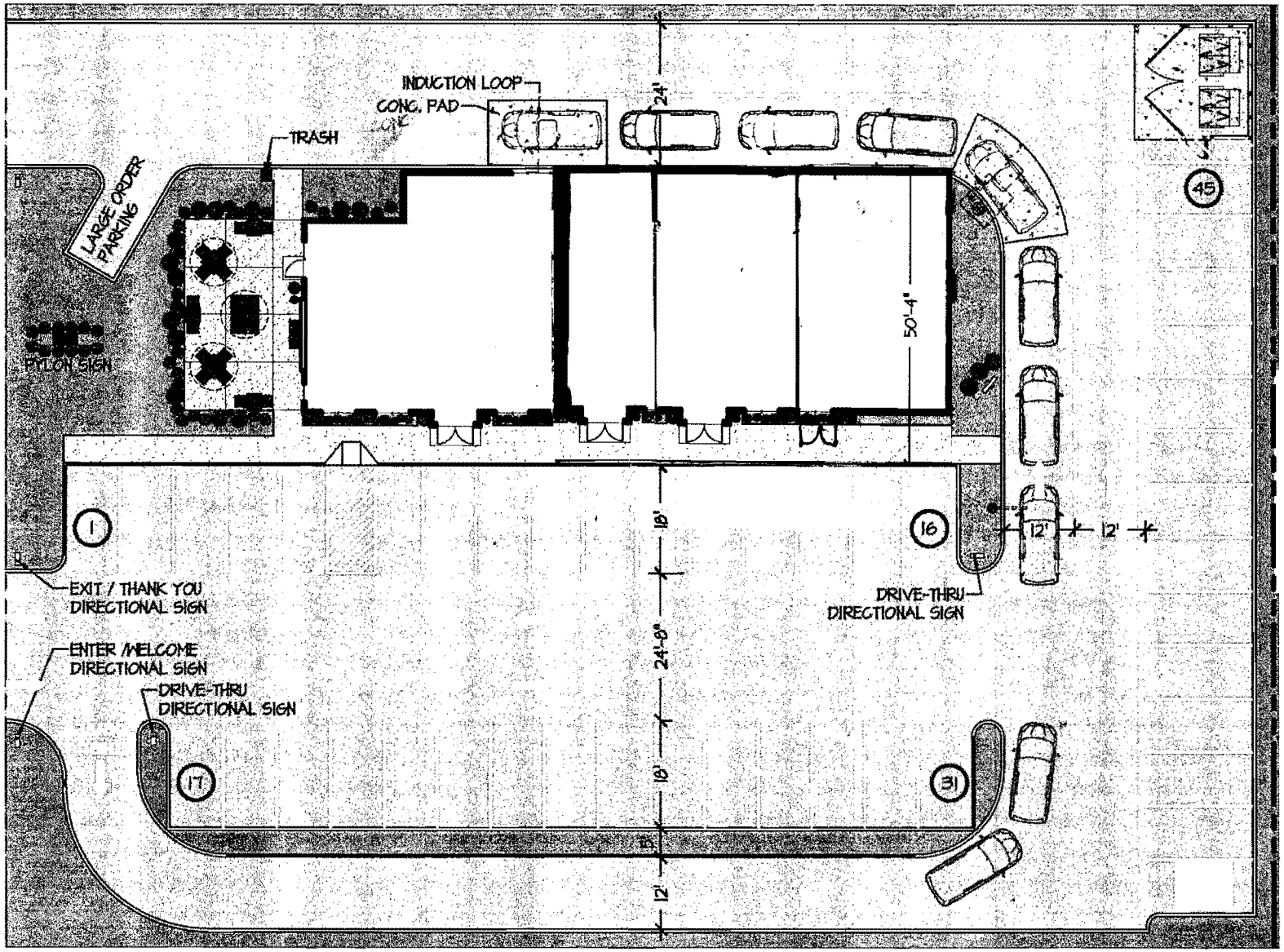
MPH 21-01 2/8/21

Towne Plaza Retail Store – Alachua, FL

Project Location & Existing Traffic

Figure 1

CONCEPT PLAN - NOT TO SCALE
ALACHUA TOWN PLAZA - RETAIL OUTPARCEL



US Highway 441

NW 147th Drive

Adam Hall <ad_hall@cityofalachua.org> 2/9/2021 12:10 PM

Re: Alachua Towne Plaza Retail Expansion Traffic study methodology memo

To mphemmen <mphemmen@comcast.net>

Mike,
This methodology is acceptable. Thank you,

Adam Hall
City of Alachua

From: mphemmen
To: ahall@cityofalachua.com, Kathy Winburn
Cc: Chris Potts PE, jay brown, Dhairya Chaudhari
Sent: Monday, February 8, 2021 2:56:54 PM
Subject: Alachua Towne Plaza Retail Expansion Traffic study methodology memo

Adam,

Please find the attached traffic study methodology memo as discussed for the proposed retail expansion with drive-thru window. The project location is the northeast outparcel within the Alachua Towne Plaza commercial center on SR 20 (US 441) at NW 147th Drive. A detailed site plan has not been developed at this time so I've included a general concept plan for the proposed 6,500 sq. ft. building.

MPH plans on collecting the required turn movement data at the intersection of US 441/NW 147th Drive on Tuesday or Wednesday of this week if that is acceptable to the City. Chris Potts informed me they are waiting on the traffic study to submit the formal re-zoning request. We'd like to get this completed ASAP to accommodate their schedule.

Please call or email me with any questions. Thanks again for taking the time to discuss the requirements with me this morning.

Mike Hemmen, AICP
MPH Transportation Planning, Inc.
1725 Riverbirch Hollow
Tallahassee, FL 32308
850.510.6461
mphemmen@comcast.net

CAUTION: This email originated from outside the City.

DO NOT respond, click, or open attachments unless you recognize the sender **(name AND email address)** and know the content is safe.

Should there still be any question on the origin of this email, contact the IT Department immediately.

Appendix B: Data Collection

ON-SITE OBSERVATION REPORT

Project/#:	Alachua Towne Plaza Retail Center, Alachua, FL	Analyst:	M. Hemmen
Location:	SR 20 (US 441) at NW 147th Drive	Date:	2/9/2021
Control:	Traffic Signal	Time:	3 pm - 6 pm
Lanes/Spd	SR 20 (US 441) 4-Ln @ 45 mph; NW 147th Drive: 2-Ln @ 15 mph		

	Isolated & Non-Isolated Intersections	No/Yes/NA	Approach
1	Does road curvature, vegetation, buildings, parked cars, etc.	N	
	block drivers' view of conflicting vehicles?		
2	Is the intersection skew angle so sharp that it makes it difficult	N	
	to view conflicting vehicles or complete turns?		
3	Do vehicle speeds appear too high?	N	
4	Does the delay for the minor road right-turn appear excessive?	N	
5	Does the delay for the minor road through appear excessive?	N	
6	Does the delay for the minor road left-turn appear excessive?	N	
7	Does the delay for the major road left-turn appear excessive?	N	
8	Does the queue for major road left-turns impede through traffic?	N	
9	As major road vehicles slow to turn do they impede other vehicles?	N	
10	Do parking maneuvers impede other vehicles?	N	
11	Are drivers not complying with the traffic control devices?	N	
12	Is there evidence that one or more curb radii are too small?	N	
13	Do pedestrians appear to cause conflict with vehicular traffic?	N	
14	Are there guidance or control problems that could be mitigated by	N	
	raised-curb channelization?		
	Non-Isolated Intersections	No/Yes/NA	Approach
15	Do queues from adjacent signalized intersections spillback into	N	
	the subject intersection?		
16	Do vehicles slowing to turn at adjacent intersections or driveways	N	
	contribute to the delay to major/minor road drivers?		
17	Is it possible that some drivers are diverting to the subject	N	
	intersection because of congestion on a nearby street?		
18	Does the arrival pattern of major road traffic platoons contribute	N	
	to the delay of minor road drivers?		

	Comments/Explanations to above responses
	120-150 second traffic signal cycle time with only 15-20 seconds for minor street approach
	Fully actuated signal phasing based on major road left turn vehicle presence indicator'
Gen.	Overall intersection operation is good with established 120-150 second signal cycle time.
	Intersection geometry includes left turn lanes at all 4 approaches, right turn lanes at 3 of 4.
	Roughly 45% of right turns are completed on red phase (RoR).
	No residual queue observed during any signal cycle over 3 hours of field observations.
	Major movement on eastbound NW 147th Drive is the left turn movement (75%)
	NW 147th EB left turn vehicles often blocked access to thru-right lane (6+ vehicles)
	All vehicles exited during single traffic cycle as phase timing was extended to meet demand

FLORIDA DEPARTMENT OF TRANSPORTATION
TRANSPORTATION STATISTICS OFFICE
2019 HISTORICAL AADT REPORT

COUNTY: 26 - ALACHUA

SITE: 5106 - SR 20 .4 MI. NW OF SR 235

YEAR	AADT	DIRECTION 1	DIRECTION 2	*K FACTOR	D FACTOR	T FACTOR
2019	24000 C	N 12000	S 12000	9.50	58.00	5.60
2018	24000 C	N 12000	S 12000	9.50	57.90	4.90
2017	24500 C	N 12500	S 12000	9.50	53.80	4.60
2016	22000 C	N 11000	S 11000	9.50	53.60	4.90
2015	19500 C	N 9700	S 9800	9.50	57.00	5.20
2014	20000 C	N 10000	S 10000	9.50	57.40	5.40
2013	19800 C	N 9800	S 10000	9.50	57.80	5.00
2012	18900 C	N 9600	S 9300	9.50	58.40	4.90
2011	18600 C	N 9200	S 9400	9.50	58.80	5.50
2010	17600 C	N 8700	S 8900	10.13	59.87	5.10
2009	19600 C	N 9600	S 10000	10.04	57.81	6.20
2008	19400 C	N 9800	S 9600	10.17	57.73	7.30
2007	20000 C	N 10000	S 10000	10.22	58.44	5.70
2006	20500 C	N 10000	S 10500	9.98	59.05	6.70
2005	20000 C	N 10000	S 10000	10.10	58.20	19.60
2004	19900 C	N 10000	S 9900	10.20	62.30	9.10

AADT FLAGS: C = COMPUTED; E = MANUAL ESTIMATE; F = FIRST YEAR ESTIMATE
S = SECOND YEAR ESTIMATE; T = THIRD YEAR ESTIMATE; R = FOURTH YEAR ESTIMATE
V = FIFTH YEAR ESTIMATE; 6 = SIXTH YEAR ESTIMATE; X = UNKNOWN
*K FACTOR: STARTING WITH YEAR 2011 IS STANDARDK, PRIOR YEARS ARE K30 VALUES

COUNTY: 26
STATION: 5106
DESCRIPTION: SR 20 .4 MI. NW OF SR 235
START DATE: 05/07/2019
START TIME: 1415

	DIRECTION: N					DIRECTION: S					COMBINED
TIME	1ST	2ND	3RD	4TH	TOTAL	1ST	2ND	3RD	4TH	TOTAL	TOTAL
0000	22	15	16	10	63	18	21	13	17	69	132
0100	20	10	11	10	51	12	8	4	10	34	85
0200	16	8	5	7	36	6	7	5	7	25	61
0300	6	6	6	14	32	6	16	11	19	52	84
0400	16	10	21	13	60	17	18	28	30	93	153
0500	30	30	40	46	146	55	55	88	107	305	451
0600	50	66	92	116	324	114	183	256	235	788	1112
0700	132	164	195	176	667	293	311	320	297	1221	1888
0800	165	171	179	193	708	268	278	232	228	1006	1714
0900	140	132	144	171	587	194	172	206	164	736	1323
1000	132	138	132	156	558	148	160	164	177	649	1207
1100	177	168	226	238	809	141	178	184	200	703	1512
1200	232	227	233	189	881	217	221	239	229	906	1787
1300	208	195	185	206	794	214	194	190	200	798	1592
1400	215	238	231	224	908	183	187	217	160	747	1655
1500	235	223	243	254	955	192	218	220	169	799	1754
1600	264	285	331	297	1177	216	161	189	222	788	1965
1700	343	345	343	294	1325	182	201	215	187	785	2110
1800	277	252	214	168	911	194	165	166	125	650	1561
1900	140	137	133	134	544	124	132	120	93	469	1013
2000	128	119	106	75	428	107	114	118	107	446	874
2100	81	75	74	47	277	94	74	62	61	291	568
2200	45	45	43	33	166	37	42	30	44	153	319
2300	16	20	22	23	81	23	13	23	18	77	158
24-HOUR TOTALS:					12488					12590	25078

PEAK VOLUME INFORMATION

	DIRECTION: N		DIRECTION: S		COMBINED DIRECTIONS	
	HOUR	VOLUME	HOUR	VOLUME	HOUR	VOLUME
A.M.	800	708	700	1221	715	1896
P.M.	1645	1328	1200	906	1645	2148
DAILY	1645	1328	700	1221	1645	2148

GENERATED BY SPS 5.0.53P

COUNTY: 26
STATION: 5106
DESCRIPTION: SR 20 .4 MI. NW OF SR 235
START DATE: 05/08/2019
START TIME: 1415

	DIRECTION: N					DIRECTION: S					COMBINED
TIME	1ST	2ND	3RD	4TH	TOTAL	1ST	2ND	3RD	4TH	TOTAL	TOTAL
0000	26	25	18	13	82	21	24	21	5	71	153
0100	10	6	13	10	39	14	15	10	8	47	86
0200	7	11	4	12	34	5	9	7	7	28	62
0300	8	5	11	10	34	7	14	13	25	59	93
0400	14	14	13	16	57	24	26	30	38	118	175
0500	25	29	51	40	145	50	59	83	110	302	447
0600	57	74	84	129	344	111	186	234	243	774	1118
0700	126	156	201	186	669	299	319	331	326	1275	1944
0800	184	164	163	187	698	296	261	271	255	1083	1781
0900	143	146	150	174	613	183	169	226	186	764	1377
1000	140	146	163	149	598	154	165	181	182	682	1280
1100	188	177	278	238	881	179	191	174	198	742	1623
1200	248	236	196	217	897	202	235	209	208	854	1751
1300	190	229	227	222	868	241	212	188	207	848	1716
1400	189	209	197	210	805	191	182	184	182	739	1544
1500	247	233	242	232	954	172	203	241	216	832	1786
1600	265	258	297	305	1125	174	207	222	215	818	1943
1700	330	350	330	307	1317	195	206	240	208	849	2166
1800	261	257	241	206	965	184	167	154	149	654	1619
1900	143	158	161	126	588	145	133	118	109	505	1093
2000	148	108	106	100	462	103	127	118	101	449	911
2100	87	103	72	69	331	90	89	80	66	325	656
2200	69	63	40	30	202	64	50	39	30	183	385
2300	32	37	9	20	98	30	26	25	24	105	203
24-HOUR TOTALS:					12806					13106	25912

PEAK VOLUME INFORMATION

	DIRECTION: N		DIRECTION: S		COMBINED DIRECTIONS	
	HOUR	VOLUME	HOUR	VOLUME	HOUR	VOLUME
A.M.	730	735	700	1275	715	1999
P.M.	1700	1317	1215	893	1645	2171
DAILY	1700	1317	700	1275	1645	2171

GENERATED BY SPS 5.0.53P

MPH Transportation Planning
Tallahassee, Florida
850-510-6461

Project No. MPH 21-01
Alachua Towne Ctr. Retail Site
US 441 at NW 147th Dr.

File Name : US441
Site Code : 00000000
Start Date : 2/9/2021

Groups Printed- Vehicles - Buses & Trucks

	US441 From North					NW147TH DR From East					US441 From South					NW147TH DR From West					
Start Time	Rt	Th	Lt	RoR	App. Total	Rt	Th	Lt	RoR	App. Total	Rt	Th	Lt	RoR	App. Total	Rt	Th	Lt	RoR	App. Total	Int. Total
Factor	1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		
16:00	17	162	21	4	204	11	2	14	17	44	12	237	16	3	268	4	6	33	3	46	562
16:15	31	183	20	4	238	7	1	14	21	43	7	219	17	7	250	0	2	35	5	42	573
16:30	30	161	18	2	211	5	2	24	15	46	12	251	17	4	284	2	6	47	6	61	602
16:45	17	158	15	12	202	12	4	12	16	44	18	218	15	4	255	9	6	38	4	57	558
Total	95	664	74	22	855	35	9	64	69	177	49	925	65	18	1057	15	20	153	18	206	2295
17:00	11	200	12	13	236	9	3	9	19	40	15	320	27	9	371	7	7	58	7	79	726
17:15	14	177	14	10	215	10	5	26	12	53	11	307	16	4	338	10	9	64	3	86	692
17:30	22	186	17	7	232	3	3	8	15	29	11	332	26	4	373	0	3	36	6	45	679
17:45	27	176	19	15	237	6	0	12	13	31	5	293	25	2	325	8	5	42	2	57	650
Total	74	739	62	45	920	28	11	55	59	153	42	1252	94	19	1407	25	24	200	18	267	2747
Grand Total	169	1403	136	67	1775	63	20	119	128	330	91	2177	159	37	2464	40	44	353	36	473	5042
Apprch %	9.5	79.0	7.7	3.8		19.1	6.1	36.1	38.8		3.7	88.4	6.5	1.5		8.5	9.3	74.6	7.6		
Total %	3.4	27.8	2.7	1.3	35.2	1.2	0.4	2.4	2.5	6.5	1.8	43.2	3.2	0.7	48.9	0.8	0.9	7.0	0.7	9.4	

	US441 From North					NW147TH DR From East					US441 From South					NW147TH DR From West					
Start Time	Rt	Th	Lt	RoR	App. Total	Rt	Th	Lt	RoR	App. Total	Rt	Th	Lt	RoR	App. Total	Rt	Th	Lt	RoR	App. Total	Int. Total
Peak Hour From 16:00 to 17:45 - Peak 1 of 1																					
Intersection	17:00																				
Volume	74	739	62	45	920	28	11	55	59	153	42	1252	94	19	1407	25	24	200	18	267	2747
Percent	8.0	80.3	6.7	4.9		18.3	7.2	35.9	38.6		3.0	89.0	6.7	1.4		9.4	9.0	74.9	6.7		
17:00 Volume	11	200	12	13	236	9	3	9	19	40	15	320	27	9	371	7	7	58	7	79	726
Peak Factor																					0.946
High Int.	17:45					17:15					17:30					17:15					
Volume	27	176	19	15	237	10	5	26	12	53	11	332	26	4	373	10	9	64	3	86	
Peak Factor					0.970					0.722					0.943					0.776	

MPH Transportation Planning
Tallahassee, Florida
850-510-6461

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Alachua Towne Ctr. Retail Site
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File Name : US441
Site Code : 00000000
Start Date : 2/9/2021

	US441 From North					NW147TH DR From East					US441 From South					NW147TH DR From West					
Start Time	Rt	Th	Lt	RoR	App. Total	Rt	Th	Lt	RoR	App. Total	Rt	Th	Lt	RoR	App. Total	Rt	Th	Lt	RoR	App. Total	Int. Total
Peak Hour From 16:00 to 17:45 - Peak 1 of 1																					
By Approach	17:00					16:30					17:00					16:30					
Volume	74	739	62	45	920	36	14	71	62	183	42	1252	94	19	1407	28	28	207	20	283	
Percent	8.0	80.3	6.7	4.9		19.7	7.7	38.8	33.9		3.0	89.0	6.7	1.4		9.9	9.9	73.1	7.1		
High Int.	17:45					17:15					17:30					17:15					
Volume	27	176	19	15	237	10	5	26	12	53	11	332	26	4	373	10	9	64	3	86	
Peak Factor					0.970					0.863					0.943					0.823	

MPH Transportation Planning
Tallahassee, Florida
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	US441 From North					NW147TH DR From East					US441 From South					NW147TH DR From West					
Start Time	Rt	Th	Lt	RoR	App. Total	Rt	Th	Lt	RoR	App. Total	Rt	Th	Lt	RoR	App. Total	Rt	Th	Lt	RoR	App. Total	Int. Total
Factor	1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		
16:00	0	8	0	0	8	0	0	0	0	0	0	8	2	0	10	0	0	1	0	1	19
16:15	0	12	1	0	13	0	0	0	0	0	0	7	0	0	7	0	0	0	0	0	20
16:30	0	4	0	0	4	0	0	0	0	0	0	9	0	0	9	0	0	0	0	0	13
16:45	0	6	0	0	6	1	0	0	0	1	1	4	0	0	5	0	0	1	0	1	13
Total	0	30	1	0	31	1	0	0	0	1	1	28	2	0	31	0	0	2	0	2	65
17:00	0	8	0	0	8	0	0	0	0	0	0	4	0	0	4	0	0	0	0	0	12
17:15	0	2	0	0	2	1	0	0	0	1	0	8	0	0	8	0	0	0	0	0	11
17:30	0	4	1	0	5	1	0	0	0	1	0	5	0	0	5	0	0	0	0	0	11
17:45	0	7	0	0	7	0	0	0	0	0	0	2	0	0	2	0	0	0	0	0	9
Total	0	21	1	0	22	2	0	0	0	2	0	19	0	0	19	0	0	0	0	0	43
Grand Total	0	51	2	0	53	3	0	0	0	3	1	47	2	0	50	0	0	2	0	2	108
Apprch %	0.0	96.2	3.8	0.0		100.0	0.0	0.0	0.0		2.0	94.0	4.0	0.0		0.0	0.0	100.0	0.0		
Total %	0.0	47.2	1.9	0.0	49.1	2.8	0.0	0.0	0.0	2.8	0.9	43.5	1.9	0.0	46.3	0.0	0.0	1.9	0.0	1.9	

	US441 From North					NW147TH DR From East					US441 From South					NW147TH DR From West					
Start Time	Rt	Th	Lt	RoR	App. Total	Rt	Th	Lt	RoR	App. Total	Rt	Th	Lt	RoR	App. Total	Rt	Th	Lt	RoR	App. Total	Int. Total
Peak Hour From 16:00 to 17:45 - Peak 1 of 1																					
Intersection	16:00																				
Volume	0	30	1	0	31	1	0	0	0	1	1	28	2	0	31	0	0	2	0	2	65
Percent	0.0	96.8	3.2	0.0		100.0	0.0	0.0	0.0		3.2	90.3	6.5	0.0		0.0	0.0	100.0	0.0		
16:15 Volume	0	12	1	0	13	0	0	0	0	0	0	7	0	0	7	0	0	0	0	0	20
Peak Factor																					0.813
High Int.	16:15					16:45					16:00					16:00					
Volume	0	12	1	0	13	1	0	0	0	1	0	8	2	0	10	0	0	1	0	1	
Peak Factor					0.596					0.250					0.775					0.500	

MPH Transportation Planning
Tallahassee, Florida
850-510-6461

Project No. MPH 21-01
Alachua Towne Ctr. Retail Site
US 441 at NW 147th Dr.

File Name : US441
Site Code : 00000000
Start Date : 2/9/2021
Page No : 2

	US441 From North					NW147TH DR From East					US441 From South					NW147TH DR From West					
Start Time	Rt	Th	Lt	RoR	App. Total	Rt	Th	Lt	RoR	App. Total	Rt	Th	Lt	RoR	App. Total	Rt	Th	Lt	RoR	App. Total	Int. Total
Peak Hour From 16:00 to 17:45 - Peak 1 of 1																					
By Approach	16:00					16:45					16:00					16:00					
Volume	0	30	1	0	31	3	0	0	0	3	1	28	2	0	31	0	0	2	0	2	
Percent	0.0	96.8	3.2	0.0		100.0	0.0	0.0	0.0		3.2	90.3	6.5	0.0		0.0	0.0	100.0	0.0		
High Int.	16:15					16:45					16:00					16:00					
Volume	0	12	1	0	13	1	0	0	0	1	0	8	2	0	10	0	0	1	0	1	
Peak Factor					0.596					0.750					0.775					0.500	

Table 2a. Traffic Impacts - Final Development Orders

Roadway Segment (FDOT Segment #, CoA Comp Plan #)	Segment Description	AADT/Peak Hour	Comp Plan MSV^^	Existing Traffic*^	Reserved Trips	Available Capacity**	Percentage of Capacity Utilized
Interstate		Min LOS Std: C					
I-75 (32, 1)	From North City Limit to US 441	AADT	91,600	45,638	508	45,454	50.38%
		Peak Hour	8,250	4,792	38	3,420	58.55%
I-75 (31, 2)	From US 441 to the South City Limit	AADT	91,600	69,956	508	21,136	76.93%
		Peak Hour	8,250	7,345	38	867	89.49%
State Roads		Min LOS Std: D					
U.S. Hwy 441 (4127, 3)	From MPO Boundary to CR 25A East Intersection	AADT	43,000	22,740	2,302	17,958	58.24%
		Peak Hour	3,870	2,047	194	1,629	57.91%
U.S. Hwy 441 (106, 4)	From CR 25A East Intersection to SR 235	AADT	45,700	20,011	6,161	19,528	57.27%
		Peak Hour	4,110	1,901	616	1,593	61.24%
U.S. Hwy 441 (107, 5)	From SR 235 to I-75	AADT	39,000	24,199	2,785	12,016	69.19%
		Peak Hour	3,510	2,299	245	966	72.48%
U.S. Hwy 441 (4107, 6)	From I-75 to CR 235A	AADT	39,000	28,490	2,785	7,725	80.19%
		Peak Hour	3,510	2,707	245	558	84.10%
U.S. Hwy 441 (14, 7)	From CR 235A to NW 188th Street	AADT	43,000	22,482	2,785	17,733	58.76%
		Peak Hour	3,870	2,136	245	1,489	61.52%
SR 235 (108, 8)	From NW 143rd Place to US 441	AADT	14,400	9,792	152	4,456	69.06%
		Peak Hour	1,290	930	40	320	75.19%
SR 235 (109, 9)	From US 441 to NW 140th Street	AADT	14,400	7,636	121	6,643	53.87%
		Peak Hour	1,290	725	18	547	57.60%
SR 235 (4109/1439, 10)	From NW 140th Street to East City Limit	AADT	14,400	4,724	121	9,555	33.65%
		Peak Hour	1,290	449	18	823	36.20%
County Facilities		Min LOS Std: D					
CR 235 (11)	From NW 143rd Place to South City Limit	AADT	15,120	5,867	542	8,711	42.39%
		Peak Hour	1,359	558	58	743	45.33%
CR 241 (12)	From NW SR 235 to North City Limit	AADT	15,120	1,486	0	13,634	9.83%
		Peak Hour	1,359	160	0	1,199	11.77%
CR 241 (13)	From CR 235 to South City Limit	AADT	20,880	10,231	0	10,649	49.00%
		Peak Hour	1,881	972	0	909	51.67%
CR 2054 (14)	From SR 235 to West City Limit	AADT	15,120	2,497	161	12,462	17.58%
		Peak Hour	1,359	167	0	1,192	12.29%
CR 2054 (15)	From SR 235 to US 441	AADT	15,120	3,007	701	11,412	24.52%
		Peak Hour	1,359	354	73	932	31.42%
CR 235A (16)	From US 441 to North City Limit	AADT	15,120	1,501	16	13,603	10.03%
		Peak Hour	1,359	166	1	1,192	12.29%
CR 235A (17)	From US 441 to CR 235	AADT	15,120	5,379	191	9,550	36.84%
		Peak Hour	1,359	486	20	853	37.23%

* Florida Department of Transportation, District Two, Level of Service Reporting Tool 2019

** *Formula: Comp Plan MSV - (Existing Traffic + Reserved Trips from Development Orders with Concurrency*

^ County Facility AADT counts provided by Alachua County Public Works, April 2020.

^^AADT & Peak Hour MSVs calculated using LOSPLAN 2012. County Facilities reflect a 10 percent reduction in the MSV calculated within LOSPLAN 2012 as set forth in the Generalized Tables for AADT / Peak Hour Volumes, FDOT 2018 Q/LOS Handbook.

Reserved Trips are automatically updated from Table 8 data.

Table 2b. Traffic Impacts - Preliminary & Final Development Orders

Roadway Segment (FDOT Segment #, CoA Comp Plan #)	Segment Description	AADT/Peak Hour	Comp Plan MSV^^	Existing Traffic*^	Project Impact (Projected & Reserved)	Available Capacity**	Percentage of Capacity Utilized
Interstate		Min LOS Std: C					
I-75 (32, 1)	From North City Limit to US 441	AADT	91,600	45,638	508	45,454	50.38%
		Peak Hour	8,250	4,792	38	3,420	58.55%
I-75 (31, 2)	From US 441 to the South City Limit	AADT	91,600	69,956	508	21,136	76.93%
		Peak Hour	8,250	7,345	38	867	89.49%
State Roads		Min LOS Std: D					
U.S. Hwy 441 (4127, 3)	From MPO Boundary to CR 25A East Intersection	AADT	43,000	22,740	2,302	17,958	58.24%
		Peak Hour	3,870	2,047	194	1,629	57.91%
U.S. Hwy 441 (106, 4)	From CR 25A East Intersection to SR 235	AADT	45,700	20,011	6,388	19,301	57.77%
		Peak Hour	4,110	1,901	640	1,569	61.82%
U.S. Hwy 441 (107, 5)	From SR 235 to I-75	AADT	39,000	24,199	6,279	8,522	78.15%
		Peak Hour	3,510	2,299	558	653	81.40%
U.S. Hwy 441 (4107, 6)	From I-75 to CR 235A	AADT	39,000	28,490	6,279	4,231	89.15%
		Peak Hour	3,510	2,707	558	245	93.02%
U.S. Hwy 441 (14, 7)	From CR 235A to NW 188th Street	AADT	43,000	22,482	6,279	14,239	66.89%
		Peak Hour	3,870	2,136	558	1,176	69.61%
SR 235 (108, 8)	From NW 143rd Place to US 441	AADT	14,400	9,792	152	4,456	69.06%
		Peak Hour	1,290	930	40	320	75.19%
SR 235 (109, 9)	From US 441 to NW 140th Street	AADT	14,400	7,636	1,662	5,102	64.57%
		Peak Hour	1,290	725	211	354	72.56%
SR 235 (4109/1439, 10)	From NW 140th Street to East City Limit	AADT	14,400	4,724	1,662	8,014	44.35%
		Peak Hour	1,290	449	211	630	51.16%
County Facilities		Min LOS Std: D					
CR 235 (11)	From NW 143rd Place to South City Limit	AADT	15,120	5,867	542	8,711	42.39%
		Peak Hour	1,359	558	58	743	45.33%
CR 241 (12)	From NW SR 235 to North City Limit	AADT	15,120	1,486	0	13,634	9.83%
		Peak Hour	1,359	160	0	1,199	11.77%
CR 241 (13)	From CR 235 to South City Limit	AADT	20,880	10,231	0	10,649	49.00%
		Peak Hour	1,881	972	0	909	51.67%
CR 2054 (14)	From SR 235 to West City Limit	AADT	15,120	2,497	161	12,462	17.58%
		Peak Hour	1,359	167	35	1,157	14.86%
CR 2054 (15)	From SR 235 to US 441	AADT	15,120	3,007	701	11,412	24.52%
		Peak Hour	1,359	354	73	932	31.42%
CR 235A (16)	From US 441 to North City Limit	AADT	15,120	1,501	16	13,603	10.03%
		Peak Hour	1,359	166	1	1,192	12.29%
CR 235A (17)	From US 441 to CR 235	AADT	15,120	5,379	191	9,550	36.84%
		Peak Hour	1,359	486	20	853	37.23%

* Florida Department of Transportation, District Two, Level of Service Reporting Tool 2019

** *Formula: Comp Plan MSV - (Existing Traffic + + Projected Trips from Preliminary Development Orders with NO Concurrency Reservation + Reserved*

^ County Facility AADT counts provided by Alachua County Public Works, April 2020.

^^AADT & Peak Hour MSVs calculated using LOSPLAN 2012. County Facilities reflect a 10 percent reduction in the MSV calculated within LOSPLAN 2012 as set forth in the Generalized Tables for AADT / Peak Hour Volumes, FDOT 2018 Q/LOS Handbook.

Reserved Trips are automatically updated from Table 8 data.

Appendix C: Highway Capacity Software Reports

SHORT REPORT

General Information		Site Information	
Analyst	*M.Hemmen*	Intersection	*US 441 at NW 147th Drive*
Agency or Co.	*MPH Transportation Planning*	Area Type	*CBD or Similar*
Date Performed	*2/10/2021*	Jurisdiction	*FDOT-D2 & City of Alachua*
Time Period	*PM Peak Hour*	Analysis Year	*2021*

Volume and Timing Input

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes	*1*	*1*	*0*	*1*	*1*	*1*	*1*	*2*	*1*	*1*	*2*	*1*
Lane Group	*L*	*TR*		*L*	*T*	*R*	*L*	*T*	*R*	*L*	*T*	*R*
Volume (vph)	*200*	*24*	*43*	*55*	*11*	*87*	*94*	*1252*	*61*	*62*	*739*	*119*
% Heavy Vehicles	*2*	*2*	*2*	*2*	*2*	*2*	*2*	*2*	*2*	*2*	*4*	*2*
PHF	*0.90*	*0.90*	*0.90*	*0.90*	*0.90*	*0.90*	*0.90*	*0.90*	*0.90*	*0.90*	*0.90*	*0.90*
Pretimed/Actuated (P/A)	*A*	*A*	*A*	*A*	*A*	*A*	*A*	*A*	*A*	*A*	*A*	*A*
Startup Lost Time	*2.0*	*2.0*		*2.0*	*2.0*	*2.0*	*2.0*	*2.0*	*2.0*	*2.0*	*2.0*	*2.0*
Extension of Effective Green	*2.0*	*2.0*		*2.0*	*2.0*	*2.0*	*2.0*	*2.0*	*2.0*	*2.0*	*2.0*	*2.0*
Arrival Type	*3*	*3*		*3*	*3*	*3*	*3*	*3*	*3*	*3*	*3*	*3*
Unit Extension	*3.0*	*3.0*		*3.0*	*3.0*	*3.0*	*3.0*	*3.0*	*3.0*	*3.0*	*3.0*	*3.0*
Ped/Bike/RTOR Volume	*0*	*0*	*18*	*0*	*0*	*59*	*0*	*0*	*19*	*0*	*0*	*45*
Lane Width	*12.0*	*12.0*		*12.0*	*12.0*	*12.0*	*12.0*	*12.0*	*12.0*	*12.0*	*12.0*	*12.0*
Parking/Grade/Parking	*N*	*0*	*N*	*N*	*0*	*N*	*N*	*0*	*N*	*N*	*0*	*N*
Parking/Hour												
Bus Stops/Hour	*0*	*0*		*0*	*0*	*0*	*0*	*0*	*0*	*0*	*0*	*0*
Minimum Pedestrian Time		*3.2*			*3.2*			*3.2*			*3.2*	

Phasing	Excl. Left	EW Perm	03	04	Excl. Left	NS Perm	07	08
Timing	G = *13.0*	G = *10.0*	G = *0.0*	G = *0.0*	G = *8.0*	G = *68.0*	G = *0.0*	G = *0.0*
	Y = *4*	Y = *6*	Y = *0*	Y = *0*	Y = *5*	Y = *6*	Y = *0*	Y = *0*

Duration of Analysis (hrs) = *0.25* | Cycle Length C = *120.0*

Lane Group Capacity, Control Delay, and LOS Determination

	EB			WB			NB			SB		
Adjusted Flow Rate	*222*	*55*		*61*	*12*	*31*	*104*	*1391*	*47*	*69*	*821*	*82*
Lane Group Capacity	*333*	*129*		*299*	*140*	*1152*	*382*	*1809*	*1199*	*210*	*1774*	*1199*
v/c Ratio	*0.67*	*0.43*		*0.20*	*0.09*	*0.03*	*0.27*	*0.77*	*0.04*	*0.33*	*0.46*	*0.07*
Green Ratio	*0.24*	*0.08*		*0.24*	*0.08*	*0.81*	*0.68*	*0.57*	*0.84*	*0.68*	*0.57*	*0.84*
Uniform Delay d_1	*41.0*	*52.3*		*35.9*	*50.8*	*2.3*	*7.8*	*20.0*	*1.6*	*13.0*	*15.3*	*1.6*
Delay Factor k	*0.24*	*0.11*		*0.11*	*0.11*	*0.11*	*0.11*	*0.32*	*0.11*	*0.11*	*0.11*	*0.11*
Incremental Delay d_2	*5.0*	*2.3*		*0.3*	*0.3*	*0.0*	*0.4*	*2.1*	*0.0*	*0.9*	*0.2*	*0.0*
PF Factor	*1.000*	*1.000*		*1.000*	*1.000*	*1.000*	*1.000*	*1.000*	*1.000*	*1.000*	*1.000*	*1.000*
Control Delay	*46.0*	*54.5*		*36.3*	*51.0*	*2.3*	*8.2*	*22.0*	*1.6*	*13.9*	*15.5*	*1.6*
Lane Group LOS	*D*	*D*		*D*	*D*	*A*	*A*	*C*	*A*	*B*	*B*	*A*
Approach Delay	*47.7*			*27.8*			*20.5*			*14.2*		
Approach LOS	*D*			*C*			*C*			*B*		
Intersection Delay	*21.2*			Intersection LOS						*C*		

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SHORT REPORT

General Information		Site Information	
Analyst	*M.Hemmen*	Intersection	*US 441 at NW 147th Drive*
Agency or Co.	*MPH Transportation Planning*	Area Type	*CBD or Similar*
Date Performed	*2/10/2021*	Jurisdiction	*FDOT-D2 & City of Alachua*
Time Period	*PM Peak Hour*	Analysis Year	*2022 Existing + Project*

Volume and Timing Input

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes	*1*	*1*	*0*	*1*	*1*	*1*	*1*	*2*	*1*	*1*	*2*	*1*
Lane Group	*L*	*TR*		*L*	*T*	*R*	*L*	*T*	*R*	*L*	*T*	*R*
Volume (vph)	*212*	*26*	*60*	*55*	*13*	*87*	*111*	*1252*	*61*	*62*	*739*	*131*
% Heavy Vehicles	*2*	*2*	*2*	*2*	*2*	*2*	*2*	*2*	*2*	*2*	*4*	*2*
PHF	*0.90*	*0.90*	*0.90*	*0.90*	*0.90*	*0.90*	*0.90*	*0.90*	*0.90*	*0.90*	*0.90*	*0.90*
Pretimed/Actuated (P/A)	*A*	*A*	*A*	*A*	*A*	*A*	*A*	*A*	*A*	*A*	*A*	*A*
Startup Lost Time	*2.0*	*2.0*		*2.0*	*2.0*	*2.0*	*2.0*	*2.0*	*2.0*	*2.0*	*2.0*	*2.0*
Extension of Effective Green	*2.0*	*2.0*		*2.0*	*2.0*	*2.0*	*2.0*	*2.0*	*2.0*	*2.0*	*2.0*	*2.0*
Arrival Type	*3*	*3*		*3*	*3*	*3*	*3*	*3*	*3*	*3*	*3*	*3*
Unit Extension	*3.0*	*3.0*		*3.0*	*3.0*	*3.0*	*3.0*	*3.0*	*3.0*	*3.0*	*3.0*	*3.0*
Ped/Bike/RTOR Volume	*0*	*0*	*18*	*0*	*0*	*59*	*0*	*0*	*19*	*0*	*0*	*45*
Lane Width	*12.0*	*12.0*		*12.0*	*12.0*	*12.0*	*12.0*	*12.0*	*12.0*	*12.0*	*12.0*	*12.0*
Parking/Grade/Parking	*N*	*0*	*N*	*N*	*0*	*N*	*N*	*0*	*N*	*N*	*0*	*N*
Parking/Hour												
Bus Stops/Hour	*0*	*0*		*0*	*0*	*0*	*0*	*0*	*0*	*0*	*0*	*0*
Minimum Pedestrian Time		*3.2*			*3.2*			*3.2*			*3.2*	

Phasing	Excl. Left	EW Perm	03	04	Excl. Left	NS Perm	07	08
Timing	G = *13.0*	G = *10.0*	G = *0.0*	G = *0.0*	G = *8.0*	G = *68.0*	G = *0.0*	G = *0.0*
	Y = *4*	Y = *6*	Y = *0*	Y = *0*	Y = *5*	Y = *6*	Y = *0*	Y = *0*

Duration of Analysis (hrs) = *0.25* | Cycle Length C = *120.0*

Lane Group Capacity, Control Delay, and LOS Determination

	EB			WB			NB			SB		
Adjusted Flow Rate	*236*	*76*		*61*	*14*	*31*	*123*	*1391*	*47*	*69*	*821*	*96*
Lane Group Capacity	*331*	*127*		*283*	*140*	*1152*	*382*	*1809*	*1199*	*210*	*1774*	*1199*
v/c Ratio	*0.71*	*0.60*		*0.22*	*0.10*	*0.03*	*0.32*	*0.77*	*0.04*	*0.33*	*0.46*	*0.08*
Green Ratio	*0.24*	*0.08*		*0.24*	*0.08*	*0.81*	*0.68*	*0.57*	*0.84*	*0.68*	*0.57*	*0.84*
Uniform Delay d_1	*41.7*	*53.1*		*36.0*	*50.8*	*2.3*	*7.9*	*20.0*	*1.6*	*13.0*	*15.3*	*1.6*
Delay Factor k	*0.28*	*0.19*		*0.11*	*0.11*	*0.11*	*0.11*	*0.32*	*0.11*	*0.11*	*0.11*	*0.11*
Incremental Delay d_2	*7.1*	*7.6*		*0.4*	*0.3*	*0.0*	*0.5*	*2.1*	*0.0*	*0.9*	*0.2*	*0.0*
PF Factor	*1.000*	*1.000*		*1.000*	*1.000*	*1.000*	*1.000*	*1.000*	*1.000*	*1.000*	*1.000*	*1.000*
Control Delay	*48.8*	*60.6*		*36.4*	*51.2*	*2.3*	*8.4*	*22.0*	*1.6*	*13.9*	*15.5*	*1.6*
Lane Group LOS	*D*	*E*		*D*	*D*	*A*	*A*	*C*	*A*	*B*	*B*	*A*
Approach Delay	*51.7*			*28.3*			*20.4*			*14.0*		
Approach LOS	*D*			*C*			*C*			*B*		
Intersection Delay	*21.8*			Intersection LOS						*C*		

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þÿ The Gainesville Sun - Apr 08, 2021 - - - A11

Obituaries

Antonio Bartee



GAINESVILLE - Age 50, Barber, Gainesville High School Graduate, left this earthly scene on Monday, March 31, 2021, while at Shands Hospital.

The Celebration of Love will be held 1:30PM, Saturday, April 10, 2021, at Newnansville Cemetery (Alachua, FL) with Minister Kenneth Graham delivering Words of Comfort. There will only be a Private Family/Friends Viewing at the Funeral Home on Friday. The Procession will form at Mr. Bartee's residence, 2120 NW 54th Terr, Gainesville, FL at 12:30PM.

Left to cherish Antonio's Memory are his Mother – Helen Cave Bartee; (Father – David Bartee, Sr. preceded him in death); Brothers – Michael Dunbar and David Bartee, Jr.; Sisters – Cathy Thomas and Michelle Dunmore (Devera Bartee preceded him in death); Aunts, Uncles, Nieces, Nephews, Cousins and Friends.

Arrangements Entrusted To
DUNCAN BROTHERS' FUNERAL HOME
428 NW 8th Street
Gainesville, Florida

Andrea Mundorff

STARKE - Andrea "Cookie" Katherine Mundorff age 79 of Starke passed away on Wednesday, March 31, 2021 at North Florida Regional Hospital in Gainesville. Ms. Cookie Mundorff was born on August 14, 1941 to the late John Baptist and Faith (Clinton) Gerard in Edwardsville, Illinois. She was an umpire for local recreation baseball and softball leagues, active with the Florida Youth Challenge, Kiwanis Club of Starke, and she was a Cub Scouts Den Mother. Cookie was also an avid St. Louis Cardinals fans.

She was preceded in death by her parents; brothers, John B. Gerard and Andre Gerard; and one sister, Georgia Lee Horan. Survivors include, her children, Terry (Elizabeth) Mundorff of Melrose, Pat (Brenda) Mundorff of Starke, and Luana Mundorff of Starke; sibling, Jules Gerard of Missouri; grandchildren, Dean, Christopher, Brittany, Travis, Tyler, Jessica, Justin, Janet, Kathie, T.J., and Alex; and 14 great-grandchildren.

A Celebration of Life will be held at 11:00 a.m. on Saturday, April 10, 2021 at the Governor Charley E. Johns Conference Center in Starke. A visitation will precede the services beginning at 10:00 a.m. Chub Johnson will be officiating.

In lieu of flowers donations may be made to Starke Kiwanis Club in care of the Kids Corner: https://www.bradfordelections.com/Kids-Corner.

Arrangements are by JONES-GALLAGHER FUNERAL HOME OF STARKE 904-964-6200.

Mrs. Heather Mayze



Mrs. Heather Mayze transitioned peacefully at home on April 5, 2021 surrounded by family and friends. A Celebration of Life Ceremony will be held at 2:30pm on Saturday, April 10, 2021 at Citizens Field in Gainesville, Florida; Pastor Nathaniel Hicks officiating. Mrs. Mayze worked at Gainesville Job Corp. and was a beloved and involved person in our community during her 33 years living in Gainesville, having relocated here from Sarasota.

She was preceded in death by a son, Javon Cobb. She will be sorely missed by her family: a loving husband, Troy Mayze; sons, Marcus Williams, Troy Mayze, Antonio Mayze; daughers, Zendaya Cobb, Darianna Babb, Tiffany Mayze; brothers, Dwayne King, Marcus Rollins, Andrew Hill; sisters Gretchen Brown, Brenda King Mitchell, Renee Hall, Yashika Cohen, Tameka Rollins; eleven grandchildren and a host of family and friends.

Mrs. Mayze will be viewably by the public on Friday, April 9, 2021 from 2pm-7pm at Chestnut Funeral Home, 18 NW 8th Ave. Gainesville, Fl 32601, 855-5 -CHESTNUT; wecare@achestnutfuneral.com. Condolences may be sent and flowers may be ordered at www.chestnutfh.com.

Santa Marie Neal



GAINESVILLE - Santa Marie Neal, 49 of Gainesville, FL, Patient Financial Consultant, departed this life March 29, 2021. Celebration of Life Service will be held 9:00 a.m. at Transformation Bible Vision Ministry, 5009 NW 34th Street, Gainesville, FL, Apostle Tony Perryman Sr. is officiating. Burial will be in Pine Grove Cemetery. Viewing on Friday at Pinkney-Smith Funeral Home, 727 NW 2nd Street, Gainesville, FL 4 p.m. - 7 p.m. and Saturday at the church 8:30 a.m. - 9 a.m. Family will meet at the church 8:45 a.m. PINKNEY-SMITH FUNERAL HOME OF GAINESVILLE.





Cecil Mack, Sr.

Mr. Cecil Mack, Sr. of Gainesville, FL entered into thy masters Joy on Friday, April 2,, 2021. Celebration of life service for Mr. Mack will be held Saturday, April 10, 2021 at 1:00pm from Faith Missionary Baptist Church, 2905 SE 21st Avenue Gainesville, FL. Pastor Kenneth Day officiating; Place of eternal rest will follow at the Pine Grove Cemetery. Public visitation will be held Friday at Phillip & Wiley Mortuary, 310 State Road 26 Melrose, Florida from 12:00-7:00pm. He leaves to cherish his memory 14 children, sisters and a host of other family and friends. Family and friends are asked to assemble at 1309 SE 20th street Gainesville, FL at 12:00pm to form the cortège.

phillipwiley.com
352-475-2000

Phillip & Wiley MORTUARY, INC.



Bias bill

Continued from Page 3A

from what it described as shielding students from views that are considered free speech by the Constitution. Shielding refers to limiting students', faculty members', or staff members' access to, or observation of, ideas and opinions that they may find uncomfortable, unwelcome, disagreeable or offensive.

Last month, Rep. Omari Hardy, D-Palm Beach, said he believed the bill's "shielding" provision was vague and could have unexpected consequences, such as removing an administrator's ability to limit speech that would disrupt classrooms.

"Nearly anything the administrator and professor would do to control the academic environment could be recast as shielding," Hardy said.

Hardy said, if the bill were passed, students could be able to show videos of American soldiers being wounded overseas, discuss pro-pedophilia positions and distribute nude photographs of ex-partners on campus, if they say their actions are contributing to class discussions or are in protest.

Roach said Hardy's criticism is outside of scope of his bill, because his examples were already legal on college campuses.

"If a student wants to show them something on their phone, whether it's an American soldier in combat or some type of pornography, there is nothing under the law to stop them from doing that right now," Roach said.

Karen Morian, the president of the United Faculty of Florida labor union, said she fears school administrators could pressure non-tenured instructors to complete the survey, disclosing their personal political beliefs. She also said the response rates from small but vocal groups of politically active students could skew results.

The bill would allow students to record lectures without a professor's permission. However, the recordings could not be published, allowing them only for personal use or as evidence in university investigations or legal proceedings. If a recording is published, the professor could seek damages up to $200,000, according to the bill.

Morian said she doubted professors could identify students who post their recordings anonymously online, which would make seeking damages for misuse impossible. A professor's syllabi, emails and course materials are generally treated as public records at state colleges and universities.

Roach said he has been surprised by the criticism the bill has received and views it as an extension of partisan politics.

"If a Republican sponsors a bill, the opposing party is going to vote down on it without examining its merits." Roach said.

This story was produced by Fresh Take Florida, a news service of the University of Florida College of Journalism and Communications.

Biden

Continued from Page 1A

– often assembled from parts and milled with a metal-cutting machine – often lack serial numbers used to trace them. It's legal to build a gun in a home or a workshop and there is no federal requirement for a background check.

The president's plans were previewed by a person familiar with the expected actions who was not authorized to publicly discuss them.

Senior administration officials confirmed that the Justice Department would issue a new proposed rule aimed at reining in ghost guns within 30 days, but offered no details on the content of the rule.

The Justice Department will also issue a proposed rule within 60 days tightening regulations on pistol-stabilizing braces, like the one used by the Boulder, Colorado, shooter in a massacre last month that left 10 dead. The rule would designate pistols used with stabilizing braces as short-barreled rifles, which, under the National Firearms Act, require a federal license to own and are subject to a more thorough application process and a $200 tax.

The Justice Department will also publish model red flag legislation within 60 days, which the administration says will make it easier for states to adopt their own red flag laws. Such laws allow for individuals to petition a court to allow the police to confiscate weapons from a person deemed to be a danger to themselves or others.

And it will begin to provide more data on firearms trafficking, starting with a new comprehensive report on the issue, which the Biden administration says it hasn't done in over two decades.

The president will also announce investments in community violence intervention programs, which are aimed at reducing gun violence in urban communities, across five federal agencies.

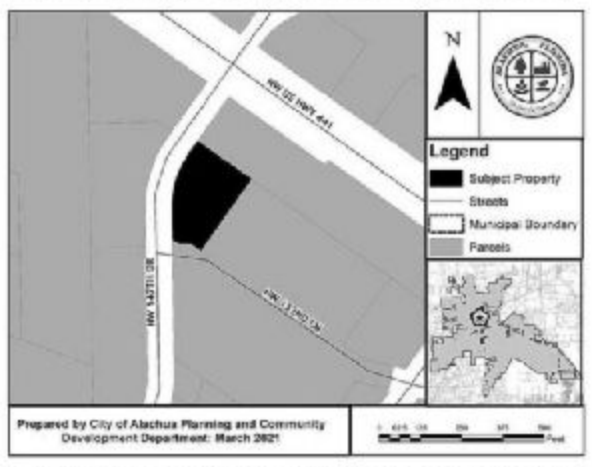
Administration officials hinted there may be more to come from the administration on guns, calling the round of executive actions "initial steps" that were completed under Garland's purview within the first few weeks of his tenure.

The ATF is currently run by acting Director Regina Lombardo. Gun-control advocates have emphasized the significance of the ATF director in enforcing the nation's gun laws, and Chipman is certain to win praise from them. During his time as a senior policy adviser with Giffords, he spent considerable effort pushing for greater regulation and enforcement on ghost guns, reforms of the background check system and measures to reduce the trafficking of illegal firearms.

Prior to that, Chipman spent 25 years as an agent at the ATF, where he worked on stopping a trafficking ring that sent illegal firearms from Virginia to New York, and served on the ATF's SWAT team. Chipman is a gun owner himself.

NOTICE OF PUBLIC HEARING BEFORE THE PLANNING AND ZONING BOARD OF THE CITY OF ALACHUA, FLORIDA

Notice is hereby given that the Planning & Zoning Board of the City of Alachua will hold a public hearing on April 20, 2021, at 6:00 p.m. The hearing will be held in the James A. Lewis Commission Chambers in City Hall, located at 15100 NW 142nd Terrace, Alachua, Florida, to consider the following: A request by Christopher Potts, P.E., of JBrown Professional Group, Inc., applicant and agent for Shakti Management, LLC, property owner, for consideration of a Site-Specific Amendment to the Official Zoning Atlas (Rezoning) to amend the Official Zoning Atlas from Central Business District (CBD) to Commercial Intensive (CI) on a ±1.09 acre subject property. The property subject to the proposed amendment is located south of US HWY 441, to the east of NW 147th Drive, and north of the Hitchcock's Plaza; Tax Parcel Number 03869-004-000; Existing Future Land Use Map (FLUM) Designation: Commercial. Existing Zoning: Central Business District (CBD).



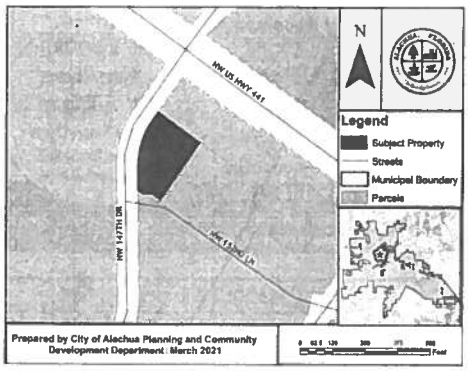
At the public hearing, all interested parties may appear and be heard with respect to the application. Copies of the application are available for public inspection at the Planning and Community Development Department, 15100 NW 142nd Terrace, Alachua, Florida, on any regular business day between the hours of 7:30 a.m. to 6:00 p.m. Written comments on the application may be sent to the following address: City of Alachua, Planning and Community Development, P.O. Box 9, Alachua, FL 32616. Notice is given pursuant to Section 286.0105, Florida Statutes, that, in order to appeal any decision made at the public hearing, you will need a record of the proceedings, and that, for such purpose, you may need to ensure that a verbatim record of the proceedings is made, which includes the testimony and evidence upon which the appeal is to be based. In accordance with the Americans with Disabilities Act, any persons with a disability requiring reasonable accommodation in order to participate in this meeting should call the City Clerk at (386) 418-6100 x 1501 at least 48 hours prior to the public hearing.

GF-GH059245

City of ALACHUA

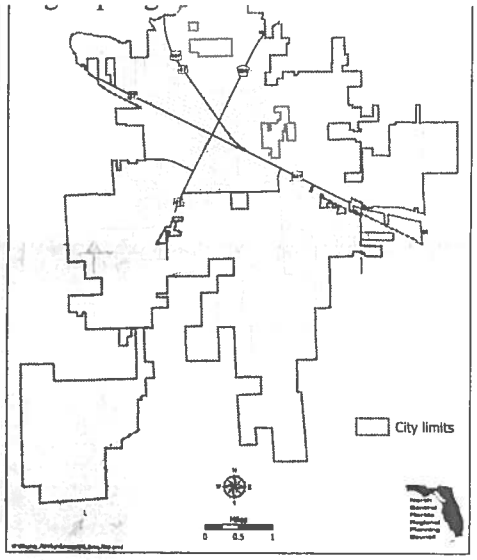
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(Published: Alachua County Today - April 08, 2021)



The first of two public hearings on the proposed amendment and the ordinance adopting said amendment will be held April 22, 2021 at 6:30 p.m., or as soon thereafter as the matter can be heard, in the City Commission Chambers, City Hall located at 23718 West U.S. Highway 27, High Springs, Florida. The title of said ordinance shall read, as follows:

ORDINANCE NO. LDC 21-02

AN ORDINANCE OF THE CITY OF HIGH SPRINGS, FLORIDA, AMENDING THE CITY OF HIGH SPRINGS LAND DEVELOPMENT CODE, APPENDIX B, LAND DEVELOPMENT CODE; ARTICLE XI ADMINISTRATION; PART 11.00.00 GENERALLY; TABLE 11.00.01 TYPE OF PROCEDURE BY DEVELOPMENT OPTION; PART 11.01.00 PROCEDURE FOR OBTAINING PERMITS; SECTION 11.01.07.01 DETERMINATION OF REQUIRED REVIEW; PART 11.02.00 PROCEDURES FOR OBTAINING DEVELOPMENT ORDERS; SECTION 11.02.02.01 LEVEL ONE, ACTION BY SITE PLAN REVIEW COMMITTEE; PART 11.04.00 SITE PLAN REVIEW; SECTION 11.04.04 PRELIMINARY SITE PLAN REQUIRED; SECTION 11.04.05 FINAL SITE PLAN REQUIRED; STRIKING PART 11.01.00 PROCEDURE FOR OBTAINING PERMITS; SECTION 11.01.07.03 PLATTED SUBDIVISION WITH NO IMPROVEMENT FACILTIIES, PROVIDING FOR SEVERABILTY; REPEALING ALL ORDINANCES IN CONFLICT AND PROVIDING AN EFFECTIVE DATE.

The public is advised to visit the website: https://www.highsprings.us for up-to-date information concerning the public hearing. If the public hearing is also held via communications media technology, all pertinent information will be made available via the City website https://www.highsprings.us or may be obtained by calling the Office of City Clerk at 386.454.1416.

The public hearing may be continued to one or more future dates. Any interested party shall be advised that the date, time and place of any continuation of the public hearing shall be announced during the public hearing and that no further notice concerning the matter will be published, unless said continuation exceeds six calendar weeks from the date of the above referenced public hearing.

At the aforementioned public hearing, all interested parties may appear to be heard with respect to the amendment.

Copies of the amendment and the ordinance adopting said amendment are available for public inspection at the Office of the City Clerk, 23718 West U.S. Highway 27, High Springs, Florida, during regular business hours.

All persons are advised that if they decide to appeal any decision made at the above referenced public hearing, they will need a record of the proceedings, and that for such purpose they may need to ensure that a verbatim record of the proceedings is made, which record includes the testimony and evidence upon which the appeal is to be based.

In accordance with the American with Disabilities Act, any person with disabilities needing any special accommodation to participate in City meetings should contact the Office of the City Clerk, 23718 West U.S. Highway 27, High Springs, Florida, 32643 or via telephone: 386.454.1416, ext. 7237, at least 48 hours prior to the date of the public hearing.

(Published: Alachua County Today - April 08, 2021)

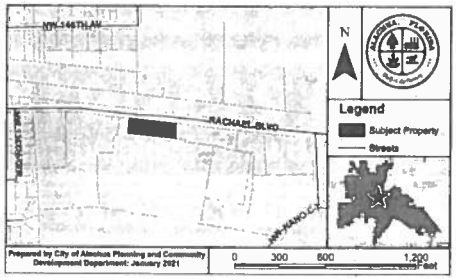
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City of ALACHUA

NOTICE OF PUBLIC HEARING BEFORE THE PLANNING AND ZONING BOARD OF THE CITY OF ALACHUA, FLORIDA

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At the public hearing, all interested parties may appear and be heard with respect to the application. Copies of the application are available for public inspection at the Planning and Community Development Department, 15100 NW 142nd Terrace, Alachua, Florida, on any regular business day between the hours of 7:30 a.m. to 6:00 p.m. Written comments on the application may be sent to the following address: City of Alachua, Planning and Community Development, P.O. Box 9, Alachua, FL 32616. Notice is given pursuant to Section 286.0105, Florida Statutes, that, in order to appeal any decision made at the public hearing, you will need a record of the proceedings, and that, for such purpose, you may need to ensure that a verbatim record of the proceedings is made, which includes the testimony and evidence upon which the appeal is to be based. In accordance with the Americans with Disabilities Act, any persons with a disability requiring reasonable accommodation in order to participate in this meeting should call the City Clerk at (386) 418-6100 x 101 at least 48 hours prior to the public hearing.

(Published: Alachua County Today - April 08, 2021)

City of ALACHUA

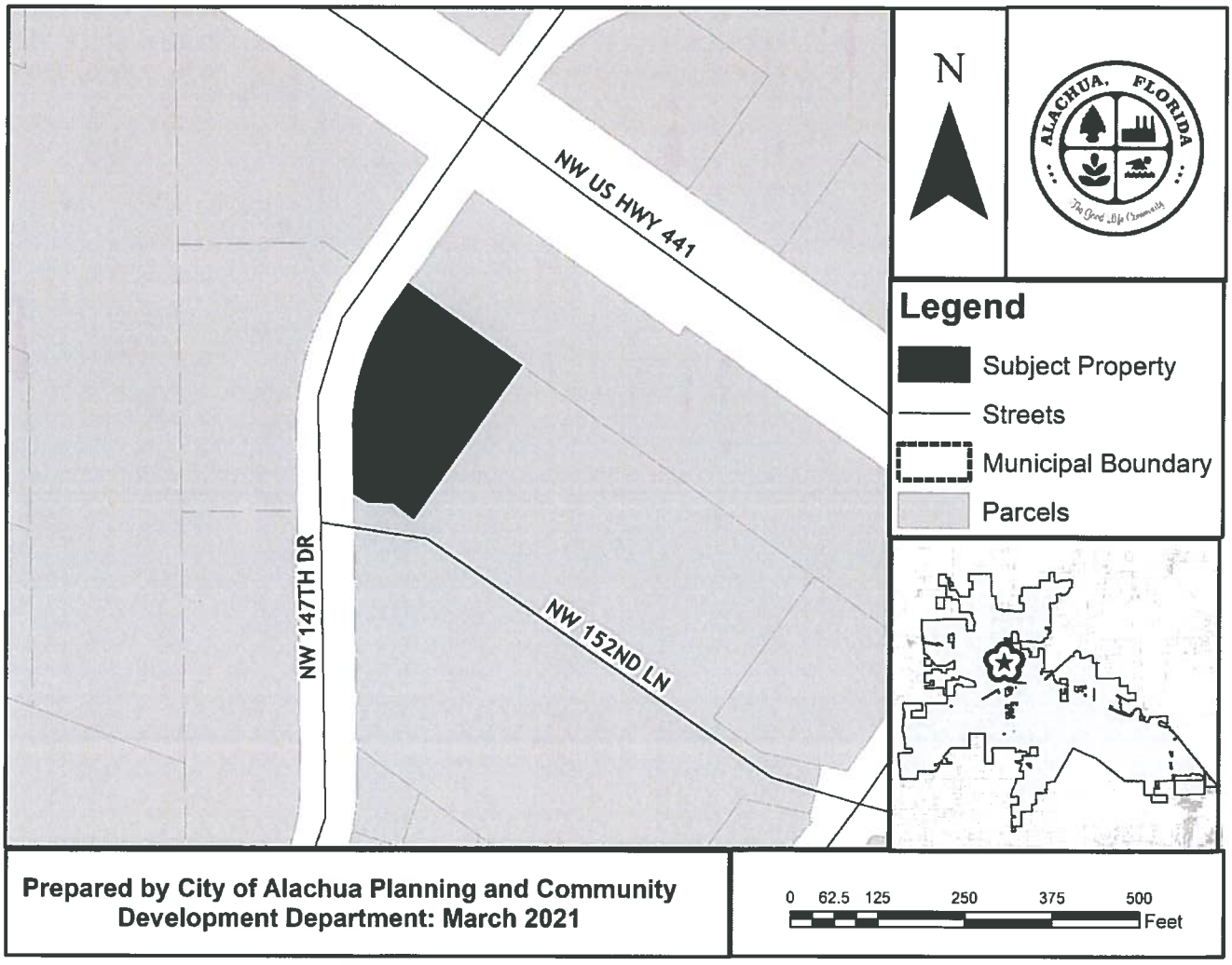
THE GOOD LIFE COMMUNITY

Mailed 4/5/2021

KC

NOTICE OF PUBLIC HEARING BEFORE THE PLANNING AND ZONING BOARD OF THE CITY OF ALACHUA, FLORIDA

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(Continues)

P.O. Box 9 ♦ Alachua, Florida 32616-0009
Phone: (386) 418-6121 ♦ Fax: (386) 418-6130

City of ALACHUA

THE GOOD LIFE COMMUNITY

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Antoinette Endelicato
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Gainesville, FL 32653

President
TCMOA
1000 Turkey Creek
Alachua, FL 32615

Tamara Robbins
PO Box 2317
Alachua, FL 32616

Dan Rhine
288 Turkey Creek
Alachua, FL 32615

Linda Dixon, AICP
PO Box 115050
Gainesville, FL 32611

Michele L. Lieberman
County Manager
12 SE 1st Street
Gainesville, FL 32601

Tom Gorman
9210 NW 59th Street
Alachua, FL 32653

Craig Parenteau
FDEP
4801 Camp Ranch Road
Gainesville, FL 32641

Bonnie Flynn
16801 NW 166th Drive
Alachua, FL 32615

Richard Gorman
5716 NW 93rd Avenue
Alachua, FL 32653

Jeannette Hinsdale
P.O. Box 1156
Alachua, FL 32616

Hugh & Jean Calderwood
P.O. Box 2307
Alachua, FL. 32616

Peggy Arnold
410 Turkey Creek
Alachua, FL 32615

Lynn Coullias
7406 NW 126th Ave
Alachua, FL 32615

Lisia Jenkins
P.O. Box 1071
Alachua, FL. 32616

David Forest
23 Turkey Creek
Alachua, FL 32615

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3727 SW 92nd Drive
Gainesville, FL 32608

Young Estates LLC
14414 NW 60th Avenue
Alachua, FL 32615

Starmax FDA LLC
C/O Family Dollar Stores Inc.
500 Volvo Parkway
Chesapeake, VA 23320

Canvas Church FL INC
P.O. Box 219
Alachua, FL 32616

Hitchcock & Sons INC
29220 NW 122nd Street
Alachua, FL 32615

Wiggins Ethel J Trustee
P.O. Box 1857
Alachua, FL 32616

Alachua Towne Centre Association INC
13505 NW 88th Place
Alachua, FL 32615

Advance Stores Co INC #9236
P.O. Box 2710 ATTN: TAX ACCOUNTING
Roanoke, VA 24001

Tomlinson Property Group LLC
3280 N. Main Street
Gainesville, FL 32609

Alachua Professional Plaza LLC
15260 NW 147th Drive, Suite A
Alachua, FL 32615

Campus USA Credit Union
14007 NW 1st Road
Newberry, FL 32669

Christopher Allan Korosic
15710 NW US Highway 441
Alachua, FL 32615

AFFIDAVIT FOR POSTED LAND USE SIGN

I Brandon Williams, POSTED THE LAND USE
(Name)
SIGN ON 5 April 2021 FOR THE Alachua Towne Centre Drive Through RZ
(Date) (State type of action and project name)
LAND USE ACTION.

AS PER ARTICLE 2.2.9 D OF THE LAND DEVELOPMENT REGULATIONS.

THIS WILL BE INCLUDED IN THE STAFF REPORT.

(Signature)

3
(Number of signs)

City of Alachua
THE GOOD LIFE COMMUNITY

Board/Committee Agenda Item

MEETING DATE: 4/20/2021
SUBJECT: Land Development Regulations Text Amendment : A request by Christopher Potts P.E.,of JBrown Professional Group, LLC to amend the City of Alachua Land Development Regulations (LDRs) as follows: amending Section 4.3.2 (I)(4), relating to the use-specific standards for minor utilities; amending Section 2.4.4 (D) (7), relating to standards for special exception permits (Legislative Hearing)

PREPARED BY: Adam Hall, AICP, Planner

RECOMMENDED ACTION:
Staff recommends that the Planning & Zoning Board find the proposed Text Amendments to the City's Land Development Regulations to be consistent with the City of Alachua Comprehensive Plan and in compliance with the City's Land Development Regulations and transmit such finding to the City Commission with a recommendation to approve upon making the following motion:

Based upon the presentation to this Board and Staff's recommendation, this Board finds the proposed text amendments to the City's Land Development Regulations to be consistent with the City of Alachua Comprehensive Plan and in compliance with the City's Land Development Regulations and transmits such finding to the City Commission with a recommendation to approve.

Summary

This application is a request to amend various sections of the Land Development Regulations (LDRs). The principle focus of the proposed amendment pertains to the use-specific standards for minor utilities. Specifically, the amendment creates additional requirements for stormwater management areas, which are defined in the City's LDRs as minor utilities, when the areas are located outside of zoning district that the principal use is permitted in. Generally, a stormwater management system is considered to be a use associated with a principal use, and therefore must be contained within a zoning district that permits the principal use. As an example, a commercial plaza would be required to be located within a zoning district, such as Commercial Intensive (CI), that permits that use. If the same property owner owned adjacent land with an Agricultural (A) zoning designation they would not be able to place the stormwater management area serving the commercial plaza in the area with the Agricultural (A) zoning designation as the principal use (the commercial plaza) is not permitted within the Agricultural (A) zoning designation.

The proposed amendment would permit, by special exception, the placement of storm water detention and retention facilities in areas of different zoning designations, with conditions. These conditions would help ensure the compatibility with surrounding properties through enhanced design and sufficient landscaped buffering.

ATTACHMENTS:

Description

- Draft Ordinance
- Staff Report and Staff Supporting Materials
- Application and Supporting Materials
- Public Notice Material for April 20, 2021 Planning and Zoning Board Hearing

ORDINANCE 21-XX

AN ORDINANCE OF THE CITY OF ALACHUA, FLORIDA, RELATING TO THE AMENDMENT OF THE CITY'S LAND DEVELOPMENT REGULATIONS ("LDRS"); AMENDING SUBPART B OF THE CITY OF ALACHUA CODE OF ORDINANCES, LAND DEVEVELOPMENT REGULATIONS; AMENDING SECTION 4.3.2 (I)(4), RELATING TO USE-SPECIFIC STANDARDS FOR MINOR UTILITIES; AMENDING SECTION 2.4.4 (D)(7) RELATING TO STANDARDS FOR SPECIAL EXCEPTIONS PROVIDING A REPEALING CLAUSE; PROVIDING SEVERABILITY; AND PROVIDING AN EFFECTIVE DATE.

R E C I T A L S

WHEREAS, a Text Amendment ("Amendment") to the City's Land Development Regulations ("LDRs"), as described below, has been proposed; and

WHEREAS, the City advertised a public hearing to be held before the Planning and Zoning Board, sitting as the Local Planning Agency ("LPA"), on April 8, 2021; and

WHEREAS, the LPA conducted a public hearing on the proposed Amendment on April 20, 2021, and the LPA reviewed and considered all comments received during the public hearing concerning the proposed Amendment and made its recommendation to the City Commission; and

WHEREAS, the City advertised public hearings to be held before the City Commission on _____, 2021, and on _____, 2021; and

WHEREAS, the City Commission conducted public hearings on the proposed Amendment on _____, 2021, and _____, 2021, and provided for public participation at both public hearings; and

WHEREAS, the City Commission has determined and found the Amendment to be consistent with the City's Comprehensive Plan and City's LDRs; and

WHEREAS, for reasons set forth in this ordinance that is hereby adopted and incorporated as findings of fact, that the Alachua City Commission finds and declares that the enactment of this Amendment is in the furtherance of the public health, safety, morals, order, comfort, convenience, appearance, prosperity, or general welfare;

NOW THEREFORE BE IT ORDAINED BY THE CITY COMMISSION OF THE CITY OF ALACHUA, FLORIDA:

Section 1. Interpretation of Recitals

The above recitals are true and correct and incorporated in this ordinance.

Section 2. Findings of Fact and Conclusions of Law

The authority for the enactment of this ordinance is Chapter 163, Part I, Florida Statutes; Sections 166.021 and 166.041; and the City's Comprehensive Plan.

Section 3. Amendment to the Land Development Regulations

The proposed Amendment to the City Land Development Regulations are attached as Exhibit "A" and are hereby incorporated herein by reference.

Section 4. Codification of and Correction of Scrivener's Errors

The City Manager or designee, without public hearing, is authorized to correct any typographical errors which do not affect the intent of this ordinance. A corrected copy shall be posted in the public record.

Section 5. Ordinance to be Construed Liberally

This ordinance shall be liberally construed in order to effectively carry out the purposes hereof which are deemed to be in the best interest of the public health, safety, and welfare of the citizens and residents of the City of Alachua, Florida.

Section 6. Repealing Clause

All ordinances or parts of ordinances in conflict herewith are, to the extent of the conflict, hereby repealed.

Section 7. Severability

It is the declared intent of the City Commission of the City of Alachua that, if any section, sentence, clause, phrase, or provision of this ordinance is for any reason held or declared to be unconstitutional, void, or inoperative by any court or agency of competent jurisdiction, such holding of invalidity or unconstitutionality shall not affect the remaining provisions of this ordinance, and the remainder of the ordinance after the exclusions of such part or parts shall be deemed to be valid.

Section 8. Effective Date

This ordinance shall take effect immediately upon its adoption by the City Commission and the signature of the Mayor.

Passed on First Reading the ____ day of _____, 2021.

PASSED and ADOPTED, in regular session, with a quorum present and voting, by the City Commission, upon second and final reading this _____day of _____, 2021.

CITY COMMISSION OF THE
CITY OF ALACHUA, FLORIDA

Gib Coerper, Mayor
SEAL

ATTEST:

Adam Boukari, City Manager/Clerk

APPROVED AS TO FORM

Marian B. Rush, City Attorney

EXHIBIT "A"

Section 4.3.2 (I) of the City's LDRs is amended as follows (text that is underlined is to be added and text that is shown as ~~strikethrough~~ is to be removed). Except as amended herein, the remainder of Section 4.3.2 (I) remains in full force and effect:

(4) *Utility, minor*. A minor utility use shall comply with the following standards:

(a) *Location*. Be located within reasonable proximity of the area to be served. Stormwater detention and retention facilities may be permitted by Special Exception Permit on properties with differing zoning designations under common ownership, subject to the following requirements:

(i) The zoning designation of the location of the stormwater detention and retention facility allows minor utilities; and,

(ii) The stormwater detention and retention facility meets the compatibility requirements as set forth in Subsection 4.3.2 (I)(4)(b); and,

(iii) The stormwater detention and retention facility provides an enhanced natural (e.g., increased topographic relief, vegetative diversity, etc) or recreational (e.g., perimeter trail, boardwalk, interpretive kiosk, etc) functional use.

(b) *Compatibility*. Provide adequate setbacks, screening and buffering around the perimeter of the proposed use if it is deemed necessary to ensure land use compatibility with surrounding uses.

Section 2.4.4 (7) of the City's LDRs is amended as follows (text that is underlined is to be added and text that is shown as ~~strikethrough~~ is to be removed). Except as amended herein, the remainder of Section 2.4.4 remains in full force and effect:

2.4.4 (7) ~~Site plan~~ Drawings. A site plan (Subsection 2.4.9 of this section) or preliminary plat (Subsection 2.4.10 (G)(3) of this section) has been prepared that demonstrates how the proposed special exception use complies with the other standards of this subsection.

City of Alachua
Planning & Community Development Department
Staff Report

Planning & Zoning Board Hearing Date: April 20, 2021
Legislative Hearing

SUBJECT:	A request to amend the City of Alachua Land Development Regulations as follows: amending Section 4.3.2 (I)(4), relating to the use-specific standards for minor utilities; amending Section 2.4.4 (D) (7), relating to standards for special exception permits
APPLICANT/AGENT:	Christopher Potts, P.E., JBrown Professional Group
PROJECT PLANNERS:	Adam Hall, AICP
RECOMMENDATION:	Staff recommends that the Planning & Zoning Board find the proposed Text Amendments to the City's Land Development Regulations to be consistent with the City of Alachua Comprehensive Plan and in compliance with the City's Land Development Regulations and transmit such finding to the City Commission with a recommendation to approve.
RECOMMENDED MOTION:	*Based upon the presentation to this Board and Staff's recommendation, this Board finds the proposed text amendments to the City's Land Development Regulations to be consistent with the City of Alachua Comprehensive Plan and in compliance with the City's Land Development Regulations and transmits such finding to the City Commission with a recommendation to approve.*

SUMMARY

This application is a request to amend two sections of the Land Development Regulations (LDRs). The principle focus of the proposed amendment pertains to the use-specific standards for minor utilities. Specifically, the amendment creates additional requirements for stormwater management areas, which are defined in the City's LDRs as minor utilities, when the areas are located outside of the zoning district that the principal use is permitted in. Generally, a stormwater management system is considered to be a use associated with a principal use, and therefore must be contained within a zoning district that permits the principal use. As an example, a commercial plaza would be required to be located within a zoning district, such as Commercial Intensive (CI), that permits that use. If the same property owner owned adjacent land with an Agricultural (A) zoning designation they would not be able to place the stormwater management area serving the commercial plaza in the area with the Agricultural (A) zoning designation as the principal use (the commercial plaza) is not permitted within the Agricultural (A) zoning designation.

The proposed amendment would permit, by special exception, the placement of storm water detention and retention facilities in areas where is a different zoning designations than the principal use, with conditions. These conditions would help ensure the compatibility with surrounding properties through enhanced design and sufficient landscaped buffering.

PROPOSED LDR TEXT AMENDMENTS

The proposed amendments are displayed below in their entirety.

AMENDING SECTION 4.3.2 (I) RELATING TO USE-SPECIFIC STANDARDS FOR MINOR UTILTIES

(4) *Utility, minor.* A minor utility use shall comply with the following standards:

(a) *Location.* Be located within reasonable proximity of the area to be served. Stormwater detention and retention facilities may be permitted by Special Exception Permit on properties with differing zoning designations under common ownership, subject to the following requirements:

(i) The zoning designation of the location of the stormwater detention and retention facility allows minor utilities; and,

(ii) The stormwater detention and retention facility meets the compatibility requirements as set forth in Subsection 4.3.2 (I)(4)(b); and,

(iii) The stormwater detention and retention facility provides an enhanced natural (e.g., increased topographic relief, vegetative diversity, etc) or recreational (e.g., perimeter trail, boardwalk, interpretive kiosk, etc) functional use.

(b) *Compatibility.* Provide adequate setbacks, screening and buffering around the perimeter of the proposed use if it is deemed necessary to ensure land use compatibility with surrounding uses.

AMENDING SECTION 2.4.4 RELATING TO SPECIAL EXCEPTION STANDARDS

2.4.4 (D) (7) ~~Site plan~~ Drawings. A site plan (Subsection 2.4.9 of this section) or preliminary plat (Subsection 2.4.10 (G)(3) of this section) has been prepared that demonstrates how the proposed special exception use complies with the other standards of this subsection.

CONSISTENCY WITH THE COMPREHENSIVE PLAN

The Goals, Objectives, and Policies (GOPs) identified below are provided to establish a basis of the application's consistency with the Comprehensive Plan. There may be additional GOPs which the application is consistent with that are not identified within this report. An evaluation and findings of consistency with the identified GOPs is also provided below.

FUTURE LAND USE ELEMENT

Goal 1: Future Land Use Map 2035

The City shall maintain a Future Land Use Map in order to effectively guide development in a sustainable manner and to ensure economic prosperity and stability while maintaining a high quality of life for all of its present and future citizens, businesses, and visitors.

Evaluation & Findings: The applicant contends that the application will be consistent with this Goal as the amendment will allow more flexibility in locating stormwater ponds, which will help to ensure economic prosperity. The proposed standards will help to maintain the City's high quality of life.

Objective 2.5: Open Space Standards

The City shall utilize open space requirements to preserve the rural character of Alachua, protect natural resources, and provide spaces for people to recreate and gather.

Policy 2.5.a: There shall be a minimum of 10% percent open space required. The City shall establish incentives for the provision of open space beyond minimum requirements.

Policy 2.5.b: Open space shall not be limited to unusable portions of project sites. A portion of open space shall be usable and functional.

Evaluation & Findings: The applicant contends that the application will be consistent with this Objective and these policies as the amendment will permit more useable open space through enhanced stormwater design.

Objective 5.2: Availability of facilities and services

The City shall utilize a concurrency management system to ensure that the adopted levels of service standards are maintained.

Policy 5.2.a: All new development shall meet level of service requirements for roadways, potable water, sanitary sewer, stormwater, solid waste, and public schools in

accordance with LOS standards adopted in the elements addressing these facilities.

Policy 5.2.b: The concurrency management system shall specify the period for which certification of level of service compliance shall be valid until actual development pursuant to a final development order or Chapter 163 Development Agreement has commenced.

Evaluation & Findings: The applicant contends that the application will be consistent with this Objective and these policies as the amendment will provide additional flexibility to meet the level of service standards for stormwater.

COMMUNITY FACILITIES ELEMENT

Goal 3: Stormwater

The City shall develop and maintain a stormwater management system that minimizes flooding and protects, preserves, and enhances desirable water quality conditions, and, where possible, preserves and utilizes existing natural features.

Objective 3.1: Drainage and Stormwater Level of Service Standards and Implementation

The City shall ensure the provision of drainage and stormwater retention through level of service standards and design requirements to minimize flooding and to protect and improve water quality.

Policy 3.1.a: The City hereby establishes the following water quantity and quality level of service standards for drainage facilities:

For all projects which are located completely within a stream or open lake watershed, detention systems must be designed such that the peak rate of post- development runoff will not exceed the peak-rate of pre-development runoff for storm events up through and including either:

1. A design storm with a 10-year, 24-hour rainfall depth with Soil Conservation Service Type II distribution falling on average antecedent moisture conditions for projects serving exclusively agricultural, silvicultural, conservation, or recreational uses; or
2. A design storm with 100-year critical duration rainfall depth for projects serving any land use other than agricultural, silvicultural, conservation, or recreational uses.
3. The level of service standard for water quality treatment shall be treatment for the "first one inch" of runoff, and shall be in compliance with the design and performance standards established in Rules 40C-42.025 and 40C-42.035, Florida Administrative Code to ensure that the receiving water quality standards of Rule 62.302.500, Florida Administrative Code are met to ensure their water quality is not degraded below the minimum

conditions necessary to maintain their classifications as established in Rule 62-302, Florida Administrative Code. These standards shall apply to all new development and redevelopment. Infill residential development within improved residential areas or subdivisions existing prior to the adoption of this Comprehensive Plan, must ensure that its post-development stormwater runoff will not contribute pollutants which will cause the runoff from the entire improved area or subdivision to degrade receiving water bodies and their water quality as stated above.

Policy 3.1.b: The City shall require the construction of roads within new subdivisions to be arranged so that the grades of the streets conform as closely as possible to the original topography to prevent the interruption of natural drainage flows, including sheet flow and flow to isolated wetland systems.

Policy 3.1.c: The City shall require a certification by the preparer of the permit plans that all construction activity undertaken shall incorporate erosion and sediment controls during construction.

Policy 3.1.d: Priorities for upgrading existing stormwater management facilities shall be scheduled in the Capital Improvements Plan as needed.

Policy 3.1.e: The City shall provide incentives for the use of pervious surfaces in new development and redevelopment in order to reduce the size of retention basins and increase the area available to receive runoff.

Policy 3.1.f: The City shall permit the use of off-site retention and detention facilities when the off-site facilities are part of previously approved master stormwater retention or detention facility designed to accommodate the stormwater runoff generated by the development.

Objective 3.2: Stormwater Facilities Monitoring and Maintenance

The City shall maintain records of all existing and newly constructed stormwater systems in the City.

Policy 3.2.a: The City shall quantify and assess any deficiencies in its existing stormwater management system, by requesting funding from the Florida Department of Environmental Protection and/or the Suwannee River Water Management District to prepare a City-wide stormwater master plan which will determine necessary design capacities and hydraulic demands for any needed stormwater management facilities and assess the performance of existing facilities with regard to flood control, water quality treatment and impact on the City's surface water and groundwater. As an alternative to a City-wide stormwater master plan, the City may fund stormwater planning needs through a comprehensive stormwater assessment program(s). If stormwater management facilities are determined by such plan to be needed, the City shall seek grant funding to fund and construct such needed facilities.

Objective 3.3: Stormwater Design Guidelines

The City shall implement design guidelines for stormwater management facilities to promote dual use, protect natural features, and provide aesthetically pleasing facilities.

Policy 3.3.a: Stormwater facility design shall incorporate the following features, where practicable:

1. Joint use of retention and detention basins for passive recreation, habitat, and open space.
2. Use of vegetation and landscaping in and around retention and detention basin to enhance stormwater management objectives and improve aesthetics.
3. Integration of on-site retention and detention facilities with other elements of the proposed development through aesthetically sensitive design and the use of landscaping.
4. Maintain and enhance the existing hydrological and ecological function of stream or drainage corridors or wetlands.
5. Where retention and detention basins are located along arterial or collector roads, the basin design shall comply with the Gainesville Urbanized Area Metropolitan Transportation Planning Organization's Urban Design Policy Manual.

Evaluation & Findings: The applicant contends that the application will be consistent with this Goal, Objectives and Policies as the amendment will allow greater flexibility in developments being able to achieve full compliance with the Comprehensive Plan. The amendment will generally provide for more land area within the City to be developed and more land area for treatment and storage of stormwater. Further, by requiring special exceptions for these uses, the City can ensure complete compliance with these Goals, Objectives, and Policies.

FINDINGS OF FACT: COMPLIANCE WITH LAND DEVELOPMENT REGULATIONS

Subsection 2.4.1(E)(1) of the Land Development Regulations (LDRs) states that, "in determining whether to approve a proposed text amendment to the Land Development Regulations, the City Commission shall find that an application is consistent with the following standards." These standards are listed below, followed by Staff's evaluation.

(a) ***Consistent with Comprehensive Plan*** – *Whether and the extent to which the proposed amendment is consistent with the Comprehensive Plan.*

Evaluation & Findings: Please refer to 'Consistency with Comprehensive Plan' section of this report.

(b) ***Consistent with Ordinances*** – *Whether the proposed amendment is in conflict with any provision of these LDRs or the City Code of Ordinances.*

Evaluation & Findings: The proposed amendments do not conflict with any other provisions of the LDRs or the City Code of Ordinances.

(c) ***Changed Conditions*** – *Whether and the extent to which there are changed conditions that require an amendment.*

Evaluation & Findings: As the City further develops, there is an increased need for greater flexibility in siting of essential uses, such as stormwater water retention and detention facilities. The allowance of such facilities through special exception will ensure that impacts to surrounding properties and non-existent or minimal.

(d) ***Community Need*** – *Whether and the extent to which the proposed amendment addresses a demonstrated community need.*

Evaluation & Findings: The proposed amendment address a demonstrated community need by providing flexibility in siting of essential facilities (stormwater retention and detention facilities) associated with future development within the City.

(e) ***Compatible with Surrounding Uses*** – *Whether and the extent to which the proposed amendment is consistent with the purpose and intent of the zone districts in these LDRs, or will improve compatibility among uses and will ensure efficient development within the City.*

Evaluation & Findings: The proposed amendments will have no discernable effect upon the purpose and intent of the zoning districts established in the LDRs or compatibility among uses. The amendments will support efficient development within the City by responding to the concerns of professionals within the various industries which support new development and redevelopment.

(f) ***Development Patterns*** – *Whether and the extent to which the proposed amendment would result in a logical and orderly development pattern.*

Evaluation & Findings: The proposed amendment will have an effect on the development pattern as it will potentially allow for greater development "footprint". However, the requirement of special exception and related standards will mitigate any impacts on adjacent properties and uses.

(g) ***Effect on Natural Environment*** – *Whether and the extent to which the proposed amendment would result in significantly adverse impacts on the natural environment, including but not limited to water, air, noise, storm water management, wildlife, vegetation, wetlands, and the natural functioning of the environment.*

Evaluation & Findings: The proposed amendment is anticipated to have a positive impact on the natural environment as it will provide great opportunity to buffer urbanized uses away from sensitive natural areas. It will also provide for enhanced stormwater treatment techniques in those areas that are under differing zoning category.

(h) ***Public Facilities*** – *Whether and the extent to which the proposed amendment would result in development that is adequately served by public facilities (roads, potable water, sewage, storm water management, parks, and solid wastes).*

Evaluation & Findings: The proposed amendments will have no effect on public facitilities.

EXHIBIT "A"

TO

USE-SPECIFIC STANDARDS FOR MINOR UTILITIES
LAND DEVELOPMENT REGULATIONS (LDR) TEXT AMENDMENTS
(2021)
STAFF REPORT

SUPPORTING APPLICATION MATERIALS SUBMITTED BY CITY STAFF TO THE PLANNING AND ZONING BOARD

City of Alachua

ADAM BOUKARI
CITY MANAGER

PLANNING & COMMUNITY DEVELOPMENT
DIRECTOR KATHY WINBURN, AICP

March 31, 2021

Christopher Potts, P.E.
JBrown Professional Group, Inc.
3530 NW 43rd Street
Gainesville, FL 32606

Also submitted electronically to chris.potts@jbprogroup.com

RE: Notice of Hearing to be Scheduled for LDR Text Amendment Application regarding Minor Utilities

Dear Mr Potts:

On March 5, 2021 the City of Alachua received your updated application for the above referenced project.

Based on review of the materials submitted, the City has determined that the application can now be scheduled for a hearing before the Planning and Zoning Board.

You must provide three (3) *double-sided, three-hole punched, color sets* of the **complete** application package, and a digital copy of all materials in PDF format on a CD *no later than 10 business days prior to the PZB Meeting at which your application is scheduled to be heard*. The application has been scheduled for the ***April 20, 2021*** Planning and Zoning Board meeting; therefore, the above referenced materials must be submitted to the City no later than 12 PM on ***Tuesday, April 6, 2021.*** Materials may be submitted earlier than this date.

Should you have any questions, please feel free to contact me at (386) 418-6100, ext. 1603 or via email at ahall@cityofalachua.com.

Sincerely,

Adam Hall, AICP
Planner

c: Kathy Winburn, AICP, Planning Director
File

City of Alachua

ADAM BOUKARI
CITY MANAGER

PLANNING & COMMUNITY DEVELOPMENT
DIRECTOR KATHY WINBURN, AICP

February 23, 2021

Christopher Potts, P.E.
JBrown Professional Group, Inc.
3530 NW 43rd Street
Gainesville, FL 32606

Also submitted electronically to chris.potts@jbprogroup.com

RE: DRT Comments for LDR Text Amendment Regarding Location of Stormwater Retention Areas

Dear Mr. Potts:

On February 8, 2021 the City of Alachua received your application for an amendment to the City of Alachua Land Development Regulations which proposes to amend Section 4.3.2 (I)(4), relating to use-specific standards for stormwater management facilities .

The application has been reviewed for compliance with the applicable review standards found in the City's Comprehensive Plan and Land Development Regulations (LDRs). Based upon Staff's review, revisions must be made to the application before it can forwarded to the Planning and Zoning Board with a recommendation of approval from Staff. Please address all insufficiencies outlined below in writing and provide an indication as to how they have been addressed by 4:00 PM on **Monday, March 8, 2021**. Upon receipt of your revised application, Staff will notify you of any remaining insufficiencies which must be resolved, if any, before the item may be approved by the City of Alachua.

Please address the following insufficiencies:

Comments

Unless otherwise noted, references to code Sections refer to City of Alachua Land Development Regulations (LDRs).

4. The amendment language that was discussed previously involved modifying these use specific standards for added flexibility and review that would permit the placement of stormwater ponds on adjacent properties with less intense zoning designations by special exception. Consider adding new subsection 4.3.2 (I)(4)(c) that creates these new use-specific standards and/or requires approval by special exception.

NEW COMMENT: After review, Staff has suggested changes to the proposed amendment that would adequately address concerns from the City staff and accomplish the goals of the applicant. The suggested language can be found below:

4.3.2 (I)(4)Utility, minor. A minor utility use shall comply with the following standards:

(a)Location. Be located within reasonable proximity of the area to be served. Stormwater detention and retention facilities may be permitted by Special Exception Permit on properties with differing zoning designations under common ownership, subject to the following requirements:

(i) The zoning designation of the location of the stormwater detention and retention facility allows minor -utilities; and,

(ii) The stormwater detention and retention facility meets the compatibility requirements as set forth in Subsection 4.3.2 (I)(4)(b); and,

(iii) The stormwater detention and retention facility provides an enhanced natural (e.g., increased topographic relief, vegetative diversity, etc) or recreational (e.g., perimeter trail, boardwalk, interpretive kiosk, etc) functional use.

(b)Compatibility. Provide adequate setbacks, screening, and buffering around the perimeter of the proposed use if it is deemed necessary to ensure land use compatibility with surrounding uses.

2.4.4 (7)~~*Site plan*~~*Drawings*. A site plan (Subsection 2.4.9 of this section) or preliminary plat (Subsection 2.4.10 (G)(3) of this section) has been prepared that demonstrates how the proposed special exception use complies with the other standards of this subsection.

If you have any questions regarding the information above, please contact me at 386-418-6100 x 1603 or via e-mail at ahall@cityofalachua.com. We look forward to receiving your revised application.

Sincerely,

Adam Hall, AICP
Planner

cc: Kathy Winburn, AICP, Planning & Community Development Director
Justin Tabor, AICP, Principal Planner
Project File

City of Alachua

ADAM BOUKARI
CITY MANAGER

PLANNING & COMMUNITY DEVELOPMENT
DIRECTOR KATHY WINBURN, AICP

January 21, 2021

Christopher Potts, P.E.
JBrown Professional Group, Inc.
3530 NW 43rd Street
Gainesville, FL 32606

Also submitted electronically to chris.potts@jbprogroup.com

RE: DRT Comments for LDR Text Amendment Regarding Location of Stormwater Retention Areas

Dear Mr. Potts:

On December 28, 2020 the City of Alachua received your application for an amendment to the City of Alachua Land Development Regulations which proposes to amend Section 4.3.2 (I)(4), relating to use-specific standards for stormwater management facilities .

The application has been reviewed for compliance with the applicable review standards found in the City's Comprehensive Plan and Land Development Regulations (LDRs). Based upon Staff's review, revisions must be made to the application before it can forwarded to the Planning and Zoning Board with a recommendation of approval from Staff. Please address all insufficiencies outlined below in writing and provide an indication as to how they have been addressed by 4:00 PM on **Monday, February 8, 2021**. Upon receipt of your revised application, Staff will notify you of any remaining insufficiencies which must be resolved, if any, before the item may be approved by the City of Alachua.

Please address the following insufficiencies:

Comments

Unless otherwise noted, references to code Sections refer to City of Alachua Land Development Regulations (LDRs).

Generally, the proposed text does not accomplish the previously discussed goal of the applicant and unnecessarily limits the scope of the City's LDRs. Significant revisions are needed that address the following:

1. Narrative of requested amendment cites Section 4.3.1a (i)(4); this should be 4.3.2(I)(4). Revise throughout.
2. The City already defines stormwater management facilities as minor utilities (see Article 10). As presented, the proposed text would limit the use-specific standards to only applying to stormwater management facilities. It is the City's intent that these use-specific standards remain in place for all minor utilities as defined in Article 10.
3. Definition of minor utility found in Section 10.2 (emphasis added):

"*Utility, minor,* means infrastructure services that need to be located in or near the neighborhood or use type where the service is provided. Examples of minor utilities include water and sewage pump stations, distribution electric substations, as defined in F.S. § 163.3208**, stormwater retention and detention facilities**, telephone exchanges, and surface transportation stops such as bus stops."

4. The amendment language that was discussed previously involved modifying these use specific standards for added flexibility and review that would permit the placement of stormwater ponds on adjacent properties with less intense zoning designations by special exception. Consider adding new subsection 4.3.2 (I)(4)(c) that creates these new use-specific standards and/or requires approval by special exception.

5. Comprehensive Plan Analysis
    a. Future Land Use Element Goal 1: Reference to stormwater management facility being defined as a minor utility under this proposed amendment. Article 10 currently includes stormwater management facility as a minor utility. Please revise.
    b. Future Land Use Element Goal 5: Reference to stormwater management facility being defined as a minor utility under this proposed amendment. Article 10 currently includes stormwater management facility as a minor utility. Please revise.
    c. Community Facilities Element Objective 3.1, Policy 3.1.e, Policy 3.5.a: Reference to stormwater management facility being defined as a minor utility under this proposed amendment. Article 10 currently includes stormwater management facility as a minor utility. Please revise.
    d. Capital Improvement Element Goal 1, Objective 1.2: Reference to stormwater management facility being defined as a minor utility under this proposed amendment. Article 10 currently includes stormwater management facility as a minor utility. Please revise.

If you have any questions regarding the information above, please contact me at 386-418-6100 x 1603 or via e-mail at ahall@cityofalachua.com. We look forward to receiving your revised application.

Sincerely,

Adam Hall, AICP
Planner

cc: Kathy Winburn, AICP, Planning & Community Development Director
Justin Tabor, AICP, Principal Planner
Project File

City of Alachua

ADAM BOUKARI
CITY MANAGER

PLANNING & COMMUNITY DEVELOPMENT
DIRECTOR KATHY WINBURN, AICP

January 5, 2021

Christopher Potts, P.E.
JBrown Professional Group, Inc.
3530 NW 43rd Street
Gainesville, FL 32606

Also submitted electronically to chris.potts@jbprogroup.com

RE: Completeness Review for Land Development Regulation Text Amendment Application

Dear Mr. Potts:

On December 28, 2020 the City of Alachua received your application for an amendment to the City of Alachua Land Development Regulations which proposes to amend Section 4.3.2 (I)(4), relating to use-specific standards for stormwater management facilities .

According to Section 2.2.6 of the Land Development Regulations (LDRs), upon receipt of an application, a completeness review shall be conducted to determine that the application contains all the necessary information and materials, is in proper form and of sufficient detail, and is accompanied by the appropriate fee. The Planning Department has reviewed the aforementioned application for completeness and finds that the application is complete.

Please note, the contents of the application **have not** been thoroughly reviewed. An in-depth review of the content of the application will be performed subsequently and any further issues with content will be provided to you at the Development Review Team meeting, which will be scheduled separately from this letter.

Should you have any questions, please feel free to contact me at (386) 418-6100, ext. 108 or via email at ahall@cityofalachua.com.

Sincerely,

Adam Hall, AICP
Planner

c: Kathy Winburn, AICP, Planning Director
Justin Tabor, AICP, Principal Planner
File

JBrown Professional Group
CIVIL ENGINEERING • PLANNING • LAND SURVEYING
3530 NW 43rd Street • Gainesville, FL 32606-6104 • Office: 352.375.8999

March 8, 2021

Mr. Adam Hall, AICP
Principal Planner, City of Alachua
15100 NW 142nd Terrace / PO Box 9
Alachua, FL 32616

RE: Tara Forest West – Text Amendment Resubmittal

Dear Adam,

We are pleased to provide a resubmittal package for **Tara Forest West – Text Amendment Application**. This resubmittal package is provided to address the RAI comments you provided in your February 23, 2021 summary. Included in this submittal package are copies of the following:

The Text Amendment submittal package includes digital copies of the following items.

- Proposed text in strikethrough/underscore format
- Explanation of need and justification.

Please find below responses to the comments provided via email, dated February 23, 2021 for the above referenced project. The comments and responses are numbered in accordance with the numbering provided. The comments have been replicated below followed by responses in bold.

1. 4.3.2 (I)(4)Utility, minor. A minor utility use shall comply with the following standards:
   (a)Location. Be located within reasonable proximity of the area to be served.
   Stormwater detention and retention facilities may be permitted by Special Exception Permit on properties with differing zoning designations under common ownership, subject to the following requirements:
   (i) The zoning designation of the location of the stormwater detention and retention facility allows minor utilities; and,
   (ii) The stormwater detention and retention facility meets the compatibility requirements as set forth in Subsection 4.3.2 (I)(4)(b); and,
   (iii) The stormwater detention and retention facility provides an enhanced natural (e.g., increased topographic relief, vegetative diversity, etc) or recreational (e.g., perimeter trail, boardwalk, interpretive kiosk, etc) functional use.
   (b)Compatibility. Provide adequate setbacks, screening, and buffering around the perimeter of the proposed use if it is deemed necessary to ensure land use compatibility with surrounding uses.
   Response: The section 4.3.1a(I)(4) has been revised.

2. 2.4.4 (7)Site plan Drawings. A site plan (Subsection 2.4.9 of this section) or preliminary plat (Subsection 2.4.10 (G)(3) of this section) has been prepared that demonstrates how the proposed special exception use complies with the other standards of this subsection.
   Response: The language has been revised to state drawings and include preliminary plat.

If you have any questions or require any additional information, please contact me. Thank you for your help with this project.

Sincerely,



Christopher Potts, P.E.
Director of Engineering
JBrown Professional Group Inc.

Cc: Sayed Moukhtara

Tara Forest West



Land Development Regulation Text Amendment

March 8, 2021

Christopher Potts, P.E.

FL Registration No. 73842

Certificate of Authorization No. 30495

3530 NW 43rd Street

Gainesville, FL 32606

(352) 375-8999

www.jbprogroup.com

JBrown Professional Group

CIVIL ENGINEERING • PLANNING • LAND SURVEYING

The applicant, Tara Forest, LLC is requesting the approval of a Text Amendment to *Article 4.3.1a (I)(4),* in order to permit the placement of "stormwater retention and detention facilities," as defined in Article 10.2 of the City ("City") of Alachua's Code of Ordinances, on properties with differing zoning designations of varying densities under common ownership, as well as adjacent properties with differing or less intense zoning designations by special exception. A stormwater retention and detention facility is defined as a "minor utility" in Article 10.2, which further describes these types of facilities as, "…infrastructure services that need to be located in or near the neighborhood or use type where the service is provided." Although integral to the City as a key element in the designing of resilient urban landscapes, stormwater management facilities are neither permitted as of right, nor by special exception, within parcels that have various zoning designations under common ownership, or on adjacent properties with differing or less intense zoning designations. The proposed text amendment shall require that all future development of stormwater retention and detention facilities on properties with either differing zoning designations under common ownership, or properties adjacent to one another with differing or less intense zoning designations must still comply with all of the standards set forth in *Article 4.3.1a (I)(4),* as well as the special exception criterion defined in *Article 2 – Administration, Subsection 2.4.4 - Special Exception permits.*

As it stands, the City has shouldered most of the responsibility for updating and maintaining its stormwater management system. The high cost of conventional infrastructure, frequency of storm surges, and the prevalence of urban flooding has made this singular approach to storm water management more and more difficult for many cities across the country to cope with. The applicant's request to permit "stormwater management facilities," via special exception on properties with differing zoning designations of varying densities under common ownership, as well as adjacent properties with differing or less intense zoning designations, will give the City's constituents an opportunity to protect their property, neighbors, and the environment. Specifically, the development of stormwater management systems on private lands with lesser densities, will significantly reduce flooding, decrease the demand on public stormwater drainage systems, support healthy streams, rivers, and wetlands, and lastly, help create healthier, and more sustainable communities. In an effort to further mitigate the impact of urbanization in the City of Alachua, the applicant has proposed the following text amendment language:

New text
~~***Text to remove***~~
Text to remain

Article 4. – Use Regulations, Subsection 4.3.I(4) – Utilities

(4) Utility, minor. A minor utility use shall comply with the following standards:

(a) Location. Be located within reasonable proximity of the area to be served. ***Stormwater detention and retention facilities may be permitted by Special Exception on properties with differing zoning designations under common ownership, subject to following requirements:***

(i) The zoning designation of the location of the stormwater detention and retention facility allows minor-utilities; and,

(ii) The stormwater detention and retention facility meets the compatibility requirement as set forth in Subsection 4.3.2 (I)(4)(b); and,

(iii) The stormwater detention and retention facility provides an enhanced natural (e.g., increased topographic relief, vegetative diversity, etc.) or recreation (e.g., perimeter trail, boardwalk, interpretive kiosk, etc.) function use.

(b) Compatibility. Provide adequate setbacks, screening and buffering around the perimeter of the proposed use if it is deemed necessary to ensure land use compatibility with surrounding uses.

Since the applicant is requesting that stormwater detention and retention facilities be permitted by Special Exception on properties with differing zoning designations under common ownership, the following text amendment language has also been proposed:

Article 2 – Administration, Subsection 2.4.4(D)(7) – Special Exception Standards

(7) ~~***Site Plan***~~ ***Drawings.*** A site plan (Subsection 2.4.9 of this section) ***or preliminary plat (Subsection 2.4.10 (G)(3) of this section)*** has been prepared that demonstrates how the proposed special exception use complies with the other standards of this subsection.

Comprehensive Plan Analysis

Pursuant to *Article 2, Section 2.4.2(E)(1)(a)* of the City's Land Development regulations ("LDRs"), the applicant is required to demonstrate how the proposed Text Amendment to *Article 4.3.1a (I)(4)* to permit the placement of stormwater retention and detention facilities by special exception, on properties with differing zoning designations of varying densities under common ownership, as well as adjacent properties with differing or less intense zoning designations, "…is consistent with, and furthers the goals, objectives and policies (G.O.P.'s) of the [City's] Comprehensive Plan." The following analysis will provide competent, and substantial evidence that the proposed

Text Amendment is consistent with the G.O.P's set forth in the following Comprehensive Plan Elements: Future Land Use, Community Facilities, Conservation/Open Space, and Capital Improvements.

Future Land Use Element:

The following G.O.P.'s are applicable to the proposed Text Amendment application:

> ***Goal 1: Future Land Use Map 2035****. The City shall maintain a Future Land Use Map in order to effectively guide development in a sustainable manner and to ensure economic prosperity and stability while maintaining a high quality of life for all of its present and future citizens, businesses, and visitors.*
>
> > **The proposed text amendment will aim to, "…ensure economic prosperity and stability, while maintaining a high quality of life for all of [the City's] present and future citizens, businesses, and visitors," by proposing to permit the placement of "stormwater retention and detention facilities," on properties with differing zoning designations of varying densities under common ownership, as well as adjacent properties with differing or less intense zoning designations by special exception. The development of stormwater management systems on private lands will significantly reduce flooding, decrease the demand on public stormwater drainage systems, support healthy streams, rivers, and wetlands, and lastly, help create healthier, and more sustainable communities.**
>
> ***Objective 2.4: Landscaping and Tree Protection Standards.*** *The City shall adopt landscaping and tree protection standards in order to achieve the aesthetic design values of the community and preserve tree canopies, as well as to protect exceptional specimens and champion trees.*
>
> ***Policy 2.4.a: Landscaping: General.*** *The City shall require landscaping plans to be submitted with each nonresidential and multiple family residential site plan. The minimum landscaped area shall be 10% of the development site, not inclusive of any designated open space areas. Landscaping designs shall incorporate principles of xeriscaping, where feasible. The City shall develop a list of preferred planting materials to assist in the landscape design. Landscape plans shall include perimeter and internal site landscaping.*
>
> ***Policy 2.4.b: Landscaping: Buffering.*** *A buffer consists of horizontal space (land) and vertical elements (plants, berms, fences, walls) that physically separate and visually screen adjacent land uses. The City shall establish buffer yard requirements that are based on the compatibility of the adjacent uses and the desired result of the buffer.*
>
> > **The proposed text amendment, which will permit the development of stormwater retention and detention facilities by special exception, on properties with differing zoning designations of varying densities under common ownership, as well as adjacent properties with differing or less intense zoning designations will be consistent with**

the landscaping and buffering standards required for all non-residential uses permitted under the City's Code of Ordinances.

Policy 2.4.c: Tree Protection: Heritage Trees and Champion Trees. *The City shall require the preservation of heritage trees and champion trees when possible. Standards shall be set for determining the health and safety risks associated with heritage and champion trees both on individual residential lots, and existing and proposed developments.*

Policy 2.4.d: Tree Protection: Regulated Trees. *The City shall establish standards for the preservation of regulated trees. Particular attention shall be given to preserving specimen and preferred species of regulated trees, where feasible.*

Policy 2.4.e Tree Protection: Removal and Mitigation. *Along with establishing standards for tree removal and mitigation, the City shall establish a tree banking program to provide flexibility for re-planting trees through the mitigation process. Funds within the tree bank may be utilized to plant landscaping on city-owned properties, in public parks, and in road rights of way, where appropriate.*

The proposed text amendment will ensure that Heritage Trees, Champion Trees, and Regulated trees are fully preserved and protected.

Objective 2.5: Open Space Standards. *The City shall utilize open space requirements to preserve the rural character of Alachua, protect natural resources, and provide spaces for people to recreate and gather.*

Policy 2.5.a: *There shall be a minimum of 10% percent open space required. The City shall establish incentives for the provision of open space beyond minimum requirements.*

Policy 2.5.b: *Open space shall not be limited to unusable portions of project sites. A portion of open space shall be usable and functional.*

The proposed text amendment will comply with the City's open space standards. These standards require a minimum 10% of open space, in order to provide spaces for constituents and visitors alike, to recreate, gather, and enjoy the amenities provided through out the City.

Objective 2.7: Discouragement of Urban Sprawl. *The City shall discourage the proliferation of urban sprawl as set for the in Chapter 163, Florida Statutes.*

Policy 2.7.a.: *The City shall discourage the proliferation of urban sprawl. Applications for an amendment to the land use category of a property shall include an analysis of the application's discouragement of the proliferation of urban sprawl. Such analysis shall consider the primary indicators or the urban form criteria as set forth in Chapter 163.317(6)(a)9.a. and b., Florida Statutes, respectively.*

The proposed text amendment will not encourage urban sprawl, and will instead be part of a logical, and cohesive growth pattern of development.

Objective 5.2: Availability of facilities and services. *The City shall utilize a concurrency management system to ensure that the adopted levels of service standards are maintained.*

Policy 5.2.a: *All new development shall meet level of service requirements for roadways, potable water, sanitary sewer, stormwater, solid waste, and public schools in accordance with LOS standards adopted in the elements addressing these facilities.*

Policy 5.2.b: *The concurrency management system shall specify the period for which certification of level of service compliance shall be valid until actual development pursuant to a final development order or Chapter 163 Development Agreement has commenced.*

The proposed text amendment does not currently necessitate a concurrency analysis as the request is not site specific. In any case, the proposed text amendment will meet satisfactory levels of service with respects to roadways, portable water, sanitary sewer, storm water, solid waste, and other applicable facilities.

Goal 5: Development Standards: *The City shall include provisions through its comprehensive plan amendment process, development review process and in its land development regulations for development standards that address natural features and availability of facilities and services. These development standards will strive to protect natural resources and public facility resources while allowing for innovative and flexible development patterns.*

By permitting the development of stormwater retention and detention facilities by special exception, on properties with differing zoning designations of varying densities under common ownership, as well as adjacent properties with differing or less intense zoning designations, there will be a reduction in need for more complex stormwater systems and large-scale retention basins that threaten the rural characteristic of Alachua. Additionally, the added flexibility in permitting this minor utility by special exception as described in the proposed text amendment, will result in less maintenance and management of public stormwater facilities in the long term.

Community Facilities Element

The following G.O.P.'s are applicable to the proposed Text Amendment application:

Goal 1: Wastewater. *The City shall plan for and provide adequate and economical wastewater service while protecting the environment, especially groundwater resources.*

Objective 1.2: Wastewater Service. *New development may only occur within areas where wastewater service is available or shall be made available as a part of the proposed development.*

Goal 2: Solid Waste. *The City will provide for solid waste disposal service in a sanitary, economic, and environmentally safe manner.*

Goal 3: Stormwater. *The City shall develop and maintain a stormwater management system that minimizes flooding and protects, preserves, and enhances desirable water quality conditions, and, where possible, preserves and utilizes existing natural features.*

Objective 3.1: *Ensure provision of drainage and stormwater retention through level of services standards and design requirements to minimize flooding and to protect and improve water quality.*

> **Permitting stormwater retention and detention facilities by special exception as described in the proposed text amendment, will protect and improve water quality, while also minimizing flooding and stormwater run-off since property owners will now have more opportunities to develop stormwater retention and detention facilities.**

Policy 3.1.e: *The City shall provide incentives for the use of pervious surfaces in developments in order to reduce the size of retention basins and increase the area available to receive runoff.*

> **The proposed text amendment will give the City's constituents more opportunities to develop stormwater retention and detention facilities on their own properties', which should reduce the size of existing retention basins, and increase the area available to receive runoff on site.**

Policy 3.5.a: *The following general requirements apply to stormwater management systems throughout the City's high aquifer recharge areas: (3): Swale conveyances shall be used to the greatest extent possible.*

> **The proposed text amendment is compatible with the City's requirement that stormwater management systems within high aquifer recharge areas, use swale conveyances to the greatest extent possible.**

Objective 3.3: Stormwater Design Guidelines*. The City shall implement design guidelines for stormwater management facilities to promote dual use, protect natural features, and provide aesthetically pleasing facilities.*

Policy 3.3.a: *Stormwater facility design shall incorporate the following features, where practicable:*

> *1. Joint use of retention and detention basins for passive recreation, habitat, and open space.*

2. Use of vegetation and landscaping in and around retention and detention basin to enhance stormwater management objectives and improve aesthetics.

3. Integration of on-site retention and detention facilities with other elements of the proposed development through aesthetically sensitive design and the use of landscaping.

4. Maintain and enhance the existing hydrological and ecological function of stream or drainage corridors or wetlands.

5. Where retention and detention basins are located along arterial or collector roads, the basin design shall comply with the Gainesville Urbanized Area Metropolitan Transportation Planning Organization's Urban Design Policy Manual.

Future development of any stormwater retention and detention facilities, whether permitted as of right or by special exception, shall comply with all the design guidelines described in Policy 3.3a, where practicable.

Policy 5.2.c: *Applicants for new development, expansions, or redevelopment shall employ one or more of the following techniques to address potential groundwater quality and quantity impacts: (2). Decreasing the amount of stormwater runoff through the use of pervious surfaces or increased open space.*

On site stormwater retention and detention facilities will reduce runoff with the use of pervious surfaces.

Conservation & Open Space Element

The following G.O.P.'s are applicable to the proposed Text Amendment application:

Objective 1.1: Conservation Designations. *The City shall provide additional protection to particularly sensitive natural resources, habitats and ecosystems by establishing conservation designations, with specific uses, buffers and management protocol.*

There are no existing or proposed conservation areas that will be impacted by the proposed text amendment, which will permit stormwater retention and detention facilities by special exception, on properties with differing zoning designations of varying densities under common ownership, as well as adjacent properties with differing or less intense zoning designations.

Objective 1.2: Native Communities and Ecosystems. *The City shall preserve and protect native communities and ecosystems, particularly those considered endangered or threatened.*

Based on available information, there are no significant native communities or sensitive ecosystems that will be impacted by the proposed text amendment.

***Objective 1.3: Listed Species**. The City shall protect species listed by State and Federal agencies as endangered, threatened or of special concern, and their habitats.*

Based on available information, there are no listed species that will be impacted by the proposed text amendment.

***Objective 1.4: Air Quality.** The City shall institute the following measures to maintain air quality at the levels established in the National Ambient Air Quality Standards (NAAQS).*

Since there are no development requests associated with this proposed text amendment, there will be no impacts to air quality.

***Objective 1.6: Mineral Resources.** The City shall ensure that extraction activities do not pose a threat to natural resources, protected habitats and ecosystems, and human health.*

Based on available information, there are no known mineral resources that will be impacted by the proposed text amendment.

***Objective 1.7: Geological Resources.** The City shall identify, protect and conserve significant geological resources and their natural functions.*

Based on available information, there are no known geographical features that will be impacted by the proposed text amendment.

***Objective 1.11: Open and Green Space.** The City shall work to preserve native ecosystems and the natural aesthetic beauty and charm of Alachua by ensuring the provision of open spaces and green linkages throughout the City, designed for the enjoyment of the citizenry.*

The proposed text amendment shall comply with all open and green space requirements.

<u>Capital Improvement Element</u>

The following G.O.P.'s are applicable to the proposed Text Amendment application:

***Goal 1: Capital Improvements Program.** The City shall maintain a Capital Improvements Program to provide needed public facilities and services to its citizens in a manner that protects investments in existing facilities, maximizes the use of existing facilities, and promotes orderly, compact growth.*

***Objective 1.2: Public Facilities Monitoring and Level of Service Standards.** The City shall require that all decisions regarding the issuance of development orders and permits shall be consistent with the development requirements of the Comprehensive Plan, the Land Development Regulations and that public facilities and services necessary to support such development are available while also maintaining the adopted level of service standards.*

The proposed text amendment will meet satisfactory levels of service with respects to roadways, portable water, sanitary sewer, storm water, solid waste, and other applicable facilities. The applicant is

aware that all development in the City must operate within the acceptable level of service standards outlined within the Comprehensive Plan.

Compliance with Zoning Code

Pursuant to *Article 2, Section 2.4.1 (E)(1)* of the City's LDRs the applicant has demonstrated in the following ways that the proposed Text Amendment to *4.3.1a (I)(4)*, is consistent with the standards found therein:

i. ***Consistent with Comprehensive Plan.*** *Whether and the extent to which the proposed amendment is consistent with the Comprehensive Plan.*

The proposed text amendment is consistent with, and furthers the goals, objectives and policies of the Comprehensive Plan, as described in the compliance analysis found herein.

ii. ***Consistent with ordinances****. Whether the proposed amendment conflicts with any portion of these LDRs or any of the City Code of Ordinances.*

The proposed text amendment fully complies with all applicable LDRs and will not result in a conflict with any of the provisions set forth in the City's Code of Ordinances.

iii. ***Changed conditions.*** *Whether and the extent to which there are changed conditions that require an amendment.*

As urbanization and frequent storm surges further exacerbate flooding and stormwater runoff concerns, the proposed text amendment will help mitigate these circumstances since stormwater retention and detention facilities will now also be permitted by special exception on properties with differing zoning designations of varying densities under common ownership, as well as adjacent properties with differing or less intense zoning designations.

iv. ***Community need.*** *Whether and the extent to which the proposed amendment addresses a demonstrated community need.*

In urban and developed areas within the City, impervious surfaces such as pavement and roofs prevent precipitation from naturally soaking into the ground. The proposed text amendment will help reduce flooding, erosion, turbidity, infrastructure damage, and storm/sanitary sewer system overflows, by now permitting stormwater retention and detention facilities by special exception, on properties with differing zoning designations of varying densities under common ownership, as well as adjacent properties with differing or less intense zoning designations City-side.

v. ***Compatible with surrounding uses.*** *Whether and the extent to which the proposed amendment is consistent with the purpose and intent of the zoning districts in these LDRs, or will improve compatibility among uses and will ensure efficient development within the city.*

The proposed text amendment is compatible with the purpose and intent of the zoning districts in these LDRs, and will improve compatibility among uses by further reducing the likelihood of flooding and contamination of existing and natural water sources.

vi. ***Development patterns.*** *Whether and the extent to which the proposed amendment would result in a logical and orderly development pattern.*

The proposed text amendment promotes logical and orderly development of land throughout the City.

vii. ***Effect on natural environment.*** *Whether and the extent to which the proposed amendment would result in significantly adverse impacts on the natural environment, including, but not limited to, water, air, noise, stormwater management, wildlife, vegetation, wetlands, and the natural functioning of the environment.*

The proposed text amendment will not result in any adverse effects to the natural environment. Conversely, the proposed text amendment will further promote the creation of stormwater retention and detention facilities on private properties throughout the City. As a result, the amendment will significantly contribute to *Goal 3: Stormwater*, of the City's *Community Facilities Element* by minimizes flooding, while also preserving and enhances desirable water quality conditions.

viii. ***Public facilities.*** *Whether and the extent to which the proposed amendment would result in development that is adequately served by public facilities (roads, potable water, sewage, stormwater management, parks and solid wastes.)*

The proposed text amendment will positively impact development in the City, as it will provide more opportunities for property owners to develop, manage, and operate stormwater retention and detention facilities, which reduce runoff of rainwater in streets, lawns, and other sites, while also improving water quality overall.

Recommendation

In view of the forgoing analysis, which is based upon the City's Comprehensive Plan and applicable LDR(s), we respectfully recommend approval of the applicant's request to amend *Article 4.3.1a (I)(4)* of the city's LDR(s), which would permit the placement of "stormwater retention and detention facilities," on properties with differing zoning designations of varying densities under common ownership, as well as adjacent properties with differing or less intense zoning designations by special exception.

Forest Grove Baptist Church
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www.ForestGroveBaptistChurch.org

Family Movie Night
Friday, April 9 at 6:45 p.m.
Worship with us via Live Stream or on-site
Sunday 9:00am - Sunday School
10:15 am - Morning Worship
Wenesday 6:00 pm - Prayer and Bible Study

St. Madeleine Cath
17155 NW US H
High Springs, Fl
386-454-23
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Sunday: 7:30 a.m. &
Saturday Vigil: 5
www.stmadeleinecatho

te project

lore information his project and its onents is available www.newberryfl.gov/ inistration/page/ gic-planning.

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Email cwalker@ alachuatoday.com

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mission, to amend the text ng Table 11.00.01 entitled , to delete Permit column; nd Plat wo improvements; ment plans, Improvement Construction plans, Final pecial exception; require ppropriateness (Historic); ination of required review no improvements'; delete ision with no improvement itled Level one – Action (a)(1)-(5) and (b); modify plan required and Section o remove 'by the SPRC in

City of ALACHUA

NOTICE OF PUBLIC HEARING BEFORE THE PLANNING AND ZONING BOARD OF THE CITY OF ALACHUA, FLORIDA

Notice is hereby given that the Planning and Zoning Board of the City of Alachua will hold a public hearing on April 20, 2021, at 6:00 p.m. The hearing will be held in the James A. Lewis Commission Chambers in City Hall, located at 15100 NW 142nd Terrace, Alachua, Florida, to consider a request by Christopher Potts, P.E., of JBrown Professional Group, Inc., to amend the City of Alachua Land Development Regulations (LDRs) as follows: amending Section 4.3.2 (I)(4), relating to the use-specific standards for minor utilities; amending Section 2.4.4 (D) (7), relating to standards for special exception permits.

At the public hearing, all interested parties may appear and be heard with respect to the application. Copies of the application are available for public inspection at the Planning and Community Development Department, 15100 NW 142nd Terrace, Alachua, Florida, on any regular business day between the hours of 7:30 a.m. to 6:00 p.m. Written comments on the application may be sent to the following address: City of Alachua, Planning and Community Development, P.O. Box 9, Alachua, FL 32616. Notice is given pursuant to Section 286.0105, Florida Statutes, that, in order to appeal any decision made at the public hearing, you will need a record of the proceedings, and that, for such purpose, you may need to ensure that a verbatim record of the proceedings is made, which includes the testimony and evidence upon which the appeal is to be based. In accordance with the Americans with Disabilities Act, any persons with a disability requiring reasonable accommodation in order to participate in this meeting should call the City Clerk at (386) 418-6100 x 101 at least 48 hours prior to the public hearing.

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